

Trenco 818 Soundside Rd Edenton, NC 27932

Re: B0419-1852 Roosevelt A

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E12919100 thru E12919130

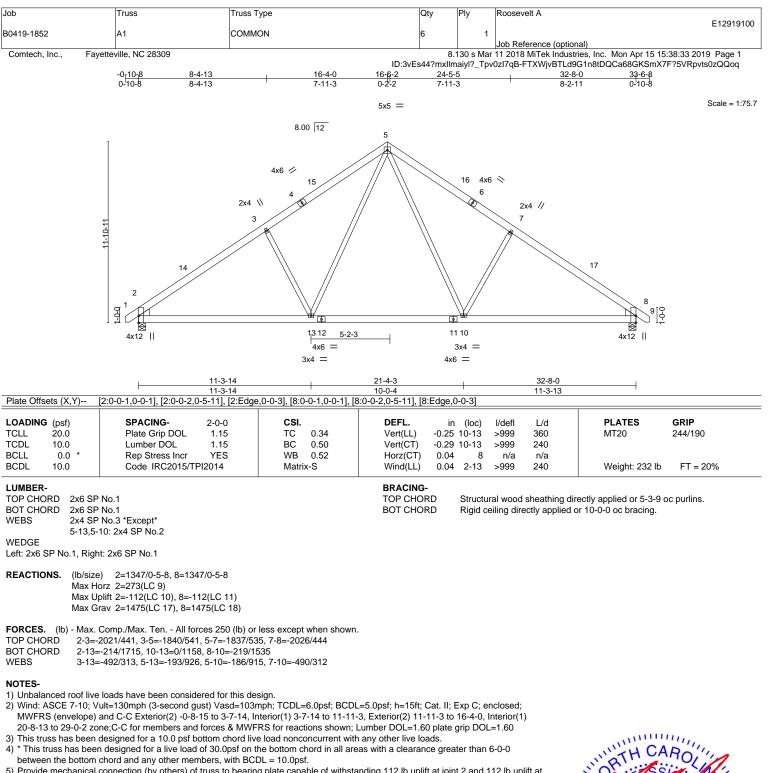
My license renewal date for the state of North Carolina is December 31, 2019.

North Carolina COA: C-0844



Lassiter, Frank

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.



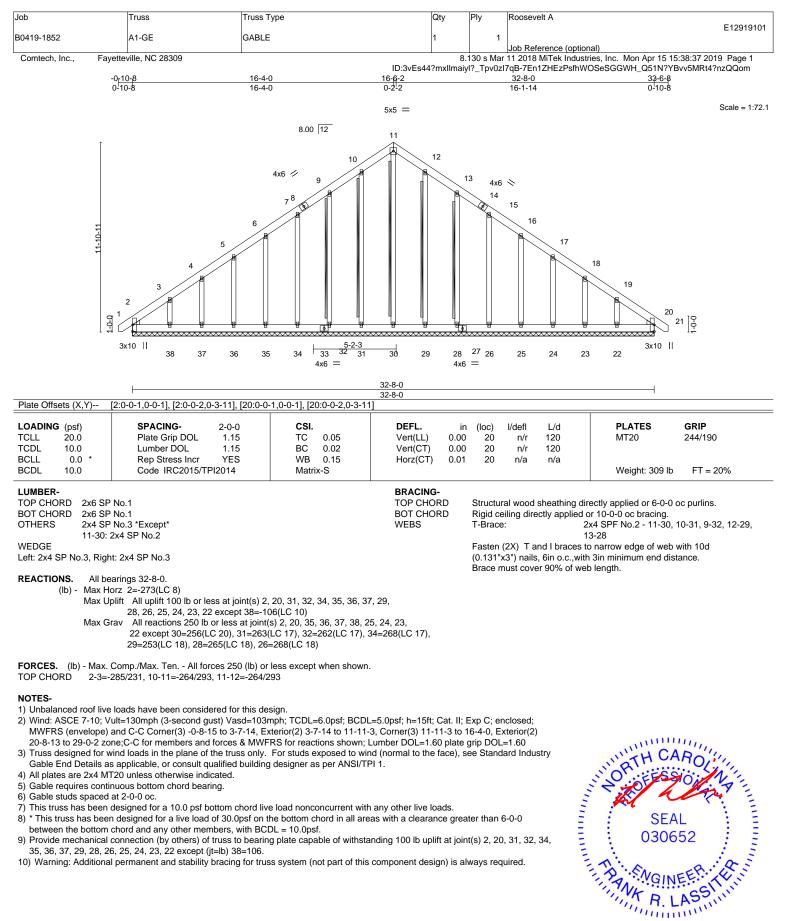
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 112 lb uplift at joint 2 and 112 lb uplift at joint 8.



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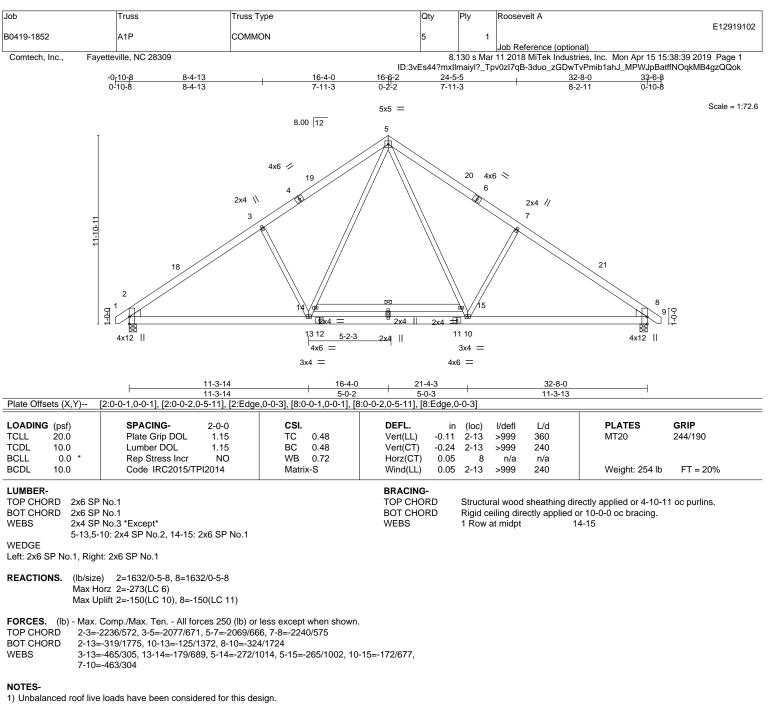




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2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-15 to 3-7-14, Interior(1) 3-7-14 to 11-11-3, Exterior(2) 11-11-3 to 16-4-0, Interior(1) 20-8-13 to 29-0-2 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 8=150.

LOAD CASE(S) Standard

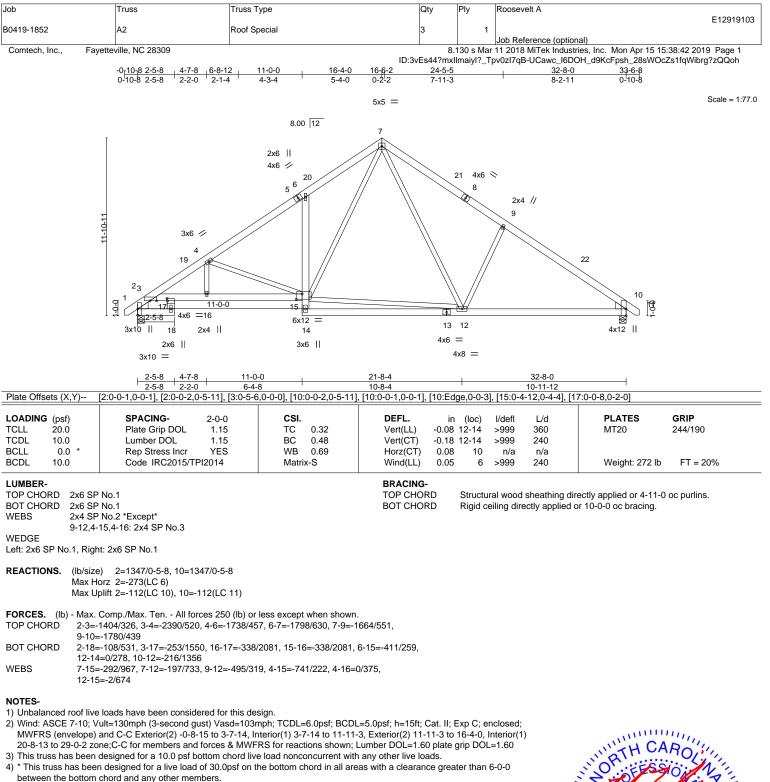
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-5=-60, 5-9=-60, 2-8=-20, 14-15=-60



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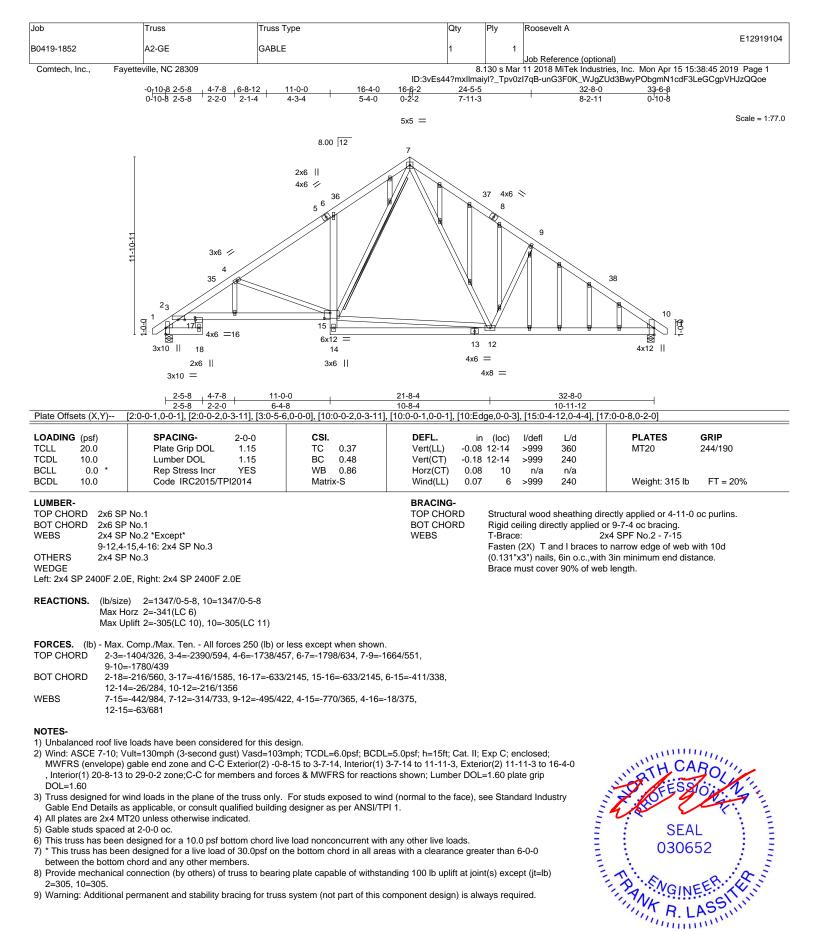


5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=112, 10=112.



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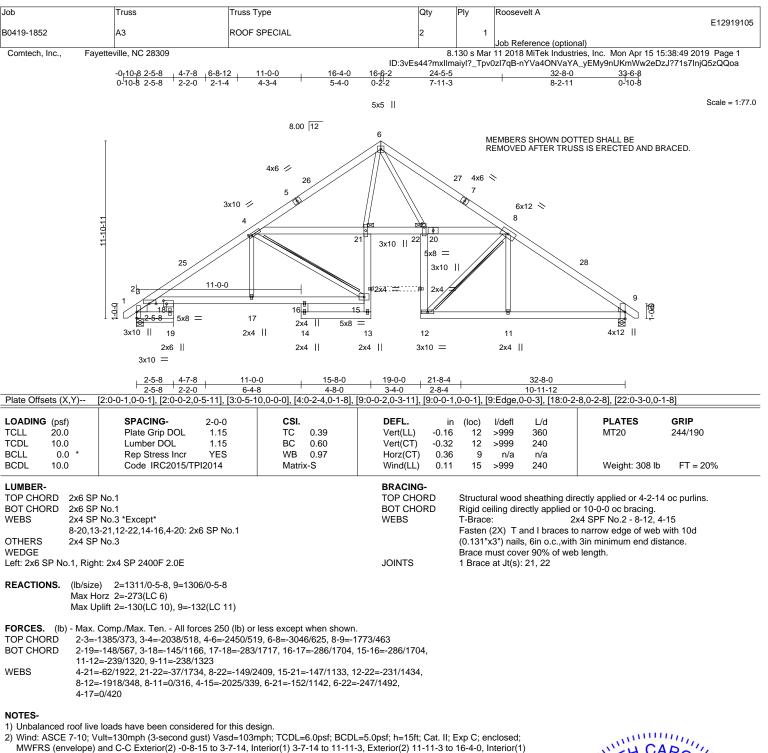




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20-8-13 to 29-0-2 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=130, 9=132.

6) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



April 16,2019

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	Truss	Truss Type		Qty	Ply	Roosevelt A		F1001010
0419-1852	B1	ROOF SPECIAL		2	1			E12919106
					100 11	Job Reference (opti		00 50 0040 D 4
Comtech, Inc., Faye	tteville, NC 28309		ID:3vEs4				stries, Inc. Mon Apr 15 15 QRsgjOdZGwM5m?qOpx	
	-0 <u>-11-0 2-5-0 6-7-4</u>			23-0-	3	31	-0-8	
	0-11-0 2-5-0 4-2-4	4-4-4 4-2-5	·	7-10-	6	. 8-	0-5	
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	2x4	2x4	8x8 =	3X4	-			
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$					31-0-8		
Plate Offsets (X,Y) [2		<u>4-4-4</u> <u>4-2-5</u> , [11:0-4-0,0-4-8], [16:0-0-8,0-2-0]	3-11	-1		11-11-4		
OADING (psf)	SPACING- 2-0-	0 CSI .	DEFL.	ir	n (loc)	l/defl L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.1	5 TC 0.31	Vert(LL)	-0.12		>999 360	MT20	244/190
CDL 10.0	Lumber DOL 1.1		Vert(CT)	-0.24		>599 240		
CLL 0.0 *	Rep Stress Incr YE Code IRC2015/TPI2014		Horz(CT) Wind(LL)			n/a n/a >999 240	Weight: 238 lb	FT = 20%
.UMBER- OP CHORD 2x6 SP N	lo 1		BRACING TOP CHO		Structur	al wood sheathing o	lirectly applied or 6-0-0 c	oc purlins
OT CHORD 2x6 SP N			BOT CHC				l or 10-0-0 oc bracing, I	
2101010 210 381	lo.3 *Except*					bracing: 9-10.	-	
VEBS 2x4 SP M			WEBS		1 Row a		6-11	13
VEBS 2x4 SP N 6-10,6-1	1: 2x4 SP No.2				I-Brace	•		
VEBS 2x4 SP N 6-10,6-1 VEDGE						(2X) T and I braces	2x4 SPF No.2 - 6-10, 4- to narrow edge of web v	with 10d
VEBS 2x4 SP N 6-10,6-1 VEDGE					Fasten (0.131")	(2X) T and I braces (3") nails, 6in o.c.,wi	to narrow edge of web with 3in minimum end dist	with 10d
VEBS 2x4 SP N 6-10,6-1 VEDGE eft: 2x4 SP No.3	1: 2x4 SP No.2	8. 9=304/0-3-0			Fasten (0.131")	(2X) T and I braces	to narrow edge of web with 3in minimum end dist	with 10d
VEBS 2x4 SP N 6-10,6-1 VEDGE eft: 2x4 SP No.3 EACTIONS. (Ib/size) Max Hor	1: 2x4 SP No.2 2=717/0-3-8, 10=1497/0-3- z 2=-255(LC 6)				Fasten (0.131")	(2X) T and I braces (3") nails, 6in o.c.,wi	to narrow edge of web with 3in minimum end dist	with 10d
VEBS 2x4 SP N 6-10,6-1 VEDGE eft: 2x4 SP No.3 EACTIONS. (Ib/size) Max Hor Max Upl	1: 2x4 SP No.2 2=717/0-3-8, 10=1497/0-3- z 2=-255(LC 6) ift 2=-72(LC 10), 10=-88(LC 1	0), 9=-85(LC 11)			Fasten (0.131")	(2X) T and I braces (3") nails, 6in o.c.,wi	to narrow edge of web with 3in minimum end dist	with 10d
VEBS 2x4 SP N 6-10,6-1 VEDGE eft: 2x4 SP No.3 REACTIONS. (Ib/size) Max Hor Max Upl	1: 2x4 SP No.2 2=717/0-3-8, 10=1497/0-3- z 2=-255(LC 6)	0), 9=-85(LC 11)			Fasten (0.131")	(2X) T and I braces (3") nails, 6in o.c.,wi	to narrow edge of web with 3in minimum end dist	with 10d
VEBS 2x4 SP N 6-10,6-1 VEDGE Left: 2x4 SP No.3 REACTIONS. (Ib/size) Max Hot Max Upl Max Gra FORCES. (Ib) - Max. C	2=717/0-3-8, 10=1497/0-3- z 2=-255(LC 6) ift 2=-72(LC 10), 10=-88(LC 1) v 2=717(LC 1), 10=1569(LC omp./Max. Ten All forces 25	0), 9=-85(LC 11) 17), 9=357(LC 22) 50 (lb) or less except when shown.			Fasten (0.131")	(2X) T and I braces (3") nails, 6in o.c.,wi	to narrow edge of web with 3in minimum end dist	with 10d
VEBS 2x4 SP N 6-10,6-1 VEDGE eft: 2x4 SP No.3 EEACTIONS. (Ib/size) Max Hon Max Upl Max Gra CORCES. (Ib) - Max. C OP CHORD 2-3=-7	1: 2x4 SP No.2 2=717/0-3-8, 10=1497/0-3- z 2=-255(LC 6) ift 2=-72(LC 10), 10=-88(LC 1 v 2=717(LC 1), 10=1569(LC omp./Max. Ten All forces 2! 24/206, 3-4=-1000/263, 4-6=-	0), 9=-85(LC 11) 17), 9=357(LC 22) 50 (lb) or less except when shown. 346/229, 6-8=-34/405	236/800		Fasten (0.131")	(2X) T and I braces (3") nails, 6in o.c.,wi	to narrow edge of web with 3in minimum end dist	with 10d
VEBS 2x4 SP N 6-10,6-1 VEDGE REACTIONS. (Ib/size) Max Hon Max Upl Max Gra FORCES. (Ib) - Max. C FOP CHORD 2-3=-7 30T CHORD 2-17=-	1: 2x4 SP No.2 2=717/0-3-8, 10=1497/0-3- z 2=-255(LC 6) ift 2=-72(LC 10), 10=-88(LC 1 v 2=717(LC 1), 10=1569(LC omp./Max. Ten All forces 24 24/206, 3-4=-1000/263, 4-6=- 121/365, 3-16=-45/598, 15-16	0), 9=-85(LC 11) 17), 9=357(LC 22) 50 (lb) or less except when shown.			Fasten (0.131")	(2X) T and I braces (3") nails, 6in o.c.,wi	to narrow edge of web with 3in minimum end dist	with 10d
VEBS 2x4 SP N 6-10,6-1 VEDGE Left: 2x4 SP No.3 REACTIONS. (Ib/size) Max Hon Max Upl Max Graves FORCES. (Ib) - Max. C TOP CHORD 2-3=-7 30T CHORD 2-17=- VEBS 6-10=-	1: 2x4 SP No.2 2=717/0-3-8, 10=1497/0-3- z 2=-255(LC 6) ift 2=-72(LC 10), 10=-88(LC 1 v 2=717(LC 1), 10=1569(LC omp./Max. Ten All forces 24 24/206, 3-4=-1000/263, 4-6=- 121/365, 3-16=-45/598, 15-16	0), 9=-85(LC 11) 17), 9=357(LC 22) 50 (lb) or less except when shown. 346/229, 6-8=-34/405 =-162/963, 14-15=-162/963, 13-14=			Fasten (0.131")	(2X) T and I braces (3") nails, 6in o.c.,wi	to narrow edge of web with 3in minimum end dist	with 10d
VEBS 2x4 SP N 6-10,6-1 VEDGE Left: 2x4 SP No.3 REACTIONS. (Ib/size) Max Hon Max Upl Max Gra FORCES. (Ib) - Max. C OP CHORD 2-3=-7: SOT CHORD 2-17=- VEBS 6-10=- IOTES-	1: 2x4 SP No.2 2=717/0-3-8, 10=1497/0-3- z 2=-255(LC 6) ift 2=-72(LC 10), 10=-88(LC 1 v 2=717(LC 1), 10=1569(LC omp./Max. Ten All forces 24 24/206, 3-4=-1000/263, 4-6=- 121/365, 3-16=-45/598, 15-16	0), 9=-85(LC 11) 17), 9=357(LC 22) 50 (lb) or less except when shown. 346/229, 6-8=-34/405 =-162/963, 14-15=-162/963, 13-14= 3=-90/467, 4-13=-861/273, 4-15=0/			Fasten (0.131")	(2X) T and I braces (3") nails, 6in o.c.,wi	to narrow edge of web with 3in minimum end dist	with 10d
VEBS 2x4 SP N 6-10,6-1 VEDGE eft: 2x4 SP No.3 EACTIONS. (Ib/size) Max Hon Max Upl Max Upl Max Gra ORCES. (Ib) - Max. C OP CHORD 2-3=-7 OT CHORD 2-3=-7 OT CHORD 2-17=- VEBS 6-10=- OTES-) Unbalanced roof live I) Wind: ASCE 7-10; Vu	1: 2x4 SP No.2 2=717/0-3-8, 10=1497/0-3- z 2=-255(LC 6) ift 2=-72(LC 10), 10=-88(LC 1 v 2=717(LC 1), 10=1569(LC comp./Max. Ten All forces 21 24/206, 3-4=-1000/263, 4-6=- 121/365, 3-16=-45/598, 15-16 1040/133, 8-10=-564/329, 6-1 bads have been considered for t=130mph (3-second gust) Va	0), 9=-85(LC 11) 17), 9=357(LC 22) 50 (lb) or less except when shown. 346/229, 6-8=-34/405 =-162/963, 14-15=-162/963, 13-14= 3=-90/467, 4-13=-861/273, 4-15=0/ or this design. asd=103mph; TCDL=6.0psf; BCDL=	402 5.0psf; h=15ft		Fasten (0.131") Brace n Exp C; er	(2X) T and I braces (3") nails, 6in o.c.,wi lust cover 90% of w not solved;	to narrow edge of web with 3in minimum end dist	with 10d
VEBS 2x4 SP N 6-10,6-1 VEDGE eft: 2x4 SP No.3 EACTIONS. (Ib/size) Max Hor Max Upl Max Gr OP CHORD 2-17 VEBS 6-10=- IOTES-) Unbalanced roof live It) Wind: ASCE 7-10; Vu MWFRS (envelope) a	1: 2x4 SP No.2 2=717/0-3-8, 10=1497/0-3- z 2=-255(LC 6) ift 2=-72(LC 10), 10=-88(LC 1 v 2=717(LC 1), 10=1569(LC omp./Max. Ten All forces 2: 24/206, 3-4=-1000/263, 4-6=- 121/365, 3-16=-45/598, 15-16 1040/133, 8-10=-564/329, 6-1 bads have been considered for t=130mph (3-second gust) Vand C-C Exterior(2) -0-8-15 to 5	0), 9=-85(LC 11) 17), 9=357(LC 22) 50 (lb) or less except when shown. 846/229, 6-8=-34/405 =-162/963, 14-15=-162/963, 13-14= 3=-90/467, 4-13=-861/273, 4-15=0/ or this design.	402 5.0psf; h=15ft Exterior(2) 10-	9-8 to 1	Fasten (0.131"; Brace n Exp C; er 5-2-5, Inte	(2X) T and I braces (3") nails, 6in o.c.,wi lust cover 90% of w not solved;	to narrow edge of web with 3in minimum end dist	with 10d ance.

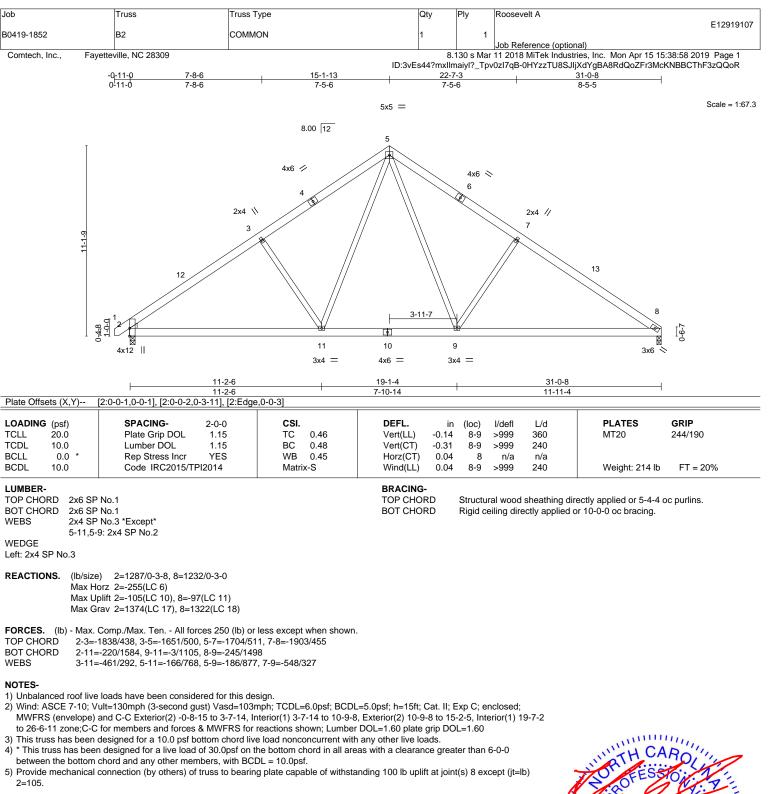
a) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
b) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 9.
b) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



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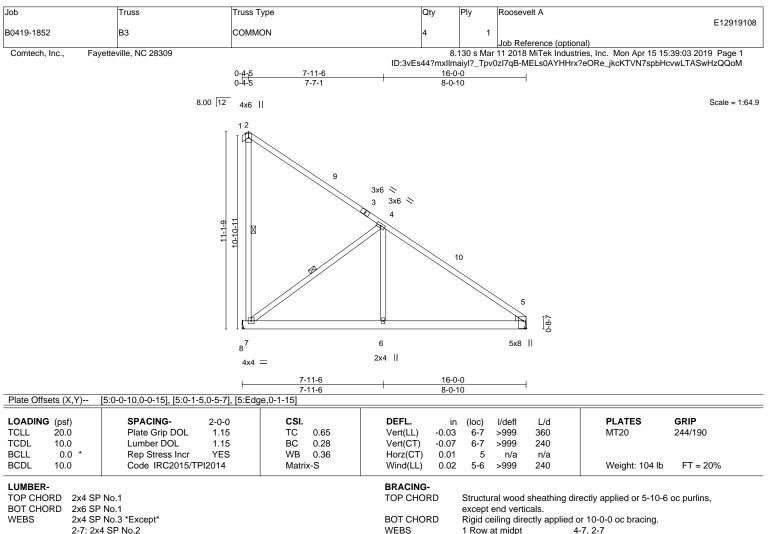




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A MiTek A 818 Soundside Road Edenton, NC 27932



2-7: 2x4 SP No.2

WEDGE Right: 2x4 SP No.3

(lb/size) 8=639/Mechanical, 5=635/Mechanical REACTIONS. Max Horz 8=-346(LC 11) Max Uplift 8=-195(LC 11) Max Grav 8=680(LC 18), 5=635(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 4-5=-794/0

BOT CHORD 7-8=-404/428, 6-7=0/551, 5-6=0/551

WEBS 4-7=-807/319, 4-6=0/411

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-9-2, Interior(1) 4-9-2 to 11-6-7, Exterior(2) 11-6-7 to 15-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

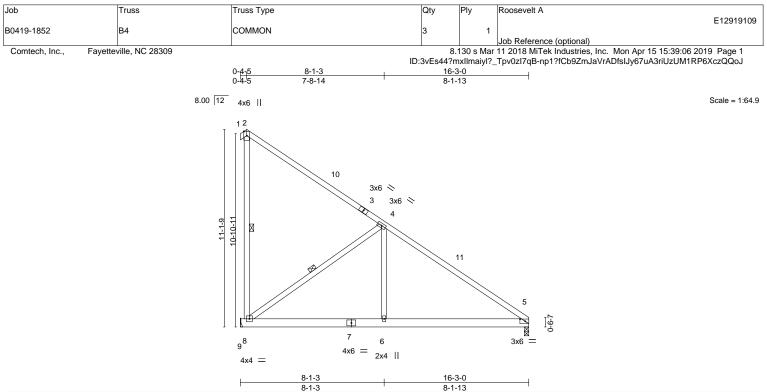
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=195



April 16,2019





OADING (psf) SP	ACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
CLL 20.0 Pla	te Grip DOL 1.15	TC 0.66	Vert(LL)	-0.03	6-8	>999	360	MT20	244/190	
CDL 10.0 Lui	mber DOL 1.15	BC 0.26	Vert(CT)	-0.08	6-8	>999	240			
CLL 0.0 * Re	p Stress Incr YES	WB 0.38	Horz(CT)	0.01	5	n/a	n/a			
CDL 10.0 Co	de IRC2015/TPI2014	Matrix-S	Wind(LL)	0.02	5-6	>999	240	Weight: 104 lb	FT = 20%	
UMBER- OP CHORD 2x4 SP No.1			BRACING- TOP CHOR	П	Structu	ural wood	sheathing di	rectly applied or 5-9-2 of	o purlins	
OT CHORD 2x6 SP No.1			TOP CHOR	D		end verti	0	rectly applied of 5-9-2 C	je puriiris,	
VEBS 2x4 SP No.3 *Ex	cept*		BOT CHOR	BOT CHORD Rigid ceilin			ligid ceiling directly applied or 10-0-0 oc bracing.			
2-8: 2x4 SP No.2	2		WEBS		1 Row	at midpt	4	4-8, 2-8		
EACTIONS. (Ib/size) 9=64	6/Mechanical, 5=642/0-3-0									
Max Horz 9=-34										
Max Uplift 9=-19	()									
	7(LC 18), 5=642(LC 1)									

TOP CHORD 4-5=-815/0

BOT CHORD

8-9=-403/427, 6-8=0/574, 5-6=0/574

WEBS 4-8=-823/318, 4-6=0/415

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-9-2, Interior(1) 4-9-2 to 11-8-11, Exterior(2) 11-8-11 to 16-1-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

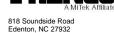
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=194.



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Job	Truss	Truss Type	Qty	Ply	Roosevelt A
					E12919110
B0419-1852	B5-GE	COMMON SUPPORTED GAB	1	1	leb Deference (entionel)
Comtech. Inc., Favette	/ille. NC 28309		8		Job Reference (optional) 11 2018 MiTek Industries, Inc. Mon Apr 15 15:39:08 2019 Page 1

ID:3vEs44?mxllmaiyl?_Tpv0zI7qB-jC8l4tcQ5NZlk9JbmHKn1XDMitbtyvRfUluDbUzQQoH 16-3-0 0-4-5 0-4-5 15-10-11 8.00 12 4x4 = Scale = 1:68.0 12 倵 3 3x4 📎 5 6 10-10-11 11-1-9 10 11 [4] 3x4 20 18 19 17 16 15 14 13 12 8x8 =

16-3-0 16-3-0

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (l	loc) l/de	fl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.15	Vert(LL)	n/a	- n/	a 999	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.02	Vert(CT)	n/a	- n/	a 999		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.22	Horz(CT)	0.02	19 n/	'a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 136 lb	FT = 20%
2-19: 2	No.1 No.3 *Except* 44 SP No.2		TOP CHOR BOT CHOR WEBS	D Ri T- Fa (0	igid ceiling -Brace: asten (2X)).131"x3") r	directly applied	irectly applied or 6-0-0 or 10-0-0 oc bracing. 2x4 SPF No.2 - 2-19, 3 to narrow edge of web h 3in minimum end dis ab length.	-18 with 10d
REACTIONS. ALLOS	arings 16-5-8.							
(lb) - Max Ho								

- Max Grav All reactions 250 lb or less at joint(s) 1, 11, 15, 14, 13, 12 except 19=364(LC 11), 18=266(LC 18), 17=262(LC 18), 16=271(LC 18)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 1-2=-375/478, 2-3=-442/549, 3-4=-404/505, 4-5=-338/421, 5-7=-283/350, 7-8=-224/275
- WEBS 2-19=-345/238

NOTES-

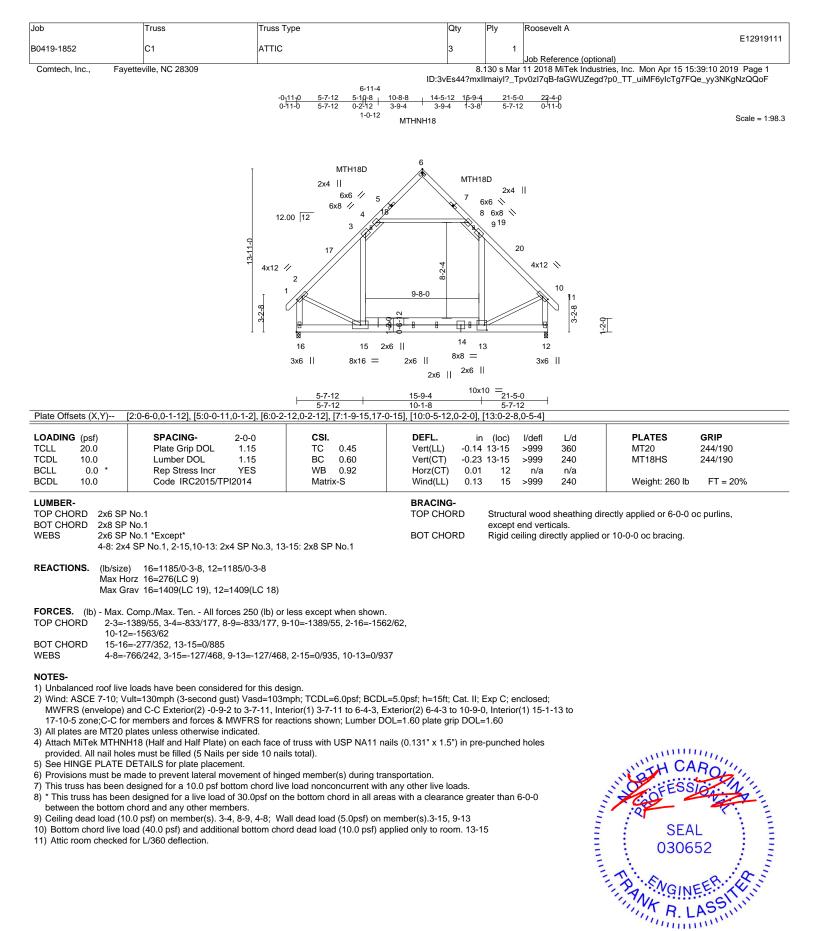
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-0-0 to 4-9-2, Exterior(2) 4-9-2 to 11-10-3, Corner(3) 11-10-3 to 16-3-0 zone: C C for members and forces & MWEPS for reactions shown: Lumber DOL=1.60 plots grip DOL=1.60
- zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 19, 18, 16, 15, 14, 13, 20 except (jt=lb) 1=114, 17=110, 12=101.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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April 16,2019



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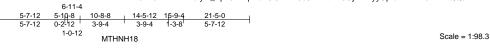
April 16,2019

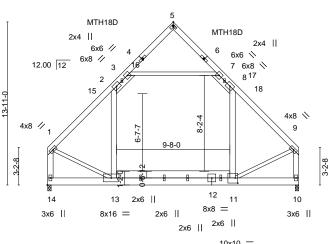
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Job	Truss	Truss Type	Qty	Ply	Roosevelt A
					E12919112
B0419-1852	C2	ATTIC	3	1	
					Job Reference (optional)
Comtech, Inc.,	Fayetteville, NC 28309		8.	130 s Mar	11 2018 MiTek Industries, Inc. Mon Apr 15 15:39:12 2019 Page 1

8.130 s Mar 11 2018 MiTek Industries, Inc. Mon Apr 15 15:39:12 2019 Page 1 ID:3vEs44?mxllmaiyl?_Tpv0zl7qB-bzOGvFfw9c3kDndN?60jBNNyyUpiuYTFPNsRkFzQQoD





	5-7-12	15-9-4	21-5-0	
	5-7-12	10-1-8	5-7-12	
Plate Offsets (X,Y) [4:0-0-11,0-1-2], [5:0-2-12,0-2-12], [6:1-9-1	15,17-0-15], [11:0			

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.45 BC 0.60 WB 0.92 Matrix-S	Vert(LL) -0.1	5 11-13 = = 3 11-13 = = 1 = 10	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 MT18HS Weight: 255 lb	GRIP 244/190 244/190 FT = 20%
		13: 2x8 SP No.1	BRACING- TOP CHORD BOT CHORD	except er	al wood sheathing dire nd verticals. ling directly applied o	ectly applied or 6-0-0 or 10-0-0 or 10-0-0 oc bracing.	oc purlins,
	e) 14=1123/0-3-8, 10=1123/0-3-8 orz 14=251(LC 7) rav 14=1368(LC 19), 10=1368(LC 18)						
TOP CHORD 1-2=-' 9-10= BOT CHORD 13-14	Comp./Max. Ten All forces 250 (lb) or 1385/31, 2-3=-835/173, 7-8=-835/173, 8 1521/12 =-278/315, 11-13=0/876	3-9=-1385/31, 1-14=-1521/	12,				
WEBS 3-7=-7	773/236, 2-13=-124/458, 8-11=-124/458	3, 1-13=0/952, 9-11=0/953					
 Unbalanced roof live Wind: ASCE 7-10; Vi MWFRS (envelope) a 	loads have been considered for this de ult=130mph (3-second gust) Vasd=103 and C-C Exterior(2) 0-3-4 to 4-8-1, Inter r members and forces & MWFRS for rea	mph; TCDL=6.0psf; BCDL: ior(1) 4-8-1 to 6-4-3, Exter	ior(2) 6-4-3 to 10-9-0,	nterior(1) 1			

3) All plates are MT20 plates unless otherwise indicated.

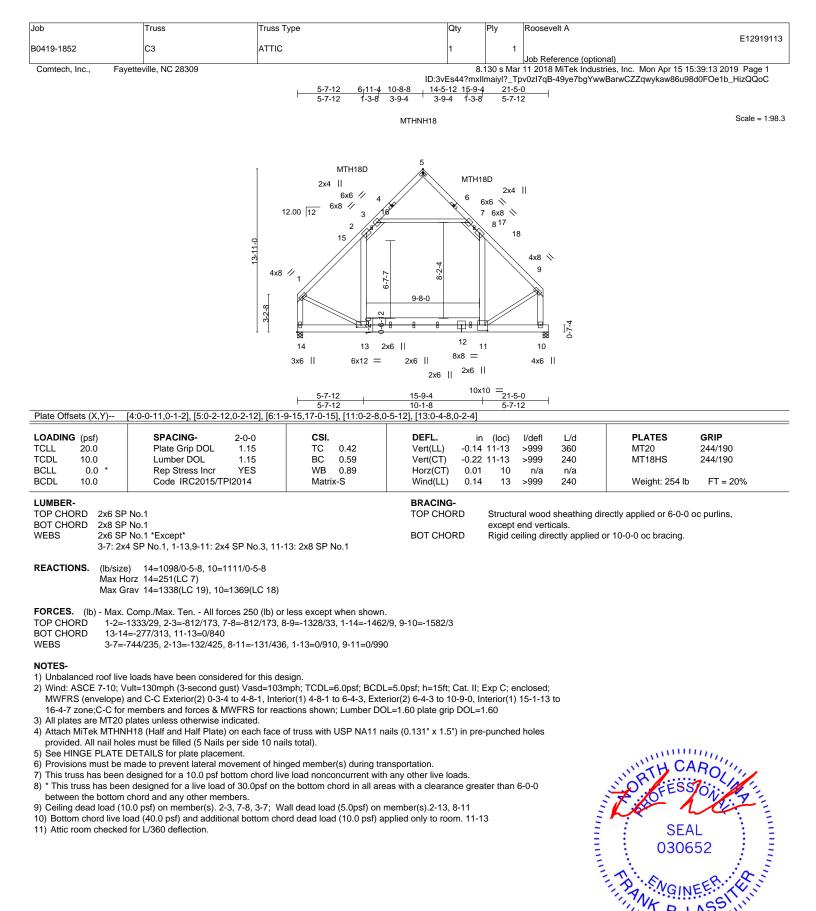
- 4) Attach MiTek MTHNH18 (Half and Half Plate) on each face of truss with USP NA11 nails (0.131" x 1.5") in pre-punched holes provided. All nail holes must be filled (5 Nails per side 10 nails total).
- 5) See HINGE PLATE DETAILS for plate placement.
- 6) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 9) Ceiling dead load (10.0 psf) on member(s). 2-3, 7-8, 3-7; Wall dead load (5.0psf) on member(s).2-13, 8-11
- 10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-1311) Attic room checked for L/360 deflection.



April 16,2019

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A MiTek Affiliat 818 Soundside Road Edenton, NC 27932

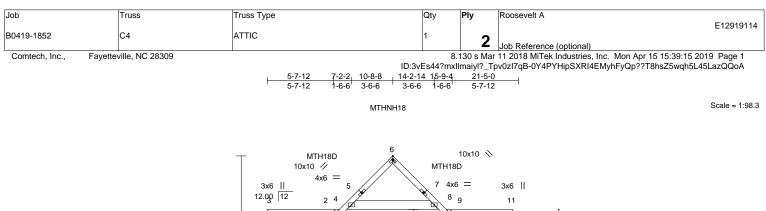


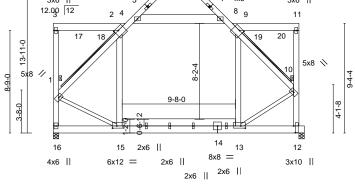
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Edenton, NC 27932





		5-7-12	15-9-4 6x12	$2 = \frac{21-5-0}{5-7-12}$		
Plate Offsets (X,Y)	[2:0-3-14,0-2-12], [5:0-0-11,0-1-2], [6:0-2				, [13:0-4-8,0-2-4], [15:0-4-8,0-2-4]
LOADING TCLL(psf)TCDL20.0TCDL10.0BCLL0.0BCDL10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.45 BC 0.46 WB 0.82 Matrix-S	Horz(CT) 0.01	15 >999 13-15 >999	L/d 240 240 n/a 360	PLATES GRIP MT20 244/190 MT18HS 244/190 Weight: 644 lb FT = 20%
BOT CHORD 2x8 SF WEBS 2x6 SF 4-8: 2x	11: 2x8 SP No.1	No.3	BRACING- TOP CHORD BOT CHORD WEBS	except end vert T-Brace:	icals. Except: 2x4 ectly applied or	etly applied or 6-0-0 oc purlins, - SPF No.2 - 1-2, 9-10 10-0-0 oc bracing. 6, 11-12
Max H Max U	e) 16=4631/0-5-8, 12=4613/0-5-8 lorz 16=-115(LC 4) Jplift 12=-232(LC 8) Grav 16=5981(LC 35), 12=6105(LC 34)					
1-3=- BOT CHORD 13-13 WEBS 4-8=- NOTES- 1) 2-ply truss to be correst bottom chords connect Bottom chords connect as 2) All loads are conside ply connections hav 2) All loads are conside ply connections hav 10 Loads are conside ply connections hav 3) Unbalanced roof live 10 Wind: ASCE 7-10; W 4) Wind: ASCE 7-10; W WWFRS (envelope) 5) Provide adequate di 6) All plates are MT20 7) Attach MiTek MTHN provided. All nail hoi 8) See HINGE PLATE 9) Provisions must be 10) This truss has bee 10) This truss has bee 11 * This truss has bee 11) * This truss has bee 120 Ceiling dead load (13) Bottom chord live I	4982/65, 2-4=-5016/212, 8-9=-5013/217 -782/0, 10-12=-6232/130, 10-11=-502/15 5=-52/3578 -3469/89, 2-15=-1258/138, 9-13=-1621/5 anected together with 10d (0.131"x3") na ted as follows: 2x6 - 2 rows staggered at tected as follows: 2x8 - 2 rows staggered at follows: 2x10 - 2 rows staggered at 0-6- ered equally applied to all plies, except if e been provided to distribute only loads to a loads have been considered for this de /ult=130mph (3-second gust) Vasd=103r ; Lumber DOL=1.60 plate grip DOL=1.60 rainage to prevent water ponding. plates unless otherwise indicated. IH18 (Half and Half Plate) on each face of les must be filled (5 Nails per side 10 nai DETAILS for plate placement. made to prevent lateral movement of hin n designed for a 10.0 psf bottom chord li een designed for a live load of 30.0psf on m chord and any other members. (10.0 psf) on member(s). 2-4, 8-9, 4-8; V load (40.0 psf) and additional bottom chord al connection (by others) of truss to beari	6 58, 1-15=-105/3926, 10- ils as follows: 0-9-0 oc, 2x8 - 2 rows state a t 0-9-0 oc. 0 oc, 2x6 - 2 rows stagge noted as front (F) or back noted as front (F) or back noted as (F) or (B), unless sign. nph; TCDL=6.0psf; BCDL of truss with USP NA11 nc ls total). ged member(s) during tra- ve load nonconcurrent with the bottom chord in all ar Vall dead load (5.0psf) on rd dead load (10.0 psf) applications on the second se	13=-18/4172 aggered at 0-9-0 oc. red at 0-9-0 oc, 2x4 - 1 n k (B) face in the LOAD (s otherwise indicated. .=5.0psf; h=15ft; Cat. II; ails (0.131" x 1.5") in pre- ansportation. th any other live loads. reas with a clearance gro member(s).2-15, 9-13 pplied only to room. 13-	CASE(S) section. Exp C; enclosed; e-punched holes eater than 6-0-0	"Presentation of the second se	SEAL 030652 April 16,2019
12=232. Continued on page 2 WARNING - Verify d Design valid for use only a truss system. Before u building design. Bracing is always required for st fabrication, storage, deli	esign parameters and READ NOTES ON THIS AND y with MiTek® connectors. This design is based only se, the building designer must verify the applicabilit j indicated is to prevent buckling of individual truss ability and to prevent collapse with possible persona very, erection and bracing of trusses and truss syst ailable from Truss Plate Institute, 218 N. Lee Street,	DINCLUDED MITEK REFERENCE / upon parameters shown, and ii y of design parameters and prop web and/or chord members only li injury and property damage. F ms. see ANS/TP/I Q	E PAGE MII-7473 rev. 10/03/2 s for an individual building com erly incorporate this design int . Additional temporary and per for general guidance regarding uality Criteria. DSB-89 and B	pots BEFORE USE. ponent, not o the overall manent bracing the	, ,	AMITEK Affiliate B18 Soundside Road Edenton, NC 27932

Job	Truss	Truss T	ype 0	Qty	Ply	Roosevelt A
						E1291911
B0419-1852	C4	ATTIC	1	1	2	
					2	Job Reference (optional)
Comtech, Inc.,	Fayetteville, NC	28309		8.1	130 s Mar	r 11 2018 MiTek Industries, Inc. Mon Apr 15 15:39:15 2019 Page 2
	-		ID:3vE	s44?mxll	maiyl?_T	pv0zI7qB-0Y4PYHipSXRI4EMyhFyQp??T8hsZ5wqh5L45LazQQoA

NOTES-

- 15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 16) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 55 lb down and 51 lb up at 0-3-4, 1212 lb down and 135 lb up at 15-5-7, and 317 lb down and 118 lb up at 19-5-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 17) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 18) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf) Vert: 15-16=-20, 13-15=-40, 12-13=-20, 2-4=-80, 4-6=-60, 6-8=-60, 8-9=-80, 4-8=-490(F=-470), 2-3=-60, 9-11=-60

Drag: 2-15=-10, 9-13=-10

Concentrated Loads (lb)

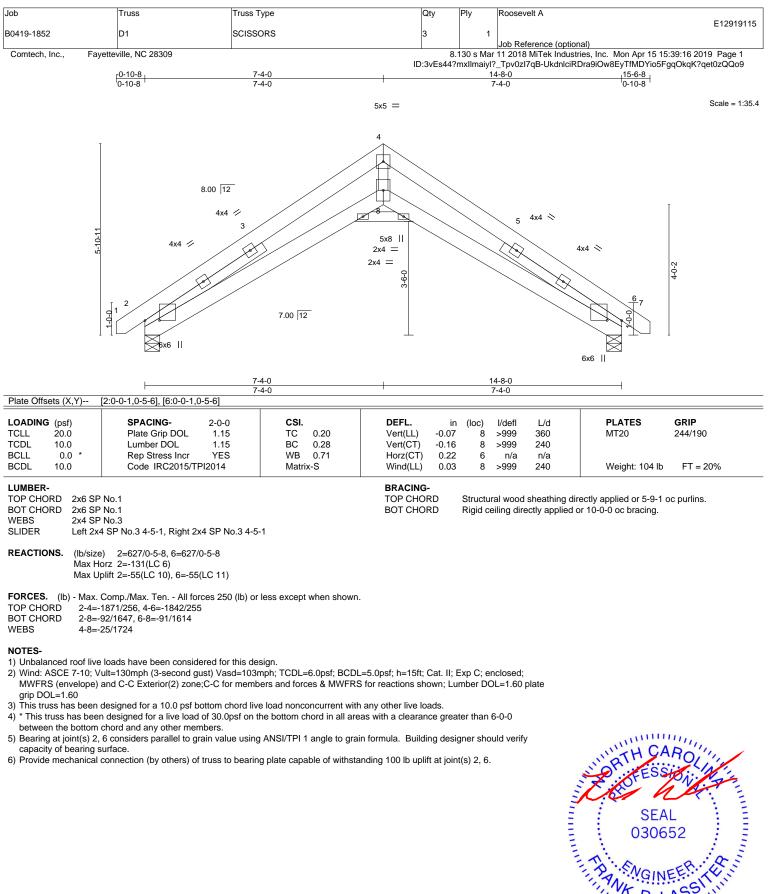
Vert: 1=-27 2=-562 9=-1152 17=-562 18=-562 19=-277(B) 20=-277(B)



April 16,2019

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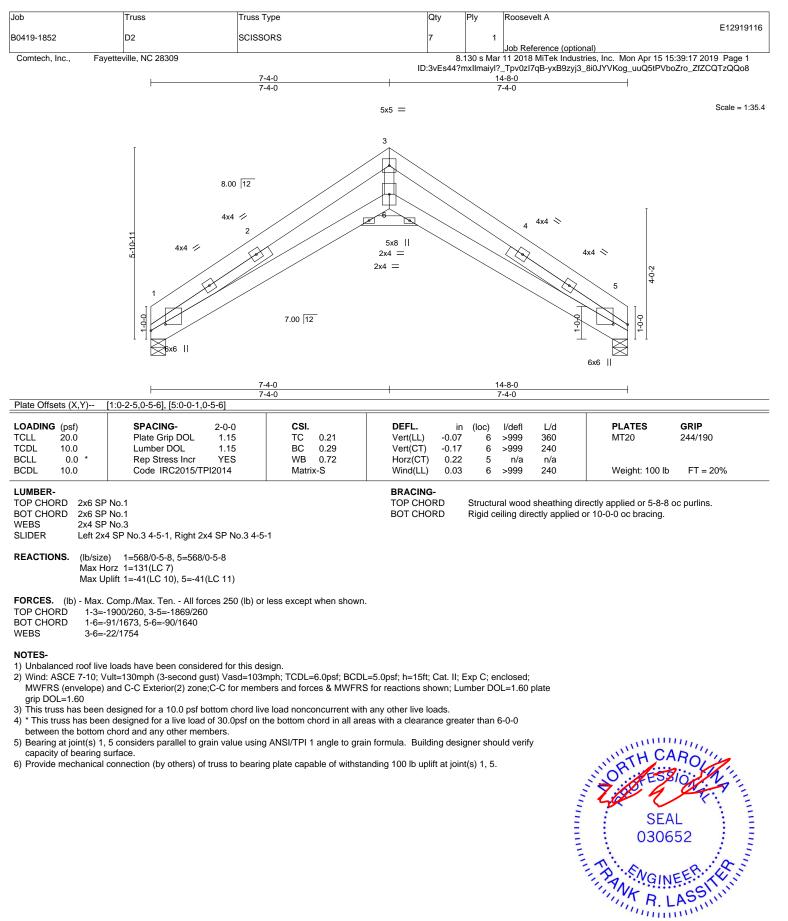






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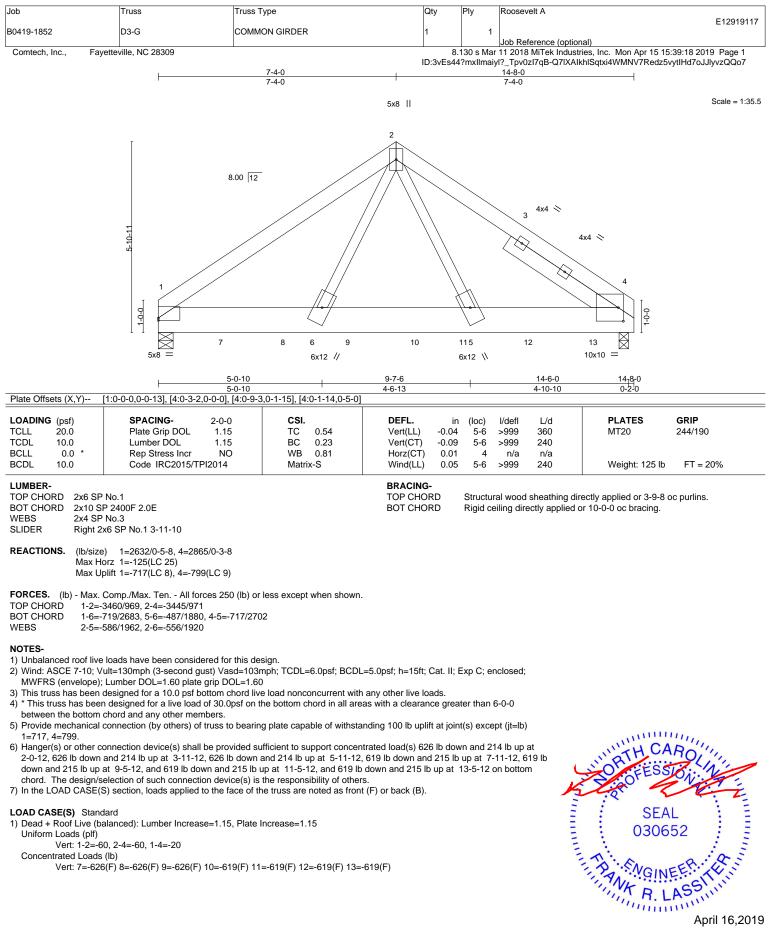




April 16,2019

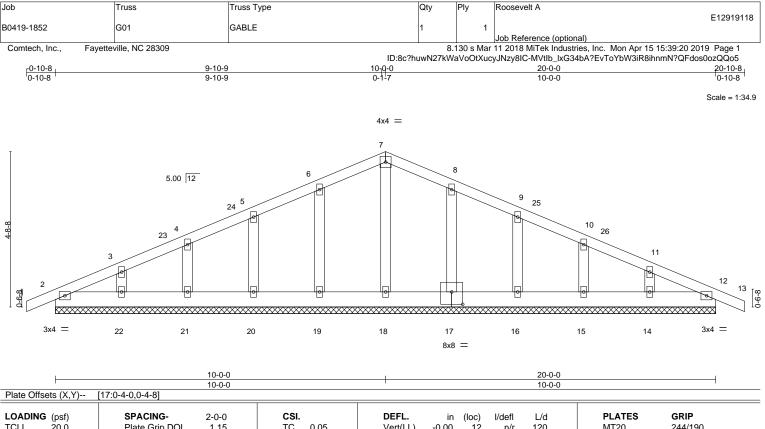
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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.05 BC 0.01 WB 0.05 Matrix-S	DEFL. in (loc) l/defl L/d Vert(LL) -0.00 12 n/r 120 Vert(CT) -0.00 12 n/r 120 Horz(CT) 0.00 12 n/a n/a
LUMBER- TOP CHORD 2x4 SP	No.1		BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.3 OTHERS

REACTIONS. All bearings 20-0-0.

(lb) - Max Horz 2=-92(LC 15)

Max Uplift All uplift 100 lb or less at joint(s) 12, 2, 19, 20, 21, 22, 17, 16, 15, 14

Max Grav All reactions 250 lb or less at joint(s) 12, 2, 18, 19, 20, 21, 22, 17, 16, 15, 14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

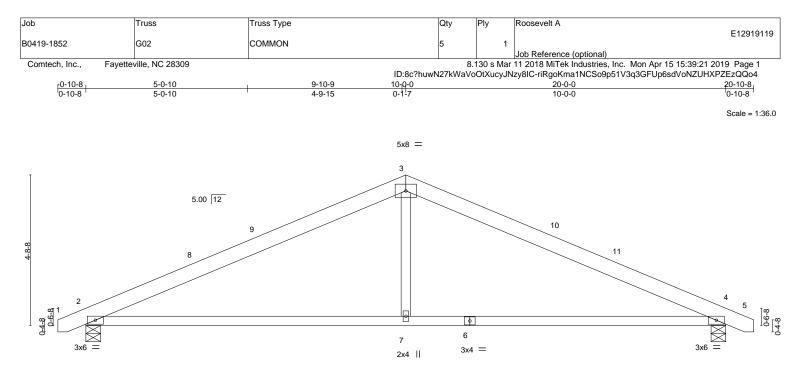
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 5-7-3, Corner(3) 5-7-3 to 10-0-0, Exterior(2) 14-4-13 to 16-5-11 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOI = 1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 2, 19, 20, 21, 22, 17, 16, 15, 14.



April 16,2019

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	<u>9-10-9</u> 9-10-9		<u> 10-0-0</u> 0-1-7		20-0-0 10-0-0		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.57 BC 0.74 WB 0.17	Vert(LL) -0.1 Vert(CT) -0.3	16 2-7	l/defl L/d >999 360 >664 240 n/a n/a	PLATES GRIP MT20 244/190	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.0		>999 240	Weight: 91 lb FT = 2	0%

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x4 SP No.1WEBS2x4 SP No.3

REACTIONS. (lb/size) 4=839/0-5-8, 2=839/0-5-8 Max Horz 2=54(LC 14) Max Uplift 4=-82(LC 11), 2=-82(LC 10)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-1197/384, 3-4=-1197/384

 BOT CHORD
 2-7=-218/1021, 4-7=-218/1021

WEBS 3-7=0/452

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-12 to 3-8-1, Interior(1) 3-8-1 to 5-7-3, Exterior(2) 5-7-3 to 10-0-0, Interior(1) 14-4-13 to 16-3-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.



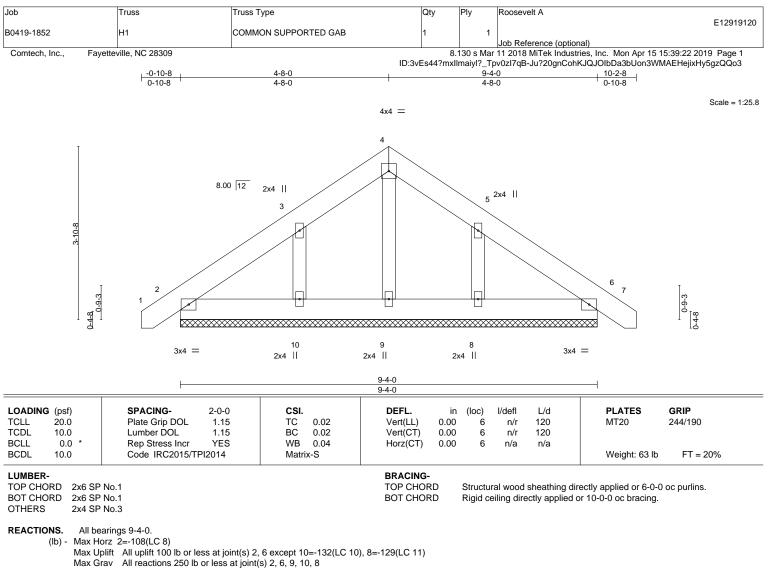
April 16,2019

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BRACING-

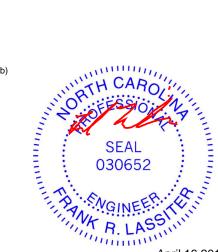
TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-7-14 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

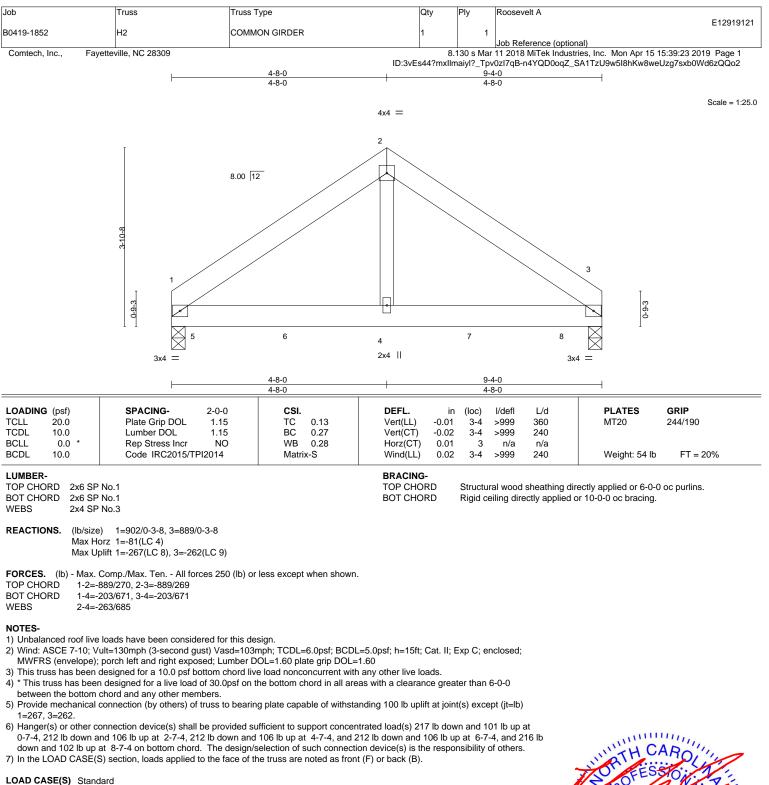
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=132, 8=129.
- 9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



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1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 1-3=-20

Concentrated Loads (lb)

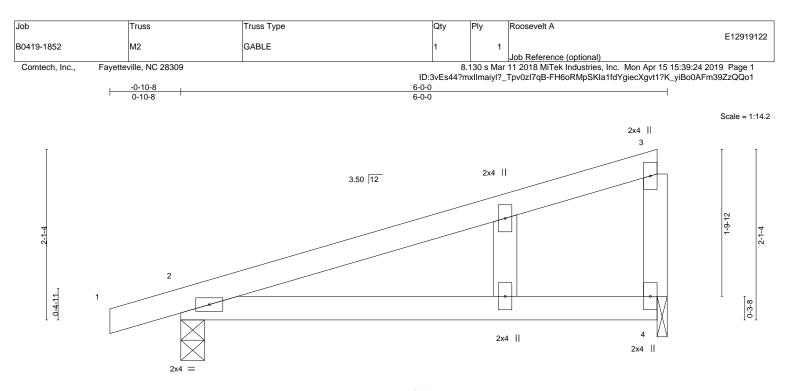
Vert: 4=-212(B) 5=-217(B) 6=-212(B) 7=-212(B) 8=-216(B)



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6-0-0 6-0-0											
OADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.15	тс	0.44	Vert(LL)	-0.06	2-4	>999	360	MT20	244/190
CDL 10.0	Lumber DOL	1.15	BC	0.32	Vert(CT)	-0.11	2-4	>615	240		
CLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCDL 10.0	Code IRC2015/TF	PI2014	Matri	x-P	Wind(LL)	0.12	2-4	>554	240	Weight: 23 lb	FT = 20%

LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.1
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING-TOP CHORD

RD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (Ib/size) 2=295/0-3-8, 4=222/0-1-8 Max Horz 2=66(LC 6)

Max Uplift 2=-125(LC 6), 4=-99(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

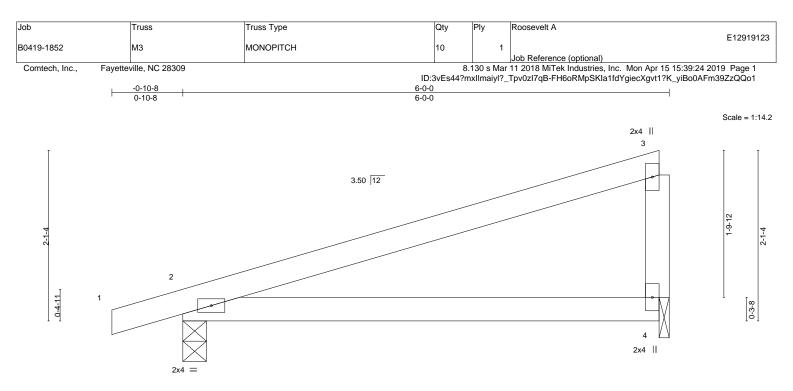
NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=125.



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		I							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.44 BC 0.32 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	```	(loc) 2-4 2-4 4 2-4	l/defl >999 >615 n/a >554	L/d 360 240 n/a 240	PLATES MT20 Weight: 22 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

BOT CHORD2x4 SP No.1WEBS2x4 SP No.3

REACTIONS. (lb/size) 2=295/0-3-8, 4=222/0-1-8 Max Horz 2=66(LC 6) Max Uplift 2=-125(LC 6), 4=-99(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=125.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

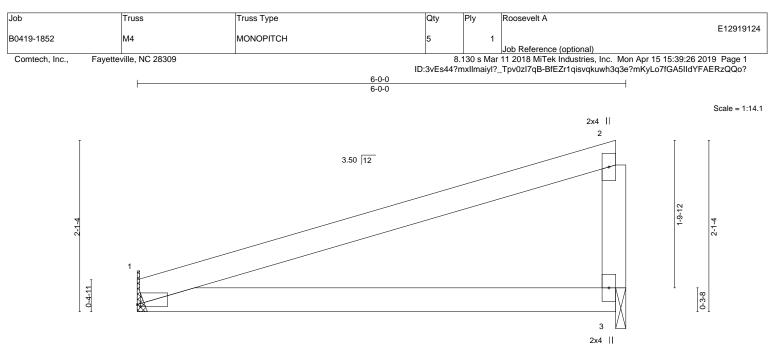
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

April 16,2019

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2x4 =

					6-0-0 6-0-0						
Plate Offsets (X,Y)	1:0-0-7,0-0-5]		1							T	
OADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.49	Vert(LL)	-0.06	<u></u> 1-3	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.33	Vert(CT)	-0.12	1-3	>589	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code IRC2015/TF	PI2014	Matri	x-P	Wind(LL)	0.13	1-3	>531	240	Weight: 20 lb	FT = 20%
LUMBER-					BRACING-						

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.3 WEBS

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=232/Mechanical, 3=230/0-1-8 Max Horz 1=58(LC 6)

Max Uplift 1=-86(LC 6), 3=-104(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

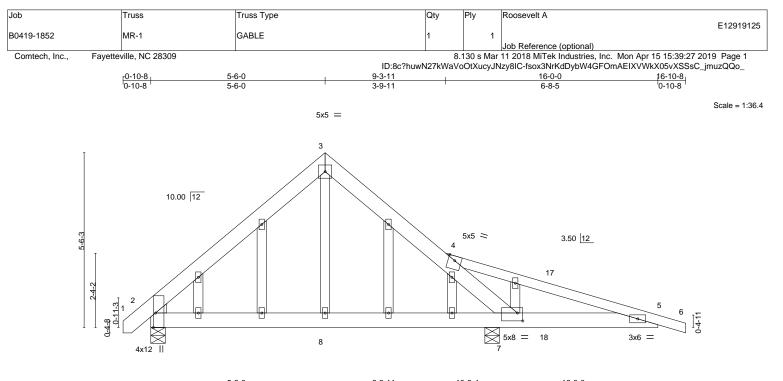
- 5) Bearing at joint(s) 3 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 3.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 3=104.



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	5-6-0		9-3-11	10-9-4		16-0-0		
	5-6-0		3-9-11	1-5-9	I	5-2-12	Ι	
Plate Offsets (X,Y)	[2:0-5-8,Edge], [2:0-1-8,0-4-9], [2:0-0-1	2,0-0-14], [7:0-2-0,0-3-0]						
LOADING (psf) ICLL 20.0 ICDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.48 BC 0.29 WB 0.07	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (lo -0.03 7 -0.02 2 0.00	8 >999	L/d 240 240 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 109 lb	FT = 20%
4-6: 2 BOT CHORD 2x6 S WEBS 2x4 S	P No.1 *Except* x4 SP No.1 P No.1 P No.3 P No.3		TOP CHOR BOT CHOR				tly applied or 5-11-1 6-0-0 oc bracing.	1 oc purlins.
OTHERS 2x4 S WEDGE Left: 2x4 SP No.3								

TOP CHORD 2-3=-366/70, 3-4=-326/129, 4-7=-1138/932, 4-5=-987/859

BOT CHORD 2-3=-300/70, 3-4=-320/729, 4-7=-1138/352, 4-3=-30

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed;

MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-14 to 9-1-15, Exterior(2) 9-1-15 to 12-5-11, Corner(3) 12-5-11 to

16-10-8 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 2x4 MT20 unless otherwise indicated.

 τ_j The places are 2x4 will 20 unless 0

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=362, 2=103.

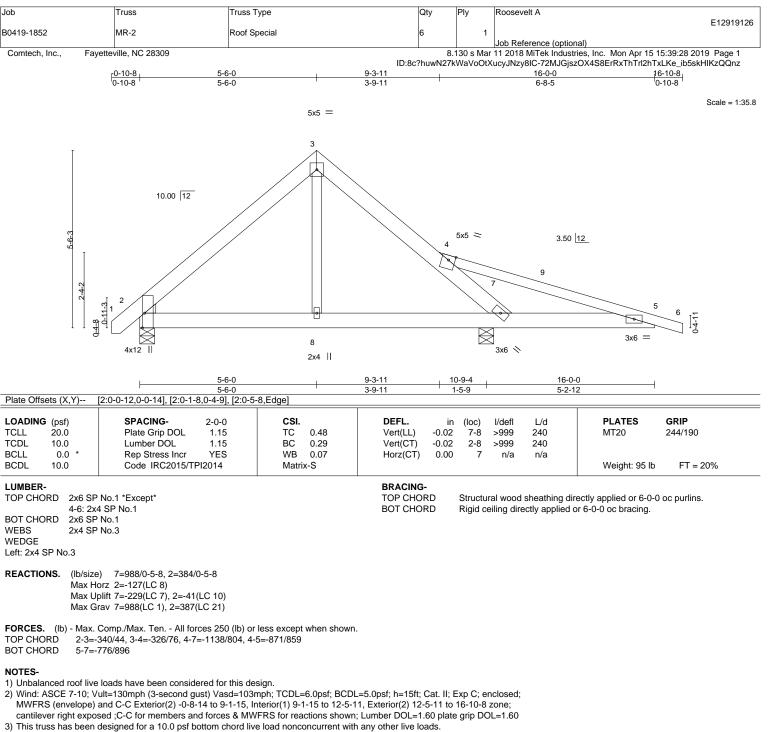
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

and an and a start of the start and a manual and SEAL 030652

April 16,2019

ENGINEERING BY ERENCED A Mi Tek Affiliate 818 Soundside Road Edenton, NC 27932

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4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 7=229.

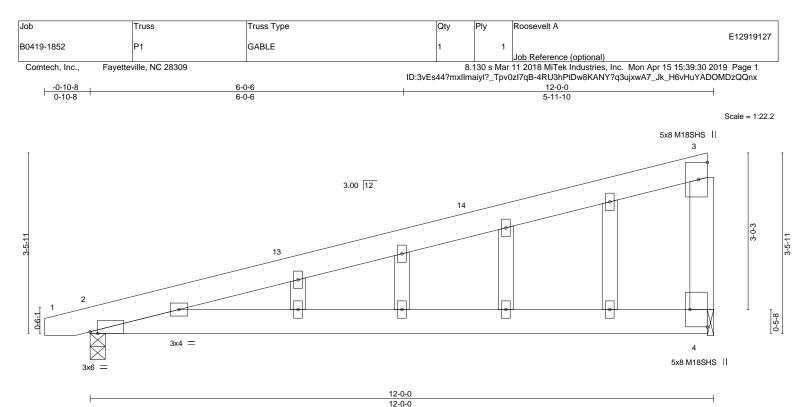
6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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[2:0-1-12,Edge], [4:Edge,0-4-0] CSI. DEFL. 2-0-0 in (loc) l/defl 1.15 тс 0.65 Vert(LL) 0.37 2-4 >380 BC 0.45 2-4 1.15 Vert(CT) -0.32 >434 \//R VES 0.00 Horz((0 00 n/2

LUMBER- BRACING- TOP CHORD 2x6 SP No.1 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.	BCDI		Code IRC2015/TPI2014	Matrix-S	1012(01) 0.0	10 4 11/a 11/a	Weight: 73 lb	FT = 20%
WEBS2x6 SP No.1BOT CHORDRigid ceiling directly applied or 9-10-2 oc bracing.OTHERS2x4 SP No.3	TOP BOT WEB	CHORD 2x6 CHORD 2x6 3S 2x6	6 SP No.1 6 SP No.1		TOP CHORD	except end verticals.	, ,,,	c purlins,

REACTIONS. (lb/size) 2=510/0-3-8, 4=464/0-1-8 Max Horz 2=146(LC 6) Max Uplift 2=-293(LC 6), 4=-291(LC 6)

SPACING-

Plate Grip DOL

Lumber DOL

Ron Stress In

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-306/184, 3-4=-310/222

Plate Offsets (X,Y)-

20.0

10.0

00

LOADING (psf)

TCLL

TCDL

BCU

- NOTES-1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-6-15 to 3-9-14, Interior(1) 3-9-14 to 7-4-7, Exterior(2) 7-4-7 to 11-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 8) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=293, 4=291,



PLATES

M18SHS

MT20

I/d

240

240

n/2

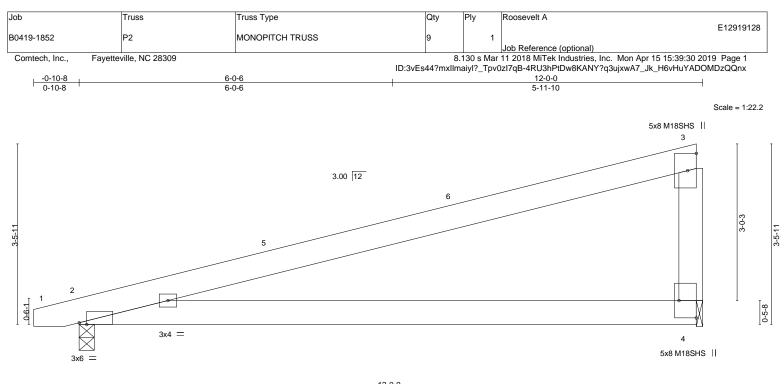
GRIP

244/190

244/190

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🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON MICLODED MITER REFERENCE PAGE mit-14/3 at 900, 1002/015 BEFORE 052. Design valid for use only with MITeR we connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



H			12-0-0		
Plate Offsets (X,Y)	[2:0-1-12,Edge], [4:Edge,0-4-0]	1 1			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.65 BC 0.45 WB 0.00 Matrix-S	DEFL. in Vert(LL) 0.37 Vert(CT) -0.32 Horz(CT) 0.00	2 2-4 >434 240	PLATES GRIP MT20 244/190 M18SHS 244/190 Weight: 65 lb FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x6 SP	No.1		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied	rectly applied or 6-0-0 oc purlins, or 9-10-2 oc bracing.

(lb/size) 2=510/0-3-8, 4=464/0-1-8 REACTIONS. Max Horz 2=102(LC 6) Max Uplift 2=-206(LC 6), 4=-203(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-306/184, 3-4=-310/222

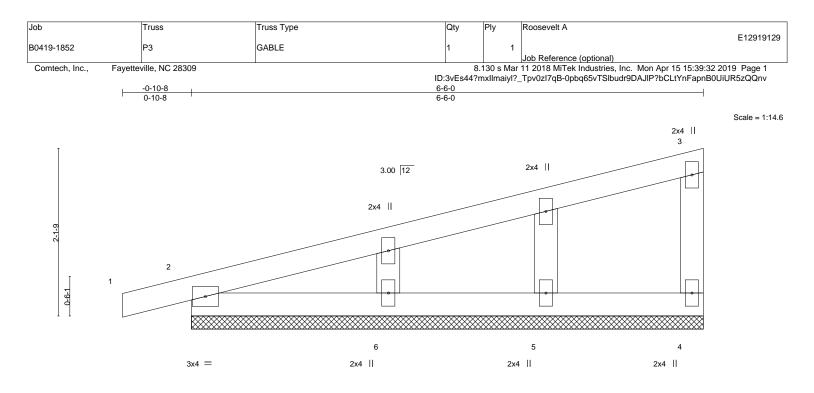
NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-6-15 to 3-9-14, Interior(1) 3-9-14 to 7-4-7, Exterior(2) 7-4-7 to 11-9-4 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=206, 4=203.



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LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.58	Vert(LL)	-0.02	1	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	0.02	1	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-P						Weight: 26 lb	FT = 20%
LUMBER-						BRACING-						
TOP CHOP	RD 2x4 SP	No.1				TOP CHOP	RD.	Structu	ral wood	sheathing di	irectly applied or 6-0-0	oc purlins,
BOT CHOP	RD 2x4 SP	No.1						except	end verti	cals.		
WEBS	2x4 SP	No.3				BOT CHOP	RD	Rigid c	eiling dire	ectly applied	or 10-0-0 oc bracing.	
OTHERS	2x4 SP	No.3						-	5		Ŭ	

REACTIONS. All bearings 6-6-0.

(lb) - Max Horz 2=85(LC 6)

- Max Uplift All uplift 100 lb or less at joint(s) except 4=-113(LC 10), 2=-136(LC 6) Max Grav All reactions 250 lb or less at joint(s) 4, 5, 6 except 2=267(LC 1)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.

NOTES-

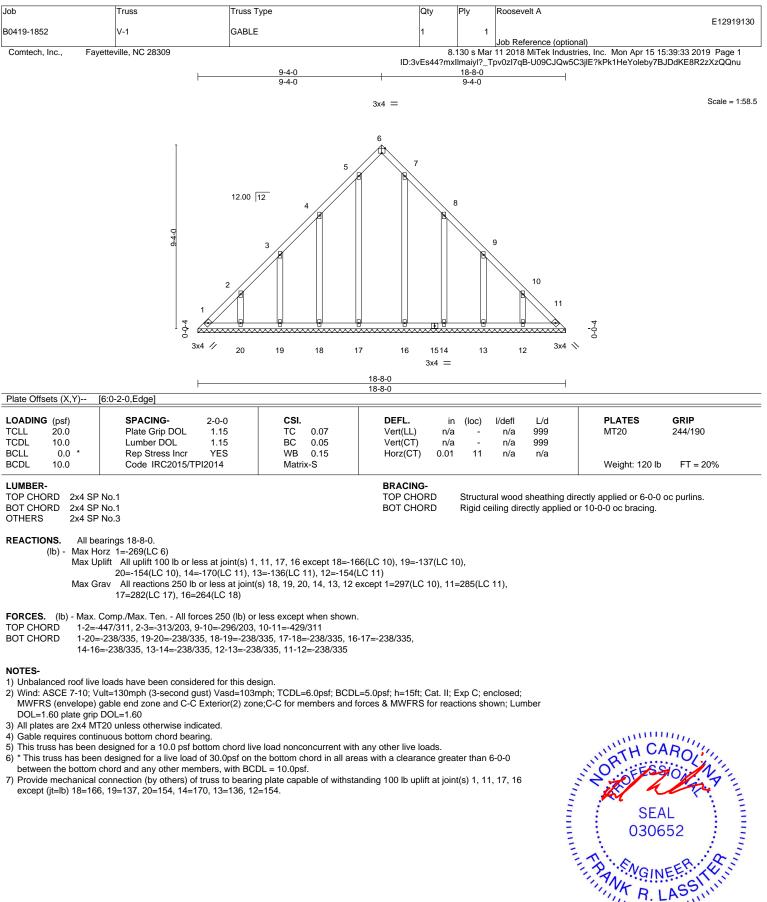
- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; enclosed;
- MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 113 lb uplift at joint 4 and 136 lb uplift at joint 2.



April 16,2019

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