

nector List	
oduct	Qty
HUS26	15
HTS20	1
TBE4	1 set
2.5A As info Only	50

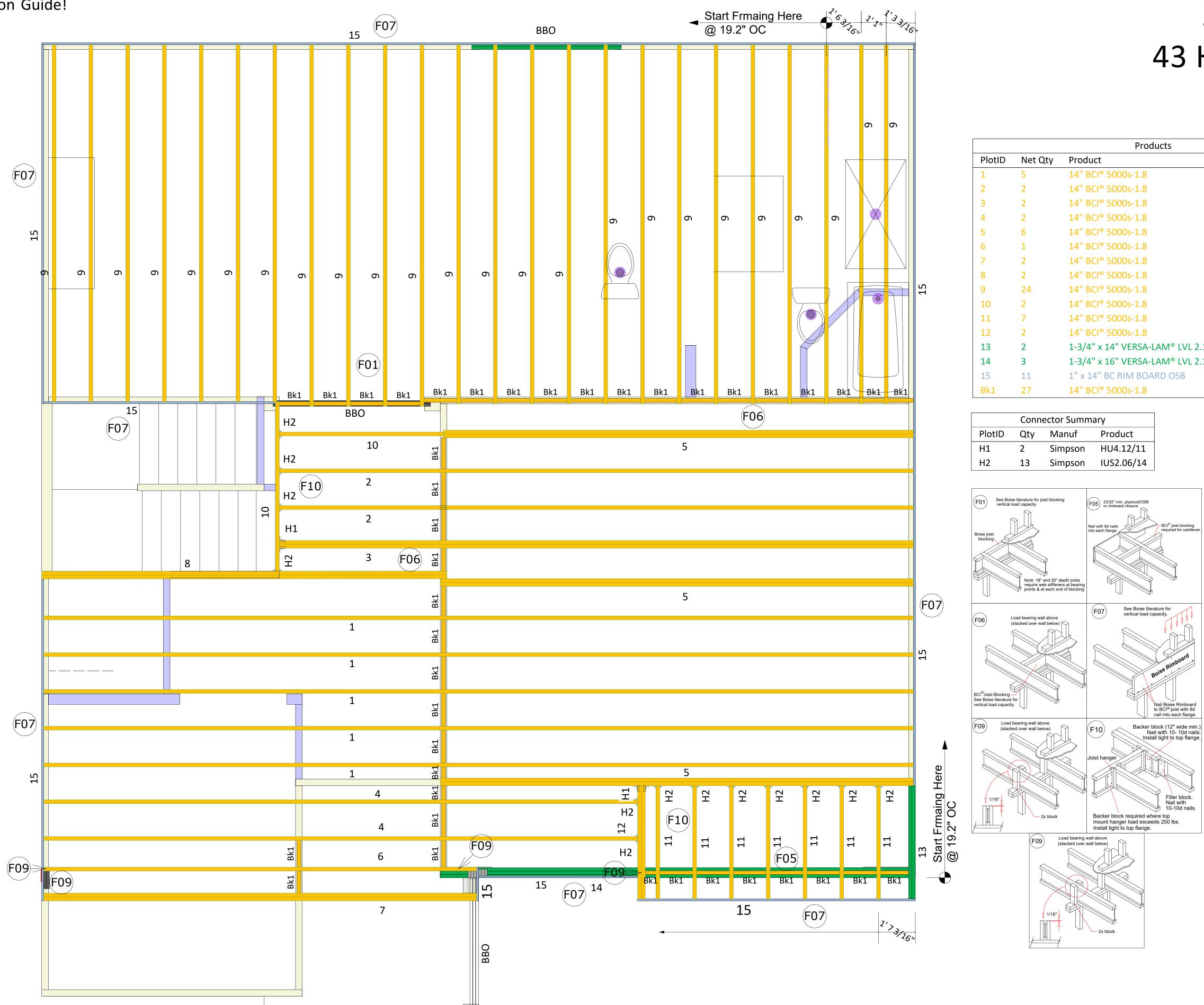


DEDICATED TO QUALITY AND EXCELLENCE 200 EMMETT ROAD DUNN, NORTH CAROLINA 28334 PHONE: 910-892-8400 FAX: 910-892-8384

ove		GOR	ORDER: 26902	SHIP DATE: 2021	
ghland Gr	KB HOME		P.O. NUMBER: PO#	REV:	
Lot 43 Highland Grove	KB	Plan 238.2338 "A"		PRINT DATE: 5/10/21	
PROJECT:	SUSTOMER:	NODEL: PI	SCALE: NOT TO SCALE	DRAWN BY: MWM	
Т	TOP LIVE: 20 PSF				
тс	)P DE	EAD:	10 P	SF	
BO	TM D	EAD	: 10 I	PSF	
WIN	ID SF	PD:	120 N	ИРН	
GENERAL NOTES: DO NOT CUT OR MODIFY TRUSSES. TRUSSES ARE SPACED 24" ON CENTER UNLESS NOTED OTHERWISE. REFER TO THE INDIVIDUAL TRUSS DESIGN DRAWINGS FOR THE LOCATION OF LATERAL BRACING AND MULTI-PLY CONNECTION REQUIREMENTS. PER ANSI TPI 1-2002 THE TRUSS ENGINEER IS RESPONSIBLE FOR TRUSS TO TRUSS CONNECTIONS AND TRUSS PLY TO PLY CONNECTIONS. THIS TRUSS PLACEMENT PLAN RECCOMENDS TRUSS TO BEARING CONNECTIONS AND TRUSS TO BEARING CONNECTIONS AND TRUSS TO BEARING CONNECTIONS WHICH SHALL BE REVIEWED BY THE BUILDING DESIGNER. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO					

ADEQUATELY TO THE FOUNDATION.

All I-Joist and Versa-Lam Beams Must be Installed per The Boise Cascade Installation Guide!



Second Floor Layout

Revisions:

# **KB** Homes 2338 Elev.A 43 Highland Grove

	Products		
ty	Product	Length	Plies
	14" BCI® 5000s-1.8	38' 0"	1
	14" BCI® 5000s-1.8	28' 0"	1
	14" BCI® 5000s-1.8	28' 0"	2
	14" BCI® 5000s-1.8	26' 0"	1
	14" BCI® 5000s-1.8	21' 0"	2
	14" BCI® 5000s-1.8	19' 0"	1
	14" BCI® 5000s-1.8	19' 0"	2
	14" BCI® 5000s-1.8	18' 0"	2
	14" BCI® 5000s-1.8	16' 0"	1
	14" BCI® 5000s-1.8	8' 0"	1
	14" BCI® 5000s-1.8	5' 0"	1
	14" BCI® 5000s-1.8	5' 0"	2
	1-3/4" x 14" VERSA-LAM® LVL 2.1E 3100 SP	6' 0"	2
	1-3/4" x 16" VERSA-LAM <sup>®</sup> LVL 2.1E 3100 SP	22' 0"	3
	1" x 14" BC RIM BOARD OSB	12' 0"	1
	14" BCI <sup>®</sup> 5000s-1.8	2' 0"	1

Squash Blocks Required Under The Ends Of All LVL And Point Loads For Load Transfer - See Details

Cascade Boise n m °

Ð KB Homes 2338 Elev.A 43 Highland Grove 84 Lumber EWP

### BC FRAMER II

Plan Date: 02/10/2020

Struc Date: 10/24/2019

By: GAT

Sheet: 2F

## **STRUCTURAL PLANS FOR:**

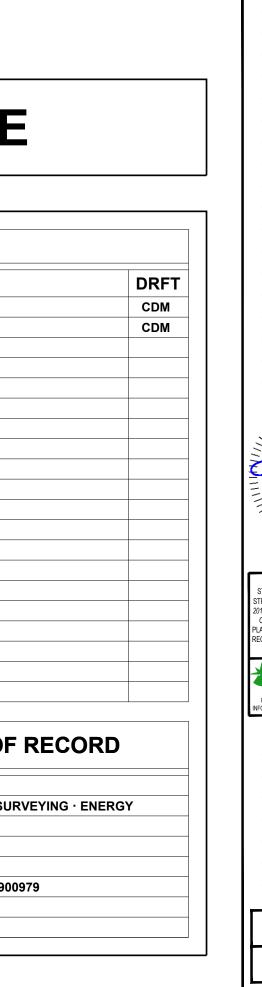


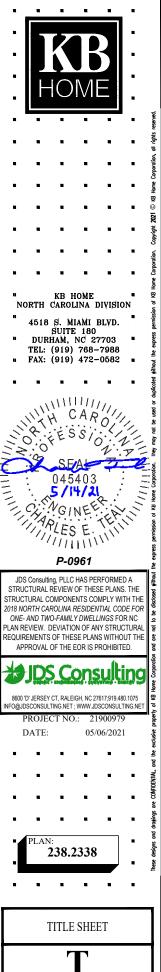
# 238.2338 - RH GARAGE

### PLAN RELEASE / REVISIONS

REV DATE	ARCH PLAN VERSION	REVISION DESCRIPTION
05/06/2021	2338-238-01350 RH D19 010621	INITIAL SETUP OF LAYOUT
05/06/2021	2338-238-01350 RH D19 010621	CREATED LOT-SPECIFIC STRUCTURAL LAYOUT FROM MASTER PLAN AND EWP LAYOUT

NO	TES	CODE	ENGINEER OF
1. ENGINEER'S SEAL APPLIES TO STRUCTURAL COMPONENTS	3. PLANS MUST HAVE SIGNED SEAL TO BE VALID AND ARE	ALL CONSTRUCTION, WORKMANSHIP,	JDS Consulting, PLLC
ONLY. ENGINEER'S SEAL DOES NOT CERTIFY DIMENSIONAL ACCURACY OR ARCHITECTURAL LAYOUT, INCLUDING ROOF	LIMITED TO THE FOLLOWING USES:	AND MATERIAL QUALITY AND SELECTION SHALL BE PER:	
GEOMETRY. JDS CONSULTING, PLLC ASSUMES NO LIABILITY FOR CHANGES MADE TO THESE PLANS BY OTHERS, OR FOR	A. IF THESE PLANS ARE ISSUED AS A MASTER-PLAN SET, THE SET IS VALID FOR 18 MONTHS FROM THE DATE ON		8600 'D' JERSEY COURT
CONSTRUCTION METHODS, OR FOR ANY DEVIATION FROM	THE SEAL, UNLESS ANY CODE-REQUIRED UPDATES ARE	2018	RALEIGH, NC 27617
THE PLANS. ENGINEER TO BE NOTIFIED PRIOR TO CONSTRUCTION IF ANY DISCREPANCIES ARE NOTED ON THE	PLACED IN EFFECT BY THE MUNICIPALITY. B. IF THESE PLANS ARE NOT ISSUED AS A MASTER-PLAN	NORTH CAROLINA	FIRM LIC. NO: P-0961
PLANS.	SET, THE SET IS VALID FOR A CONDITIONAL, ONE-TIME USE FOR THE LOT OR ADDRESS SPECIFIED ON THE	STATE BUILDING CODE: RESIDENTIAL CODE	PROJECT REFERENCE: 219009
2. DIMENSIONS SHALL GOVERN OVER SCALE, AND CODE SHALL GOVERN OVER DIMENSIONS.	TITLE BLOCK.		





NOTE: ALL CHAPTERS, SECTIONS, TABLES, AND FIGURES CITED WITHOUT A PUBLICATION TITLE ARE FROM THE APPLICABLE RESIDENTIAL CODE (SEE TITLE SHEET).

#### GENERAL

- 1. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION. FURTHERMORE, CONTRACTOR IS ULTIMATELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, AND SAFETY ON SITE. NOTIFY JDS Consulting, PLLC IMMEDIATELY IF DISCREPANCIES ON PLAN EXIST.
- 2. BRACED-WALL DESIGN IS BASED ON SECTION R602.10 WALL BRACING. PRIMARY PRESCRIPTIVE METHOD TO BE CS-WSP. SEE WALL BRACING PLANS AND DETAILS FOR ADDITIONAL INFORMATION

ALL NON-PRESCRIPTIVE SOLUTIONS ARE BASED ON GUIDELINES ESTABLISHED IN THE AMERICAN SOCIETY OF CIVIL ENGINEERS PUBLICATION ASCE 7 AND THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION - SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC.

SEISMIC DESIGN SHALL BE PER SECTION R301.2.2 - SEISMIC 3. PROVISIONS, INCLUDING ASSOCIATED TABLES AND FIGURES, BASED ON LOCAL SEISMIC DESIGN CATEGORY.

#### DESIGN LOADS

ASSUMED SOIL BEARING-CAPACITY	2,000 PSF
	LIVE LOAD
ULTIMATE DESIGN WIND SPEED	115 MPH, EXPOSURE B
GROUND SNOW	15 PSF
ROOF	20 PSF
RESIDENTIAL CODE TABLE R301.5	LIVE LOAD (PSF)
DWELLING UNITS	40
SLEEPING ROOMS	30
ATTICS WITH STORAGE	20
ATTICS WITHOUT STORAGE	10
STAIRS	40
DECKS	40
EXTERIOR BALCONIES	60
PASSENGER VEHICLE GARAGES	50
FIRE ESCAPES	40
GUARDS AND HANDRAILS	200 (pounds, concentrated)

COMPONENT AND CLADDING LOADS. INCLUDING THOSE FOR DOORS AND WINDOWS, SHALL BE DERIVED FROM TABLES R301.2(2) AND R301.2(3) FOR A BUILDING WITH A MEAN ROOF HEIGHT OF 35 FEET, LOCATED IN EXPOSURE B

KS

KING STUD COLUMN

LAMINATED VENEER

PRESSURE TREATED

SQUARE FOOT (FEET)

SHELF / SHELVES SHEATHING

MECHANICAL

MANUFACTURER

NOT TO SCALE

REFRIGERATOR

ROUGH OPENING

ROOF SUPPORT

STUD COLUMN

SINGLE JOIST

STUD POCKET

THICK(NESS)

TRIPLE JOIST

TRIPLE RAFTER

TOP OF CURB / CONCRETE

UNLESS NOTED OTHERWISE CLOTHES WASHER WATER HEATER WELDED WIRE FABRIC EXTRA JOIST

TREAD TEMPERED GLASS

#### ABBREVIATIONS

ADDR	EVIATIONS	LVL	LAMINATED
		LVL	LUMBER
ABV	ABOVE	МАХ	MAXIMUM
AFF	ABOVE FINISHED FLOOR	MECH	MECHANICA
ALT	ALTERNATE	METR	MANUFACT
BRG	BEARING	MIN	MINIMUM
BSMT CANT	BASEMENT	NTS	NOT TO SC
CANT	CANTILEVER	OA	OVERALL
CLG	CEILING JOIST CEILING	OC	ON CENTER
CLG	CONCRETE MASONRY UNIT	PT	PRESSURE
CNIU	CASED OPENING	R	RISER
COL	COLUMN	REF	REFRIGERA
COL	CONCRETE	RFG	ROOFING
CONC	CONTINUOUS	RO	ROUGH OPE
D	CLOTHES DRYER	RS	ROOF SUPP
DBL	DOUBLE	SC	STUD COLU
DIAM	DIAMETER	SF	SQUARE FC
DJ	DOUBLE JOIST	SH	SHELF / SHI
DN	DOWN	SHTG	SHEATHING
DP	DEEP	SHW	SHOWER
DR	DOUBLE RAFTER	SIM	SIMILAR
DSP	DOUBLE STUD POCKET	SJ	SINGLE JOI
EA	EACH	SP	STUD POCK
EE	EACH END	SPEC'D	SPECIFIED
EQ	EQUAL	SQ	SQUARE
EX	EXTERIOR	т	TREAD
FAU	FORCED-AIR UNIT	TEMP	TEMPERED
FDN	FOUNDATION	THK	THICK(NES
FF	FINISHED FLOOR	TJ	TRIPLE JOIS
FLR	FLOOR(ING)	тос	TOP OF CU
FP	FIREPLACE	TR	TRIPLE RAP
FTG	FOOTING	TYP	TYPICAL
HB	HOSE BIBB	UNO	UNLESS NO
HDR	HEADER	W	CLOTHES W
HGR	HANGER	WH	WATER HEA
JS	JACK STUD COLUMN	WWF	WELDED W
		XJ	EXTRA JOIS

#### MATERIALS

INTERIOR / TRIMMED FRAMING LUMBER SHALL BE #2 SPRUCE PINE FIR (SPF) WITH THE FOLLOWING DESIGN PROPERTIES (#2 SOUTHERN YELLOW PINE MAY BE SUBSTITUTED):

Fb = 875 PSI Fv = 70 PSI E = 1.4E6 PSI

2. FRAMING LUMBER EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND, CONCRETE, OR MASONRY SHALL BE PRESSURE TREATED #2 SOUTHERN YELLOW PINE (SYP) WITH THE FOLLOWING DESIGN PROPERTIES:

Fb = 975 PSI Fv = 95 PSI E = 1.6E6 PSI

3. LVL STRUCTURAL MEMBERS TO BE LAMINATED VENEER LUMBER WITH THE FOLLOWING MINIMUM DESIGN PROPERTIES:

Fb = 2600 PSI Fv = 285 PSI E = 1.9E6 PSI

PSL STRUCTURAL MEMBERS TO BE PARALLEL STRAND LUMBER 4. WITH THE FOLLOWING MINIMUM DESIGN PROPERTIES:

Fb = 2900 PSI Fv = 290 PSI E = 2.0E6 PSI

5. LSL STRUCTURAL MEMBERS TO BE LAMINATED STRAND LUMBER WITH THE FOLLOWING MINIMUM DESIGN PROPERTIES:

Fb = 2250 PSI Fv = 400 PSI E = 1.55E6 PSI

- STRUCTURAL STEEL WIDE-FLANGE BEAMS SHALL CONFORM TO ASTM A992. Fy = 50 KSI
- REBAR SHALL BE DEFORMED STEEL CONFORMING TO ASTM A615, GRADE 60.
- 8. POURED CONCRETE COMPRESSIVE STRENGTH TO BE A MINIMUM 3,000 PSI AT 28 DAYS. MATERIALS USED TO PRODUCE CONCRETE SHALL COMPLY WITH THE APPLICABLE STANDARDS LISTED IN AMERICAN CONCRETE INSTITUTE STANDARD ACI 318 OR ASTM C1157.
- CONCRETE SUBJECT TO MODERATE OR SEVERE WEATHERING PROBABILITY PER TABLE R301.2(1) SHALL BE AIR-ENTRAINED WHEN REQUIRED BY TABLE R402.2.
- 10. CONCRETE MASONRY UNITS (CMU) SHALL CONFORM TO AMERICAN CONCRETE INSTITUTE PUBLICATION 530: BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES AND COMPANION COMMENTARIES AND THE MASONRY SOCIETY PUBLICATION TMS 402/602: BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES.
- 11. MORTAR SHALL COMPLY WITH ASTM INTERNATIONAL STANDARD
- 12. INDICATED MODEL NUMBERS FOR ALL METAL HANGERS, STRAPS, FRAMING CONNECTORS, AND HOLD-DOWNS ARE SIMPSON STRONG-TIE BRAND, EQUIVALENT USP BRAND PRODUCTS ARE ACCEPTABLE.
- 13. REFER TO I-JOIST EQUIVALENCE CHART ON I-JOIST DETAIL SHEET FOR SUBSTITUTION OF MANUFACTURER SERIES.

#### FOUNDATION

- MINIMUM ALLOWABLE SOIL BEARING CAPACITY IS ASSUMED TO BE 2,000 PSF. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY SOIL BEARING CAPACITY IF UNSATISFACTORY CONDITIONS FXIST
- 2. CONCRETE FOUNDATION WALLS TO BE SELECTED AND CONSTRUCTED PER SECTION R404 OR AMERICAN CONCRETE INSTITUTE STANDARD ACI 318.
- MASONRY FOUNDATION WALLS TO BE SELECTED AND CONSTRUCTED PER SECTION R404 AND/OR AMERICAN CONCRETE INSTITUTE PUBLICATION 530: BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES AND COMPANION COMMENTARIES AND/OR THE MASONRY SOCIETY PUBLICATION TMS 402/602: BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES.
- CONCRETE WALL HORIZONTAL REINFORCEMENT TO BE PER TABLE R404.1.2(1) OR AS NOTED OR DETAILED. CONCRETE WALL VERTICAL REINFORCEMENT TO BE PER TABLES R404.1.2(3 AND 4) OR AS NOTED OR DETAILED, ALL CONCRETE WALLS SHALL COMPLY WITH APPLICABLE PROVISIONS OF CHAPTER 6.
  - TABLES ASSUME THAT WALLS HAVE PERMANENT LATERAL SUPPORT AT THE TOP AND BOTTOM. FOUNDATION DRAINS ARE ASSUMED AT ALL WALLS PER В. SECTION R405.
- 5. PLAIN-MASONRY WALL DESIGN TO BE PER TABLE R404.1.1(1) OR AS NOTED OR DETAILED. MASONRY WALLS WITH VERTICAL REINFORCEMENT TO BE PER TABLES R404.1.1 (2 THROUGH 4) OR AS NOTED OR DETAILED. ALL MASONRY WALLS SHALL COMPLY WITH APPLICABLE PROVISIONS OF CHAPTER 6.
  - A. TABLES ASSUME THAT WALLS HAVE PERMANENT LATERAL SUPPORT AT THE TOP AND BOTTOM.
  - WALL REINFORCING SHALL BE PLACED ACCORDING TO B FOOTNOTE (c) OF THE TABLES (REINFORCING IS NOT CENTERED IN WALL).
  - C. FOUNDATION DRAINS ARE ASSUMED AT ALL WALLS PER SECTION R405.
- 6. WOOD SILL PLATES TO BE ANCHORED TO THE FOUNDATION WITH 1/2" DIAMETER ANCHOR BOLTS WITH MINIMUM 7" EMBEDMENT. SPACED A MAXIMUM OF 6'-0" OC AND WITHIN 12" FROM THE ENDS OF EACH PLATE SECTION. INSTALL MINIMUM (2) ANCHOR BOLTS PER SECTION. SEE SECTION R403.1.6 FOR SPECIFIC CONDITIONS.
- 7. THE UNSUPPORTED HEIGHT OF SOLID MASONRY PIERS SHALL NOT EXCEED TEN TIMES THEIR LEAST DIMENSION. UNFILLED, HOLLOW PIERS MAY BE USED IF THE UNSUPPORTED HEIGHT IS NOT MORE THAN FOUR TIMES THEIR LEAST DIMENSION
- CENTERS OF PIERS TO BEAR IN THE MIDDLE THIRD OF THE FOOTINGS, AND GIRDERS SHALL CENTER IN THE MIDDLE THIRD OF THE PIERS.
- 9. ALL FOOTINGS TO HAVE MINIMUM 2" PROJECTION ON EACH SIDE OF FOUNDATION WALLS (SEE DETAILS).
- 10. ALL REBAR NOTED IN CONCRETE TO HAVE AT LEAST 2" COVER FROM EDGE OF CONCRETE TO EDGE OF REBAR
- 11. FRAMING TO BE FLUSH WITH FOUNDATION WALLS.
- 12. WITH CLASS 1 SOILS, VAPOR BARRIER AND CRUSHED STONE MAY BE OMITTED

#### FRAMING

- 3.
- STRUCTURAL COMPONENTS.
- CONSTRUCTION.

7

- LUMBER.

  - DETAILS.
- SPECIFICATIONS

- C.
- D.
- DRAWINGS.

- EACH END OF FLITCH BEAM.

- EXTERIOR RIM JOIST / BOARD
- SHALL BE MET.

ALL BEARING HEADERS TO BE (2) 2x6 SUPPORTED W/ MIN (1) JACK STUD AND (1) KING STUD EACH END, UNO.

ALL NON-BEARING HEADERS TO BE (2) 2x4, UNO.

NON-BEARING INTERIOR WALLS NOT MORE THAN 10' NOMINAL HEIGHT AND NOT SHOWN AS BRACED WALLS MAY BE FRAMED WITH 2x4 STUDS @ 24" OC.

SOLID BLOCKING TO BE PROVIDED AT ALL POINT LOADS THROUGH FLOOR LEVELS TO THE FOUNDATION OR TO OTHER

5. ALL BEAMS SPECIFIED ARE MINIMUM SIZES ONLY. LARGER MEMBERS MAY SUBSTITUTED AS NEEDED FOR EASE OF

6. ALL EXTERIOR WALLS TO BE FULLY SHEATHED WITH 7/16" OSB

PORCH / PATIO COLUMNS TO BE 4x4 MINIMUM PRESSURE-TREATED

A. ATTACH PORCH COLUMNS TO SLAB / FDN WALL USING ABA, ABU, ABW, OR CPT SIMPSON POST BASES TO FIT COLUMN SIZES NOTED ON PLAN -OR- ANY OTHER COLUMN CONNECTION WITH 500# UPLIFT CAPACITY.

B. ATTACH PORCH COLUMNS TO PORCH BEAMS USING AC OR BC SIMPSON POST CAPS TO FIT COLUMN SIZES NOTED ON PLAN -OR- ANY OTHER COLUMN CONNECTION WITH 500# UPLIFT CAPACITY.

C. TRIM OUT COLUMN(S) AND BEAM(S) PER BUILDER AND

8. ALL ENGINEERED WOOD PRODUCTS (LVL, PSL, LSL, ETC.) SHALL BE INSTALLED WITH CONNECTIONS PER MANUFACTURER

ENGINEERED WOOD FLOOR SYSTEMS AND ROOF TRUSS SYSTEMS: A. SHOP DRAWINGS FOR THE SYSTEMS SHALL BE PROVIDED TO THE ENGINEER OF RECORD FOR REVIEW AND COORDINATION BEFORE CONSTRUCTION. TRUSS PROFILES SHALL BE SEALED BY THE TRUSS MANUFACTURER

INSTALLATION OF THE SYSTEMS SHALL BE PER MANUFACTURER'S INSTRUCTIONS. TRUSS LAYOUT AND PLACEMENT BY MANUFACTURER TO

COINCIDE WITH THE SUPPORT LOCATIONS SHOWN IN THESE

10. ALL BEAMS TO BE CONTINUOUSLY SUPPORTED LATERALLY AND SHALL BEAR FULL WIDTH ON THE SUPPORTING WALLS OR COLUMNS INDICATED, WITH A MINIMUM OF THREE STUDS, UNO.

11. ALL STEEL BEAMS TO BE SUPPORTED AT EACH END WITH A MIN BEARING LENGTH OF 3 1/2" AND FULL FLANGE WIDTH. BEAMS MUST BE ATTACHED AT EACH END WITH A MINIMUM OF FOUR 16d NAILS OR TWO 1/2" x 4" LAG SCREWS, UNO.

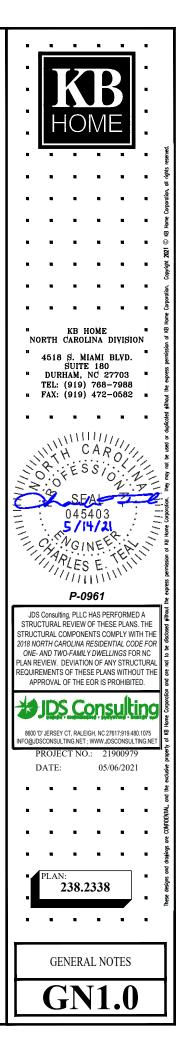
12. STEEL FLITCH BEAMS TO BE BOLTED TOGETHER USING (2) ROWS OF 1/2" DIAMETER BOLTS (ASTM 307) WITH WASHERS PLACED UNDER THE THREADED END OF THE BOLT. BOLTS TO BE SPACED AT 24" OC (MAX) AND STAGGERED TOP AND BOTTOM OF BEAM (2' EDGE DISTANCE, WITH TWO BOLTS TO BE LOCATED AT 6" FROM

13. WHEN A 4-PLY LVL BEAM IS USED, ATTACH WITH (1) 1/2" DIAMETER BOLT, 12" OC, STAGGERED TOP AND BOTTOM, 1 1/2" MIN FROM ENDS. ALTERNATE EQUIVALENT ATTACHMENT METHOD MAY BE USED, SUCH AS SDS, SDW, OR TRUSSLOK SCREWS (SEE MANUFACTURER SPECIFICATIONS).

14. FOR STUD COLUMNS OF 4-OR-MORE STUDS, INSTALL SIMPSON STRONG-TIE CS16 STRAPS ACROSS STUDS @ 30" OC, 6" MAX FROM PLATES, ON INSIDE FACE OF COLUMN (EXTERIOR WALL), ON BOTH FACES OF COLUMN (INTERIOR WALL).

15. FLOOR JOISTS ADJACENT AND PARALLEL TO THE EXTERIOR FOUNDATION WALL SHALL BE PROVIDED WITH FULL-DEPTH SOLID BLOCKING, NOT LESS THAN TWO (2) INCHES NOMINAL IN THICKNESS, PLACED PERPENDICULAR TO THE JOIST AT SPACING NOT MORE THAN FOUR (4) FEET. THE BLOCKING SHALL BE NAILED TO THE FLOOR SHEATHING, THE SILL PLATE, THE JOIST, AND THE

16. BRACED WALL PANELS SHALL BE FASTENED TO MEET THE UPLIFT-RESISTANCE REQUIREMENTS IN CHAPTERS 6 AND 8 OF THE APPLICABLE CODE (SEE TITLE SHEET). REQUIREMENTS OF THE STRUCTURAL DRAWINGS THAT EXCEED THE CODE MINIMUM



FASTENER SCHEDULE				
CONNECTION 3" x 0.131" NAIL 3" x 0.120" N				
JOIST TO SILL PLATE	(4) TOE NAILS	(4) TOE NAILS		
SOLE PLATE TO JOIST / BLOCKING	NAILS @ 8" OC (typical) (4) PER 16" SPACE (at braced panels)	NAILS @ 8" OC (typical) (4) PER 16" SPACE (at braced panels)		
STUD TO SOLE PLATE	(4) TOE NAILS	(4) TOE NAILS		
TOP OR SOLE PLATE TO STUD	(3) FACE NAILS	(4) FACE NAILS		
RIM JOIST OR BAND JOIST TO TOP PLATE OR SILL PLATE	TOE NAILS @ 6" OC	TOE NAILS @ 4" OC		
BLOCKING BETWEEN JOISTS TO TOP PLATE OR SILL PLATE	(4) TOE NAILS	(4) TOE NAILS		
DOUBLE STUD	NAILS @ 8" OC	NAILS @ 8" OC		
DOUBLE TOP PLATES	NAILS @ 12" OC	NAILS @ 12" OC		
DOUBLE TOP PLATES LAP (24" MIN LAP LENGTH)	(12) NAILS IN LAPPED AREA, EA SIDE OF JOINT	(12) NAILS IN LAPPED AREA, EA SIDE OF JOINT		
TOP PLATE LAP AT CORNERS AND INTERSECTING WALLS	(3) FACE NAILS	(3) FACE NAILS		
OPEN-WEB TRUSS BOTTOM CHORD TO TOP PLATES OR SILL PLATE (PARALLEL TO WALL)	NAILS @ 6" OC	NAILS @ 4" OC		
BOTTOM CHORD OF TRUSS TO TOP PLATES OR SILL PLATE (PERPENDICULAR TO WALL)	(3) TOE NAILS	(3) TOE NAILS		

#### SEE <u>TABLE R602.3(1)</u> FOR ADDITIONAL STRUCTURAL-MEMBER FASTENING REQUIREMENTS.

DETAILS AND NOTES ON DRAWINGS GOVERN.

#### BALLOON WALL FRAMING SCHEDULE (USE THESE STANDARDS UNLESS NOTED OTHERWISE ON THE FRAMING PLAN SHEETS)

	MAX HEIGHT (PLATE TO PLATE)
FRAMING MEMBER SIZE	115 MPH ULTIMATE DESIGN WIND SPEED
2x4 @ 16" OC	10'-0"
2x4 @ 12" OC	12'-0"
-	
2x6 @ 16" OC	15'-0"
2x6 @ 12" OC	17'-9"
2x8 @ 16" OC	19'-0"
2x8 @ 12" OC	22'-0"
(2) 2x4 @ 16" OC	14'-6"
(2) 2x4 @ 12" OC	17'-0"
(2) 2x6 @ 16" OC	21'-6"
(2) 2x6 @ 12" OC	25'-0"
(2) 2x8 @ 16" OC	27'-0"
(2) 2x8 @ 12" OC	31'-0"
(2) 2x4 @ 12" OC (2) 2x6 @ 16" OC (2) 2x6 @ 12" OC (2) 2x8 @ 16" OC	17'-0" 21'-6" 25'-0" 27'-0"

- a. ALL HEIGHTS ARE MEASURED SUBFLOOR TO TOP OF WALL PLATE.
- b. WHEN SPLIT-FRAMED WALLS ARE USED FOR HEIGHTS OVER 12', THE CONTRACTOR SHALL ADD 6' MINIMUM OF CS16 COIL STRAPPING (FULLY NAILED), CENTERED OVER THE WALL BREAK.
- c. FINGER-JOINTED MEMBERS MAY BE USED FOR CONTINUOUS HEIGHTS WHERE TRADITIONALLY MILLED LUMBER LENGTHS ARE LIMITED.
- d. FOR GREATER WIND SPEED, SEE ENGINEERED SOLUTION FOR CONDITION IN DRAWINGS.

#### ROOF SYSTEMS

TRUSSED ROOF - STRUCTURAL NOTES

1. PROVIDE CONTINUOUS BLOCKING THROUGH STRUCTURE FOR ALL POINT LOADS.

DENOTES OVER-FRAMED AREA

- 2.
- 3. MINIMUM 7/16" OSB ROOF SHEATHING
- 4. TRUSS LAYOUT AND PLACEMENT BY MANUFACTURER TO COINCIDE WITH THE SUPPORT LOCATIONS SHOWN. TRUSS PROFILES SHALL BE SEALED BY THE TRUSS MANUFACTURER. TRUSS PLANS TO BE COORDINATED WITH THE SEALED STRUCTURAL DRAWINGS. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS.
- 5. MANUFACTURER TO PROVIDE REQUIRED UPLIFT CONNECTION.
- 6. PROVIDE H2.5A (MINIMUM) OR EQUIVALENT AT EACH TRUSS-TO-TOP PLATE CONNECTION AT OVER-FRAMED AREAS, UNLESS NOTED OTHERWISE.
- 7. UPLIFT CONNECTION TO BE CARRIED THROUGH TO FLOOR SYSTEM.

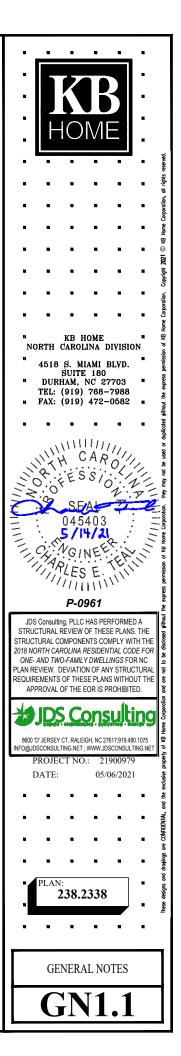
#### STICK-FRAMED ROOF - STRUCTURAL NOTES

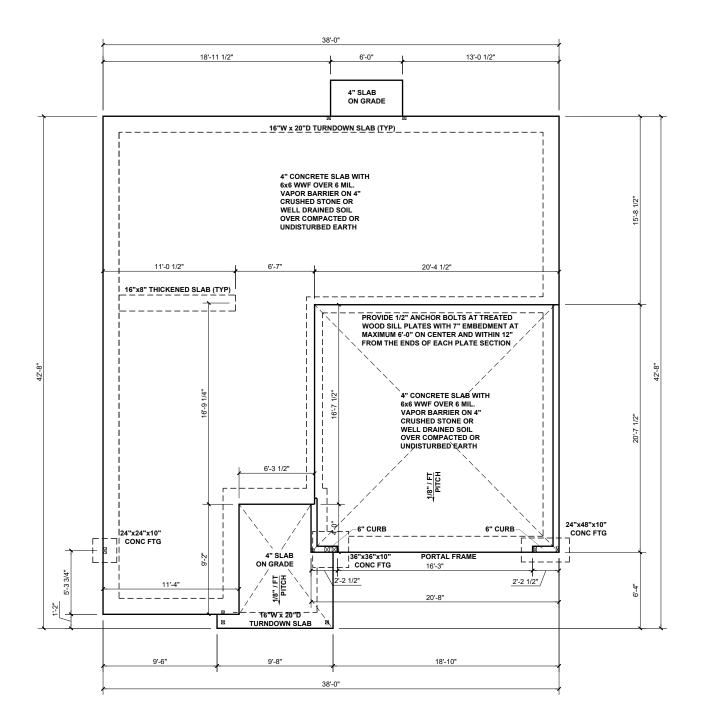
- 1. PROVIDE 2x4 COLLAR TIES AT 48" OC AT UPPER THIRD OF RAFTERS, UNLESS NOTED OTHERWISE.
- 2. FUR RIDGES FOR FULL RAFTER CONTACT.
- 3. PROVIDE CONTINUOUS BLOCKING THROUGH STRUCTURE FOR ALL POINT LOADS.
- 4. DENOTES OVER-FRAMED AREA
- 5. MINIMUM 7/16" OSB ROOF SHEATHING
- PROVIDE 2x4 RAFTER TIES AT 16" OC AT 45° BETWEEN RAFTERS AND CEILING JOISTS. USE (4) 16d NAILS AT EACH CONNECTION. RAFTER TIES MAY BE SPACED AT 48" OC AT LOCATIONS WHERE NO KNEE WALLS ARE INSTALLED.
- 7. PROVIDE H2.5A (MINIMUM) OR EQUIVALENT AT EACH RAFTER-TO-TOP PLATE CONNECTION AT OVER-FRAMED AREAS, UNLESS NOTED OTHERWISE.
- 8. UPLIFT CONNECTION TO BE CARRIED THROUGH TO FLOOR SYSTEM.

BRICK VENEER LINTEL SCHEDULE			
SPAN	N STEEL ANGLE SIZE END BEARING LENGTH		
UP TO 42"	L3-1/2"x3-1/2"x1/4"	8" (MIN. @ EACH END)	
UP TO 72"	L6"x4"x5/16"* (LLV) 8" (MIN. @ EACH END)		
OVER 72" L6"x4"x5/16"* (LLV) ATTACH LINTEL w/ 1/2" THRU BOLT @ 12" OC, 3" FROM EACH END			

\* FOR QUEEN BRICK: LINTELS AT THIS CONDITION MAY BE 5"x3-1/2"x5/16"

NOTE: BRICK LINTELS AT SLOPED AREAS TO BE 4"x3-1/2"x1/4" STEEL ANGLE WITH 16D NAILS IN 3/16" HOLES IN 4" ANGLE LEG AT 12" OC TO TRIPLE RAFTER. WHEN THE SLOPE EXCEEDS 4:12 A MINIMUM OF 3"x3"x1/4" PLATES SHALL BE WELDED AT 24" OC ALONG THE STEEL ANGLE.





**SLAB FOUNDATION PLAN - 'A'** 

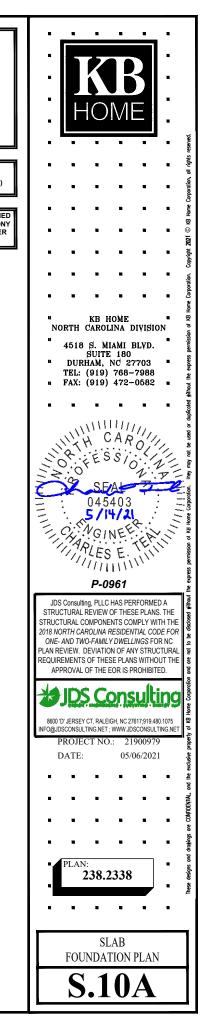
SCALE: 1/8"=1'-0"

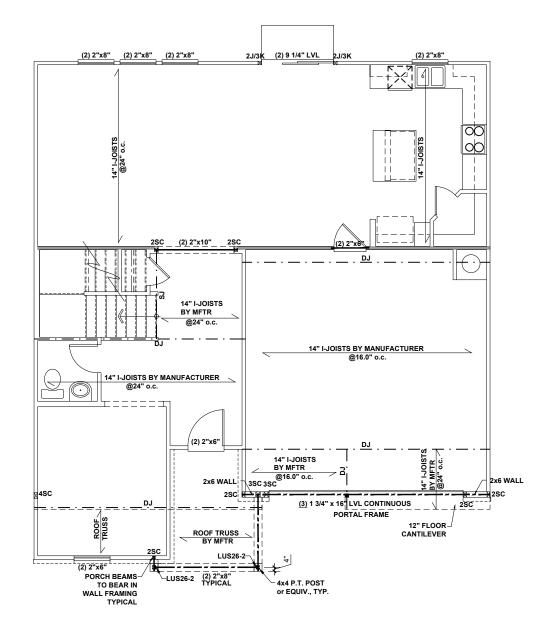
#### BEAM & POINT LOAD LEGEND

INTERIOR LOAD BEARING WALL
 ROOF RAFTER / TRUSS SUPPORT
 DOUBLE RAFTER / DOUBLE JOIST
 STRUCTURAL BEAM / GIRDER
 WINDOW / DOOR HEADER
POINT LOAD TRANSFER
POINT LOAD FROM ABOVE BEARING ON BEAM / GIRDER

(1) #5 REBAR @ CENTER OF ALL PERIMETER AND INTERNAL LOAD BEARING FOOTINGS. (2" C.C. MIN)

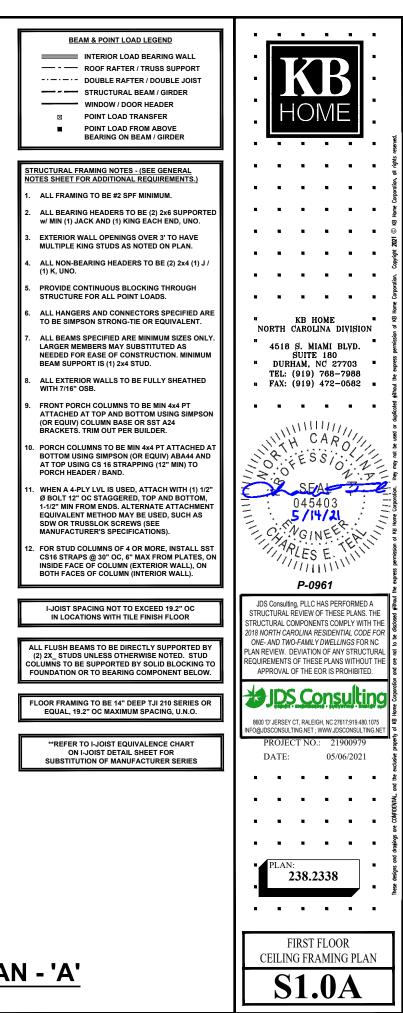
ALL CONCRETE CURBS SUPPORTING PORTAL FRAMED OR ENGINEERED OPENINGS IN GARAGES WITH A PONY WALL OVER 24" ABOVE THE GARAGE DOOR HEADER SHALL BE REQUIRED TO BE AT LEAST 8" WIDE.

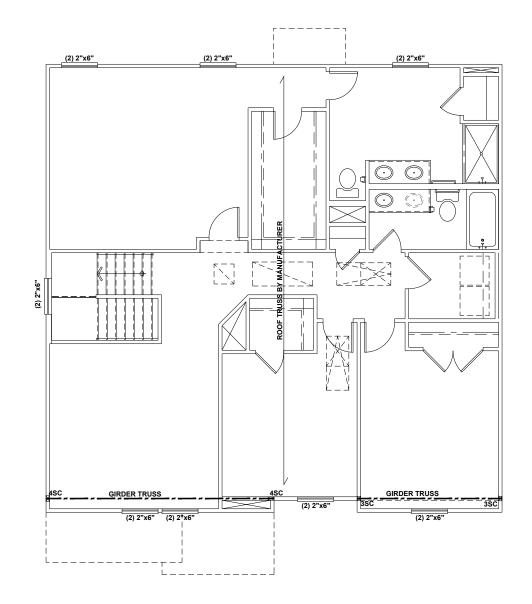




**FIRST FLOOR CEILING FRAMING PLAN - 'A'** 

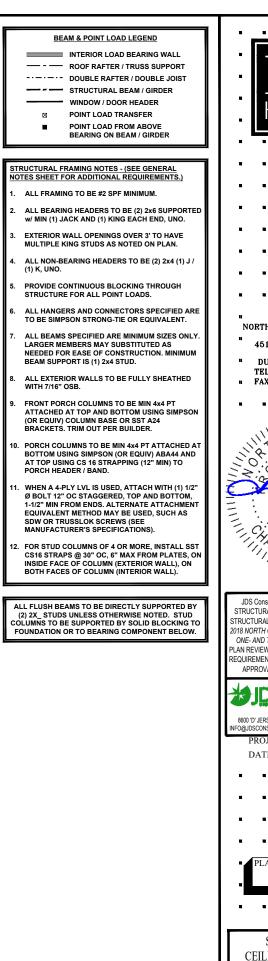
SCALE: 1/8"=1'-0"

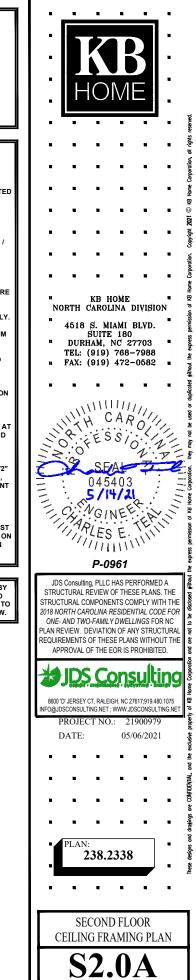


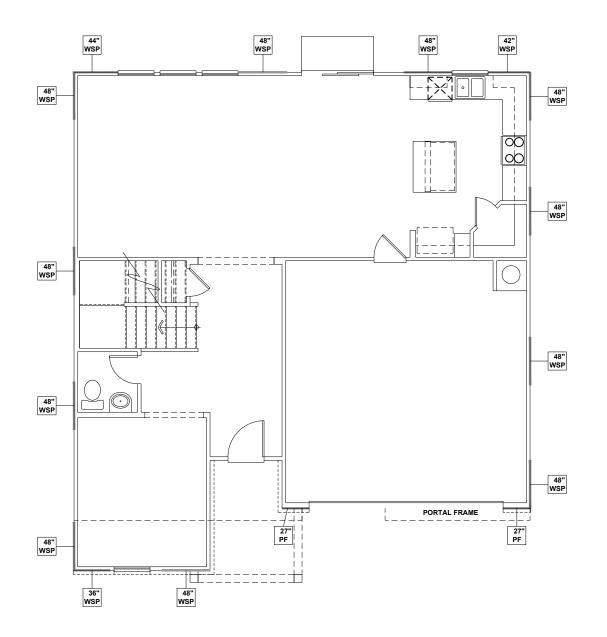


**SECOND FLOOR CEILING FRAMING PLAN - 'A'** 

SCALE: 1/8"=1'-0"

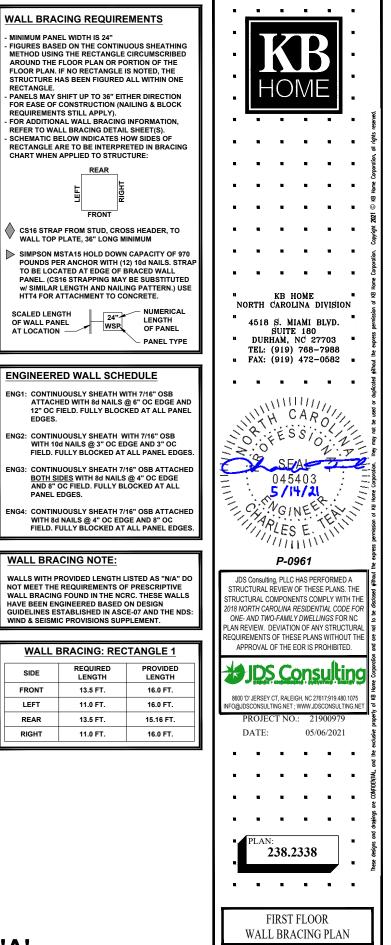




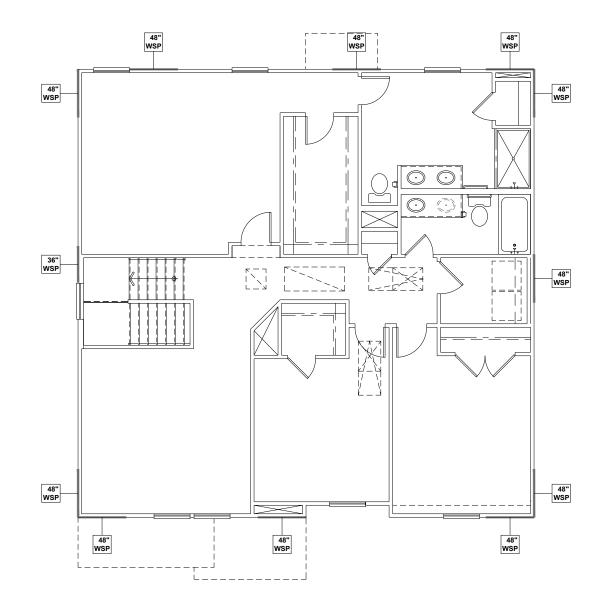


**FIRST FLOOR WALL BRACING PLAN - 'A'** 

SCALE: 1/8"=1'-0"

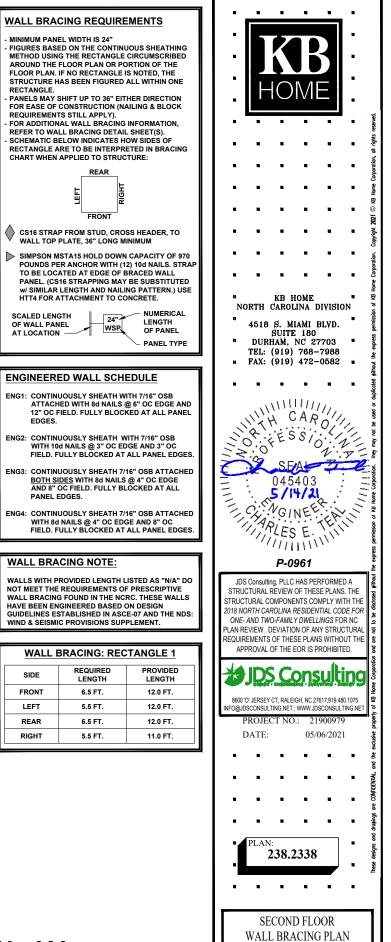


**S4.0**A

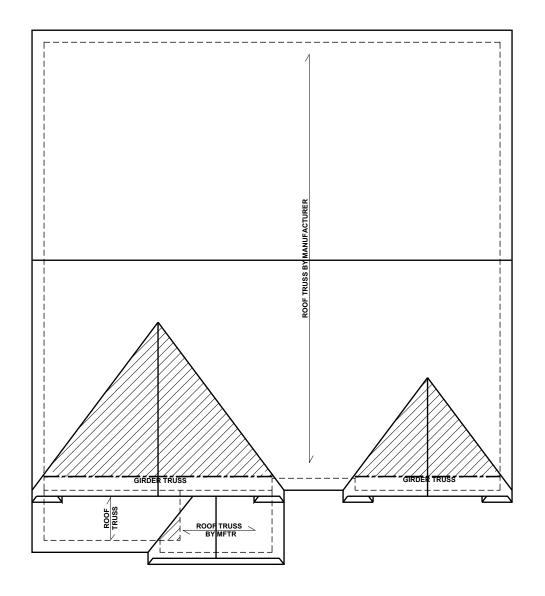


SECOND FLOOR WALL BRACING PLAN - 'A'

SCALE: 1/8"=1'-0"



**S5.0A** 



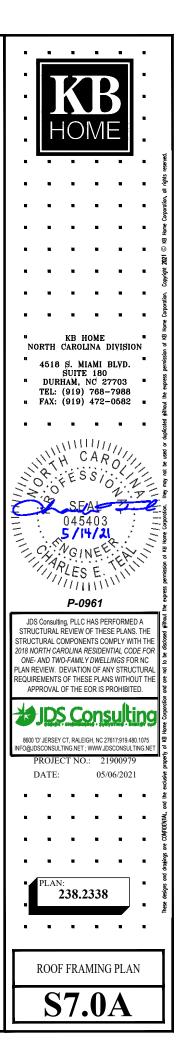
**ROOF FRAMING PLAN - 'A'** 

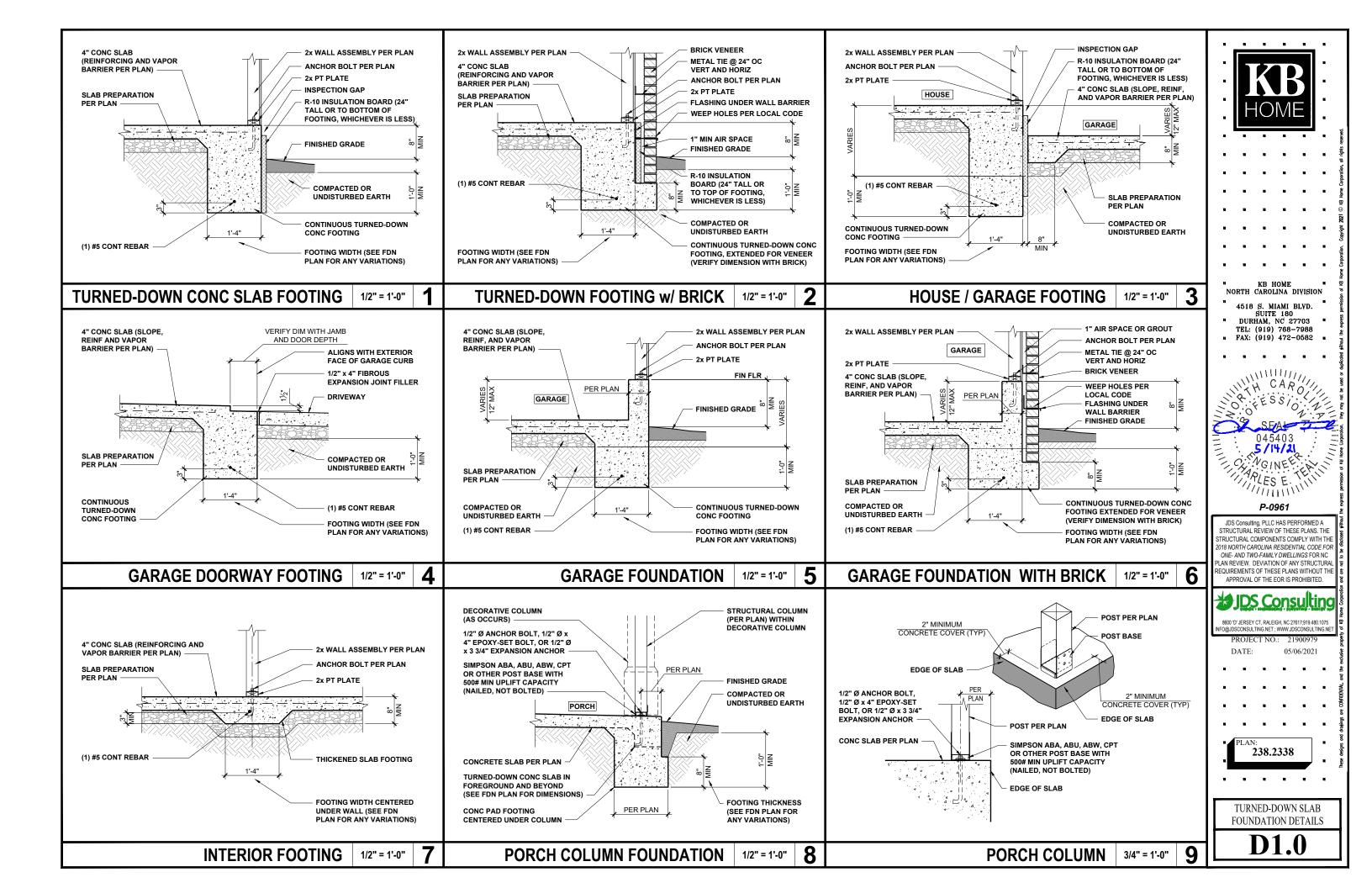
SCALE: 1/8"=1'-0"

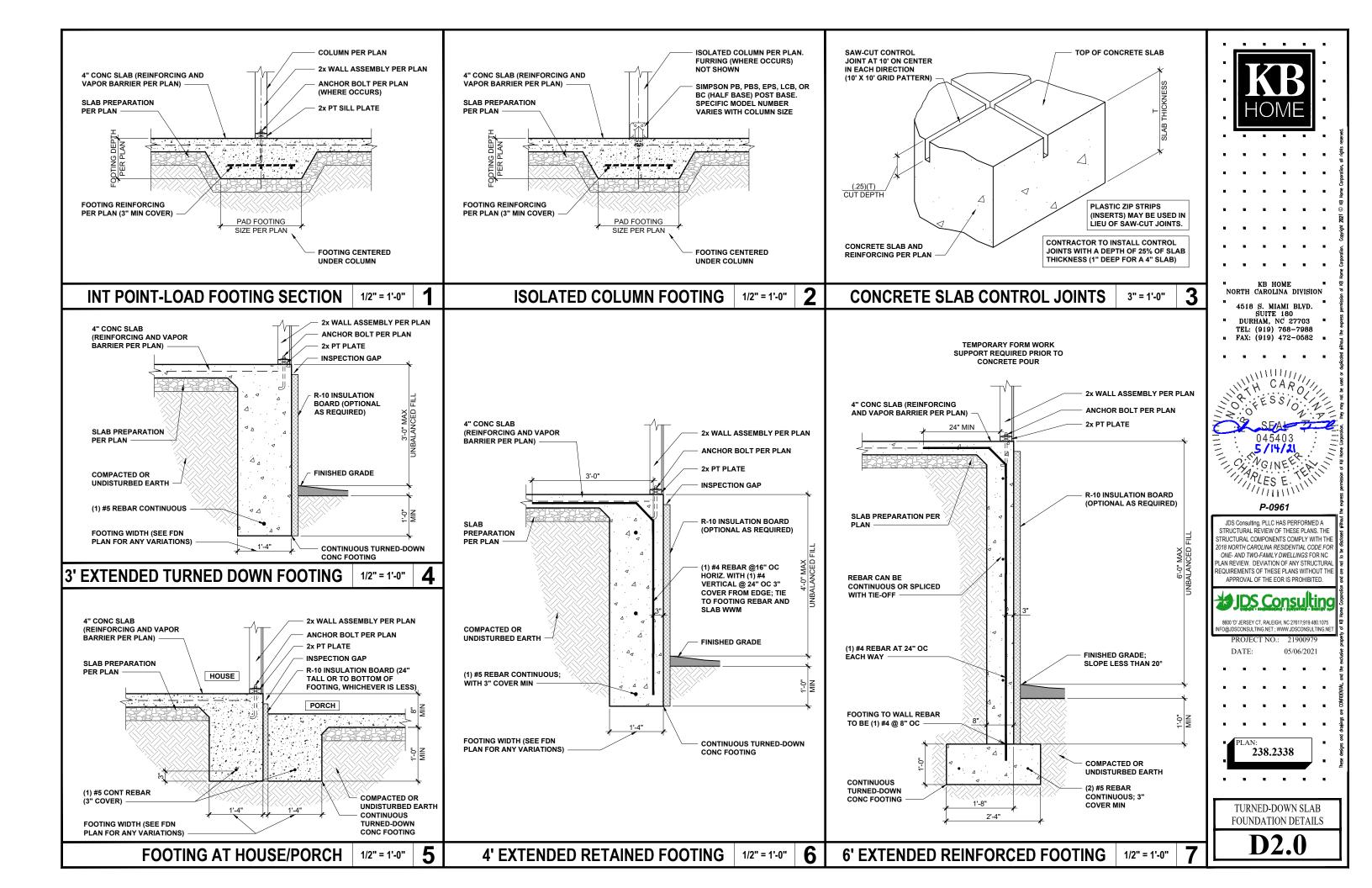
#### BEAM & POINT LOAD LEGEND

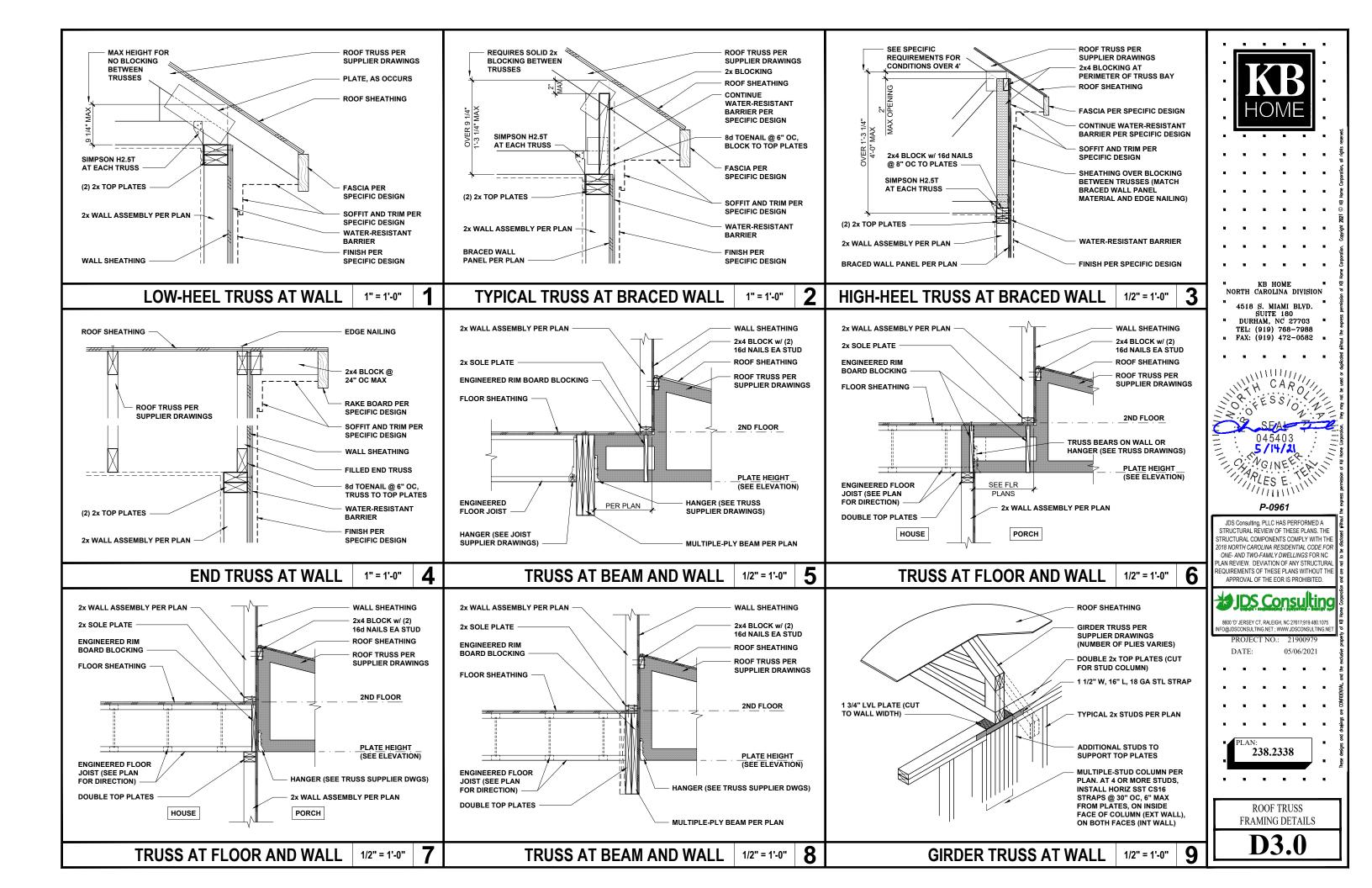
# X

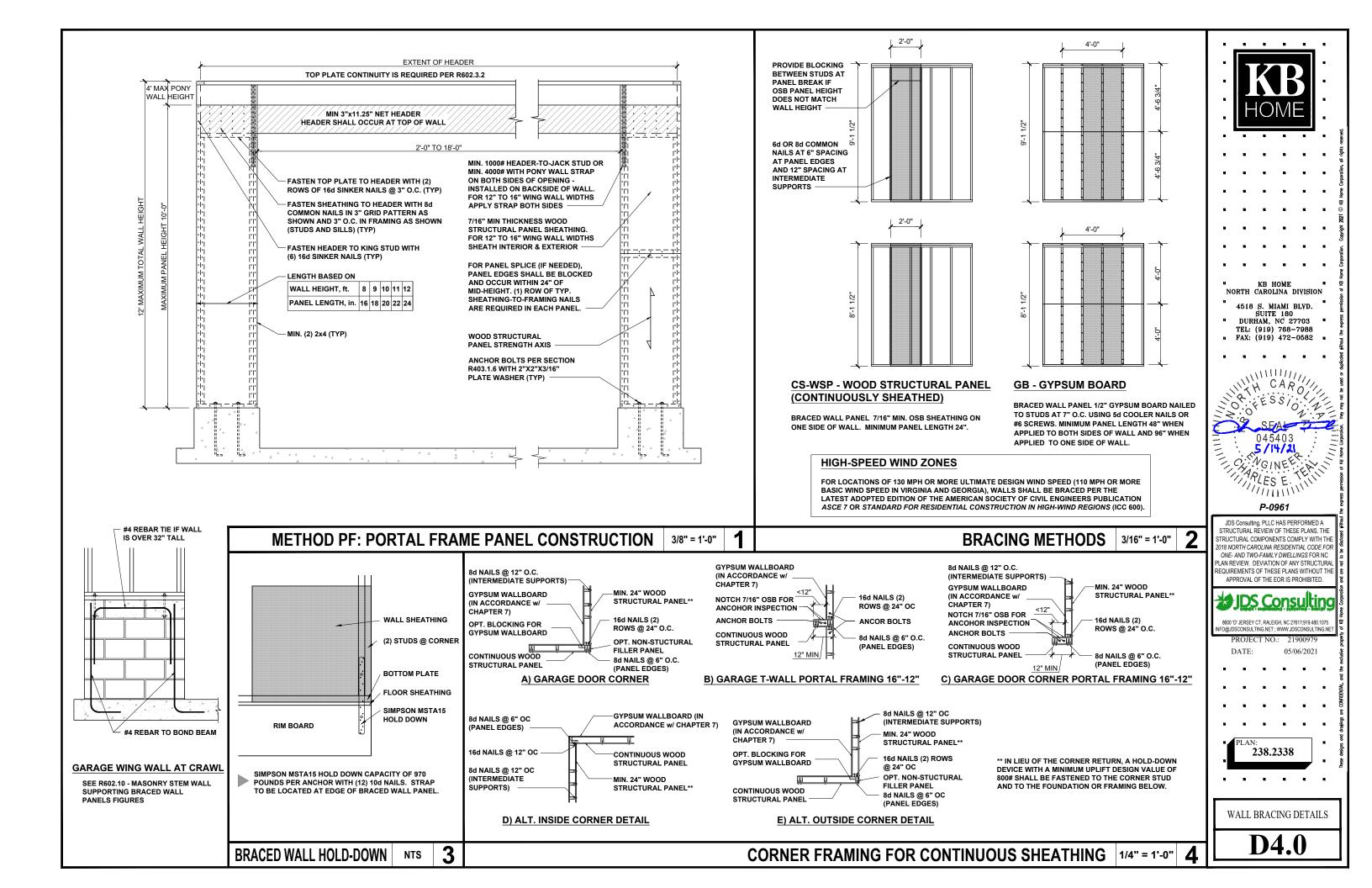
INTERIOR LOAD BEARING WALL ----- ROOF RAFTER / TRUSS SUPPORT ----- DOUBLE RAFTER / DOUBLE JOIST - STRUCTURAL BEAM / GIRDER WINDOW / DOOR HEADER POINT LOAD TRANSFER POINT LOAD FROM ABOVE BEARING ON BEAM / GIRDER

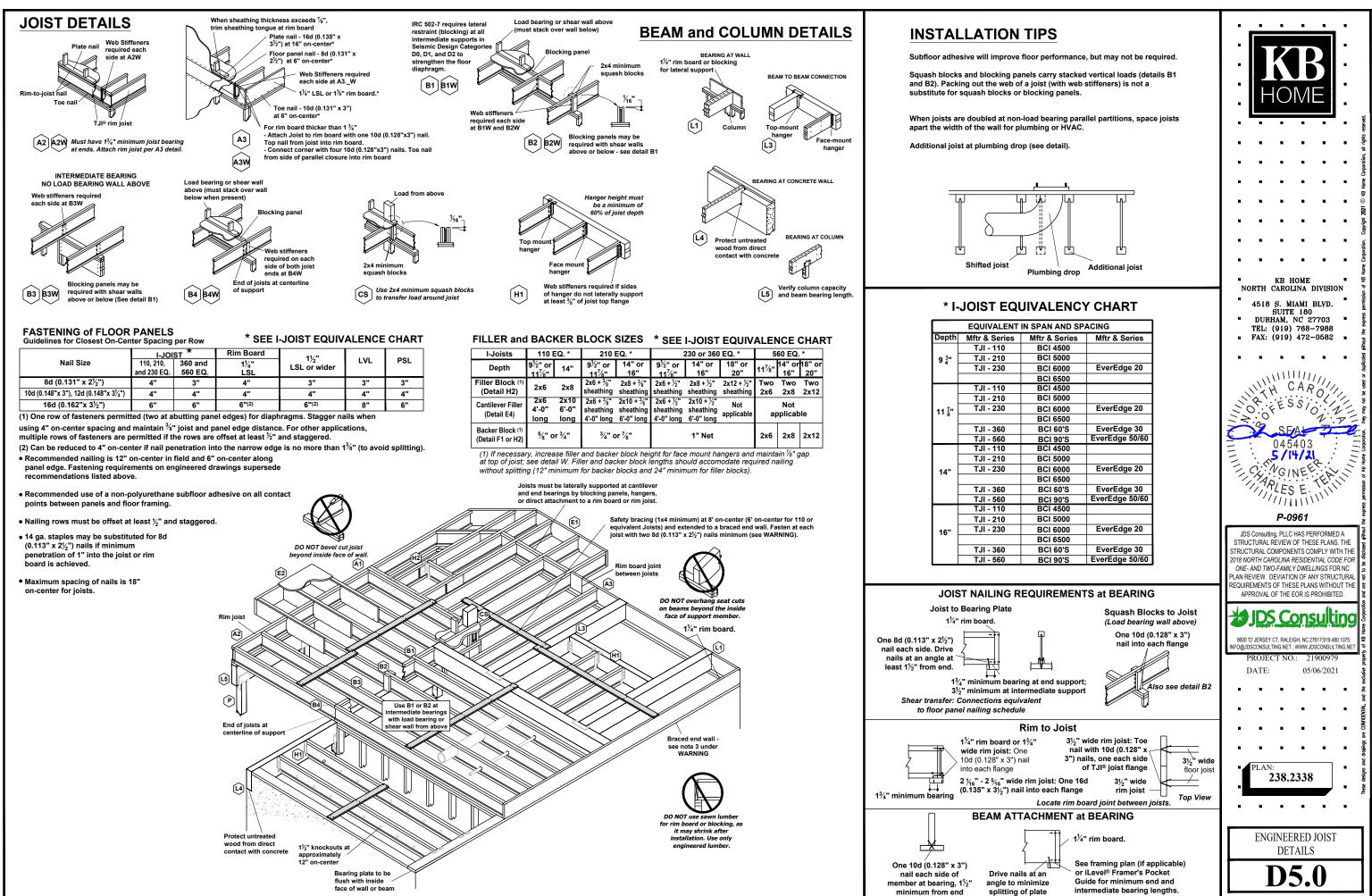


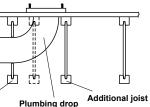












NT	NT IN SPAN AND SPACING				
s	Mftr & Series	Mftr & Series			
	BCI 4500				
	BCI 5000				
	BCI 6000	EverEdge 20			
	BCI 6500				
	BCI 4500				
	BCI 5000				
	BCI 6000	EverEdge 20			
	BCI 6500				
	BCI 60'S	EverEdge 30			
	BCI 90'S	EverEdge 50/60			
	BCI 4500				
	BCI 5000				
	BCI 6000	EverEdge 20			
	BCI 6500				
	BCI 60'S	EverEdge 30			
	BCI 90'S	EverEdge 50/60			
	BCI 4500				
	BCI 5000				
	BCI 6000	EverEdge 20			
	BCI 6500				
	BCI 60'S	EverEdge 30			
	BCI 90'S	EverEdge 50/60			