

соттесн										
TRUSSES & BEAMS Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444										
Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds the support system for all reactions that exceed 15000#.										
		Davi	id Lai	ndry						
LU7 NJA	AD CH. (BASEI ABER OF CA	MRTEU DONITABU ACKISTUDS MEADER	JK JA(ES R5025(REQUIREI /SIRDER	U & (5)) 5 & EA END	:0F					
PEADER/SINER NO 00										
Harnett	Lot 5 C.P. Stewart Rd.	Roof / 3GRF, 4BR	05/12/21	David Landry	J Lenny Norris					
COUNTY	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALESMAN					
Weaver Development Co. Inc.	Lot 5 C.P. Stewart Rd.	Halifax II	Seal Date		J0521-2900					
-	ш		ATE	#						

permanent bracing of the roor and noor system an for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.con



соттесн ROOF & FLOOR **TRUSSES & BEAMS Reilly Road Industrial Park** Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444 earing reactions less than or equal to 3000# are semed to comply with the prescriptive Code quirements. The contractor shall refer to the tached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the current output for any strategies that expende upport system for any reaction that exceeds ose specified in the attached Tables. A sign the support system for all reactions that ceed 15000#. Signature David Landry LOAD CHART FOR JACK STUDS (BASED ON TABLES R502 5(1) & (6)) NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/SIRDER FND RFACTION (UP TO) REQ'D STUDS FOR (3) PLY PLADLR FNN RFACTTO (UP TO) RCQ'D STUDS I (4) N V UFAIN -006 #7 ΕE A REQ.15.5 (2) PLV END E 1700 1 2550 1 3400 1 3400 2 6800 2 5100 2 5100 3 7650 3 10200 3 6800 4 10200 4 13600 4 8500 5 12750 5 17000 5 10200 6 15300 6 11900 7 13600 8 15300 9 Rd. Stewart 4BR Jdry Roof / 3GRF, Lenny Norris 05/12/21 5 C.P. David Lan nett Lot Har DATE REV. DRAWN BY SALESMAN ADDRESS COUNTY MODEL Inc. Ŝ Rd. Development Stewart J0521-2900 Halifax II Seal Date C.P. Weaver വ Lot JOB NAME SEAL DATE # BUILDER QUOTE # PLAN JOB

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

russ delivery package or online @ sbcindustry.c

-	•	(Client: Project:	Weaver De	velopment			Da	ate:	5/12/20 David I	21 andry				Page 1 of 8
lis	Design	/	Address:					Jo	b Name	e: Lot 5 C	P. Stewa	rt Road			
BM1	S_P_F #2	2 00	א ייחו	10 000	ייי 2_6)) /	PAS	Pr SEC	oject #:	J0521-2 Level: Leve	2900 el				
	0-1 -1 #2	2.00		10.000	μ Ζ-Ι	ıy -									

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			-		atter		-							X X	9 1/4"
•	The second second	•	and there	•	and the second second	11								(V)	
1 SPF							2 SPF								
			5'6"											1 1	3"
			5.6					I							
Member In	formation						Rea	action	s UN	PATTER	NED Ib	(Uplift)			
Type:	Girder		Applica	tion: Mothod:	Floor		Brg	Dire	ection	Live	e	Dead	Snow	Wind	Const
Moisture Cond	z dition: Dry		Building	g Code:	IBC/IRC 2015	5	1	Verti Verti	ical		2	919 919	919 919	0	0
Deflection LL:	480 360		Load SI	haring:	No Not Checked										
Importance:	Normal - II		Deck.		Not Checked										
Temperature:	Temp <= 100°F						Bea	arings	;						
							В	earing	Lengt	h Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1	- SPF - SPF	3.500" 3.500"	Vert Vert	41% 41%	919 / 919 919 / 919	1837 1837	L	D+S D+S
Analysis Re	sults		A II.e	Orarait	Oamh	0									
Moment	2122 ft-lb	2'9" 3	3946 ft-Ib	0.538 (54	%) D+S	L									
Unbraced	2122 ft-lb	2'9" 3	3654 ft -l b	0.581 (58	%) D+S	L									
Shear	1127 lb 0.018 (1/3452)	1'3/4" 2 2'9" (2872 lb 1 1 26 (1 /48(0.392 (39	%) D+S %) S	L									
TL Defl inch	0.035 (L/1726)	2'9" (D.168 (L/360	0) 0.209 (21	%) D+S	L									
Design Not	es														
1 Provide sup may also b 2 Fasten all p	oport to prevent lateral e required at the interio lies using 2 rows of 10	movement or bearings Od Box nail	t and rotatic by the buil s (.128x3")	on at the end ding code. at 12" o.c. M	bearings. Late aximum end d	eral suppo listance no	ort								
3 Refer to las	t page of calculations	for fastene	rs required	for specified	loads.										
4 Girders are 5 Top loads r	designed to be suppo nust be supported equ	orted on the ally by all p	e bottom edg plies.	ge only.											
6 Top must b 7 Bottom mu	e laterally braced at er st be laterally braced a	nd bearings at end beari	s. ings.												
8 Lateral sler	derness ratio based o	n single ply	y width.		Side	Deed		Live	1 6 2 2		\Alind 1	6 Const 1	25 Com	monto	
1	Uniform	L		mb width	Тор	334 P	LF		- 310	334 PLF	0 Pl	LF 0	PLF A4	ments	
										Manufactu	rer Info		Comtech, Ir 1001 S. Rei	c. ly Road, Suite #	639
													28314 010-864-TE		
													313-004-1N		
					This	design is v	valid until ∠	/7/2024					C	от	есн
Version 21.40.305	Powered by iStruct™ Data	aset: embed	ded										CSD	DESER	

	1	Client:	Weaver Developm	nent		Date:	5/12/2021	Page 2 of 8
4		Project:				Input by:	David Landry	
	isDesign	Address:				Job Name:	Lot 5 C.P. Stewart Road	
-						Project #:	J0521-2900	
BM1	S-P-F #2	2.000" X	10.000"	2-Ply	- PASSE	D L	evel: Level	
				-				
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						12		IVIVI I
						$\overline{\Sigma}$		∧ ∧ 9 1/4"
•	•	•	•	•	• -			
	SPF .				2 SPF			,
		FIOI			2 01 1	<i>,</i>		
		5.0.						3"
1		5'6"				1		
Multi-Ph	v Analysis							
Easten all	I plies using 2 row	us of 10d Box nails	(128v3") at 12'	oc Mavir	num and dis	tance no	t to exceed 6"	
Capacity	plies using 2 rov			0.0				
Load		0.0 PLF						
Yield Limit p	er Foot	157.4 PLF						
Yield Limit p	er Fastener	78.7 lb.						
Edge Distan	ice	1 1/2"						
Min. End Dis	stance	3"						
Load Combin	ination	1.00						
Duration rat		1.00						
							Manufacturer Info	Comtech, Inc. 1001 S. Rei∎y Road, Suite #639
						Г		Fayetteville, NC USA
								28314 910-864-TRUS
								le entre et l
				This design	is valid until 4/7/20	024		COMTECH

1973	/	Client: Weaver De	velopment		Date:	5/12/2021				Page 3 of 8
1		Project:	·		Input by:	David Landry				-
is	Design	Address:			Job Nam	ne: Lot 5 C.P. Ster	wart Road			
-					Project #	U0521-2900				
BM2	Kerto-S LVL	1.750" X 9.2	50" 2-PIv	- PA	SSED	Level: Level				
			,							
						1				
		1								
•	•	• •	•						N A A	
									IXIXI	0.1/4
	Contraction and		- Carton						MA	9 1/4
									<u> </u>	\rightarrow
1 SPF	End Grain	2	SPF End Grain							
/		4'8"		•						3 1/2"
ł		4'8"								
1		40	ľ							
Member In	formation			Rea	ctions UN	IPATTERNED	lb (Uplift)			
Type:	Girder	Application:	Floor	Brg	Direction	Live	Dead	Snow	Wind	Const
Plies: Moisture Con	Z adition: Dry	Design Method: Building Code:	ASD IBC/IBC 2015	1	Vertical	0	1526	1510	0	0
Deflection LL	.: 480	Load Sharing:	No	2	vertical	U	1526	1510	U	U
Deflection TL	.: 360	Deck:	Not Checked							
Importance:	Normal - II									
Temperature	: Temp <= 100°F			_	•					
				Bea	rings					
				Be	aring Leng	th Dir. Cap	React D/L lb	Total L	d. Case	Ld. Comb.
				1 - En	SPF 3.500	" Vert 28%	6 1526 / 1510) 3036 L		D+S
Analysis Re	esults			Gr	ain					
Analysis	Actual Location	Allowed Capacity	Comb. Cas	e 2-	SPF 3.500	" Vert 28%	6 1526 / 1510) 3036 L		D+S
Moment	2881 ft-lb 2'4"	14423 ft-lb 0.200 (20	%) D+S L	En	d ain					
Unbraced	2881 ft-lb 2'4"	12555 ft-lb 0.229 (23	%) D+S L							
Shear	1659 lb 3'7 1/4"	7943 lb 0.209 (21	%) D+S L							
LL Defl inch	0.015 (L/3370) 2'4 1/16"	0.105 (L/480) 0.142 (14	%)S L							
TL Defl inch	0.030 (L/1676) 2'4 1/16"	0.140 (L/360) 0.215 (21	%) D+S L							
Design No	tes									
1 Provide su	pport to prevent lateral movement	ent and rotation at the end	bearings. Lateral supp	ort						
2 Fasten all	plies using 2 rows of 10d Box n	ails (.128x3") at 12" o.c. M	aximum end distance	not						
to exceed	6".		la a da							
4 Girders and	e designed to be supported on t	hers required for specified he bottom edge only.	loads.							
5 Top loads	must be supported equally by a	Il plies.								
6 Top must t	be laterally braced at end bearin	igs. oringo								
8 Lateral sle	nderness ratio based on single	ply width.								
ID	Load Type	Location Trib Width	Side Dead	0.9	Live 1 Sn	ow 1.15 Wind	1.6 Const.	1.25 Comr	nents	
1	Uniform		Тор 647	PLF	0 PLF	647 PLF 0	PLF 0	PLF A3		
	Self Weight		7	PLF						
Notes	chen	nicals	For flat roofs pro ponding	ovide proper dra	inage to prevent	Manufacturer Info)	1001 S. Reilly	Road, Suite #63	39
structural adequacy design criteria an	of this component based on the 1 LVL of this component based on the 1 LVL doadings shown it is the 2 Dec	beams must not be cut or dri∎ed	-			301 Merritt 7 Build	ng, 2nd Floor	USA 28314		
responsibility of the ensure the compo	customer and/or the contractor to regardenent suitability of the intended faste	ning installation requirements, ning details, beam strength values a	multi-ply ind code			Norwalk, CT 0685 (800) 622-5850	I	910-864-TRU	S	
application, and to ve Lumber	erify the dimensions and loads. appro 3. Dam	ovals aged Beams must not be used				WWW.metsawood.co	om/us 3			
 Dry service condi LVL not to be tre 	itions, unless noted otherwise 5 Provi ated with fire retardant or corrosive	gn assumes top edge is laterally restrai ide lateral support at bearing points al displacement and rotation	to avoid		7/2024			CO	оте	CH
			This design is	valid until 4/	//2024					
rersion 21.40.305	o rowerea by iStruct™ Dataset: emb	euuea						CSD	545A	

	1	Client:	Weaver Developmer	nt	Date:	5/12/2021	Page 4 of 8
Ť	is Design	Project:			Input by:	David Landry	
- 1	ispesign	Address:			Job Nam Project #	te: Lot 5 C.P. Stewart Road	
DM2	Karta S I	VI 1 750"	V 0 250"	2 DIV		Level: Level	
	Rerio-5 L	VL 1./50	A 9.230	2-Piy	- PASSED		
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							9 1/
•	•	•	•	• —	<u> </u>		V V V
	PE End Grain		2 SPE En		<u> </u>		
		4'8"					3 1/2"
		40		,			3 1/2
		4.8.		I			
L							
Multi-Ply	/ Analysis						
Fasten all	plies using 2 row	s of 10d Box nails (.128x3") at 12" c	o.c Maximu	m end distance r	not to exceed 6".	
Capacity Load		0.0 % 0.0 PLF					
Yield Limit pe	er Foot	163.7 PLF					
Yield Limit pe Yield Mode	er Fastener	81.9 lb. IV					
Edge Distanc	ce	1 1/2"					
Min. End Dist	tance	3"					
Duration Fac	tor	1.00					
Notes		chemicals	6.	. For flat roofs provid	e proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Struct structural adequa	tured Designs is responsible only acy of this component based o	of the Handling & Installation	DN ut or drilled	ponding		Metsä Wood 301 Merritt 7 Building 2nd Floor	Fayetteville, NC USA
design criteria responsibility of ensure the cor	and loadings shown. It is the customer and/or the contrac	the 2 Refer to manufacture tor to regarding installation	r's product information requirements, multi-ply			Norwalk, CT 06851 (800) 622-5850	28314 910-864-TRUS
application, and to	o verify the dimensions and loads.	 astening details, beam s approvals Damaged Beams must no 	strength values, and code t be used			www.metsawood.com/us	
1. Dry service co 2. LVL not to be	onditions, unless noted otherwise • treated with fire retardant or con	4. Design assumes top edge 5. Provide lateral support a ateral diedecement of a	is laterally restrained at bearing points to avoid otation	-			соттесн
Version 21.40	305 Powered by iStruct™ [)ataset: embedded		i nis design is va	alia until 4///2024		
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lis	Design	C P A	lient: roject: .ddress:	Weaver Dev	elopment			Date: Input by: Job Name Project #:	5/12/20 David La : Lot 5 C. J0521-2	21 andry P. Stewar 2900	t Road			Page 5 of 8
GDH	Kerto-S LV	/L 1.	750")	(11.87	'5'' 3-l	Ply - P	ASS	ED	Level: Leve	ł				
	· · ·	3			1	• • •	•••••			•			M	11 7/0"
1 SPF End	۰۰۰ d Grain		•			• • • • • •	•	• 14	•	•	2 SPF E	•• Ind Grain		
<i> </i>					18'10"							,		5 1/4"
/					18'10"							,	ſ	
Member In	formation		Annlingti	[React	ions UNI	PATTERI	NED Ib	(Uplift)	0	105	Ormat
Plies:	Girder 3		Design N	Aethod:	ASD		Brg	Direction Vertical	LIVE) I	Jead 2720	5now 188	vvina 0	Const
Moisture Con	dition: Dry		Building	Code: I	BC/IRC 2015		2	Vertical	()	2720	188	0	0
Deflection LL:	480		Load Sh	aring:	Yes									
Deflection TL:	360 Normal - II		Deck:	Г	Not Checked									
Temperature:	Temp <= 100°	°F												
							Beari	ngs						
							Bear	ing Length	n Dir. Vort	Cap. F	React D/L I	b Tota	Ld. Case	Ld. Comb.
							End	PF 3.300	ven	10 /0	2720710	0 2900		D+3
Analysis Re	sults						Grair) DE 3.500"	Vort	18%	2720 / 18	8 2009	. 1	D+S
Analysis	Actual	Location A	llowed	Capacity	Comb.	Case	End	FF 3.300	ven	10 /0	2720710	0 2300	, ,	013
Unbraced	13035 ft-Ib	9'5" 1: 9'5" 1:	3043 ft-lb	0.999 (100%)	D+S	L	Grair	1						
Shear	2364 lb	17'6 5/8" 1	1970 I b	0 197 (20%	%) D	Uniform								
LL Defl inch	0.037 (L/6029)	9'5 1/16" 0.	.459 (L/480) 0.080 (8%)) S	L								
TL Defl inch	0.565 (L/390)	9'5 1/16" 0.	.612 (L/360) 0.922 (92%	%) D+S	L	-							
1 Provide su may also b 2 Fasten all p to exceed 6 3 Refer to las 4 Girders are 5 Top loads r	pport to prevent later e required at the inte blies using 2 rows of 5". It page of calculation designed to be supp must be supported er	al movement rior bearings 10d Box nails is for fastener ported on the qually by all pl	and rotation by the build (.128x3") a s required f bottom edg lies.	n at the end b ing code. at 12" o.c. Ma or specified l e only.	pearings. Later Iximum end dis oads.	al support								
7 Bottom mu	st be laterally braced at	a maximum o I at end bearir	n 1011-13/1 ngs.	υ U.C.										
8 Lateral sler	nderness ratio based	on single ply	width.		Sida	Dead 0.0	<u> </u>	No 1 Cro	NA/ 1 15	Wind 1	6 Const	1.25 0	mmonto	
	Loau Type Uniform	L	υσαιιση	THE WILLT	Тор	60 PI F	LI N	PLF	0 PLF	0 PI	F r	1.20 U	all	
2	Tie-In	0-0-0 to	18-10-0	1-0-0	Тор	20 PSF	0	PSF	20 PSF	0 PS	F C	PSF Ro	of	
3	Uniform				Тор	195 PLF	0	PLF	0 PLF	0 PL	F (PLF B1	GE	
	Self Weight					14 PLF								
Notes		chemicals	5		6. For flat	roofs provide p	roper draina	ge to prevent	Manufactu	rer Info		Comtec	n, Inc. Reilly Road, Suite #	639
Calculated Structured structural adequacy design criteria and responsibility of the ensure the compor	Designs is responsible only o of this component based on d loadings shown. It is customer and/or the contracto ent suitability of the inter	the the the the the the the the the the	& Installatic ns must not be cu o manufacturer installation details, beam s	n tordri∎ed 's product info requirements, n trength values, an	ponding mation nulti-ply d code				Metsä Woo 301 Merritt Norwalk, C (800) 622-5	d 7 Building, T 06851 850	2nd Floor	Fayetter USA 28314 910-864	-TRUS	
application, and to ver Lumber 1. Dry service condit 2. LVL not to be treat	my me dimensions and loads. ions, unless noted otherwise ited with fire retardant or corro	approvals 3. Damaged 4. Design as 5. Provide I lateral dis	s Beams must not ssumes top edge lateral support a splacement and ro	t be used is laterally restraine t bearing points to otation	ad avoid This c	lesign is valid	until 4/7/2	2024	www.metsa ICC-ES: ES	wood.com/ SR-3633	us		comt	есн
Version 21 40 305	Powered by iStruct D	ataset: embedd	lod										A DEALE	

	/		Client:	Weaver Developr	nent	Date	e:	5/12/2021	Page 6 of 8
Í	isDesign		Project:			Inpu	ut by: Name	David Landry	
+	Ispesign		Address.			Proj	ject #:	J0521-2900	
GDH	Kerto-S	LVL	1.750"	X 11.875"	3-Plv -	- PASSED	L	₋evel: Level	
					· · ·)				
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r	• • •	• •	• •	• •	• •	• •	•	• • • •	
.							•		
1 SPF	End Grain							2 SPF End	Grain A
/					18'10"				
/					18'10"				
'					1010				I
Multi-Dly	/ Analysis								
Easten all	nliec using 2 i	rows of 1	Od Boy nails	(128v3") at 12	" o.c. Nail fro	om hoth sides	Mavi	mum and distance not to	exceed
6".	plies using 21			(.120X3) at 12	U.C. Mail IIC	JIII DOUI SIGES.	Ινιάλι		exceeu
Capacity		0.0 %	6						
Load Yield Limit pe	er Foot	0.0 F 163.1	²LF 7 PLF						
Yield Limit pe	er Fastener	81.9	lb.						
Yield Mode Edge Distand	ce	IV 1 1/2							
Min. End Dis	tance	3"							
Load Combir Duration Fac	nation tor	1.00							
								Manufacturor late	Comtech Inc
Notes Calculated Struct	tured Designs is responsible	e on l y of the H	chemicals andling & Installa	ition	 For flat roofs pro- ponding 	vide proper drainage to pro	revent	Manuracturer Into	1001 S. Reilly Road, Suite #639 Fayetteville, NC
structural adequa design criteria	acy of this component ba and loadings shown.	it is the 2	LVL beams must not be Refer to manufact	e cut or dri∎ed urer's product information				301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-864-TRUS
ensure the cor application, and t	mponent suitability of the coverify the dimensions and	ne intended	regardıng installatio fastening details, bea approvals	n requirements, multi-ply m strength values, and code				(800) 622-5850 www.metsawood.com/us	
Lumber 1. Dry service of	onditions, un l ess noted othe	3. 4. erwise 5	Damaged Beams must Design assumes top en Provide lateral surges	t not be used dge is laterally restrained rt at bearing points to avoid				ICC-ES: ESR-3633	loom to out
2. LVL not to be	e treated with fire retardant	or corrosive	lateral displacement ar	nd rotation	This design is	valid until 4/7/2024			соттесн
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isDesign	Client: Weaver Development Project: Address:	Date: Input by: Job Nam Project #	5/12/2021 David Landry ie: Lot 5 C.P. Stewart Road : J0521-2900	Page 7 of 8
GDH2 Kerto-S LVL	1.750" X 11.875" 2-Ply -	PASSED	Level: Level	
2 · · · · · · · · · · · · · · · · · · ·	1 1 9'10" 9'10"		2 SPF End Grain	11 7/8" 11 7/8"
Member Information Type: Girder Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II	Application:FloorDesign Method:ASDBuilding Code:IBC/IRC 2015Load Sharing:NoDeck:Not Checked	Reactions UN Brg Direction 1 Vertical 2 Vertical	IPATTERNED Ib (Uplift) Live Dead 0 1653 0 1653	Snow Wind Const 1313 0 0 1313 0 0
Temperature: Temp <= 100°F				
		Bearing Lengi 1 - SPF 3.500 End	th Dir. Cap. React D/L I t " Vert 28% 1653 / 1313	o Total Ld. Case Ld. Comb. 3 2966 L D+S
Analysis Kesults Analysis Actual Location Moment 6627 ft-lb 4'11" Uppraced 6627 ft-lb 4'11"	Allowed Capacity Comb. Case 22897 ft-lb 0.289 (29%) D+S L 9857 ft-lb 0.672 (67%) D+S L	2 - SPF 3.500' End Grain	" Vert 28% 1653 / 1313	3 2966 L D+S
Shear 2202 lb 1'3 3/8" LL Defl inch 0.056 (L/2022) 4'11" TL Defl inch 0.126 (L/895) 4'11" Design Notes 1 Provide support to prevent lateral movem may also be required at the interior bearing	10197 lb 0.216 (22%) D+S L 0.234 (L/480) 0.237 (24%) S L 0.312 (L/360) 0.402 (40%) D+S L	_		
 2 Fasten all plies using 2 rows of 10d Box r to exceed 6". 3 Refer to last page of calculations for faste 4 Girders are designed to be supported on 5 Top loads must be supported equally by a 6 Top must be laterally braced at end bearin 7 Bottom must be laterally braced at end be 8 Lateral slenderness ratio based on single 	ails (.128x3") at 12" o.c. Maximum end distance not ners required for specified loads. he bottom edge only. Il plies. gs. arings. ply width.			
ID Load Type 1 Uniform 2 Uniform Self Weight	Location Trib Width Side Dead 0.1 Top 60 PLI Top 267 PLI 9 PLI	 ¿ive 1 Sn 0 PLF 0 PLF 	ow 1.15 Wind 1.6 Const. 0 PLF 0 PLF 0 267 PLF 0 PLF 0	1.25 Comments PLF Wall Above PLF G1
Notes cheat Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. 1. UVL 2000 (10	icels 6, For flat roofs provide ponding S. Installation ponding same must not be cut or drilled r to manufacturer's product information dring installation requirements, multi-ply ning details, beam strength values, and code vals gade Beams must not be used gade Beams must not be used de lateral support at bearing points to avoid	proper drainage to prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	Comtech, Inc. 1001 S. Relly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS
Version 21 40 305 Powered by iStruct TM Dataset: emb	ar displacement and rotation This design is val	id until 4/7/2024		

isDesign	Client: Weaver Developm Project: Address:	ient Date: Input b Job Na	5/12/2021 by: David Landry ame: Lot 5 C.P. Stewart Road	Page 8 of 8
GDH2 Kerto-S LVL	1.750'' X 11.875'	" 2-Ply - PASSED	t #: J0521-2900 Level: Level	
• • •	• •	• • •	• • •	
SPE End Grain	• •	• • •		
	9'10) ⁿ		3 1/2"
ſ	9'10	טיי סיי	1	
Multi-Ply Analysis Fasten all plies using 2 rows of 10d Capacity 0.0 % Load 0.0 PLF Yield Limit per Foot 163.7 PLI Yield Limit per Fastener 81.9 lb. Yield Mode IV Edge Distance 3" Load Combination Duration Factor 1.00	Box nails (.128x3") at 12" F	o.c Maximum end distance	not to exceed 6".	
Notes chem Cackulated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and badings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. 1. ULL to the transfer of the customer and/or the contractor to paper and/or the customer a	nicals ing & Installation seams must not be cut or drilled r to manufacturer's product information dring installation requirements, multi-ply ming details, beam strength values, and code vals aged Beams must not be used aged Beams must not be used aged Beams must not be used de lateral support at bearing points to avoid d displacement and rotation	 For flat roofs provide proper drainage to prever ponding This design is valid until 4/7/2024 	nt Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	Contech, Inc. 1001 S. Relly Road, Suite #639 Fayetteville, NC USA 28314 910-964-TRUS

Version 21.40.305 Powered by iStruct[™] Dataset: embedded



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0521-2900 Lot 5 C.P. Stewart Road

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15721358 thru E15721386

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



May 12,2021

Gilbert, Eric **IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



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818 Soundside Road Edenton, NC 27932



Job	Truss	Truss Type	Qty	Ply	Lot 5 C.P. Stewart Road		
10524 2000	4405	CARLE			E15721359		
J0521-2900	AIGE	GABLE	1	1	lob Reference (entionel)		
					Job Reference (optional)		
Comtech, Inc, Fayette	ville, NC - 28314,		8.	330 s Oct	7 2020 MiTek Industries, Inc. Wed May 12 11:22:32 2021 Page 2		
		ID:G?Mgu2wAOefhMIzVCCS4xvzzRiE-b01z5OIVwjnwARCvEAfNSyVFntHrH3JaN9jMWMzHF					

NOTES-

Refer to girder(s) for truss to truss connections.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18.

13) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclidual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, terction and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





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Job	Truss	Truss Type	Qty	Ply	Lot 5 C.P. Stewart Road
10504 0000		17710			E15721362
J0521-2900	A3A	ATTIC	1	2	lob Reference (ontional)
Comtach Inc Equation	IIO NC - 28214			220 c Oct	7 2020 MiTak Industrias Inc. Wed May 12 11:22:41 2021 Page 2

ID:G?Mgu2wAOefhMIzVCCS4xvzzRiE-ql4M_TP8oUwempPeGZKUKrNrvVHOu7GuR3OKKLzHFwi

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-5=-60, 5-6=-80, 6-7=-60, 7-8=-60, 8-9=-80, 9-12=-60, 2-16=-20, 13-16=-40, 12-13=-20, 6-8=-20

Drag: 5-16=-10, 9-13=-10

Concentrated Loads (lb)

Vert: 23=-1837(F)

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Design valid for use only with MiTek® connectors. This and inscribe and only and inscribe the set of the set o



Job	Truss	Truss Type	Qty	Ply	Lot 5 C.P. Stewart Roa	ad	
.10521-2900	A6	ROOF TRUSS	2	- 1			E15721366
			5	1	Job Reference (optiona	al)	
Comtech, Inc, Fayett	eville, NC - 28314,		8.3 ID:G?Mgu2wAOefhM	330 s Oct IIzVCCS4	/ 2020 MiTek Industries xvzzRiE-?sEWIEY2DtJ4	s, inc. Wed May 12 1 [.] aVkIPN03G9KfQx2_z	1:22:52 2021 Page 1 4sWzGZPDCzHFwX
	\vdash	<u>3-10-0 9-7-0 10-10</u> 3-10-0 5-9-0 1-3-0	013-7-1 19-10-0 2-9-1 6-2-15		28-3-8 8-5-8	<u>29-2-</u> 8 0-11-0	
							Scale - 1.87 0
		6x10 M18SHS =					Scale = 1.07.0
	9.00 12						
		2					
	5x8 II 3x10 =	18 3x 18 3x 15 17 6x8 11 2x6 1 	$6 = \frac{1}{8} \frac{6\times8}{5} \frac{11}{5} \frac{5}{5} \frac{1}{5} \frac{1}{$	0 <u>12</u> 2x	4 // 4x6 ≈		
	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	6 - - 0 -		6	7		
				/		2-910 B	
	14	13	12 11 ²⁰ 21 10)	4x8		
	5x8	= 3x10	$5x8 = {}^{8x8} = {}^{4x6}$	=			
	⊢	3-10-0 10-10-0	17-10-0		28-3-8		
Plate Offsets (X,Y) [4	:0-7-14,Edge], [13:0-7-12,0-1-	3-10-0 7-0-0 3], [14:0-3-12,0-3-0]	7-0-0		10-5-8		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.67 BC 0.90 WB 0.94	DEFL. in Vert(LL) -0.22 Vert(CT) -0.46 Horz(CT) 0.03	(loc) 10-12 10-12 8	l/defl L/d >999 360 >736 240 n/a n/a	PLATES MT20 M18SHS	GRIP 244/190 244/190
BCDL 10.0	Code IRC2015/1PI2014	Matrix-S	Wind(LL) 0.15	10-12	>999 240	Weight: 322 lb	F1 = 20%
LUMBER- TOP CHORD 2x8 SP N 5-7,7-9:2 BOT CHORD 2x8 SP N 11-14:22 WEBS 2x6 SP N 2-17,5-12	lo.1 *Except* 2x6 SP No.1 lo.1 *Except* 10 SP No.1 lo.1 *Except* 2,5-10,6-10: 2x4 SP No.2		BRACING- TOP CHORD BOT CHORD WEBS JOINTS	Structura except e Rigid ce 1 Row a 1 Brace	al wood sheathing dire end verticals. iling directly applied or it midpt 3-4 at Jt(s): 15	ctly applied or 4-5-1 5-11-2 oc bracing. 15, 14-15	3 oc purlins,
REACTIONS. (size) Max Hor Max Gra	14=0-3-8, 8=0-3-8 z 14=-386(LC 13) v 14=1899(LC 21), 8=1327(L0	C 2)					
FORCES. (lb) - Max. C	omp./Max. Ten All forces 250) (lb) or less except when shown					
TOP CHORD 1-2=-46 14-16=	6/909, 2-3=-81/883, 3-4=-845/0 -69/1026, 1-16=-13/598	0, 4-5=-1740/0, 5-6=-2479/0, 6-8	=-2689/6,				
BOT CHORD 13-14= WEBS 13-15= 14-15=	0/1154, 12-13=0/1196, 10-12= 0/1674, 4-12=0/1284, 15-16=- -3791/83, 2-17=-1396/254, 5-1	0/1729, 8-10=0/2401 703/80, 15-17=-2008/133, 3-17= 2=-1480/147, 5-10=-173/947, 6-	-1755/94, -10=-420/248				
NOTES- 1) Unbalanced roof live la 2) Wind: ASCE 7-10; Vul MWFRS (envelope) an MWFRS for reactions 3) All plates are MT20 pl 4) This truss has been de 5) * This truss has been de 5) * This truss has been de will fit between the bot 6) Ceiling dead load (10. 7) Bottom chord live load 8) Attic room checked for	bads have been considered for t=130mph (3-second gust) Vand C-C Exterior(2) 0-2-12 to 8- shown; Lumber DOL=1.60 pla ates unless otherwise indicated signed for a 10.0 psf bottom c designed for a live load of 30.0 tom chord and any other mem 0 psf) on member(s). 3-4, 4-5, (40.0 psf) and additional botto L/360 deflection.	this design. sd=103mph; TCDL=6.0psf; BCD 2-13, Interior(1) 8-2-13 to 28-11- te grip DOL=1.60 d. hord live load nonconcurrent with psf on the bottom chord in all are bers, with BCDL = 10.0psf. 15-16, 15-17, 3-17; Wall dead k m chord dead load (10.0 psf) ap	L=6.0psf; h=15ft; Cat. II; 14 zone;C-C for member h any other live loads. eas where a rectangle 3-6 pad (5.0psf) on member(s plied only to room. 12-13	Exp C; Er s and for 6-0 tall by s).13-15,	nclosed; ces & 2-0-0 wide 4-12	A A A A A A A A A A A A A A A A A A A	y 12,2021

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Job	Truss	Truss Type	Qty	Ply	Lot 5 C.P. Stewart Road	
J0521-2900	A6GE	GABLE	1	1		E15721368
Comtech, Inc, Faye	tteville, NC - 28314,		8.;	330 s Oct	Job Reference (optional) 7 2020 MiTek Industries, Inc.	. Wed May 12 11:22:55 2021 Page 1
	1	3-10-0 J 9-7-0 10-10-0	ID:G?Mgu2wAOefhM 013-7-1 19-10-0	IzVCCS4x	vzzRiE-QRwfwFawVohfRzTK 28-3-8 29-21	(4VamuoyCO85hAVbyfEn4qXzHFwU 18
	F=	3-10-0 5-9-0 1-3-0	2-9-1 6-2-15	1	8-5-8 0 ⁻ 11-	[!] 0
		6x10 M18SHS =				Scale = 1:87.0
	9.00 12					
		2				
	$5x8 1$ $3x10 = 29$ $\frac{1}{1}$	28 30 35 36 6x6 11	$ \begin{array}{c} 4x12 \\ 6 \\ 7 \\ 8 \\ 2x4 \\ 38 \\ 9 \\ 38 \\ 38 \\ 38 \\ 38 \\ 38 \\ 38 \\ 38 \\ 38$	0 <u>12</u> 2x4 2x 10	4 <i>//</i> 4x6 ≂	
		Ø 1 4×8 Ⅲ 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8 3x10 18		12 2X4 12 13 2X4 14 2X4 15 16 16	31-12 31-12 32-12
	27	26 2 - 3×10 II	25 24 23 22 21 $5 \times 8 - 6 \times 8 = 8 \times 8$	_	20 19 18 4x8 =	0
	5X8	<u> </u>	17-10-0		28-3-8	
Plate Offsets (X,Y) [8:0-5-8,0-4-0], [21:0-4-0,0-3-8],	<u>3-10-0</u> 7-0-0 [26:0-7-8,0-1-8], [38:0-2-8,0-1-7]	7-0-0		10-5-8	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-C Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.56 BC 0.83 WB 0.68 Matrix-S	DEFL. in Vert(LL) -0.17 Vert(CT) -0.37 Horz(CT) 0.03 Wind(LL) 0.18	(loc) 25 23-25 16 23-25	l/defl L/d >999 360 >898 240 n/a n/a >999 240	PLATES GRIP MT20 244/190 M18SHS 244/190 Weight: 372 lb FT = 20%
LUMBER-			BRACING-			
TOP CHORD 2x8 SP 8-12.12	No.1 *Except* -17: 2x6 SP No.1		TOP CHORD	Structura except e	al wood sheathing directly a and verticals.	applied or 4-11-1 oc purlins,
BOT CHORD 2x8 SP	No.1 *Except*		BOT CHORD	Rigid ce	iling directly applied or 10-0)-0 oc bracing, Except:
WEBS 2x6 SP	No.1 *Except*		WERS	6-11-2 c	c bracing: 20 27 c bracing: 25-26.	0.05
OTHERS 2x4 SP	No.2		JOINTS	1 Brace	at Jt(s): 28, 35, 39	5-25
REACTIONS. (size) Max Ho Max Up Max Gr) 27=0-3-8, 16=0-3-8 yrz 27=-563(LC 13) plift 27=-62(LC 13), 16=-134(LC av 27=1798(LC 21), 16=1288(13) _C 1)				
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-128/768, 2-3=-152/797, 3-4=-176/666, 4-5=-229/603, 5-6=-810/21, 6-7=-1473/0, 7-8=-1553/0, 8-9=-2182/301, 9-10=-2203/254, 10-11=-2244/236, 11-13=-2332/264, 13-14=-2405/262, 14-15=-2401/211, 15-16=-2447/172, 27-29=-204/868, 1-29=-80/527 BOT CHORD 26-27=0/1083, 25-26=0/1123, 23-25=0/1668, 22-23=0/1668, 21-22=0/1668, 20-21=-104/2197, 19-20=-104/2197, 18-19=-104/2197 WEBS 26-28=-65/1610, 6-25=-26/999, 28-29=-589/141, 28-30=-1741/298, 30-35=-1523/241, 35-36=-1524/241, 5-36=-1525/239, 27-28=-3429/396, 2-30=-1299/326, 25-37=-1571/470, 8-37=-1650/498, 8-38=-491/1192, 38-39=-311/673, 21-39=-325/714, 11-21=-413/251, 23-38=-204/583						
 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. 4) All plates are MT20 plates unless otherwise indicated. 6) Gable studs spaced at 2-0-0 oc. 7) This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 9) Ceiling dead load (10.0 psf) on member(s). 5-6, 28-29, 28-30, 30-35, 35-36, 5-36; Wall dead load (5.0psf) on member(s).26-28, 6-25 10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 25-26 Continued on page 2 						
WARNING - Verify des Design valid for use only a truss system. Before us building design. Bracing is always required for sta fabrication, storage, deliv Safety Information ava	ign parameters and READ NOTES ON T with MiTek® connectors. This design is e, the building designer must verify the indicated is to prevent buckling of indivi- bility and to prevent collapse with possil ery, erection and bracing of trusses and lable from Truss Plate Institute, 2670 C	HIS AND INCLUDED MITEK REFERENCE F based only upon parameters shown, and i applicability of design parameters and prop dual truss web and/or chord members only le personal injury and property damage. F truss systems, see ANS/TP/1 Q ain Highway, Suite 203 Waldorf, MD 2060	PAGE MII-7473 rev. 5/19/2020 f is for an individual building com perly incorporate this design ini y. Additional temporary and pe For general guidance regarding tuality Criteria, DSB-89 and B 11	BEFORE US aponent, not to the overa rmanent bra the CSI Building	E. Il Icing <i>g Component</i>	AND

Job	Truss	Truss Type	Qty	Ply	Lot 5 C.P. Stewart Road
J0521-2900	A6GE	GABLE	1	1	E15721368
					Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8.	330 s Oct	7 2020 MiTek Industries, Inc. Wed May 12 11:22:55 2021 Page 2
		ID:G?Mgu2wAOefhMlzVCCS4xvzzRiE-QRwfwFawVohfRzTK4VamuoyCO85hAVbyfEn4qXzHFwU			

NOTES-

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 27 except (jt=lb) 16=134.

12) Attic room checked for L/360 deflection.

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	⊢	<u>4-8-12</u> <u>10-9-8</u> <u>4-8-12</u> <u>6-0-12</u>	16-10-4	4-8-12	
Plate Offsets (X,Y)	[1:0-1-4,0-2-0], [7:0-1-4,0-2-0], [9:0-4-0	,0-4-12], [11:0-4-0,0-4-12]	0-0-12	4-0-12	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.78 BC 0.75 WB 0.20 Matrix-S	DEFL. in Vert(LL) -0.23 Vert(CT) -0.39 Horz(CT) 0.01 Wind(LL) 0.07	(loc) I/defl L/d 9-11 >999 360 9-11 >653 240 8 n/a n/a 9-11 >999 240	PLATES GRIP MT20 244/190 Weight: 223 lb FT = 20%
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x10 S WEBS 2x6 SF 4-13,1-	? No.1 P No.1 ? No.1 *Except* 11,7-9: 2x4 SP No.2	· · · ·	BRACING- TOP CHORD BOT CHORD JOINTS	Structural wood sheathing d except end verticals. Rigid ceiling directly applied 1 Brace at Jt(s): 13	lirectly applied or 4-2-11 oc purlins, or 9-6-8 oc bracing.
REACTIONS. (size Max H Max G	e) 12=0-3-8, 8=Mechanical orz 12=313(LC 11) rav 12=1446(LC 21), 8=1446(LC 20)				
FORCES. (lb) - Max. TOP CHORD 1-2=- BOT CHORD 11-12 WEBS 6-9=-	Comp./Max. Ten All forces 250 (lb) o 1600/0, 2-3=-984/147, 5-6=-984/147, 6 2=-303/406, 9-11=0/997 6/678, 2-11=-7/678, 3-13=-1036/187, 5	r less except when shown. -7=-1600/0, 1-12=-1600/0, 7 -13=-1036/187, 1-11=0/915,	7-8=-1601/0 , 7-9=0/919		
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) to 21-4-4 zone; end plate grip DOL=1.60 3) This truss has been	e loads have been considered for this de fult=130mph (3-second gust) Vasd=103 and C-C Exterior(2) 0-2-12 to 4-8-12, Ir vertical left and right exposed;C-C for n	esign. imph; TCDL=6.0psf; BCDL= iterior(1) 4-8-12 to 10-9-8, E embers and forces & MWFf re load popconcurrent with a	6.0psf; h=15ft; Cat. II; xterior(2) 10-9-8 to 15 RS for reactions show	Exp C; Enclosed; -2-5, Interior(1) 15-2-5 n; Lumber DOL=1.60	
 4) * This truss has been 	n designed for a live load of 30.0psf on	the bottom chord in all areas	s where a rectangle 3-	6-0 tall by 2-0-0 wide	

(i) This trass been besigned for a live load of 50.505 of the bottom chord in an areas where a rectangle 2000 will fit between the bottom chord and any other members.
(5) Ceiling dead load (10.0 psf) on member(s). 2-3, 5-6, 3-13, 5-13; Wall dead load (5.0psf) on member(s).6-9, 2-11
(6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 9-11
(7) Refer to girder(s) for truss to truss connections.
(8) Attic room checked for L/360 deflection.



May 12,2021

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SI IDER Left 2x4 SP No.2 -x 2-6-0. Right 2x4 SP No.2 -x 2-6-0

REACTIONS. All bearings 13-5-0.

(lb) - Max Horz 2=-224(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 15, 13 except 16=-256(LC 12), 12=-251(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 10, 14, 15, 13 except 16=270(LC 19), 12=265(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 4-16=-280/263, 8-12=-280/260

NOTES-1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 15, 13 except (jt=lb) 16=256, 12=251.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



May 12,2021

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6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



May 12,2021

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3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11.



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ss Truss Type	Qty P	Ply	Lot 5 C.P. Stewart Road
			E15721378
GR COMMON GIRDE	. 1	2	
		J	Job Reference (optional)
NC - 28314,	8.33	30 s Oct	7 2020 MiTek Industries, Inc. Wed May 12 11:23:15 2021 Page 2
ī	SS Truss Type GR COMMON GIRDER NC - 28314.	ss Truss Type Qty F GR COMMON GIRDER 1 1 NC - 28314. 8.33 8.33	ss Truss Type Qty Ply GR COMMON GIRDER 1 3 NC - 28314. 8.330 s Oct

ID:G?Mgu2wAOefhMIzVCCS4xvzzRiE-qI7D75pToxCpr2?AFixTi0nWvC18saMvGLd7XMzHFwA

LOAD CASE(S) Standard Concentrated Loads (lb)

Vert: 13=-1547(B) 12=-1645(B) 15=-1547(B) 16=-1547(B) 17=-1547(B) 18=-1547(B) 19=-1547(B) 20=-1547(B) 21=-1645(B) 22=-1645(B) 23=-1645(B) 23=-1645(B)

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Plate Offsets (X,Y)	[3:0-3-8,0-2-8]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.20 BC 0.14 WB 0.37 Matrix-S	DEFL. in (loc) I/defl L/d Vert(LL) -0.02 13-15 >999 360 Vert(CT) -0.03 12-13 >999 240 Horz(CT) 0.01 12 n/a n/a Wind(LL) 0.01 12-13 >999 240	PLATES GRIP MT20 244/190 Weight: 259 lb FT = 20%

LUMBER- TOP CHORD BOT CHORD WEBS OTHERS SLIDER REACTIONS.	2x6 SP No.1 2x6 SP No.1 2x4 SP No.2 *Except* 9-16: 2x6 SP No.1 2x4 SP No.2 Left 2x4 SP No.2 Left 2x4 SP No.2 -x 8-5-2, Right 2x4 SP No.2 -x 4-8-11 All bearings 11-1-8 except (jt=length) 12=0-3-8, 15=0-3-8.	BRACING- TOP CHORD BOT CHORD WEBS JOINTS	Structural wood sheathing Rigid ceiling directly applie T-Brace: Fasten (2X) T and I brace (0.131*x3*) nails, 6in o.c., Brace must cover 90% of 1 Brace at Jt(s): 21, 22, 29	g directly applied or 6-0-0 oc purlins. ed or 10-0-0 oc bracing. 2x4 SPF No.2 - 5-16, 4-17 es to narrow edge of web with 10d with 3in minimum end distance. web length.
(lb) -	Max Uplift All uplift 100 lb or less at joint(s) 1, 12, 17, 20 except 16=-205(LC	; 13),		

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-3=-503/279, 3-4=-349/173, 4-5=-269/210, 8-9=-311/176, 9-12=-683/95

BOT CHORD 1-20=-204/371, 19-20=-204/371, 18-19=-204/371, 17-18=-205/372, 16-17=-205/372, 15-16=0/417, 13-15=0/417, 12-13=0/417

Max Grav All reactions 250 lb or less at joint(s) 17, 19, 20 except 1=385(LC 21), 12=663(LC 20), 16=287(LC 1), 18=434(LC 19), 15=352(LC 18)

WEBS 16-25=-528/327, 21-25=-506/310, 9-21=-552/358, 9-13=0/298, 3-18=-507/461

NOTES-

1) Unbalanced roof live loads have been considered for this design.

18=-446(LC 12)

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 12, 17, 20 except (jt=lb) 16=205, 18=446.
- 9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.







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					10-11-0		
8], [4:0-6-7,0-1-8]							
ING- 2-0-0 Grip DOL 1.15 er DOL 1.15 tirtess Incr YES IRC2015/TPI2014	CSI. TC 0.63 BC 0.50 WB 0.15 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.08 4-7 -0.18 4-7 0.02 4 0.06 2-7	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 122 lb	GRIP 244/190 FT = 20%
	8], [4:0-6-7,0-1-8] XING- 2-0-0 Grip DOL 1.15 er DOL 1.15 Stress Incr YES IRC2015/TPI2014	8], [4:0-6-7,0-1-8] XING- 2-0-0 CSI. Grip DOL 1.15 TC 0.63 er DOL 1.15 BC 0.50 Stress Incr YES WB 0.15 IRC2015/TPI2014 Matrix-S	BI. [4:0-6-7,0-1-8] ZING- 2-0-0 CSI. DEFL. Grip DOL 1.15 TC 0.63 Vert(LL) ver DOL 1.15 BC 0.50 Vert(CT) Stress Incr YES WB 0.15 Horz(CT) IRC2015/TPI2014 Matrix-S Wind(LL)	B. [4:0-6-7,0-1-8] ZING- 2-0-0 CSI. DEFL. in (loc) Grip DOL 1.15 TC 0.63 Vert(LL) -0.08 4-7 er DOL 1.15 BC 0.50 Vert(CT) -0.18 4-7 Stress Incr YES WB 0.15 Horz(CT) 0.02 4 IRC2015/TPI2014 Matrix-S Wind(LL) 0.06 2-7	BI. [4:0-6-7,0-1-8] ZING- 2-0-0 CSI. DEFL. in (loc) I/defl Grip DOL 1.15 TC 0.63 Vert(LL) -0.08 4-7 >999 ver DOL 1.15 BC 0.50 Vert(CT) -0.18 4-7 >999 stress Incr YES WB 0.15 Horz(CT) 0.02 4 n/a IRC2015/TPI2014 Matrix-S Wind(LL) 0.06 2-7 >999	BI. [4:0-6-7,0-1-8] ZING- 2-0-0 CSI. DEFL. in (loc) l/defl L/d Grip DOL 1.15 TC 0.63 Vert(LL) -0.08 4-7 >999 360 ver DOL 1.15 BC 0.50 Vert(CT) -0.18 4-7 >999 240 Stress Incr YES WB 0.15 Horz(CT) 0.02 4 n/a n/a IRC2015/TPI2014 Matrix-S Wind(LL) 0.06 2-7 >999 240	BI. [4:0-6-7,0-1-8] ZING- 2-0-0 CSI. DEFL. in (loc) l/defl L/d PLATES Grip DOL 1.15 TC 0.63 Vert(LL) -0.08 4-7 >999 360 MT20 er DOL 1.15 BC 0.50 Vert(CT) -0.18 4-7 >999 240 Stress Incr YES WB 0.15 Horz(CT) 0.02 4 n/a n/a IRC2015/TPI2014 Matrix-S Wind(LL) 0.06 2-7 >999 240 Weight: 122 lb

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 4=0-3-8 Max Horz 2=76(LC 11) Max Uplift 2=-64(LC 12), 4=-64(LC 13) Max Grav 2=953(LC 2), 4=953(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1379/292, 3-4=-1379/292

BOT CHORD 2-7=-93/1123, 4-7=-93/1123 WEBS 3-7=0/655

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-2 to 3-7-11, Interior(1) 3-7-11 to 10-11-8, Exterior(2) 10-11-8 to 15-4-5, Interior(1) 15-4-5 to 22-8-2 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Structural wood sheathing directly applied or 5-6-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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818 Soundside Road

Edenton, NC 27932



			21-11-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.03 BC 0.02 WB 0.06	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) 0.00 12 n/r 120 MT20 244/190 Vert(CT) 0.00 12 n/r 120 MT20 244/190 Horz(CT) 0.00 12 n/a n/a n/a N	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Weight: 154 lb FT = 20%	
LUMBER-			BRACING-	

TOP CHORD

BOT CHORD

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. All bearings 21-11-0.

- Max Horz 2=119(LC 12) (lb) -
 - Max Uplift All uplift 100 lb or less at joint(s) 2, 20, 21, 22, 17, 16, 15, 12 except 23=-109(LC 12), 14=-106(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 19, 20, 21, 22, 23, 17, 16, 15, 14, 12
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 21, 22, 17, 16, 15, 12 except (jt=lb) 23=109, 14=106.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🛕 WARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTerk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses systems, see **ANS/TPI Quality Criteria, DSB-89 and BCSI Building Compore Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. All bearings 12-8-3.

(lb) - Max Horz 1=144(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-161(LC 12), 6=-161(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=342(LC 19), 6=342(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 2-8=-356/291, 4-6=-355/291

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 6-4-1, Exterior(2) 6-4-1 to 10-8-14, Interior(1) 10-8-14 to 12-3-15 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=161.6=161.



May 12,2021

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REACTIONS. (size) 1=9-8-3, 3=9-8-3, 4=9-8-3

Max Horz 1=-108(LC 8) Max Uplift 1=-27(LC 13), 3=-27(LC 13)

Max Grav 1=204(LC 1), 3=204(LC 1), 4=311(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate arip DOL=1.60

- 3) Gable requires continuous bottom chord bearing. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=6-8-3, 3=6-8-3, 4=6-8-3

Max Horz 1=-72(LC 8)

Max Uplift 1=-26(LC 13), 3=-26(LC 13)

Max Grav 1=146(LC 1), 3=146(LC 1), 4=187(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

arip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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BRACING-

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 BOT CHORD

LUMBER-

2x4 SP No.2 OTHERS

REACTIONS. (size) 1=3-8-3, 3=3-8-3, 4=3-8-3

Max Horz 1=-36(LC 8)

Max Uplift 1=-13(LC 13), 3=-13(LC 13)

Max Grav 1=72(LC 1), 3=73(LC 1), 4=93(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate arip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 3-8-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 12,2021

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