

Dimension Notes

 Dimension Notes

 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise

 2. All interior wall dimensions are to face of frame wall unless noted otherwise

 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

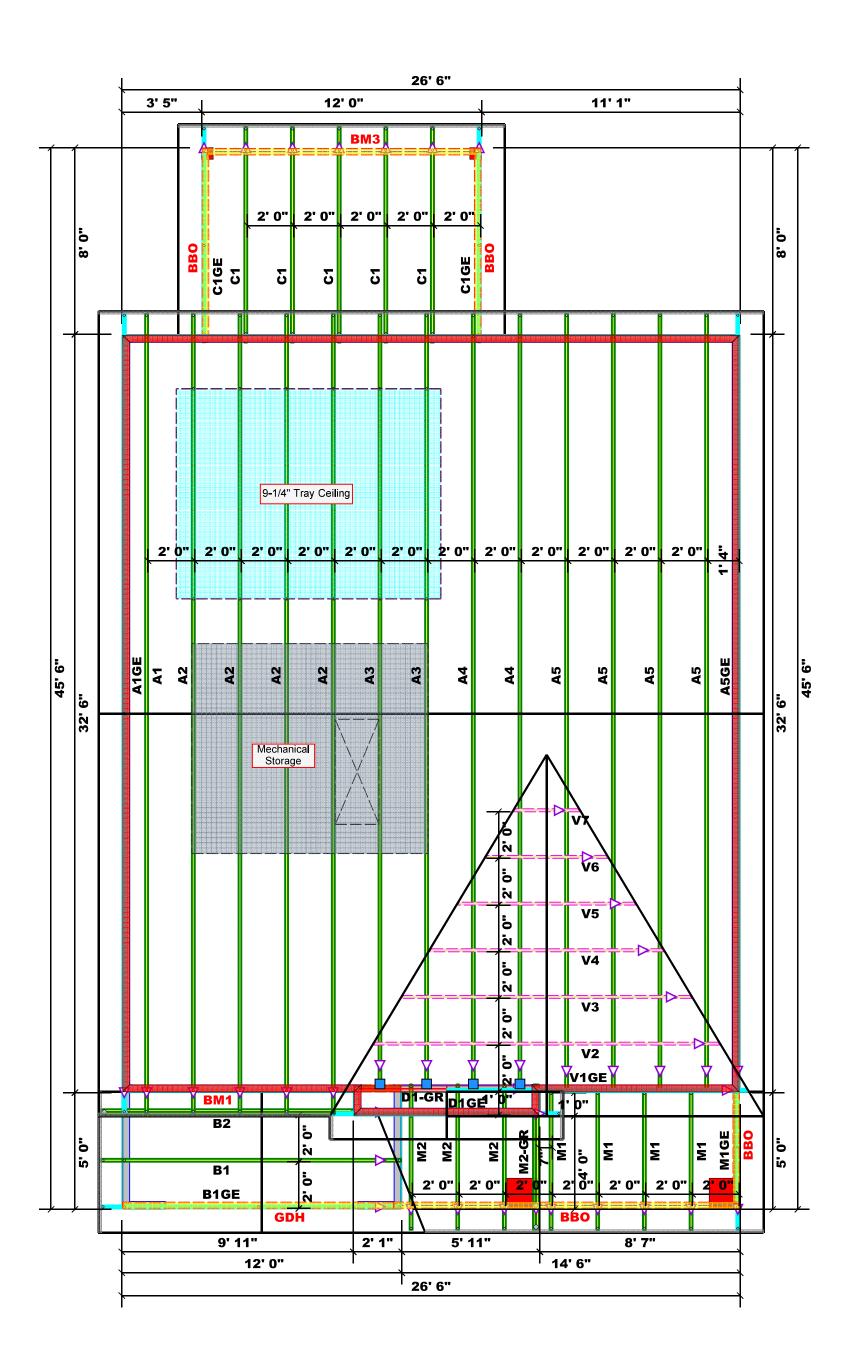
All Walls Shown Are Considered Load Bearing

					Hatch	Legend
	Area = 1468.1 sc	.ft.			Pad	ded HVAC
Hip L	e Line = 52.07 ft. ine = 0 ft. . OH = 98.57 ft.				2nd	Floor Walls
Rake	d OH = 159.04 ft ing = 50 sheets				Tray	y Ceiling
					Dro	p Beam
	Conne	ctor Info	rmati	ion	Nail Info	ormation
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	4	NA	16d/3-1/2"	16d/3-1/2"

		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM2	15' 0''	1-3/4"x 16" LVL Kerto-S	2	2
BM3	12' 0"	2x10 SPF No.2	2	2
GDH	12' 0"	2x12 SPF No.2	2	2

Truss Placement Plan Scale: 1/4"=1'





Bearing deemed requirem size and restions 15000#, / retained	eilly R Fayet Phon Fax: reactions to compl ents. Thu Tables (ents) to number s greater A registe to design	SES toad In teville e: (91) : (910)		EAN ial Par 28309 -8787 4444 tive Code efer to th rescriptiv imum fou aired to s t greater f ional sha im for any	AS k * are e re Code indation upport than II be							
Tables. A retained reactions	retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#. Signature											
END REACTION (UP TO)	NUMBER Z <thz< th=""> Z Z <th< th=""></th<></thz<>											
CITY / CO . Sanford / Harnett	4903 Barbecue Church Rd.	Roof	05/13/21	DRAWN BY David Landry	SALES REP. Lenny Norris							
CITY / CO .	ADDRESS	MODEL	DATE REV . 05/13/21	DRAWN BY	SALES REP.							
Weaver Development Co. Inc.	Lot 3 Barbecue Church Rd.	Hickory "C"	Seal Date	Quote #	J0521-2896							
BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #							
These t compor design See ind identifie	russes an nents to b at the sp ividual de ed on the	re design be incorpo ecification esign she	MENT DIA ed as indi orated inte n of the b ets for ea nt drawing or tempor	vidual bu o the buil uilding de ch truss g. The bu	ilding ding signer. design							

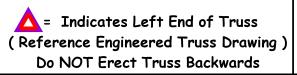
	Dimension Notes
face of shea 2. All interio frame wall u 3. All exterio	or wall to wall dimensions are to athing unless noted otherwise or wall dimensions are to face of unless noted otherwise or wall to truss dimensions are to ne wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

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	Area = 1468.1 sc e Line = 52.07 ft.	Į.ft.			Pad	lded HVAC
Hip L	= 0 ft OH = 98.57 ft.				2nd	Floor Walls
Rake	d OH = 159.04 ft ing = 50 sheets				Tra	y Ceiling
					Dro	p Beam
	Conne	ctor Info	rmati	ion	Nail Info	ormation
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	4	NA	16d/3-1/2"	16d/3-1/2"

	Products									
PlotID	Length	Product	Plies	Net Qty						
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2						
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2						
BM3	12' 0"	2x10 SPF No.2	2	2						
GDH	12' 0''	2x12 SPF No.2	2	2						

Truss Placement Plan Scale: 1/4"=1'



See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



Trenco 818 Soundside Rd Edenton, NC 27932

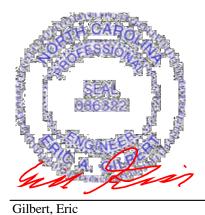
Re: J0521-2896 Lot 3 Barbecue Church Rd.

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15725374 thru E15725398

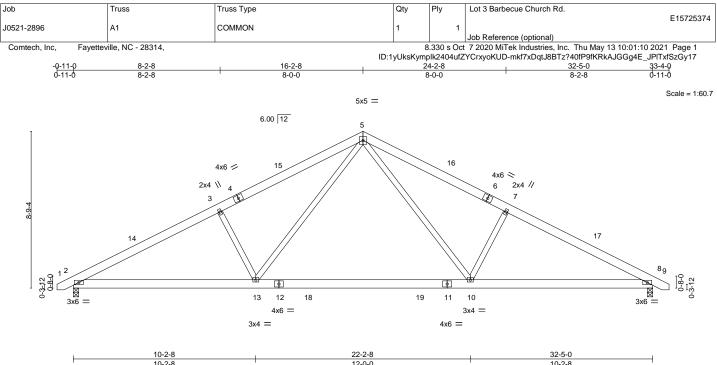
My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



May 13,2021

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



	1	10-2-8	1	12-0-0	1	10-2-8
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc)	l/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.28	Vert(LL) -0.34 10-13	>999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.64	Vert(CT) -0.47 10-13	>824 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.27	Horz(CT) 0.05 8	n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.05 2-13	>999 240	Weight: 208 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-110(LC 10) Max Uplift 2=-89(LC 12), 8=-89(LC 13)

Max Grav 2=1337(LC 1), 8=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2307/486, 3-5=-2125/534, 5-7=-2125/534, 7-8=-2307/486

BOT CHORD 2-13=-316/2007, 10-13=-106/1303, 8-10=-320/1964

WEBS 5-10=-147/921, 7-10=-454/288, 5-13=-147/921, 3-13=-454/288

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Frovide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2 and 89 lb uplift at joint 8.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

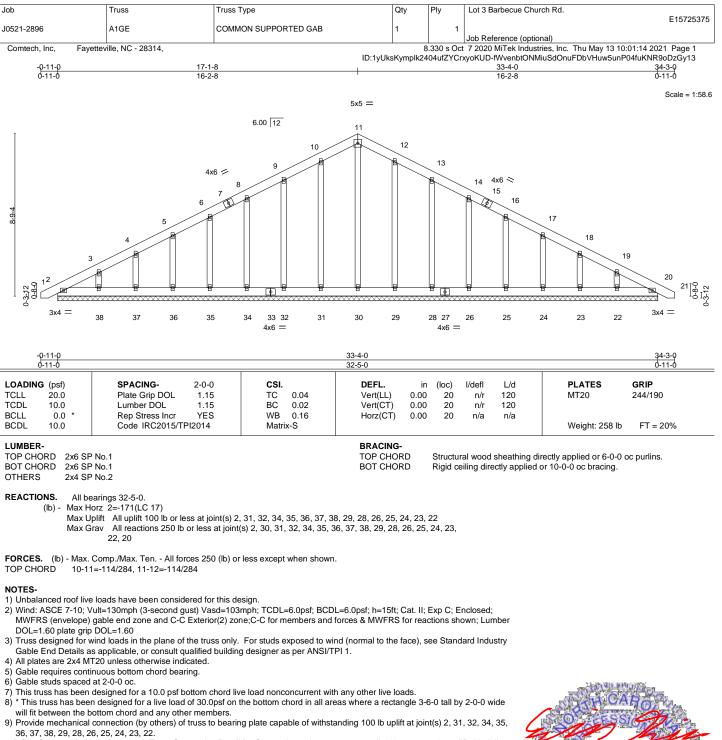


Structural wood sheathing directly applied or 4-11-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



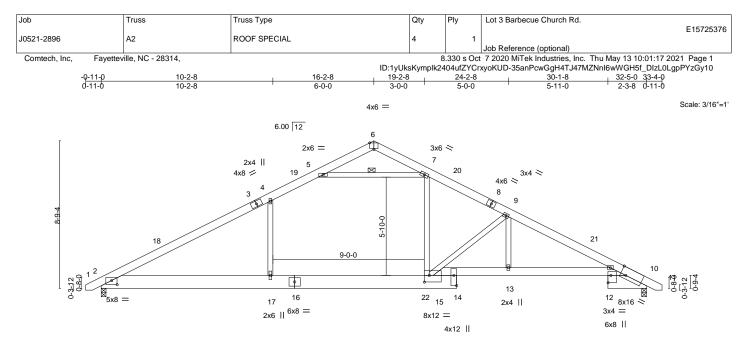


10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



🛕 WARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTE k® connectors. This sket on LCULCUL MIT IN EXPECTED EXECT PAGE MIT-F473 (eV. 319/2/20 BEFORE USE. Design valid for use only with MTE k® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Comport Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





H	10-2-8 10-2-8	<u>16-2-8</u> 6-0-0	<u>19-2-8</u> 21-1- 3-0-0 1-11-		30-1-8		H
Plate Offsets (X,Y)	[2:0-4-0,0-2-14], [6:0-3-0,Edge], [10:0-4	-0,Edge], [12:Edge,0-2-0],	, [14:Edge,0-2-0], [15:0-3	3-8,0-4-12]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.75 BC 0.67 WB 0.70 Matrix-S	DEFL. in Vert(LL) -0.21 Vert(CT) -0.38 Horz(CT) 0.09 Wind(LL) 0.17	17 >999 10 n/a	9 360 9 240 a n/a	PLATES MT20 Weight: 247 lb	GRIP 244/190 FT = 20%
BOT CHORD 2x10 10-15	P No.1 SP No.1 *Except* : 2x6 SP 2400F 2.0E P No.2		BRACING- TOP CHORD BOT CHORD WEBS		directly applied or 1	ttly applied or 4-0-8 c 10-0-0 oc bracing.	oc purlins.
Max Max	ze) 2=0-3-8, 10=0-3-8 Horz 2=110(LC 11) Uplift 2=-90(LC 12), 10=-90(LC 13) Grav 2=1393(LC 2), 10=1353(LC 2)						
TOP CHORD 2-4= BOT CHORD 2-17	.: Comp./Max. Ten All forces 250 (lb) or 2217/403, 4-5=-1870/483, 7-9=-2258/5 7=-193/1848, 15-17=-195/1860, 13-15=-3 7=-29/402, 7-15=-114/967, 9-15=-1075/2	19, 9-10=-2889/551 71/2525, 10-13=-380/252	5				

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

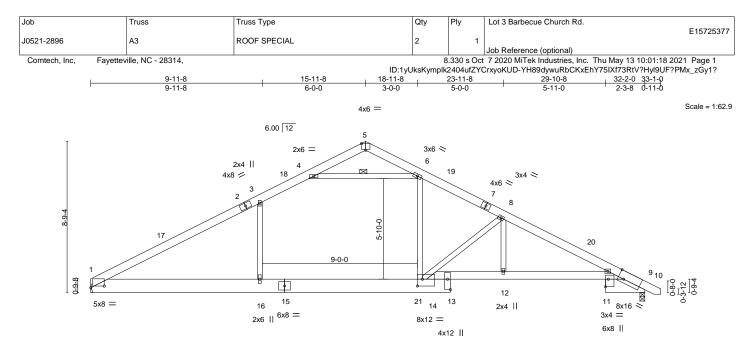
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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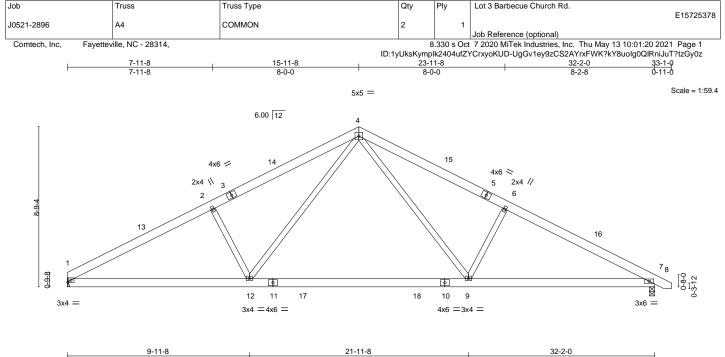


	<u>9-11-8</u> 9-11-8	18-11-8		0-10-8	23-1 3-1-		29-10-8 5-11-0		32-2-0	
Plate Offsets (X,Y)					-	-0	5-11-0		2-3-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.76 BC 0.66 WB 0.70 Matrix-S	Vert(CT) Horz(CT)	in -0.21 -0.36 0.09 0.16	16 16 9	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLA ⁻ MT20 Weig		GRIP 244/190 FT = 20%
BOT CHORD 2x10 9-14	SP No.1) SP No.1 *Except* I: 2x6 SP 2400F 2.0E SP No.2	1	BRACING- TOP CHORE BOT CHORE WEBS	C	Rigid c		heathing dire tly applied or 4-i	10-0-0 oc		oc purlins.
Max Max Max FORCES. (lb) - Ma TOP CHORD 1 BOT CHORD 11	size) 1=Mechanical, 9=0-3-8 < Horz 1=-111(LC 8) < Upift 1=-76(LC 12), 9=-90(LC 13) < Grav 1=1345(LC 2), 9=1347(LC 2) ax. Comp./Max. Ten All forces 250 (lb) or 3=-2189/401, 3-4=-1853/491, 6-8=-2237/5 16=-198/4827, 14-16=-200/1839, 12-14=-3 14=-117/966, 3-16=-53/392, 4-6=-1931/47	17, 8-9=-2874/549 875/2511, 9-12=-384/251	1							
 2) Wind: ASCE 7-10 MWFRS (envelop to 32-10-10 zone; 3) This truss has bee 4) * This truss has be will fit between the 5) Refer to girder(s) 	live loads have been considered for this de b; Vult=130mph (3-second gust) Vasd=103 be) and C-C Exterior(2) 0-0-12 to 4-5-9, Int ;C-C for members and forces & MWFRS fo en designed for a 10.0 psf bottom chord liv een designed for a live load of 30.0psf on t e bottom chord and any other members, w for truss to truss connections. cal connection (by others) of truss to bearing	mph; TCDL=6.0psf; BCD erior(1) 4-5-9 to 15-11-8, or reactions shown; Lumb re load nonconcurrent with the bottom chord in all are the BCDL = 10.0psf.	Exterior(2) 15-11-8 er DOL=1.60 plate h any other live load eas where a rectand	to 20-4 grip D0 ds. gle 3-6	4-5, Inte DL=1.60 -0 tall b	erior(1) 20-) y 2-0-0 wie			H CA Z	Hor



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			12-0-0					10-2-8	
(,Y) [1:0-1-14,0-1-8]									
) SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
) Plate Grip D0	DL 1.15	TC 0.28	Vert(LL)	-0.34	9-12	>999	360	MT20	244/190
) Lumber DOL	1.15	BC 0.64	Vert(CT)	-0.47	9-12	>822	240		
) * Rep Stress Ir	nor YES	WB 0.27	Horz(CT)	0.05	7	n/a	n/a		
) Code IRC20	15/TPI2014	Matrix-S	Wind(LL)	0.05	12	>999	240	Weight: 204 lb	FT = 20%
f) 0 0	f) SPACING- 0 Plate Grip DO 0 Lumber DOL 0 * Rep Stress Ir	SPACING- 2-0-0 0 Plate Grip DOL 1.15 0 Lumber DOL 1.15 0 * Rep Stress Incr YES	f) SPACING- Plate Grip DOL 2-0-0 1.15 CSI. 0 Plate Grip DOL 1.15 TC 0.28 0 Lumber DOL 1.15 BC 0.64 0 * Rep Stress Incr YES WB 0.27	f) SPACING- Plate Grip DOL 2-0-0 1.15 CSI. DEFL. 0 Plate Grip DOL 1.15 TC 0.28 Vert(LL) 0 Lumber DOL 1.15 BC 0.64 Vert(CT) 0 * Rep Stress Incr YES WB 0.27 Horz(CT)	f) SPACING- Plate Grip DOL 2-0-0 1.15 CSI. DEFL. in 0 Plate Grip DOL 1.15 TC 0.28 Vert(LL) -0.34 0 Lumber DOL 1.15 BC 0.64 Vert(CT) -0.47 0 Rep Stress Incr YES WB 0.27 Horz(CT) 0.05	f) SPACING- Plate Grip DOL 2-0-0 1.15 CSI. DEFL. in (loc) 0 Plate Grip DOL 1.15 TC 0.28 Vert(LL) -0.34 9-12 0 Lumber DOL 1.15 BC 0.64 Vert(CT) -0.47 9-12 0 Rep Stress Incr YES WB 0.27 Horz(CT) 0.05 7	f) SPACING- Plate Grip DOL 2-0-0 1.15 CSI. DEFL. in (loc) l/defl 0 Plate Grip DOL 1.15 TC 0.28 Vert(LL) -0.34 9-12 >999 0 Lumber DOL 1.15 BC 0.64 Vert(CT) -0.47 9-12 >822 0 * Rep Stress Incr YES WB 0.27 Horz(CT) 0.05 7 n/a	f) SPACING- Plate Grip DOL 2-0-0 CSI. DEFL. in (loc) l/defi L/d 0 Plate Grip DOL 1.15 TC 0.28 Vert(LL) -0.34 9-12 >999 360 0 Lumber DOL 1.15 BC 0.64 Vert(CT) -0.47 9-12 >822 240 0 * Rep Stress Incr YES WB 0.27 Horz(CT) 0.05 7 n/a n/a	SPACING- 2-0-0 CSI. DEFL. in (loc) I/defl L/d 0 Plate Grip DOL 1.15 TC 0.28 Vert(LL) -0.34 9-12 >999 360 MT20 0 Lumber DOL 1.15 BC 0.64 Vert(LT) -0.47 9-12 >822 240 MT20 0 * Rep Stress Incr YES WB 0.27 Horz(CT) 0.05 7 n/a n/a

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No 2 WEBS

REACTIONS. 1=Mechanical, 7=0-3-8 (size) Max Horz 1=-111(LC 8) Max Uplift 1=-76(LC 12), 7=-89(LC 13)

Max Grav 1=1278(LC 1), 7=1331(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2276/496, 2-4=-2096/546, 4-6=-2113/532, 6-7=-2294/484

BOT CHORD 1-12=-319/1973, 9-12=-109/1291, 7-9=-324/1953

WEBS 4-9=-147/922, 6-9=-454/288, 4-12=-144/897, 2-12=-437/286

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.

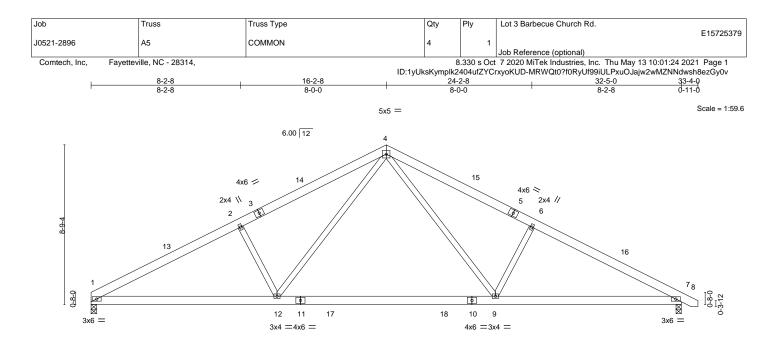


Structural wood sheathing directly applied or 4-11-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MARNING - Verify design pa ameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTerk® connectors. This design is back into ALD DED will the REFERENCE FAGE MIF/473 few. 519/2020 beFVRE USE. Design valid for use only with MTerk® connectors. This design is backed only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Compon Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	10-2-8 10-2-8	22-2-8 12-0-0				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. DEFL. TC 0.29 Vert(LL) BC 0.65 Vert(CT) WB 0.27 Horz(CT) Matrix-S Wind(LL)	in (loc) -0.34 9-12 -0.47 9-12 0.05 7 0.05 12	l/defl L/d >999 360 >822 240 n/a n/a >999 240	PLATES MT20 Weight: 206 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD 2x4 SP No.2 WEBS

REACTIONS. (size) 1=0-3-8, 7=0-3-8 Max Horz 1=-111(LC 10) Max Uplift 1=-77(LC 12), 7=-89(LC 13) Max Grav 1=1284(LC 1), 7=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2310/503, 2-4=-2129/551, 4-6=-2126/535, 6-7=-2308/487

BOT CHORD 1-12=-327/2012, 9-12=-111/1304, 7-9=-326/1966

WEBS 4-9=-147/921, 6-9=-454/288, 4-12=-149/924, 2-12=-458/292

NOTES-

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to

33-1-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf. 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.

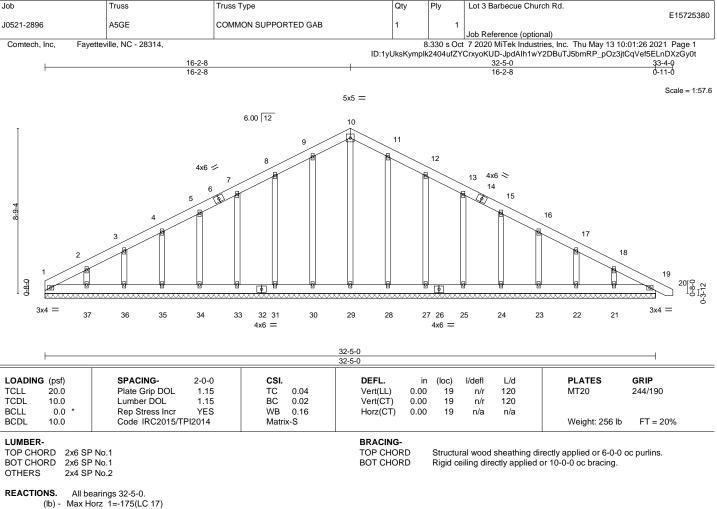
Structural wood sheathing directly applied or 4-10-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



APPENDED. 110



- Max Uplift All uplift 100 lb or less at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except 37=-101(LC 12) Max Grav All reactions 250 lb or less at joint(s) 1, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22,
- 21, 19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 9-10=-114/284, 10-11=-114/284

NOTES-

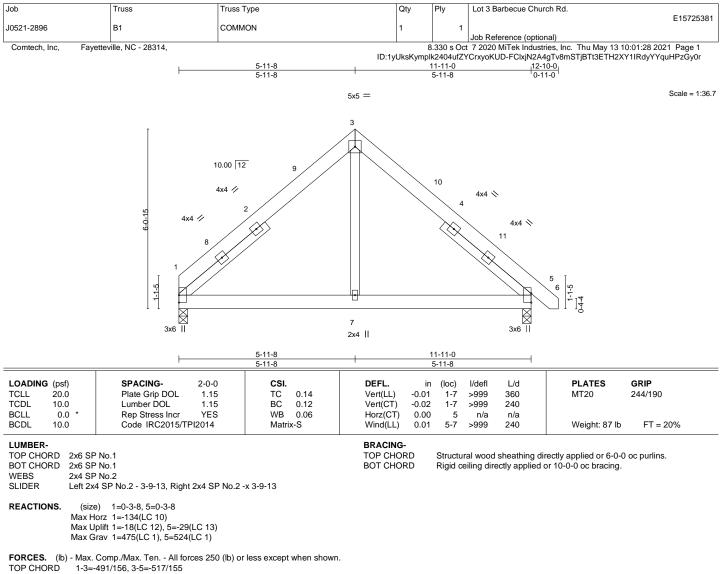
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=101.



May 13,2021

🛕 WARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid of use only with MTek® connectors. This skew invCLOBED with REFERENCE FAGE MIF 4/3 FeV, 519/2020 BEFORE 052. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Braching indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Compore Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BOT CHORD 1-7=0/307, 5-7=0/307 WEBS 3-7=0/277

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 5-11-8, Exterior(2) 5-11-8 to 10-4-5, Interior(1) 10-4-5 to 12-8-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

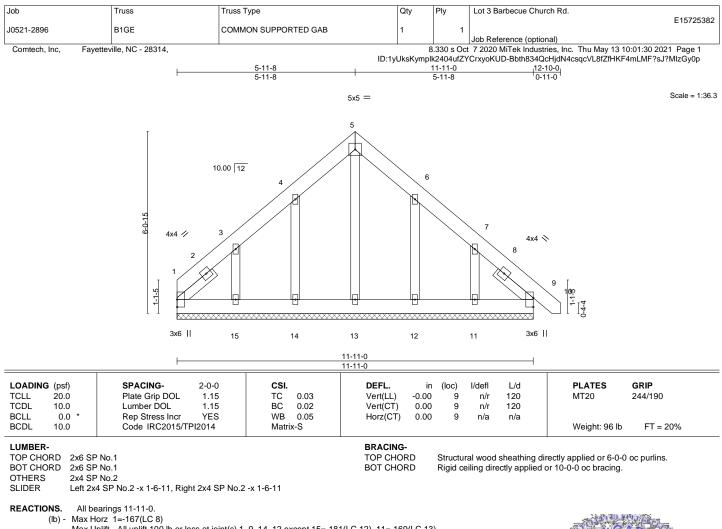
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oulapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 14, 12 except 15=-181(LC 12), 11=-169(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

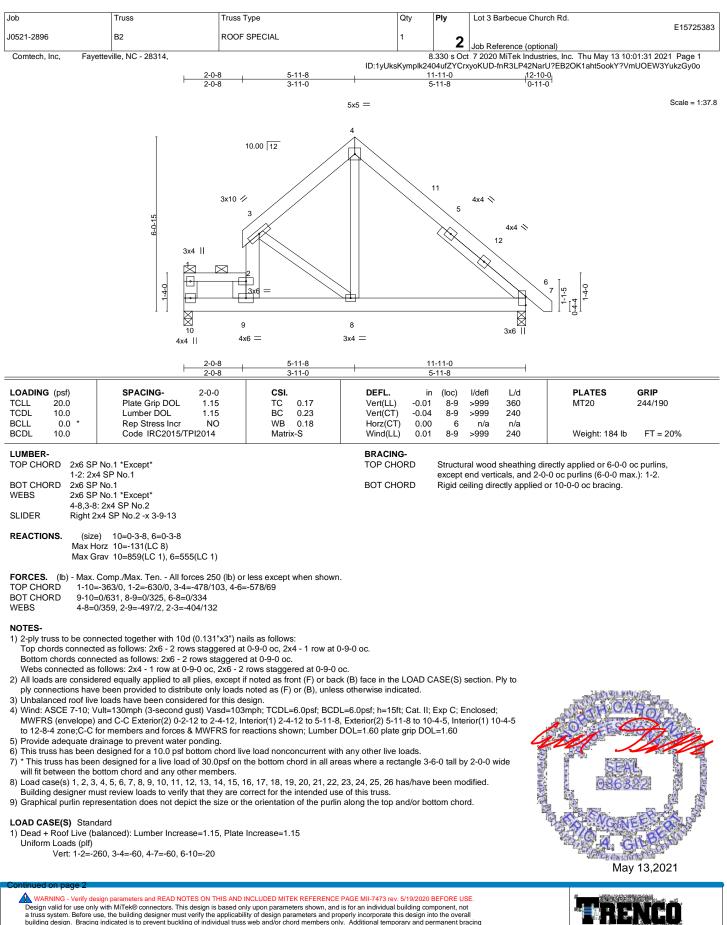
- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 14, 12 except (it=lb) 15=181, 11=169.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 9.



May 13,2021

🛕 WARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Coulity Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Design valid for use only with MTerk® connectors. This design is back into ALD DED will the REFERENCE FAGE MIF/473 few. 519/2020 beFVRE USE. Design valid for use only with MTerk® connectors. This design is backed only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Compon Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
					E15725383
J0521-2896	B2	ROOF SPECIAL	1	2	
				_	Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8	.330 s Oc	7 2020 MiTek Industries, Inc. Thu May 13 10:01:32 2021 Page 2

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:32 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-7z_SZl5g8uzLcOmFy1YpD4ezY8tEDkYTAo6QAzGy0n

	ID:1yUksKymplk2404ufZYCr:
LOAD CASE(S) Standard	
2) Dead + 0.75 Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (plf) Vert: 1-2=-250, 3-4=-50, 4-7=-50, 6-10=-20	
3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25	
Uniform Loads (plf)	
Vert: 1-2=-220, 3-4=-20, 4-7=-20, 6-10=-40	
4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf) Vert: 1-2=-156, 3-4=27, 4-12=35, 6-12=27, 6-7=20, 6-10=-12	
Horz: 3-4=-39, 4-12=47, 6-12=39, 6-7=32	
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-170, 3-4=35, 4-11=27, 6-11=35, 6-7=58, 6-10=-12 Horz: 3-4=-47, 4-11=39, 6-11=47, 6-7=70	
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-235, 3-4=-58, 4-6=-58, 6-7=-51, 6-10=-20	
Horz: 3-4=38, 4-6=-38, 6-7=-31	
 Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) 	
Vert: 1-2=-235, 3-4=-58, 4-6=-58, 6-7=11, 6-10=-20	
Horz: 3-4=38, 4-6=-38, 6-7=31	
8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf) Vert: 1-2=-179, 3-4=-13, 4-6=11, 6-7=4, 6-10=-12	
Horz: 3-4=1, 4-6=23, 6-7=16	
9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60	C
Uniform Loads (plf)	
Vert: 1-2=-191, 3-4=11, 4-6=-13, 6-7=2, 6-10=-12 Horz: 3-4=-23, 4-6=-1, 6-7=14	
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60	0
Uniform Loads (plf)	
Vert: 1-2=-201, 3-4=-35, 4-6=-11, 6-7=-4, 6-10=-20	
Horz: 3-4=15, 4-6=9, 6-7=16 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.	60
Uniform Loads (plf)	60
Vert: 1-2=-213, 3-4=-11, 4-6=-35, 6-7=-28, 6-10=-20	
Horz: 3-4=-9, 4-6=-15, 6-7=-8	
12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increa	se=1.60
Uniform Loads (plf) Vert: 1-2=-179, 3-4=21, 4-6=9, 6-7=2, 6-10=-12	
Horz: 3-4=-33, 4-6=21, 6-7=14	
13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increa	ase=1.60
Uniform Loads (plf)	
Vert: 1-2=-191, 3-4=9, 4-6=21, 6-7=14, 6-10=-12 Horz: 3-4=-21, 4-6=33, 6-7=26	
14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increa	use=1.60
Uniform Loads (plf)	
Vert: 1-2=-179, 3-4=21, 4-6=9, 6-7=2, 6-10=-12	
Horz: 3-4=-33, 4-6=21, 6-7=14	4.00
 Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increa Uniform Loads (plf) 	Se=1.60
Vert: 1-2=-191, 3-4=9, 4-6=21, 6-7=14, 6-10=-12	
Horz: 3-4=-21, 4-6=33, 6-7=26	
16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increa	ise=1.60
Uniform Loads (plf) Vert: 1-2=-201, 3-4=-1, 4-6=-13, 6-7=-6, 6-10=-20	
Horz: 3-4=-19, 4-6=7, 6-7=14	
17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Incre	ase=1.60
Uniform Loads (plf)	
Vert: 1-2=-213, 3-4=-13, 4-6=-1, 6-7=6, 6-10=-20	
Horz: 3-4=-7, 4-6=19, 6-7=26 18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90	
Uniform Loads (plf)	
Vert: 1-2=-220, 3-4=-20, 4-7=-20, 6-10=-20	
19) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1	.60, Plate Increase=1.60
Uniform Loads (plf) Vert: 1-2=-236, 3-4=-61, 4-6=-43, 6-7=-38, 6-10=-20	
Horz: 3-4=11, 4-6=7, 6-7=12	
20) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=	1.60, Plate Increase=1.60
Uniform Loads (plf)	
Vert: 1-2=-245, 3-4=-43, 4-6=-61, 6-7=-56, 6-10=-20 Horz: 3-4=-7, 4-6=-11, 6-7=-6	
H0r2: 3-4=-7, 4-b=-11, 6-7=-b 21) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Incr	ease=1.60 Plate Increase=1

21) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60

WARNIG - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
					E15725383
J0521-2896	B2	ROOF SPECIAL	1	2	
				_	Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 10:01:32 2021 Page 3

Thu May 13 $ID:1yUksKymplk2404ufZYCrxyoKUD-7z_SZI5g8uzLcOmFy1YpD4ezY8tEEDkYTAo6QAzGy0n$

LOAD CASE(S) Standard Uniform Loads (plf)

Vert: 1-2=-236, 3-4=-36, 4-6=-45, 6-7=-40, 6-10=-20

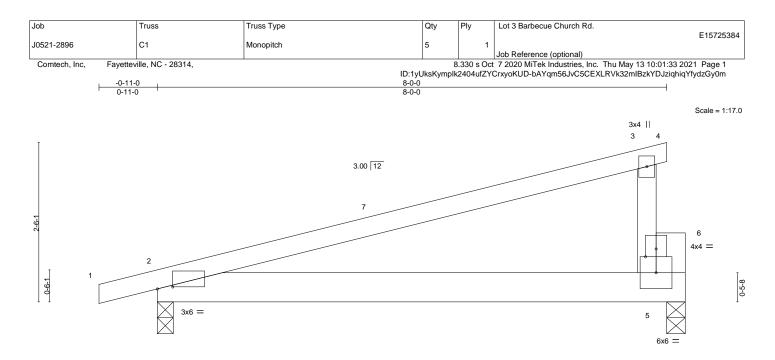
Horz: 3-4=-14, 4-6=5, 6-7=10

22) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

- Uniform Loads (plf)
 - Vert: 1-2=-245, 3-4=-45, 4-6=-36, 6-7=-31, 6-10=-20 Horz: 3-4=-5, 4-6=14, 6-7=19
- 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf) Vert: 1-2=-260, 3-4=-60, 4-7=-20, 6-10=-20
- 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-2=-220, 3-4=-20, 4-7=-60, 6-10=-20
- 25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-2=-250, 3-4=-50, 4-7=-20, 6-10=-20
- 26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
 - Vert: 1-2=-220, 3-4=-20, 4-7=-50, 6-10=-20

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			0-3-0			
	1		8-3-8			
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [6:0-2-0,0-1-8]					
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. jr	n (loc) l/defl L/	d PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.84	Vert(LL) -0.05	(,		244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.24	Vert(CT) -0.10	2-5 >969 24	0	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00) 5 n/a n/a	a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.10	0 2-5 >886 24	0 Weight: 37 lb	FT = 20%
LUMBER-		1	BRACING-			
TOP CHORD 2x4 SP	' No.1		TOP CHORD	Structural wood shea	athing directly applied or 5-3-4	l oc purlins,
BOT CHORD 2x6 SP	' No.1			except end verticals.		
WEBS 2x4 SP	No.2		BOT CHORD	Rigid ceiling directly a	applied or 10-0-0 oc bracing.	
OTHERS 2x6 SP	' No.1					

8-3-8

REACTIONS. (size) 2=0-3-0, 5=0-3-8 Max Horz 2=74(LC 8) Max Uplift 2=-150(LC 8), 5=-127(LC 8) Max Grav 2=375(LC 1), 5=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

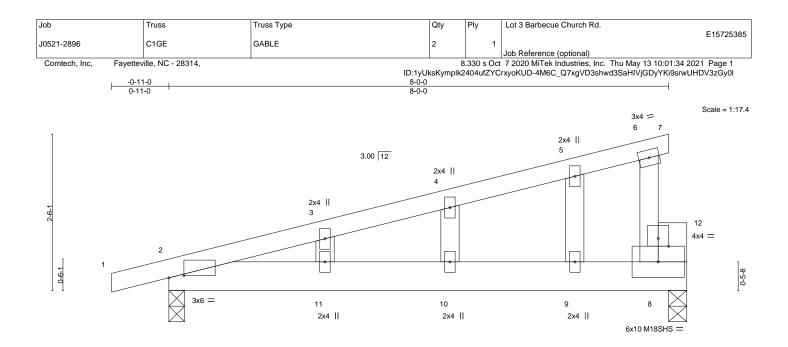
NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 8-0-0 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 5=127.



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			<u>8-3-8</u> 8-3-8		
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [12:0-2-0,0-1-8]		0-3-0		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.35 BC 0.26 WB 0.01 Matrix-S	Vert(LL) 0.09	n (loc) I/defl L/d 9 10-11 >999 240 3 10-11 >999 240 9 8 n/a n/a	PLATES GRIP MT20 244/190 M18SHS 244/190 Weight: 41 lb FT = 20%
	No.1		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	ectly applied or 6-0-0 oc purlins, r 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 8=0-3-8 Max Horz 2=105(LC 8) Max Uplift 2=-216(LC 8), 8=-188(LC 8)

Max Grav 2=375(LC 1), 8=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

BOT CHORD 2-11=-284/207, 10-11=-284/207, 9-10=-284/207, 8-9=-284/207

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are MT20 plates unless otherwise indicated.

4) Gable studs spaced at 2-0-0 oc.

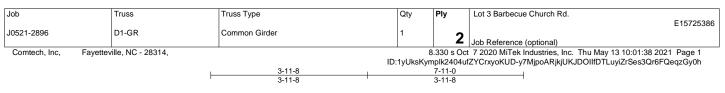
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216.8=188.



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4x4 ||



2 10.00 12 3 1-0-3 5 6 7 8 4 3x10 || 3x10 || 3x10 ||

				<u>3-11</u> 3-11	-8			'-11-0 -11-8				
Plate Offs	ets (X,Y)	[1:0-0-4,0-0-5], [1:0-0-8,0	-3-6], [3:0-0-4	,0-0-5], [3:0-0	-8,0-3-6]							
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.38	Vert(LL)	-0.02	3-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.57	Vert(CT)	-0.04	3-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.39	Horz(CT)	0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matrix	(-P	Wind(LL)	0.01	3-4	>999	240	Weight: 100 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No 2 WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 3=0-3-8

Max Horz 1=91(LC 26) Max Uplift 1=-191(LC 8), 3=-180(LC 9) Max Grav 1=2919(LC 1), 3=2779(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 1-2=-2418/177, 2-3=-2418/177 TOP CHORD 1-4=-100/1678, 3-4=-100/1678 BOT CHORD 2-4=-154/3142 WEBS

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=191, 3=180.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1261 lb down and 93 lb up at 0-9-12, 1258 lb down and 96 lb up at 2-9-12, and 1325 lb down and 96 lb up at 4-9-12, and 1325 lb down and 96 lb up at 6-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 1-3=-20

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1-0-3

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



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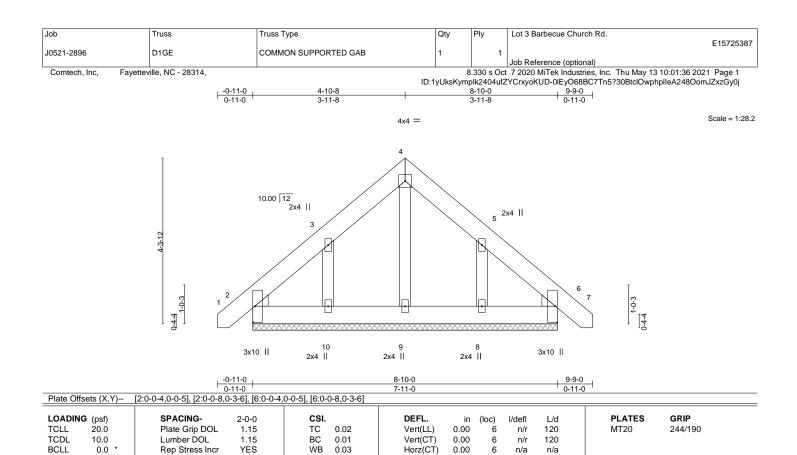
Job		Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.		
J0521-2896		D1-GR	Common Girder	1	2	E15725386		
					Z	Job Reference (optional)		
Comtech, Inc, Fayetteville, NC - 28314,				. 8	3.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 10:01:38 2021 Page 2		
			ID:1yUksKymplk2404ufZYCrxyoKUD-y7MjpoARjkjUKJDOIIfDTLuyiZrSes3Qr6FQeqzGy0h					

LOAD CASE(S) Standard Concentrated Loads (lb)

Vert: 5=-1261(B) 6=-1258(B) 7=-1258(B) 8=-1258(B)

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BRACING-

TOP CHORD

BOT CHORD

 184	IRF	- 0
 JM	в	-к-

BCDL

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No 2

10.0

OTHERS WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS. All bearings 7-11-0.

(lb) - Max Horz 2=-118(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-152(LC 12), 8=-148(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code IRC2015/TPI2014

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-P

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=152.8=148.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Weight: 60 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins.

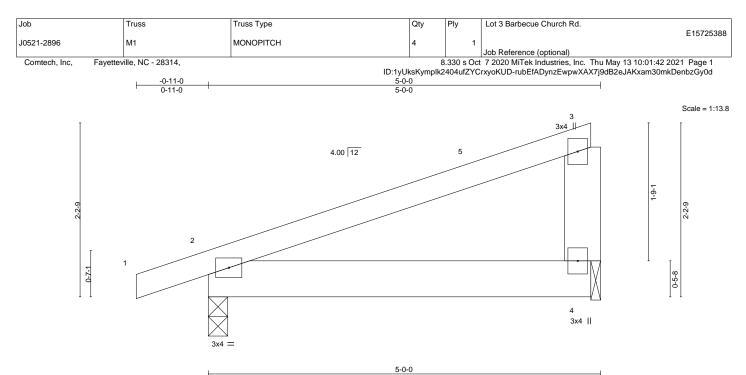
Rigid ceiling directly applied or 10-0-0 oc bracing.

FT = 20%

May 13,2021

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	5-0-0				1							
LOADING (p	sf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20	0.0	Plate Grip DOL	1.15	TC	0.28	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL 10	0.0	Lumber DOL	1.15	BC	0.08	Vert(CT)	-0.01	2-4	>999	240		
BCLL 0	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL 10	0.0	Code IRC2015/TF	PI2014	Matri	x-P	Wind(LL)	0.01	2-4	>999	240	Weight: 24 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=63(LC 8) Max Uplift 2=-102(LC 8), 4=-79(LC 8)

Max Grav 2=255(LC 1), 4=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

- capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=102.



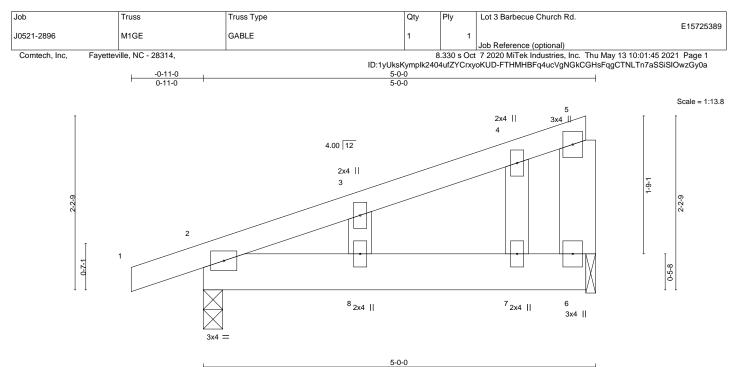
Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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LOADING (psf)	SPACING- 2-0-0	CSI. DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.09 Vert(LL) 0.01 8 >999 240	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.09 Vert(CT) -0.01 8 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.02 Horz(CT) -0.00 6 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Weight: 27 lb FT = 20%

LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1
OTHERS	2x4 SP No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD

REACTIONS. (size) 2=0-3-0, 6=0-1-8

Max Horz 2=90(LC 8)

Max Uplift 2=-147(LC 8), 6=-115(LC 8) Max Grav 2=255(LC 1), 6=179(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

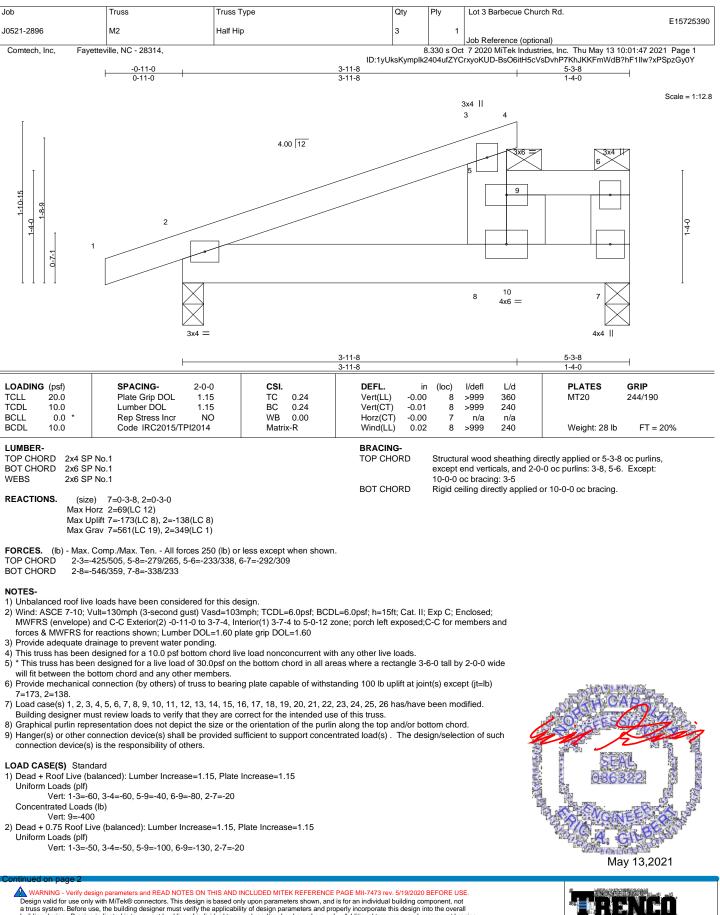
7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=147.6=115.



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Design valid for use only with MTerk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses systems, see **ANS/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

J	ob	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
						E15725390
J	0521-2896	M2	Half Hip	3	1	
						Job Reference (optional)
	Comtech, Inc, Fayetteville, NC - 28314,			8	.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 10:01:47 2021 Page 2

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:47 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-BsO6itH5cVsDvhP7KhJKKFmWdB?hF1Ilw?xPSpzGy0Y

Concentrated Loads (lb)	
Vert: 9=-350	
 Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25 	
Uniform Loads (plf)	
Vert: 1-3=-20, 3-4=-20, 5-6=-40, 2-7=-40	
Concentrated Loads (lb) Vert: 9=-300	
4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=12, 2-8=52, 8-10=115, 7-10=52	
Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55	
Concentrated Loads (lb)	
Vert: 9=548	
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf) Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=42, 2-8=52, 8-10=115, 7-10=52	
Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55	
Concentrated Loads (Ib)	
Vert: 9=566	
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9	
Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51	
Concentrated Loads (lb) Vert: 9=-420	
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9	
Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51	
Concentrated Loads (lb)	
Vert: 9=-420	
 Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) 	
Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-11, 2-8=10, 8-10=33, 7-10=10	
Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7	
Concentrated Loads (lb)	
Vert: 9=154	
9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12	
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27	
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12	
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb)	
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)	
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2	
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34	
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb)	
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339	
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 Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=9, 3-4=-2, 5-6=1, 2-7=-12 Horz: 1-2=-2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 	
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$ \begin{array}{l} \mbox{Vert: } 1-2=6, 2\cdot3=12, 3\cdot4=28, 5\cdot6=1, 2\cdot7=\cdot12 \\ \mbox{Hor: } 1\cdot2=\cdot18, 2\cdot3=\cdot24, 3\cdot4=\cdot40, 3\cdot5=\cdot27 \\ \mbox{Concentrated Loads (lb) \\ \mbox{Vert: } 9=43 \\ \mbox{10} \mbox{Dead } + 0.6 \mbox{MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 1\cdot2=-6, 2\cdot3=\cdot1, 3\cdot4=6, 5\cdot6=\cdot33, 2\cdot8=2, 8\cdot10=25, 7\cdot10=2 \\ \mbox{Hor: } 1\cdot2=\cdot26, 2\cdot3=\cdot19, 3\cdot4=\cdot26, 3\cdot5=34 \\ \mbox{Concentrated Loads (lb) } \\ \mbox{Vert: } 9=-339 \\ \mbox{11} \mbox{Dead } + 0.6 \mbox{MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 1\cdot2=-78, 2\cdot3=-9, 3\cdot4=\cdot2, 5\cdot6=-21, 2\cdot7=\cdot20 \\ \mbox{Hor: } 1\cdot2=-78, 2\cdot3=-11, 3\cdot4=-18, 3\cdot5=-0 \\ \mbox{Concentrated Loads (lb) } \\ \mbox{Vert: } 9=-234 \\ \mbox{12} \mbox{Dead } 0.6 \mbox{MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 1\cdot2=-126, 2\cdot3=-21, 3\cdot4=-14, 5\cdot6=-11, 2\cdot7=-12 \\ \mbox{Hor: } 1\cdot2=-26, 2\cdot3=-33, 3\cdot4=-26, 3\cdot5=-39 \\ \mbox{Concentrated Loads (lb) } \\ \mbox{Vert: } 9=43 \\ \mbox{13} \mbox{Dead } 0.6 \mbox{MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 1\cdot2=-14, 2\cdot3=21, 3\cdot4=-14, 3\cdot5=-27 \\ \mbox{Concentrated Loads (lb) } \\ \mbox{Vert: } 9=43 \\ \mbox{14} \mbox{Dead } 0.6 \mbox{MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 1\cdot2=-14, 2\cdot3=21, 3\cdot4=-14, 3\cdot5=-27 \\ \mbox{Concentrated Loads (lb) } \\ \mbox{Vert: } 9=43 \\ \mbox{14} \mbox{Dead } 0.6 \mbox{MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 9=43 \\ \mbox{14} \mbox{Dead } 0.6 \mbox{MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 9=-43 \\ \mbox{14} \mbox{Dead } 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, $	
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (pl) Vert: 1-2=6, 2-3=-11, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=2, 2-3=-9, 3-4=2, 5-6=-1, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=14, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 5-6=-11, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 5-6=-11, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 5-6=-39 Concentrated Loads (lb) Vert: 1-2=-14, 2-3=-3, 3-4=-26, 3-5=-39 Concentrated Loads (lb)	
$ \begin{array}{l} \mbox{Vert: } 1-2=6, 2\cdot3=12, 3\cdot4=28, 5\cdot6=1, 2\cdot7=\cdot12 \\ \mbox{Hor: } 1\cdot2=\cdot18, 2\cdot3=\cdot24, 3\cdot4=\cdot40, 3\cdot5=\cdot27 \\ \mbox{Concentrated Loads (lb) \\ \mbox{Vert: } 9=43 \\ \mbox{10} \mbox{Dead } + 0.6 \mbox{MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 1\cdot2=-6, 2\cdot3=\cdot1, 3\cdot4=6, 5\cdot6=\cdot33, 2\cdot8=2, 8\cdot10=25, 7\cdot10=2 \\ \mbox{Hor: } 1\cdot2=\cdot26, 2\cdot3=\cdot19, 3\cdot4=\cdot26, 3\cdot5=34 \\ \mbox{Concentrated Loads (lb) } \\ \mbox{Vert: } 9=-339 \\ \mbox{11} \mbox{Dead } + 0.6 \mbox{MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 1\cdot2=-78, 2\cdot3=-9, 3\cdot4=\cdot2, 5\cdot6=-21, 2\cdot7=\cdot20 \\ \mbox{Hor: } 1\cdot2=-78, 2\cdot3=-11, 3\cdot4=-18, 3\cdot5=-0 \\ \mbox{Concentrated Loads (lb) } \\ \mbox{Vert: } 9=-234 \\ \mbox{12} \mbox{Dead } 0.6 \mbox{MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 1\cdot2=-126, 2\cdot3=-21, 3\cdot4=-14, 5\cdot6=-11, 2\cdot7=-12 \\ \mbox{Hor: } 1\cdot2=-26, 2\cdot3=-33, 3\cdot4=-26, 3\cdot5=-39 \\ \mbox{Concentrated Loads (lb) } \\ \mbox{Vert: } 9=43 \\ \mbox{13} \mbox{Dead } 0.6 \mbox{MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 1\cdot2=-14, 2\cdot3=21, 3\cdot4=-14, 3\cdot5=-27 \\ \mbox{Concentrated Loads (lb) } \\ \mbox{Vert: } 9=43 \\ \mbox{14} \mbox{Dead } 0.6 \mbox{MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 1\cdot2=-14, 2\cdot3=21, 3\cdot4=-14, 3\cdot5=-27 \\ \mbox{Concentrated Loads (lb) } \\ \mbox{Vert: } 9=43 \\ \mbox{14} \mbox{Dead } 0.6 \mbox{MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 9=43 \\ \mbox{14} \mbox{Dead } 0.6 \mbox{MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 \\ \mbox{Uniform Loads (plf) } \\ \mbox{Vert: } 9=-43 \\ \mbox{14} \mbox{Dead } 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, $	

LOAD CASE(S) Standard

WARNIG - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



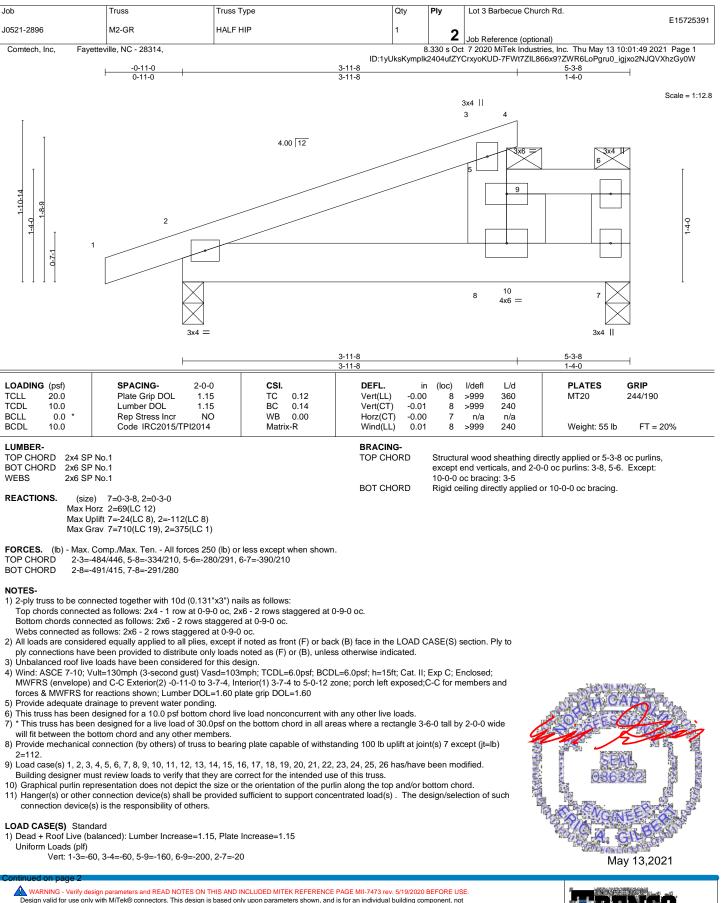
Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
					E15725390
J0521-2896	M2	Half Hip	3	1	
					Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 10:01:47 2021 Page 3

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:47 2021 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-BsO6itH5cVsDvhP7KhJKKFmWdB?hF1Ilw?xPSpzGy0Y

	ID: TyUKSKYMPIK2404UIZ Y CFXyOK UD-BSODitH5CVSDVNP7
LO	AD CASE(S) Standard
	Uniform Loads (plf)
	Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12
	Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
	Concentrated Loads (lb)
	Vert: 9=43
16)	Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-7=-20
	Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12
	Concentrated Loads (lb)
	Vert: 9=-234
17)	Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-21, 2-7=-20
	Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0
	Concentrated Loads (lb) Vert: 9=-234
18)	Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
,	Uniform Loads (pf)
	Vert: 1-3=-20, 3-4=-20, 5-6=-120, 2-7=-20
	Concentrated Loads (lb)
	Vert: 9=-200
19)	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-8=-3, 8-10=13, 7-10=-3 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26
	Concentrated Loads (Ib)
	Vert: 9=-454
20)	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-86, 6-9=-116, 2-7=-20
	Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0
	Concentrated Loads (lb) Vert: 9=-375
21)	Dead + 0.75 Root Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60
,	Uniform Loads (plf)
	Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-7=-20
	Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9
	Concentrated Loads (lb)
22)	Vert: 9=-375 Dead + 0.25 Read Live (kel) + 0.25 Attic Floor + 0.25(0.6 MWERS Mind (Neg. Jak) 2nd Received Live (kel) + 0.25 Attic Floor + 0.25(0.6 MWERS Mind (Neg. Jak) 2nd Received Live (kel)
22)	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
	Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-86, 6-9=-116, 2-7=-20
	Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0
	Concentrated Loads (lb)
	Vert: 9=-375
23)	1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-6=-40, 2-7=-20
	Veit. 155-50, 545-50, 5-6-40, 2-7-20 Concentrated Loads (lb)
	Verte 9=400
24)	2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1-3=-20, 3-4=-20, 5-9=-40, 6-9=-80, 2-7=-20
	Concentrated Loads (lb)
25)	Vert: 9=-400 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
20)	Uniform Loads (pf)
	Vert: 1-3=-50, 3-4=-50, 5-6=-100, 2-7=-20
	Concentrated Loads (lb)
	Vert: 9=-350 ´
26)	4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1-3=-20, 3-4=-20, 5-9=-100, 6-9=-130, 2-7=-20 Concentrated Loads (lb)
	Vert: 9=-350

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses sand truss systems, see **AVSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Design valid for use only with MTeRe® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **AVSUPTI Quality Criteria, DSB-89 and BCSI Building Compor Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.	
						E15725391
J0521-2896	M2-GR	HALF HIP	1	2		
					Job Reference (optional)	
Comtech, Inc,	-ayetteville, NC - 28314,			3.330 s Oct	t 7 2020 MiTek Industries, Inc. Thu May 13 10:01:49 202	21 Page 2

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:49 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-7FWt7ZIL866x9?ZWR6LoPgru0_igjxo2NJQVXhzGy0W

Uniform Loads (plf) Vert: 1-3=-50, 3-4=-50, 5-9=-220, 6-9=-250, 2-7=-20
Concentrated Loads (lb)
Vert: 9=-350
3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-20, 3-4=-20, 5-6=-160, 2-7=-40
Concentrated Loads (lb)
Vert: 9=-300
4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=-108, 2-8=52, 8-10=115, 7-10=52
Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55
Concentrated Loads (lb)
Vert: 9=548
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=-78, 2-8=52, 8-10=115, 7-10=52
Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55
Concentrated Loads (lb) Vert: 9=566
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51
Concentrated Loads (lb)
Vert: 9=-420
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51
Concentrated Loads (lb)
Vert: 9=-420
8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-131, 2-8=10, 8-10=33, 7-10=10 Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7
Concentrated Loads (lb)
Vert: 9=154 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60. Plate Increase=1.60
9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
 Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
 Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12
9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb)
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-39 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-39 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb)
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-28, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=-14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (lb) Vert: 9=-324 12) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (lb) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb)
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=-14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=43
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=-14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=-14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-234 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (b) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (b) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-334 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-3, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=-2, 5-6=-119, 2-7=-12
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=-14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-131, 2-7=-12 Horz: 1-2=-2, 2-3=9, 3-4=-2, 5-6=-119, 2-7=-12 Horz: 1-2=-2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (b) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (b) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-334 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-3, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=-2, 5-6=-119, 2-7=-12
 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=-14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=4, 2-3=9, 3-4=-2, 5-6=-119, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 1-2=-2, 2-3=9, 3-4=-2, 5-6=-119, 2-7=-12 Horz: 1-2=-2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12 Horz: 1-2=-2, 2-3=9, 3-4=-2, 5-6=-119, 2-7=-12 Horz: 1-2=-2, 2-3=, 3-4=-2, 5-6=-119, 2-7=-12 Horz: 1-2=-2, 2-3=, 3-4=-2, 5-6=-119, 2-7=-12 Horz: 1-2=-2, 2-3=, 3-4=-2, 5-6=-119, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (b)

2) Dead + 0.75 Roof Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15

LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 9=-400

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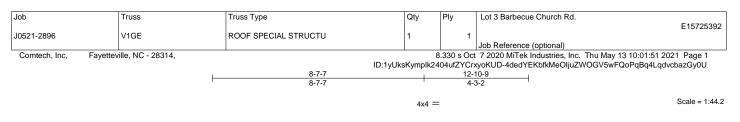
Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
					E15725391
J0521-2896	M2-GR	HALF HIP	1	2	
				_	Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 10:01:50 2021 Page 3

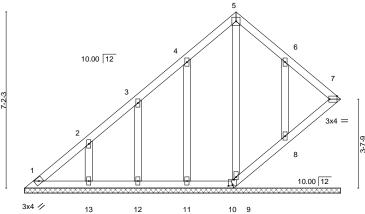
8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:50 2021 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-cR4FKvJzvQEnm98i?ps1yt03m02vS02Bcz9338zGy0V

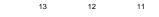
10	AD CASE(S) Standard
	Uniform Loads (plf)
	Vent: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12
	Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
	Concentrated Loads (lb)
	Vert: 9=43
15	Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12
	Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
	Concentrated Loads (Ib)
	Vert: 9=43
16	Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20
	Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12
	Concentrated Loads (lb)
47	Vert: 9=-234
17	Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf) Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-141, 2-7=-20
	Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0
	Concentrated Loads (lb)
	Vert: 9=-234
18	Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
	Uniform Loads (plf)
	Vert. 1-3=-20, 3-4=-20, 5-6=-240, 2-7=-20
	Concentrated Loads (lb)
	Vert: 9=-200
19	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-8=-3, 8-10=13, 7-10=-3
	Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26
	Concentrated Loads (lb)
	Vert: 9=-454
20	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-206, 6-9=-236, 2-7=-20
	Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0 Concentrated Loads (lb)
	Vert 9=-375
21	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (pf)
	Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-7=-20
	Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9
	Concentrated Loads (lb)
	Vert: 9=-375
22	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-206, 6-9=-236, 2-7=-20
	Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0
	Concentrated Loads (lb)
	Vert: 9=-375
23	1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1-3=-60, 3-4=-60, 5-6=-160, 2-7=-20
	Concentrated Loads (b)
24	Vert: 9=-400
24	2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
	Vent: 1-3=-20, 3-4=-20, 5-9=-160, 6-9=-200, 2-7=-20
	Concentrated Loads (lb)
	Vert 9=-400
25	3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1-3=-50, 3-4=-50, 5-6=-220, 2-7=-20
	Concentrated Loads (lb)
	Vert: 9=-350
26	4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1-3=-20, 3-4=-20, 5-9=-220, 6-9=-250, 2-7=-20
	Concentrated Loads (lb)
	Vert: 9=-350

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8-6-5 12-10-9 8-6-5 4-4-4

3x4 =

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.06	Vert(LL) n/a - n/a 999	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) n/a - n/a 999	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.08	Horz(CT) 0.00 7 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 75 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No 2 BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 12-10-9.

(lb) - Max Horz 1=231(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10 except 11=-112(LC 12), 12=-107(LC 12), 13=-133(LC 12), 8=-126(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 9, 11, 12, 13, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-295/189

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

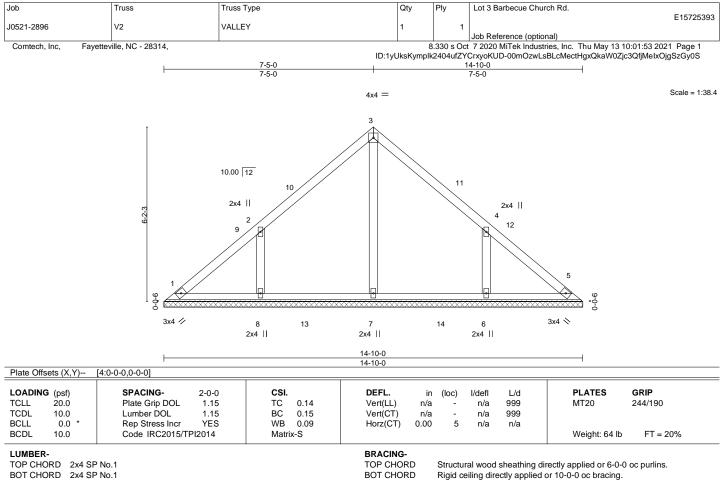
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 7, 9, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10 except (jt=lb) 11=112, 12=107, 13=133, 8=126.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 7, 9, 8.



May 13,2021

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2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. All bearings 14-10-0.

(lb) - Max Horz 1=-140(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-135(LC 12), 6=-135(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=400(LC 19), 8=393(LC 19), 6=393(LC 20)

WEBS 2-8=-338/247, 4-6=-338/247

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-5-0, Exterior(2) 7-5-0 to 11-9-13, Interior(1) 11-9-13 to 14-5-3 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

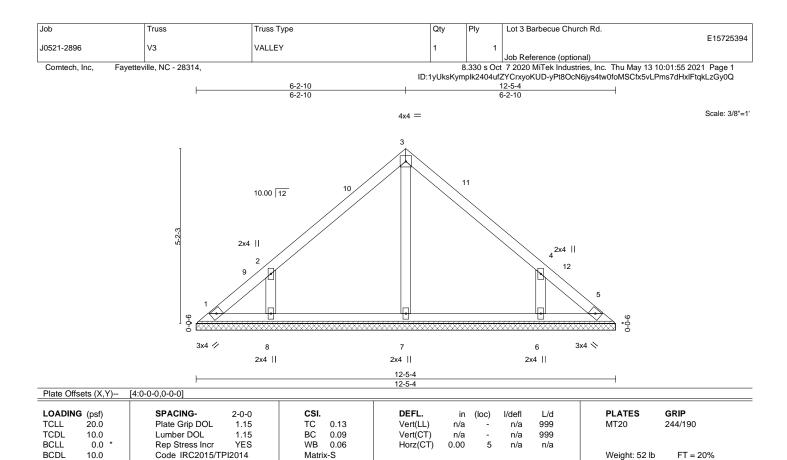
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=135, 6=135,



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.1
OTHERS	2x4 SP No 2

REACTIONS. All bearings 12-5-4.

(lb) - Max Horz 1=-116(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-123(LC 12), 6=-123(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=326(LC 19), 6=326(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-312/241, 4-6=-312/241

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-2-10, Exterior(2) 6-2-10 to 10-7-7, Interior(1) 10-7-7 to 12-0-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123, 6=123.

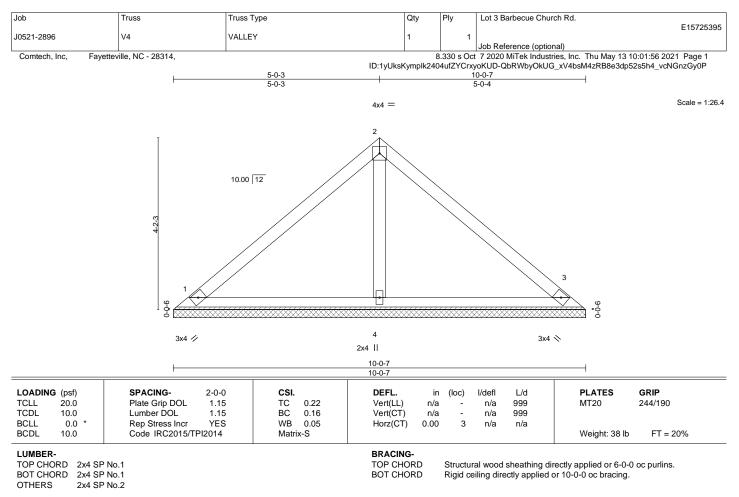


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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REACTIONS. (size) 1=10-0-7, 3=10-0-7, 4=10-0-7

Max Horz 1=-92(LC 8)

Max Uplift 1=-22(LC 13), 3=-30(LC 13)

Max Grav 1=197(LC 1), 3=197(LC 1), 4=344(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

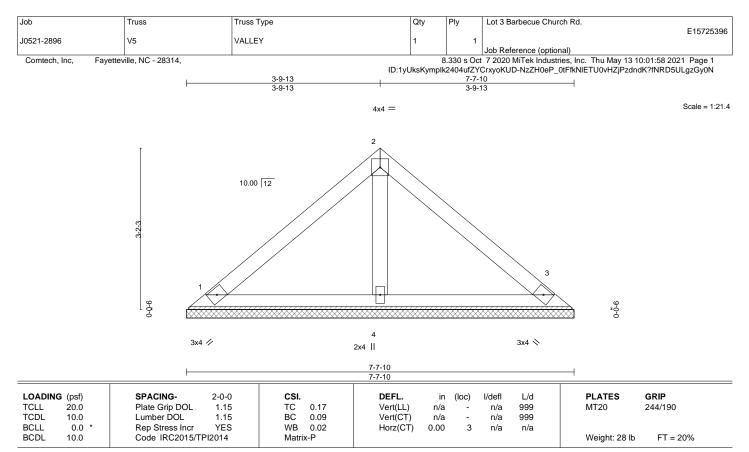
will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. (size) 1=7-7-10, 3=7-7-10, 4=7-7-10

Max Horz 1=68(LC 9)

Max Uplift 1=-24(LC 13), 3=-30(LC 13)

Max Grav 1=158(LC 1), 3=158(LC 1), 4=230(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

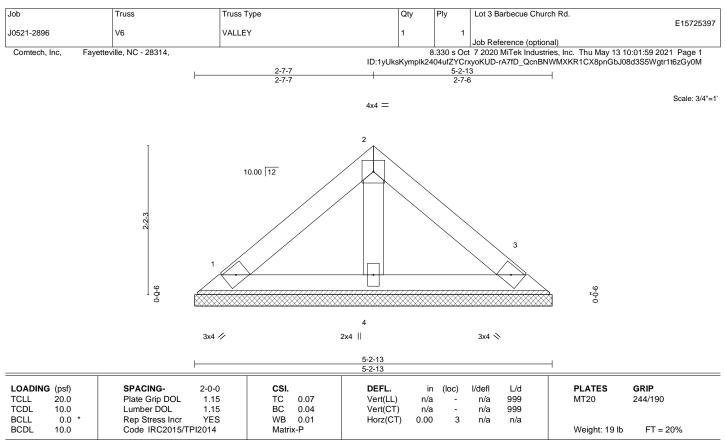


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component and the prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component and the prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component and the prevent personal personal injury and property damage. For general guidance regarding the fabrication and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component and the personal persona





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=5-2-13, 3=5-2-13, 4=5-2-13

Max Horz 1=-44(LC 8)

Max Uplift 1=-15(LC 13), 3=-19(LC 13) Max Grav 1=102(LC 1), 3=102(LC 1), 4=149(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

arip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



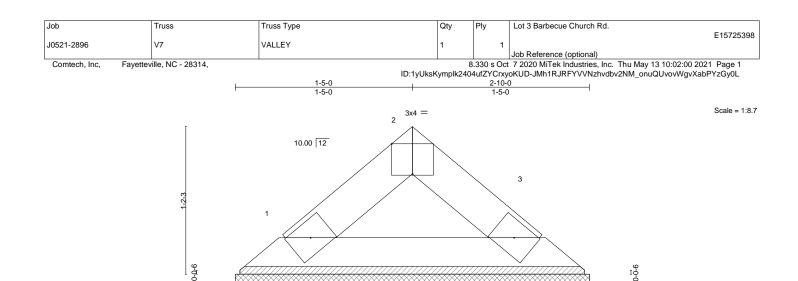
Structural wood sheathing directly applied or 5-2-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

May 13,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





3x4 🥢

3x4 📎

Plate Offsets (X,Y)	[2:0-2-0,Edge]		-		1					Г	
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.01	Vert(LL)	n/a	· -	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code IRC2015/T	PI2014	Matri	к-Р						Weight: 8 lb	FT = 20%

2-10-0

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. (size) 1=2-10-0, 3=2-10-0

Max Horz 1=-20(LC 8) Max Uplift 1=-4(LC 12), 3=-4(LC 13) Max Grav 1=81(LC 1), 3=81(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



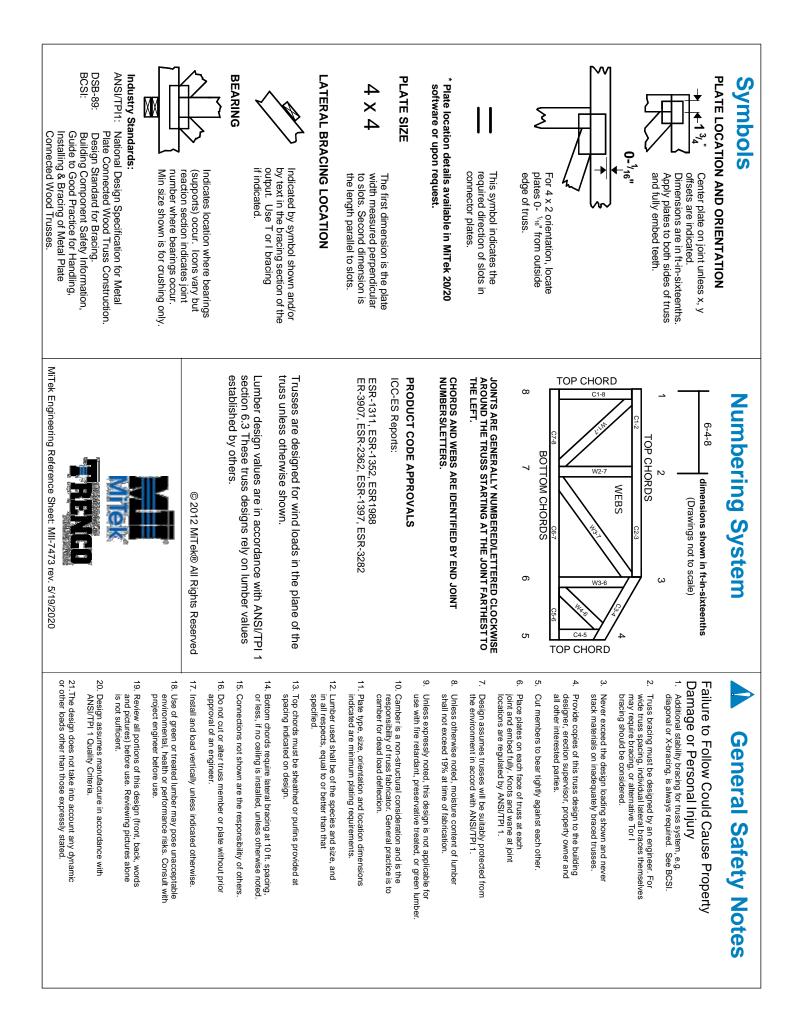
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

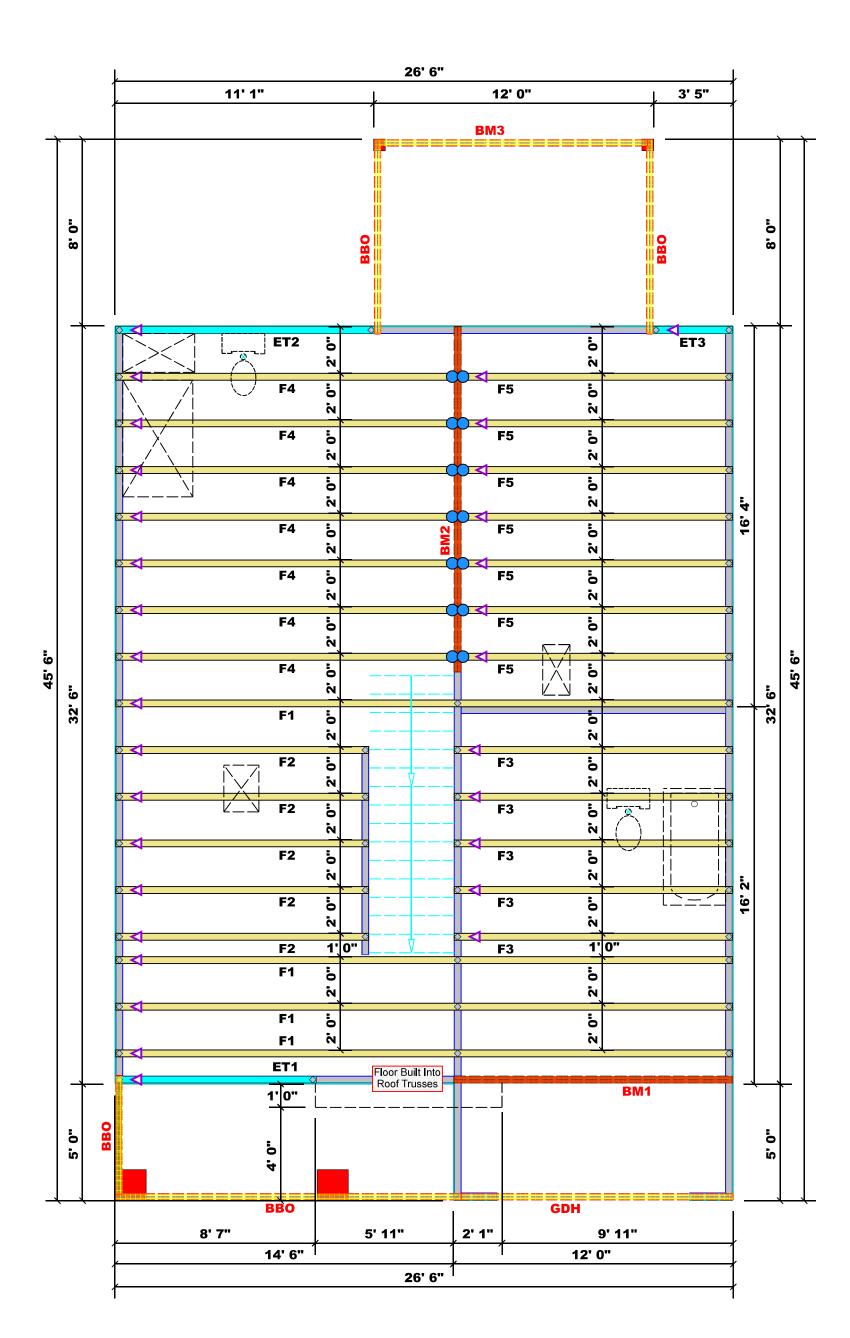


BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 2-10-0 oc purlins.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.





Phone: (910) 864-8787 Fax: (910) 864-4444 Bearing reactions less than or equal to 3000 are determined that the prescriptive Code requirements. The control shall be the the prescriptive Code requirement for all code shall be another to the Cade requirement for all code shall be another to support solution system for all code shall be another to all co

Dimension Notes All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes

1. Plumbing drop locations shown are NOT exact.

Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
 Adjust spacing as needed not to exceed 24"oc.

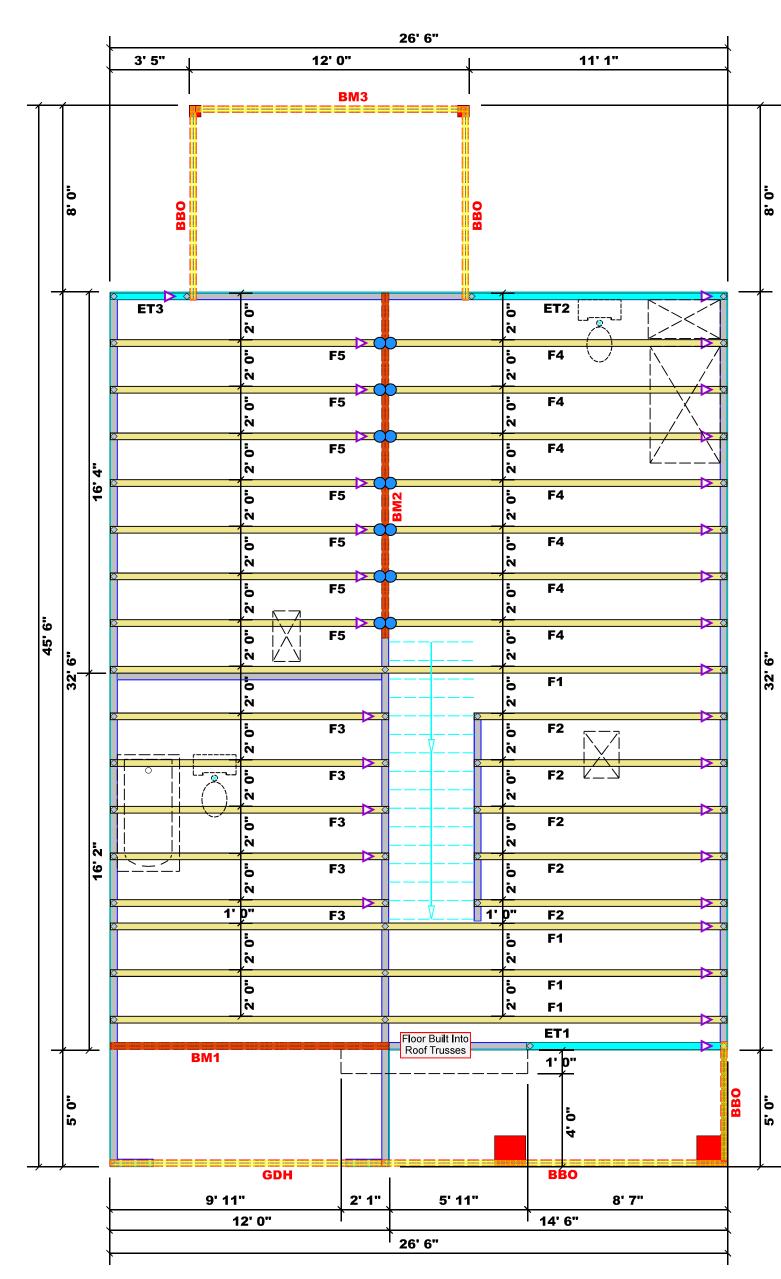
	Conne	Nail Info	ormation			
Sym	Product	Manuf	Header	Truss		
\bigcirc	HUS410	USP	14	NA	16d/3-1/2"	16d/3-1/2"

	Products									
PlotID	Length	Product	Plies	Net Qty						
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2						
BM2	15' 0''	1-3/4"x 16" LVL Kerto-S	2	2						
BM3	12' 0"	2x10 SPF No.2	2	2						
GDH	12' 0"	2x12 SPF No.2	2	2						

Truss Placement Plan Scale: 1/4"=1'



See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



			Te						
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TRUSSES & BEAMS Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787									
	Fax reactions	: (910) s less tha	864-4	1444 to 3000#					
Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.									
Signatu	rel		<i>d La</i> id La	<i>indr</i> ndry	y				
	(BASEI	D ON TABU	Es 85025(CK STU 1) & (b)) 5 @ EA END					
NO ECC 2010 1700 3400 5100 6800 8500 10200 11900	agy 241 Kia (2) 1 2 3 4 5 6 7	PEAD33 NO LLOV 44 0 2550 5100 7650 1020 12750	2 3 24 25	NOLLOVE 412 340 680 1021 1360 1700)0 1)0 2)0 3)0 4				
/ Harnett	4903 Barbecue Church Rd.			Jdry	rris				
Sanford /	4903 Bar	Floor	05/13/21	David Lan	Lenny Noi				
CITY / CO. Sanford / Harnett	ADDRESS	WODEL	DATE REV.	DRAWN BY David Landry	SALES REP. Lenny Norris				
ent Co. Inc.	Lot 3 Barbecue Church Rd.	₹,	2	#	-2897				
Weaver Development Co. In	Lot 3 Barbec	Hickory "C"	Seal Date	Quote #	J0521-2897				

Dimension Notes All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes Plumbing drop locations shown are NOT exact.
 Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
 Adjust spacing as needed not to exceed 24"oc.

	Conne	Nail Info	ormation			
Sym	Product	Header	Truss			
\bigcirc	HUS410	USP	14	NA	16d/3-1/2"	16d/3-1/2"

	Products		
Length	Product	Plies	Net Qty
12' 0"	1-3/4"x 16" LVL Kerto-S	2	2
15' 0''	1-3/4"x 16" LVL Kerto-S	2	2
12' 0"	2x10 SPF No.2	2	2
12' 0"	2x12 SPF No.2	2	2
	12' 0" 15' 0" 12' 0"	Length Product 12' 0" 1-3/4"x 16" LVL Kerto-S 15' 0" 1-3/4"x 16" LVL Kerto-S 12' 0" 2x10 SPF No.2	Length Product Plies 12' 0" 1-3/4"x 16" LVL Kerto-S 2 15' 0" 1-3/4"x 16" LVL Kerto-S 2 12' 0" 2x10 SPF No.2 2

Truss Placement Plan Scale: 1/4"=1'

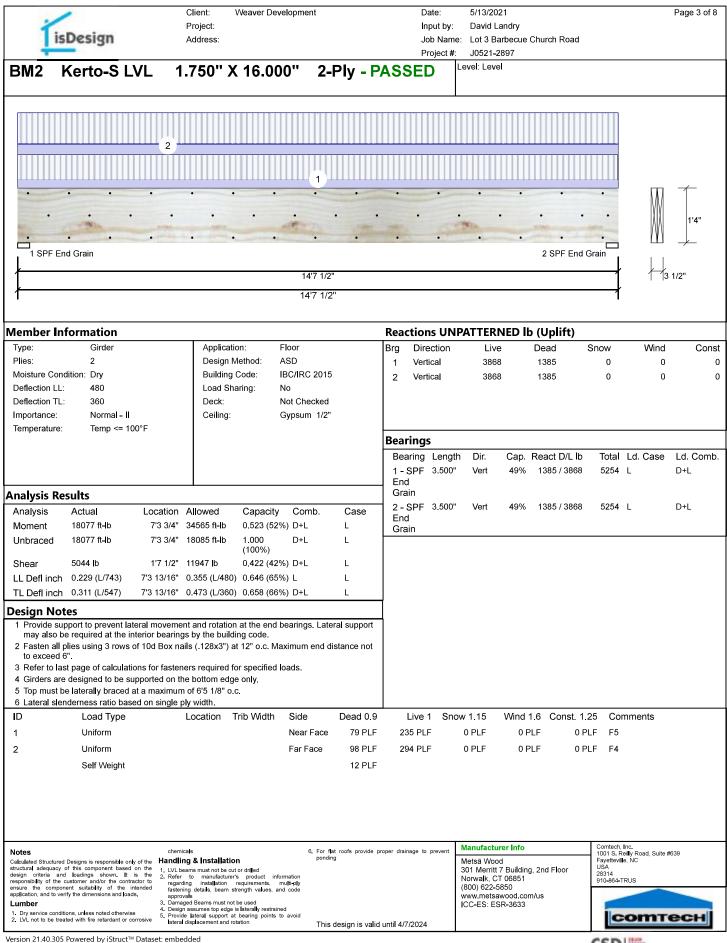
▲ = Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Design	Р	Client: V Project: Address:	Veaver Deve	lopment		lnı Jo	ate: out by: b Name: oject #:	5/13/2021 David Landry Lot 3 Barbecu J0521-2897	ue Church Road	l		Page 1 of
(erto-S L\	/L 1.1	750" X	16.00)" 2-1	Ply - P		<u> </u>					
	2											
				an co			40	2 SPF]			1'4"
			11'11"						ł		13	1/2"
			11'11"						1			
		Applicatio				1				0	146	0
2 lition: Dry 480 360 Normal - II		Design M Building (ethod: A Code: IE ring: N N	SD 8C/IRC 2015 o ot Checked		1 Verti	cal	0 0	2869 2869	2079 2079 2079	0	Cor
Temp <= 100	۴F					Bearings						
						Bearing 1 - SPF	Length 3.500"	Vert 95	% 2869 / 207	9 4948	L	Ld. Com D+S
sults		1				2 - SPF	3.500"	Vert 95	% 2869 / 207	9 4948	L	D+S
Actual 13679 ft-lb 13679 ft-lb 3615 lb 0.069 (L/2000)	5'11 1/2" 3 5'11 1/2" 1 1'7 1/2" 1 5'11 1/2" 0	9750 ft-lb 3699 ft-lb 3739 lb .287 (L/480)	0.999 (100%) 0.263 (26% 0.240 (24%	D+S) D+S) S	Case L L L L							
	5'11 1/2" 0	.383 (L/360)	0.428 (43%) D+S	L	1						
es port to prevent late required at the inte lies using 3 rows of	erior bearings f 10d Box nails	by the buildii s (.128x3") at	ng code. 12" o.c. Max r specified lo	timum end dis	••							
". t page of calculation designed to be sup nust be supported e a laterally braced at derness ratio based	oported on the equally by all p t a maximum o	lies. of 8'8 3/8" o.c	-									
t page of calculation designed to be sup nust be supported e a laterally braced at	pported on the equally by all p t a maximum o d on single ply	lies. of 8'8 3/8" o.c width.	-	Side	Dead 0.9	Live 1	I Snow	1.15 Win	d 1.6 Const.	1.25 Co	mments	
	i i	2 Cormation Girder 2 ition: Dry 480 360 Normal - II Temp <= 100°F Sults Actual Location A 13679 ft-lb 5'11 1/2" 3 13679 ft-lb 5'11 1/2" 1 3615 lb 1'7 1/2" 1 0.069 (L/2000) 5'11 1/2" 0 0.164 (L/840) 5'11 1/2" 0	Image: Constraint of the second state of th	2 1 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 6ormation Application: Girder Application: 2 Building Code: 480 Building Code: 360 Deck: Normal - II Deck: Temp <= 100°F	2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 11 12 13679 ft-lb 5 11 13679 ft-lb 5 5 11 13679 ft-lb 5 5 11 11 12 13679 ft-lb 5 5	Image: constraint of the second sec	2 1 2 1 2 1 11'11" 11'11" ormation Reaction Cirder Application: Floor 2 Dire Building Code: IBC/IRC 2015 Load Sharing: No Deck: Not Checked Ceiling: Gypsum 1/2" Easting: Bearings 1:S679 ft-lb 5'11 1/2" 3950 ft-lb 0.344 (34%) D+S L 13679 ft-lb 5'11 1/2" 13699 ft-lb 0.999 D+S L .3615 lb 1'7 1/2" 13739 lb 0.263 (26%) D+S L 0.069 (L/2000) 5'11 1/2" 0.247 (L/480) 0.240 (24%) S L	Application: Floor Brg Direction 1''11" 1''11"	Image: construction of the second s	Application: Floor Provide 2 1 2 1 2 2 1 2 2 1 2 2 1 2 2 1 2 2 1 2 2 1 2 2 11'11" 2 2 11'11" 11'11" 2 11'11" 11'11" 2 11'11" 11'11" 2 11'11" 11'11" 11'11" 0 2 2 480 360 Deck: No Checked Ceiling: Gypsum 1/2" Brig Direction Live Dead 1 - 0 2869 2 Vertical 0 2869 360 Deck: No Checked 0 1379 fl-b<	Application: Floor String 2 1 2 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" 11'11" Direction Live Dead Snow 1 Verical 0 2869 2079 2 Uerical 0 2869 2079 2 Verical 0 2869 2079 2 Verical 0 2869 2079 2 Verical 0 2869 2079 1 Sprintla Sidon' Veri 95% 2869/2079 1 Sprintla Sidon' Veri <td< td=""><td>Application: Provide Provide</td></td<>	Application: Provide Provide

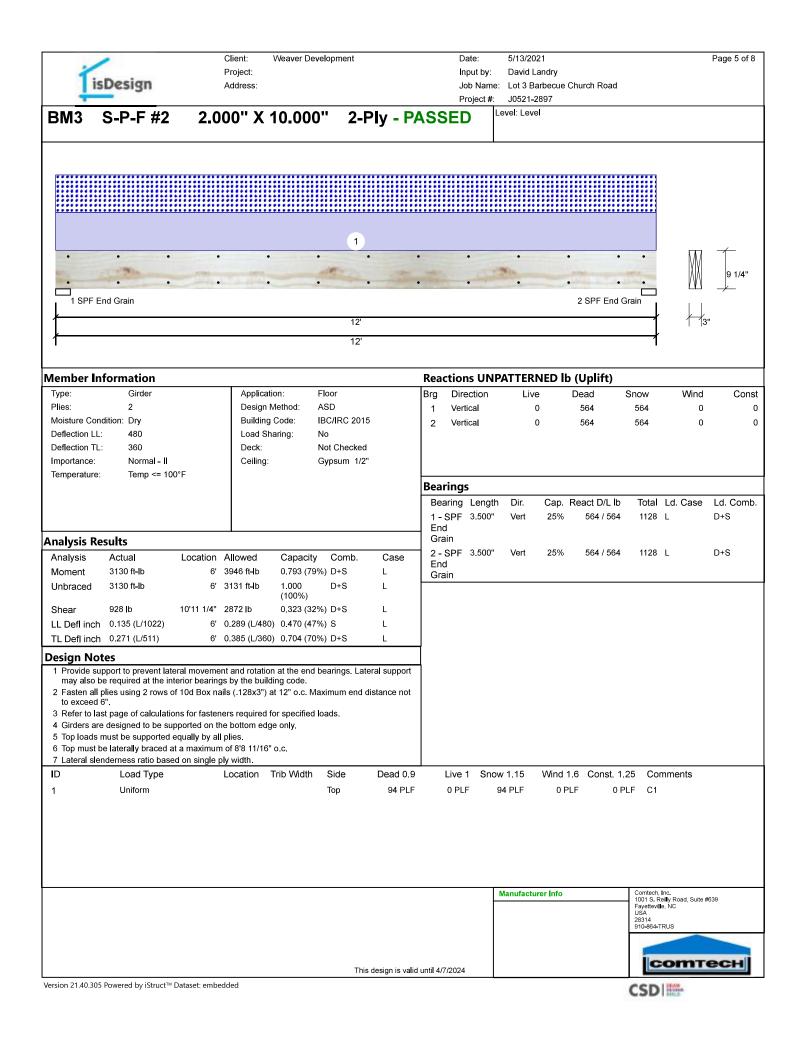
	1		Client:	Weaver Developm	ient		Date:	5/13/2021	Page 2 of 8
4			Project:				Input by:	David Landry	
	isDesign		Address:					E: Lot 3 Barbecue Church Road	
-							Project #:	J0521-2897	
BM1	Kerto-S	LVL	1.750"	X 16.000"	2-Ply	- PASS	ED	Level: Level	
					•				
•	• •	•	•	• •	•	• •	•	•••	NAA 1
.	•	•	• •	•	• •	•	•	. 1/2	1'4"
								$\overline{\nabla}$	M 14
	• •	•	٠	• •	•	• •	•		
1 SPF	F							2 SPF //	
1				11'11"				f	3 1/2"
/				11'11"				ł	
•									
-	y Analysis								
	plies using 3			(.128x3") at 12"	o.c Maxim	um end di	stance no	ot to exceed 6".	
Capacity		0.0							
Load Yield Limit p	er Foot	0.0 F 245	PLF 6 PLF						
Yield Limit p		81.9							
Yield Mode		IV							
Edge Distan Min. End Dis		1 1/2 3"	2"						
Load Combi		3							
Duration Fac		1.00							
Notes			chemicals		6. For flat roofs on	ovide proper drainag	e to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Rei∎y Road, Suite #639
Calculated Struc	ctured Designs is responsibluacy of this component ba		landling & Installa		ponding		-	Metsä Wood	Fayetteville, NC USA
design criteria	uacy of this component ba and loadings shown. the customer and/or the c	It is the	2. LVL beams must not be 2. Refer to manufacture regarding installation	urer's product information				301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	28314 910-864-TRUS
ensure the co	the customer and/or the component suitability of the to verify the dimensions and	ne intended	regarding installation fastening details, bear approvals	n requirements, mu l ti-ply n strength values, and code				(800) 622-5850 www.metsawood.com/us	
Lumber		. 4	 Damaged Beams must Design assumes top ed 	ge is aterally restrained				ICC-ES: ESR-3633	
2. LVL not to b	conditions, unless noted othe be treated with fire retardant	erwise g	5. Provide lateral suppor lateral displacement an	t at bearing points to avoid	This design is	s valid until 4/7/2	024		соттесн
Version 21.40	305 Powered by iStru		ombaddad						

Version 21.40.305 Powered by iStruct™ Dataset: embedded



	1		Client:	Weaver Developm	nent	Da	te:	5/13/2021	Page 4 of 8
1			Project:				ut by:	David Landry	
1	isDesign		Address:					Lot 3 Barbecue Church Road	
			4 3501	<u></u>			oject #:	J0521-2897 evel: Level	
BM2	Kerto-S	LVL	1.750"	X 16.000"	2-Ply	- PASSED			
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							_		. I V M I *
	End Grain	•	•	• •	• •	•	•	• • • • • • • • • • • • • • • • • • •	
								2 311 Ella	
					14'7 1/2"				1 3 1/2"
/					14'7 1/2"				
Multi-Ply	y Analysis								
-	-	rows of 10	d Box nails	(.128x3") at 12'	'oc Maxim	um end distar	nce no	t to exceed 6"	
Capacity	p	79.8 %							
Load		196.0 F							
Yield Limit pe Yield Limit pe		245.6 F 81.9 lb.							
Yield Mode		IV							
Edge Distand		1 1/2" 3"							
Min. End Dis Load Combir		3 D+L							
Duration Fac		1.00							
Notes			emicals		6. For flat roofs pro	ovide proper drainage to p	levent	Manufacturer Info	Comtech, Inc. 1001 S. Rei∎y Road, Suite #639
structura adequ	tured Designs is responsib lacy of this component b	ased on the 1 LV	dling & Installa /L beams must not be	cut or dried	ponding			Metsä Wood 301 Merritt 7 Building, 2nd Floor	Fayetteville, NC USA
responsibility of	and loadings shown the customer and/or the imponent suitability of t	It is the 2 Re contractor to re	efer to manufacti garding insta ll atior	urer's product information n requirements, multi-ply				Norwalk, CT 06851 (800) 622-5850	28314 910-864-TRUS
application, and t	to verify the dimensions and	dloads. ap	stening details, bear provals amaged Beams must	n strength values, and code not be used			· ·	www.metsawood.com/us ICC-ES: ESR-3633	
1. Dry service c	conditions, unless noted oth e treated with fire retardant	erwise 5. Pr	esign assumes top eo ovide atera suppor	lge is laterally restrained t at bearing points to avoid				100-LO. LON-3033	соттесн
		a la	teral displacement an	d rotation	This design is	valid until 4/7/2024			

Version 21.40.305 Powered by iStruct™ Dataset: embedded



	1	Client	t: Wea	iver Developm	ient		Date:	5/13/2021	Page 6 of 8
1		Proje					Input by:	David Landry	
	isDesign	Addre	ess:					: Lot 3 Barbecue Church Road	
-							Project #:	J0521-2897	
BM3	S-P-F #2	2.000'	' X 10	.000"	2-Ply ·	- PASSE	ED	Level: Level	
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Multi-Ply	y Analysis								
	ہ ۔ I plies using 2 rov	ws of 10d Box r	nails (128	x3") at 12"	oc Maxir	num end di	stance no	ot to exceed 6"	
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Load		0.0 PLF							
Yield Limit pe Yield Limit pe		157.4 PLF 78.7 lb.							
Yield Mode		IV							
Edge Distand		1 1/2"							
Min. End Dis Load Combir		3"							
Duration Fac		1.00							
							г	Manufactures Info	Comtach Inc
							ŀ	Manufacturer Info	Comtech, Inc. 1001 S. Rei∎y Road, Suite #639 Fayetteville, NC
									USA 28314 910-864-TRUS
									910-864-TRUS
									соттесн
					This design	is valid until 4/7/2	024		

Ti	Design	Clier Proje Addr	ect:	velopment				₋andry arbecue Chu	rch Road			Page 7 c
GDH	S-P-F #2	2.000	" X 12.00)" 2-F	Ply - P/		Level: Lev					
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	End Grain	•	• • •	1	-		2 SDE 0	End Grain			\mathbb{W}	
			8	'10"			20111		-r			
				10"					7		0	
1			o	10					I			
ember Ir	nformation					Reactions	UNPATTER	NED lb (I	Inlift)			
Туре:	Girder		Application:	Floor		Brg Direct			-	now	Wind	Co
Plies:	2		-	ASD		1 Vertica	ıl	0 7	751	88	0	
Moisture Cor Deflection LL	-		J	IBC/IRC 2015 No		2 Vertica	l	0 7	751	88	0	
Deflection TL			-	Not Checked								
mportance:	Normal - II			Gypsum 1/2"								
emperature	: Temp <= 100°	F				_ ·						
						Bearings						
						Bearing L 1 - SPF 3 End	-	Cap. Rea 19%	act D/L lb 751 / 88	Total Lo 839 L	d. Case	Ld. Co D+S
nalysis R	esults	I				Grain						
Analysis	Actual	Location Allo	wed Capacity	Comb.	Case	2-SPF 3 End	.500" Vert	19%	751 / 88	839 L		D+S
Moment	1490 ft -l b	4'5" 4153	,		Uniform	Grain						
Unbraced	1490 ft-lb	4'5" 3539	· ·		Uniform							
Shear LL Defl inch	542 lb	1'2 3/4" 2734	lb 0.198 (20) 9 (L/480) 0.021 (2%)	,	Uniform							
LL Defi Incr	(L/22622)	451/16 0.20	9 (L/480) 0.021 (2%	o) S	L							
L Defl inch	0.042 (L/2381)	4'5 1/16" 0.27	9 (L/360) 0.151 (15	%) D+S	L							
esign No	tes					1						
may also 2 Fasten all to exceed 3 Refer to la 4 Girders ar 5 Top loads 6 Top must	ast page of calculations e designed to be supp must be supported eq be laterally braced at e	rior bearings by 10d Box nails (.1 s for fasteners re ported on the bot jually by all plies and bearings.	the building code. 28x3") at 12" o.c. M equired for specified tom edge only.	aximum end di								
7 Lateral sle ID	enderness ratio based Load Type	0 1 7	ation Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1 6	Const. 1.2	5 Comm	nents	
1	Uniform	2000		Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PL			
2	Uniform			Тор	90 PLF	0 PLF	0 PLF	0 PLF	0 PL			
3	Tie-In	0-0-0 to 8-	10-0 1-0-0	Тор	20 PSF	0 PSF	20 PSF	0 PSF	0 PS	F Roof L	oad	
							Manufacti	irer Info		Comtech, Inc. 1001 S. Reilly Fayetteville, N USA	Road, Suite #6 C	339
									·	28314 910-864-TRU		есн

Ť	isDesign	Client: Project: Address:	Weaver Developme	nt			5/13/2021 David Landry Lot 3 Barbecue Church Road	Page 8 of 8
GDH	S-P-F #2	2.000" X	12.000"	2-Ply -	PASSE	Project #:	J0521-2897 evel: Level	
•	•	• •	•	•	•	•	••]	
							11/2"	11 1/4"
	•	• •	•	•	•	•		
	F End Grain		8'10"				2 SPF End Grain	3"
∤			8'10"					
Multi-Ply	Analysis							
Fasten all		vs of 10d Box nails	(.128x3") at 12" (o.c Maxim	um end dis	tance no	t to exceed 6".	
Capacity Load Yield Limit pe	r Foot	0.0 % 0.0 PLF 157.4 PLF						
Yield Limit pe Yield Mode		78.7 lb.						
Edge Distanc Min. End Dist		1 1/2" 3"						
Load Combin Duration Fact	ation	1.00						
	-							
						P	Manufacturer Info	Comtech, Inc. 1001 S. Relity Road, Suite #639 Fayetteville, NC
								Fayetteville, NC USA 28314 910-864-TRUS
				This design is	valid until 4/7/20	024		соттесн



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0521-2897 Lot 3 Barbecue Church Rd.

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15725976 thru E15725983

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



May 13,2021

Gilbert, Eric **IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

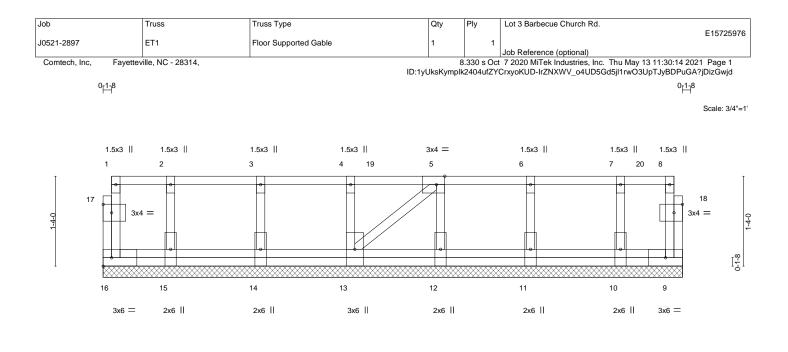


Plate Offsets (X,Y)	[5:0-1-8,Edge], [17:0-1-8,0-1-8], [18:0-1	-8 0-1-81	8-7-0 8-7-0			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.00 WB 0.05 Matrix-P	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	n - n/a 999 n - n/a 999	PLATES MT20 Weight: 54 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	<i>,</i> , , , , , , , , , , , , , , , , , ,	oc purlins,	

REACTIONS. All bearings 8-7-0.

2x4 SP No.3(flat)

(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

OTHERS

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

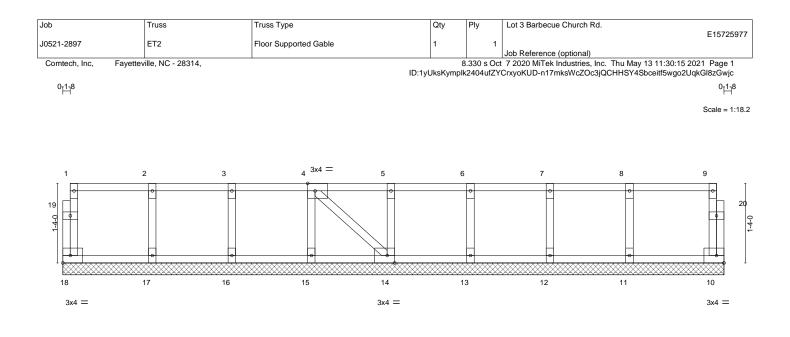
Uniform Loads (plf) Vert: 9-16=-10, 1-8=-100 Concentrated Loads (lb) Vert: 3=-71 6=-71 19=-71 20=-77



May 13,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			<u>11-1-0</u> 11-1-0			
Plate Offsets (X,Y)	[4:0-1-8,Edge], [14:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.01 WB 0.04 Matrix-S	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	a - n/a 999 a - n/a 999	PLATES MT20 Weight: 52 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) OTHERS 2x4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o		•	

REACTIONS.

DNS. All bearings 11-1-0. (lb) - Max Grav All reactions 250 lb or less at joint(s) 18, 10, 17, 16, 15, 14, 13, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

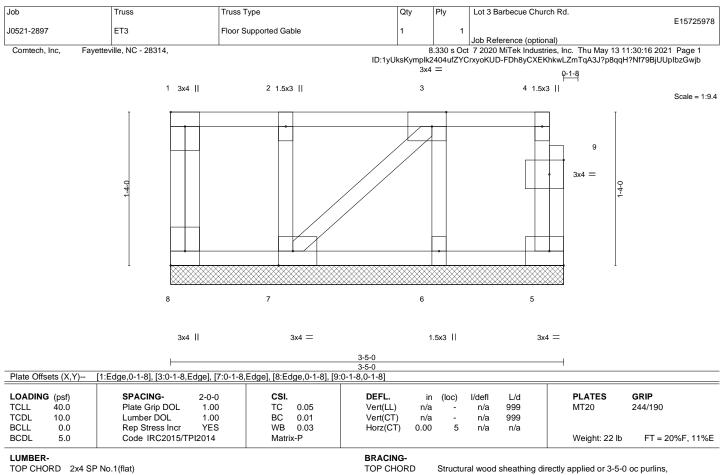
6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



May 13,2021

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BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.3(flat) WEBS 2x4 SP No.3(flat) OTHERS

REACTIONS. All bearings 3-5-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

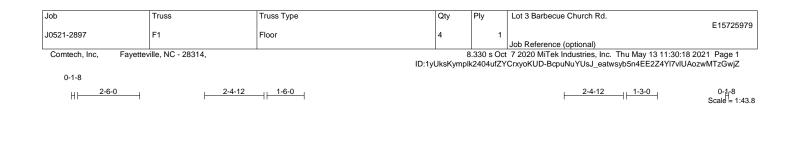
Strongbacks to be attached to walls at their outer ends or restrained by other means.

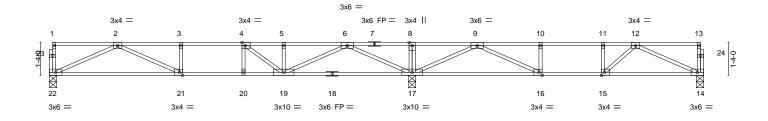
6) CAUTION, Do not erect truss backwards.



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 	<u>14-7-12</u> 14-7-12					<u>26-5-</u> 11-9-		
Plate Offsets (X,	') [4:0-1-8,Edge], [15:0-1-8,Edge], [16:0-7	-8,Edge], [21:0-1-8,Edge]						
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.55 BC 0.59 WB 0.53 Matrix-S	Vert(CT)	in (loc) -0.17 21-22 -0.25 21-22 0.03 14	l/defl >999 >697 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 129 lb	GRIP 244/190 FT = 20%F, 11%E
BOT CHORD 2 WEBS 2 REACTIONS.	x4 SP No.1(flat) x4 SP No.1(flat) x4 SP No.3(flat) (size) 22=0-3-8, 17=0-3-8, 14=0-3-8 /lax Grav 22=728(LC 10), 17=1669(LC 1), 1-	I=562(LC 7)	BRACING- TOP CHORE BOT CHORE	except D Rigid c	end vertie eiling dire	cals.	ectly applied or 6-0-0 d or 10-0-0 oc bracing, 1	•
TOP CHORD	Max. Comp./Max. Ten All forces 250 (lb) o 2-3=-1987/0, 3-4=-1987/0, 4-5=-1740/0, 5-6 9-10=-1183/0, 10-11=-1183/0, 11-12=-1183, 21-22=0/1314, 20-21=0/1987, 19-20=0/198; 15-16=0/1183, 14-15=0/954 8-17=-284/0, 2-22=-1440/0, 2-21=0/745, 3-2 4-19=-646/0, 9-17=-1465/0, 9-16=0/917, 10-	1740/0, 6-8=0/1282, 8-9 0 7, 17-19=-191/818, 16-17= 1=-260/0, 6-17=-1781/0, 6	=0/1282, 366/574, 5-19=0/1122,					
NOTES-								

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.





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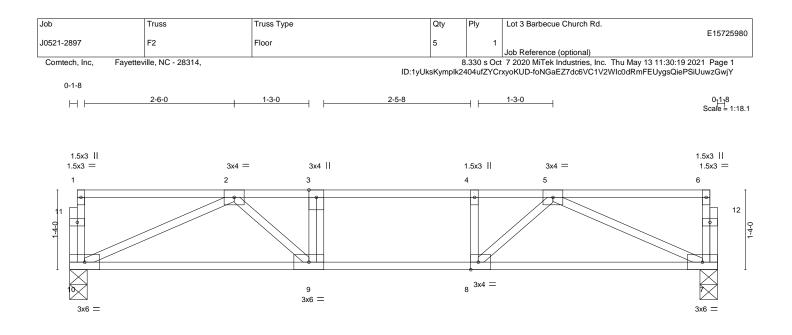


Plate Offsets (X,Y)	[8:0-1-8,Edge]		10-10-0	
Plate Olisets (A, F)	[0.0-1-o,Euge]			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl	L/d PLATES GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.42	Vert(LL) -0.08 9-10 >999	480 MT20 244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.35	Vert(CT) -0.11 9-10 >999	360
BCLL 0.0	Rep Stress Incr YES	WB 0.30	Horz(CT) 0.02 7 n/a	n/a
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S		Weight: 56 lb FT = 20%F, 11%E
LUMBER-			BRACING-	
TOP CHORD 2x4 SF	P No.1(flat)		TOP CHORD Structural wood sh	heathing directly applied or 6-0-0 oc purlins,
BOT CHORD 2x4 SF	P No.1(flat)		except end vertica	ıls.
WEBS 2x4 SF	P No.3(flat)		BOT CHORD Rigid ceiling direct	tly applied or 10-0-0 oc bracing.

REACTIONS. (size) 10=0-3-8, 7=0-3-8 Max Grav 10=576(LC 1), 7=576(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1234/0, 3-4=-1234/0, 4-5=-1234/0

BOT CHORD 9-10=0/981, 8-9=0/1234, 7-8=0/982

WEBS 2-10=-1073/0, 5-7=-1075/0, 5-8=0/485, 2-9=0/478, 3-9=-255/0, 4-8=-266/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

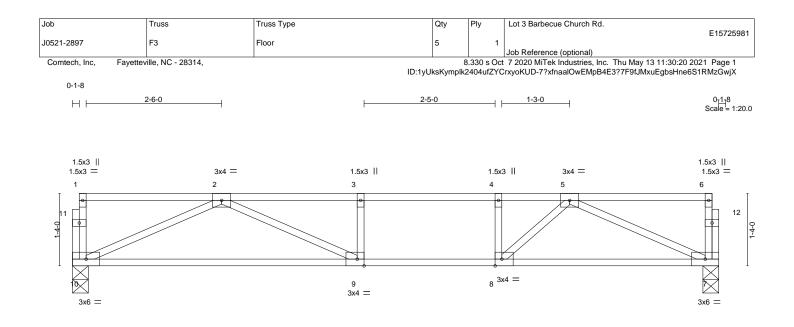


May 13,2021

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10-10-0 10-10-0



			11-11-0			
Plate Offsets (X,Y)	[8:0-1-8,Edge], [9:0-1-8,Edge]		11-11-0			
		1				
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. ir	n (loc) l/defl L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.68	Vert(LL) -0.19	9 9-10 >740 480	MT20	244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.56	Vert(CT) -0.29	9-10 >490 360		
BCLL 0.0	Rep Stress Incr YES	WB 0.34	Horz(CT) 0.02	2 7 n/a n/a		
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 59 lb	FT = 20%F, 11%E
LUMBER-			BRACING-			
TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)		TOP CHORD	Structural wood sheathing dir except end verticals.	rectly applied or 6-0-0) oc purlins,	
WEBS 2x4 SF	PNo.3(flat)		BOT CHORD	Rigid ceiling directly applied	or 10-0-0 oc bracing.	
REACTIONS. (size	e) 10=0-3-8, 7=0-3-8					

11-11-0

Max Grav 10=635(LC 1), 7=635(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1508/0, 3-4=-1508/0, 4-5=-1508/0

BOT CHORD 9-10=0/1112, 8-9=0/1508, 7-8=0/1121

WEBS 2-10=-1219/0, 2-9=0/558, 5-7=-1228/0, 5-8=0/655, 4-8=-353/0

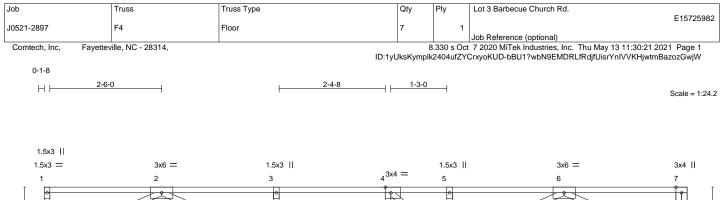
NOTES-

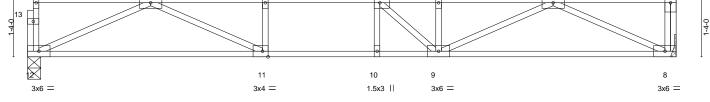
 Unbalanced floor live loads have been considered for this design.
 Plates checked for a plus or minus 1 degree rotation about its center.
 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



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			14-6-0			
Plate Offsets (X,Y) [4:0-1-8,Edge], [11:0-1-8,Edge]		14-6-0			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc) I/defl L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.61	Vert(LL) -0.20	9-10 >838 480	MT20	244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.84	Vert(CT) -0.25	9-10 >684 360		
BCLL 0.0	Rep Stress Incr YES	WB 0.46	Horz(CT) 0.03	8 n/a n/a		
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 73 lb	FT = 20%F, 11%E
LUMBER-		1	BRACING-			
	No.1(flat) No.1(flat)		TOP CHORD	Structural wood sheathing dir except end verticals.	ectly applied or 6-0-0	oc purlins,
	No.3(flat)		BOT CHORD	Rigid ceiling directly applied of	or 10-0-0 oc bracing.	
REACTIONS. (size) 12=0-3-8, 8=Mechanical					
Max Gr	rav 12=778(LC 1), 8=784(LC 1)					

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2243/0, 3-4=-2243/0, 4-5=-2186/0, 5-6=-2186/0

11-12=0/1424, 10-11=0/2243, 9-10=0/2243, 8-9=0/1429 BOT CHORD WEBS 2-12=-1561/0, 2-11=0/958, 3-11=-303/0, 6-8=-1573/0, 6-9=0/836, 5-9=-271/41, 4-9=-428/186

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

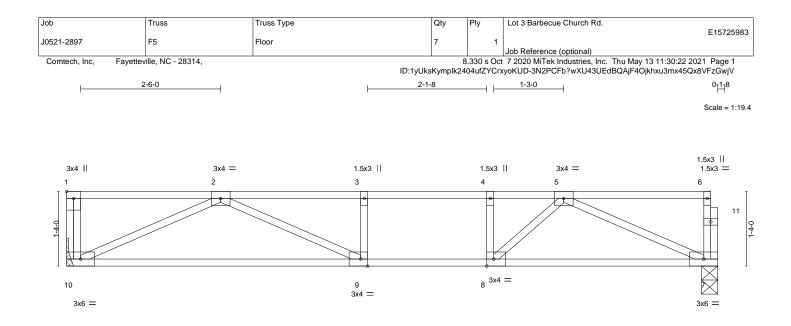
5) CAUTION, Do not erect truss backwards.





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 			<u>11-7-8</u> 11-7-8			
Plate Offsets (X,Y)	[1:Edge,0-1-8], [8:0-1-8,Edge], [9:0-1-8	,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.60 BC 0.51 WB 0.33 Matrix-S	DEFL. ir Vert(LL) -0.16 Vert(CT) -0.25 Horz(CT) 0.02	9-10 >540 360	PLATES MT20 Weight: 58 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	<i>y</i>) oc purlins,	
REACTIONS. (size Max G	e) 10=Mechanical, 7=0-3-8 irav 10=626(LC 1), 7=619(LC 1)					

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1441/0, 3-4=-1441/0, 4-5=-1441/0

BOT CHORD 9-10=0/1081, 8-9=0/1441, 7-8=0/1087

WEBS 2-10=-1190/0, 2-9=0/515, 5-7=-1190/0, 5-8=0/606, 4-8=-323/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



May 13,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



