

Client: Project: Address:

Weaver Homes Magnolia Elev. A Magnolia Elev. A Date: 1/26/2021 Input by:

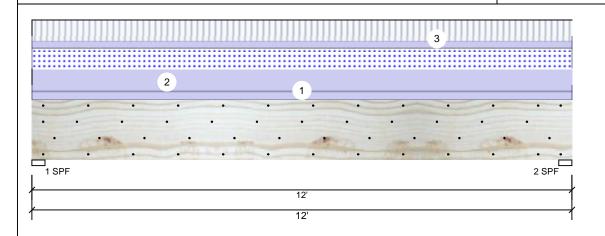
Christine Shivy Job Name: Magnolia Elev. A

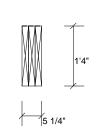
Reactions UNPATTERNED Ib (Uplift)

Project #:

**Kerto-S LVL** 1.750" X 16.000" BM<sub>1</sub>

Level: Level 3-Ply - PASSED





Ld. Comb.

D+0.75(L+S)

D+0.75(L+S)

Page 1 of 1

Member Information							
Type:	Girder						
Plies:	3						
Moisture Condition:	Dry						
Deflection LL:	480						
Deflection TL:	360						
Importance:	Normal						
Temperature:	Temp <= 100°F						

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: Yes Deck: Not Checked

Brg Wind Const Live Dead Snow 1932 3454 1950 0 0 1 0 2 1932 3454 1950 0

# **Bearings** Bearing Length 1 - SPF 3.500"

2 - SPF 3.500"

Cap. React D/L lb Total Ld. Case 3454 / 2912 6366 L 82% 3454 / 2912 6366 L

## **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	17729 ft-lb	6'	62010 ft-lb	0.286 (29%)	D+0.75(L+S)	L
Unbraced	17729 ft-lb	6'	17732 ft-lb	1.000 (100%)	D+0.75(L+S)	L
Shear	4565 lb	1'6 5/8"	17920 lb	0.255 (25%)	D+L	L
LL Defl inch	0.066 (L/2116)	6'	0.289 (L/480)	0.230 (23%)	0.75(L+S)	L
TL Defl inch	0.143 (L/968)	6'	0.385 (L/360)	0.370 (37%)	D+0.75(L+S)	L

### Design Notes

- 1 Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 10'4 1/8" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

		3 - 1 7									_
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Wall	
2	Uniform			Тор	325 PLF	0 PLF	325 PLF	0 PLF	0 PLF	A2	
3	Uniform			Far Face	107 PLF	322 PLF	0 PLF	0 PLF	0 PLF	F1	
	Self Weight				19 PLF						

### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- approvals
  Damaged Beams must not be used
  Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 1/8/2023



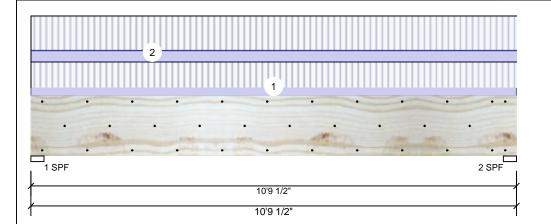
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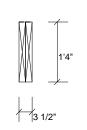
Date: 1/26/2021 Input by: Christine Shivy Job Name: Magnolia Elev. A

Project #:

1.750" X 16.000" 2-Ply - PASSED **Kerto-S LVL** BM<sub>2</sub>

Level: Level





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Member Inform	nation			Reactions UNPATTERNED Ib (Uplift)					
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	3389	1200	0	0	0
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015	2	3389	1200	0	0	0
Deflection LL:	480	Load Sharing:	No						
Deflection TL:	360	Deck:	Not Checked						
Importance:	Normal								
Temperature:	Temp <= 100°F								
				Bearings	6				
				Bearing	Length	Cap. React	D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	3.500"	88% 1200	/ 3389	4589 L	D+L
				2 - SPF	3.500"	88% 1200	/ 3389	4589 L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	11397 ft-lb	5'4 3/4"	34565 ft-lb	0.330 (33%)	D+L	L
Unbraced	11397 ft-lb	5'4 3/4"	11746 ft-lb	0.970 (97%)	D+L	L
Shear	4386 lb	1'6 5/8"	11947 lb	0.367 (37%)	D+L	L
LL Defl inch	0.085 (L/1457)	5'4 3/4"	0.259 (L/480)	0.330 (33%)	L	L
TL Defl inch	0.115 (L/1076)	5'4 3/4"	0.345 (L/360)	0.330 (33%)	D+L	L

# **Design Notes**

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6. Lateral slenderness ratio based on single ply width

o Edicial sicilaciness ratio based on single ply width.											
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Far Face	89 PLF	267 PLF	0 PLF	0 PLF	0 PLF	F4	
2	Uniform			Near Face	121 PLF	361 PLF	0 PLF	0 PLF	0 PLF	F2	
	Self Weight				12 PLF						

## Notes

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- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Handling & Installation

  1. UVI beams must not be out or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

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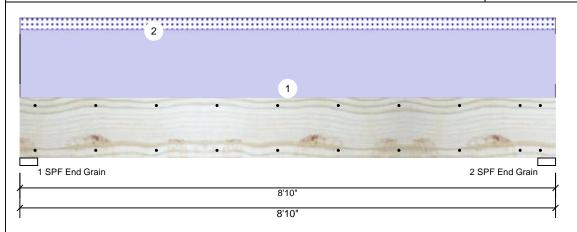
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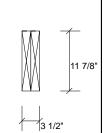
Date: 1/26/2021 Input by: Christine Shivy Job Name: Magnolia Elev. A

Project #:

1.750" X 11.875" **Kerto-S LVL** 2-Ply - PASSED **GDH** 

Level: Level





Const

Ld. Case Ld. Comb. D+S

D+S

0

0

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/lember Infor	mation			Reactio	Reactions UNPATTERNED lb (Uplift)				
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	
Plies:	2	Design Method:	ASD	1	0	1101	177	0	
Moisture Condition	on: Dry	Building Code:	IBC/IRC 2015	2	0	1101	177	0	
Deflection LL:	480	Load Sharing:	No						
Deflection TL:	360	Deck:	Not Checked						
Importance:	Normal								
Temperature:	Temp <= 100°F								
				Bearing	gs				
				Bearing	Length	Cap. Rea	act D/L lb	Total Ld. C	
				1 - SPF End	3.500"	12%	1101 / 177	1277 L	
Analysis Resu	lts			Grain					
Analysis A	ctual Location	Allowed Capac	•	I End	3.500"	12%	1101 / 177	1277 L	

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2185 ft-lb	4'5"	17919 ft-lb	0.122 (12%)	D	Uniform
Unbraced	2536 ft-lb	4'5"	10756 ft-lb	0.236 (24%)	D+S	L
Shear	797 lb	7'7 3/8"	7980 lb	0.100 (10%)	D	Uniform
LL Defl inch	0.006 (L/18257)	4'5 1/16"	0.209 (L/480)	0.030 (3%)	S	L
TL Defl inch	0.040 (L/2525)	4'5 1/16"	0.279 (L/360)	0.140 (14%)	D+S	L

# **Design Notes**

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

		F-)									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	200 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Loads	
2	Uniform			Тор	40 PLF	0 PLF	40 PLF	0 PLF	0 PLF	2'-0" Gable End	
	Self Weight				9 PLF						

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- Handling & Installation

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Grain

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