PLANS DESIGNED TO THE 2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE.

BCR LOT 3 4903 BARBECUE CHURCH RD SANFORD,NC

GENERAL NOTES

- 1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AND REGULATIONS.
- 2. CONTRACTOR SHALL THOROUGHLY REVIEW ALL SHEETS IN PLAN SET AND VERIFY ALL DETAILS AND DIMENSIONS BEFORE BEGINNING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO RENAISSANCE RESIDENTIAL PROCEEDING WITH WORK. CONTRACTORS SHALL ASSUME RESPONSIBILITY FOR ERRORS THAT ARE NOT REPORTED PRIOR TO CONSTRUCTION.
- 3. ALL DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED.
- 4. CONTRACTOR SHALL ENSURE COMPATIBILITY OF THE BUILDING WITH ALL



THEY TO BE ASSIGNED TO ANY THIRD PARTY WITHOUT FIRST OBTAINING SAIL WRITTEN PERMISSION AND CONSENT.

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DATE: AUGUST 25, 2020

SCALE: 1/4" = 1'-0" DRAWN BY: WG

REVIEWED BY:

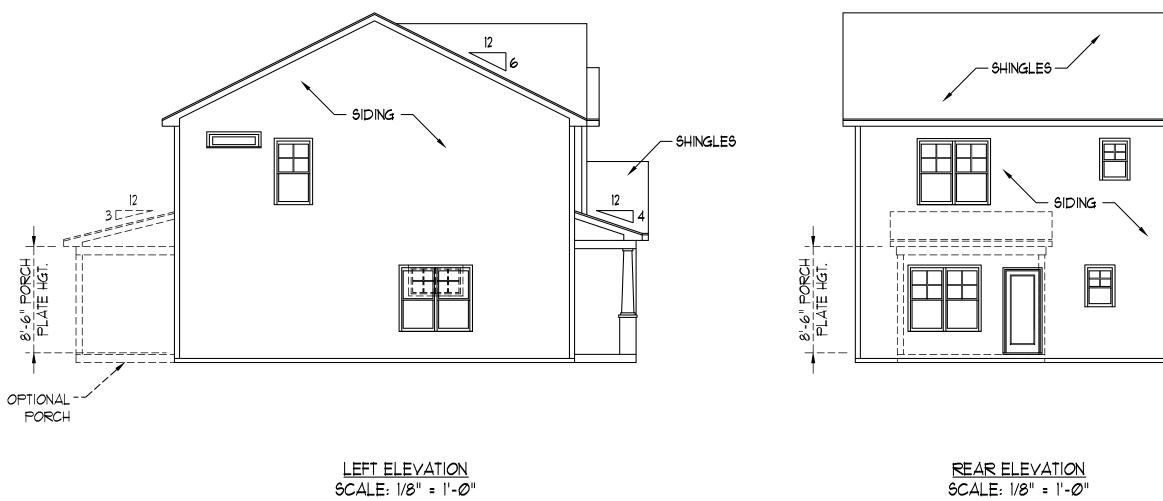
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C - ELEVATIONS

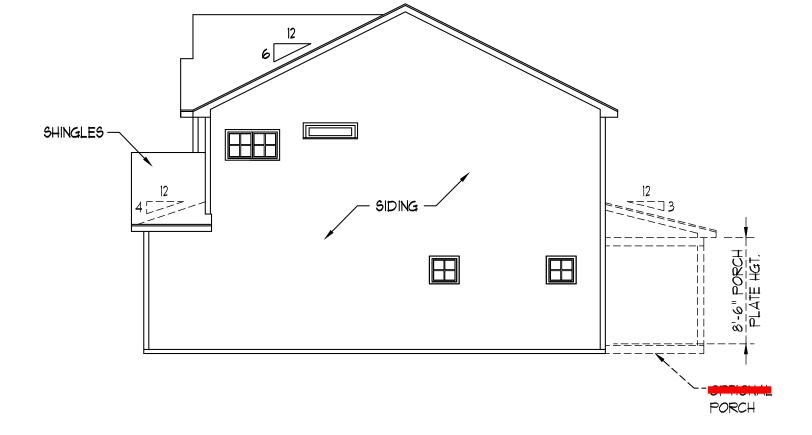
A-3

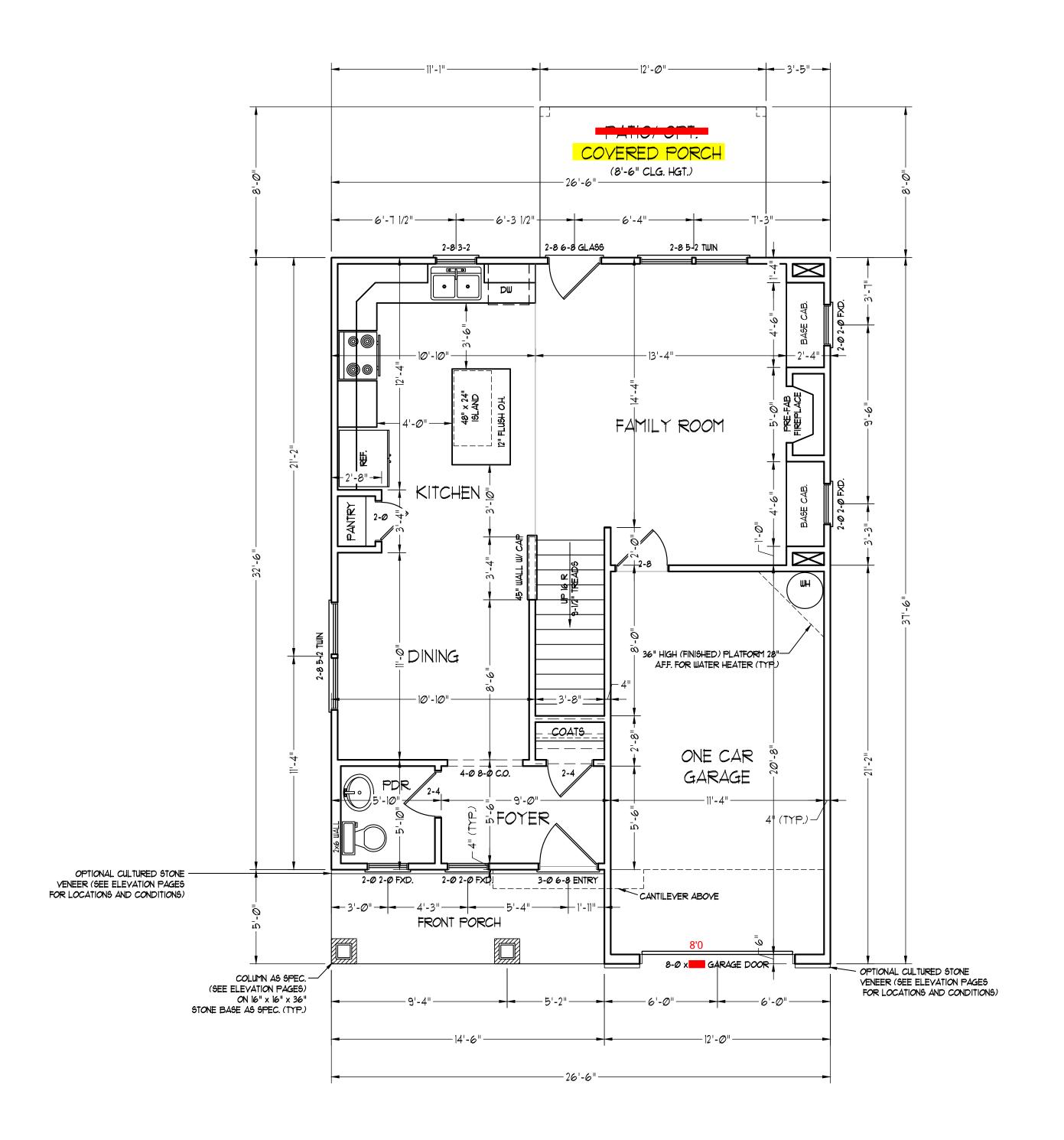


FRONT ELEVATION-C SCALE: 1/4" = 1'-0"



SCALE: 1/8" = 1'-0"





SQUARE FOOTAGE (I.F.S.)

 Ist FLOOR:
 639 SQ. FT.

 2nd FLOOR:
 195 SQ. FT.

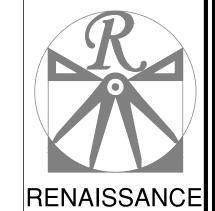
 TOTAL:
 1434 SQ. FT.

 GARAGE:
 232 SQ. FT.

 FRONT PORCH:
 13 SQ. FT.

 STD. REAR PATIO:
 96 SQ. FT.

 OPT. REAR PORCH:
 96 SQ. FT.



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VEST ON LOT WILL BE DETERMINED BY THE SITE PLAN AND

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ENGINEERED BY:

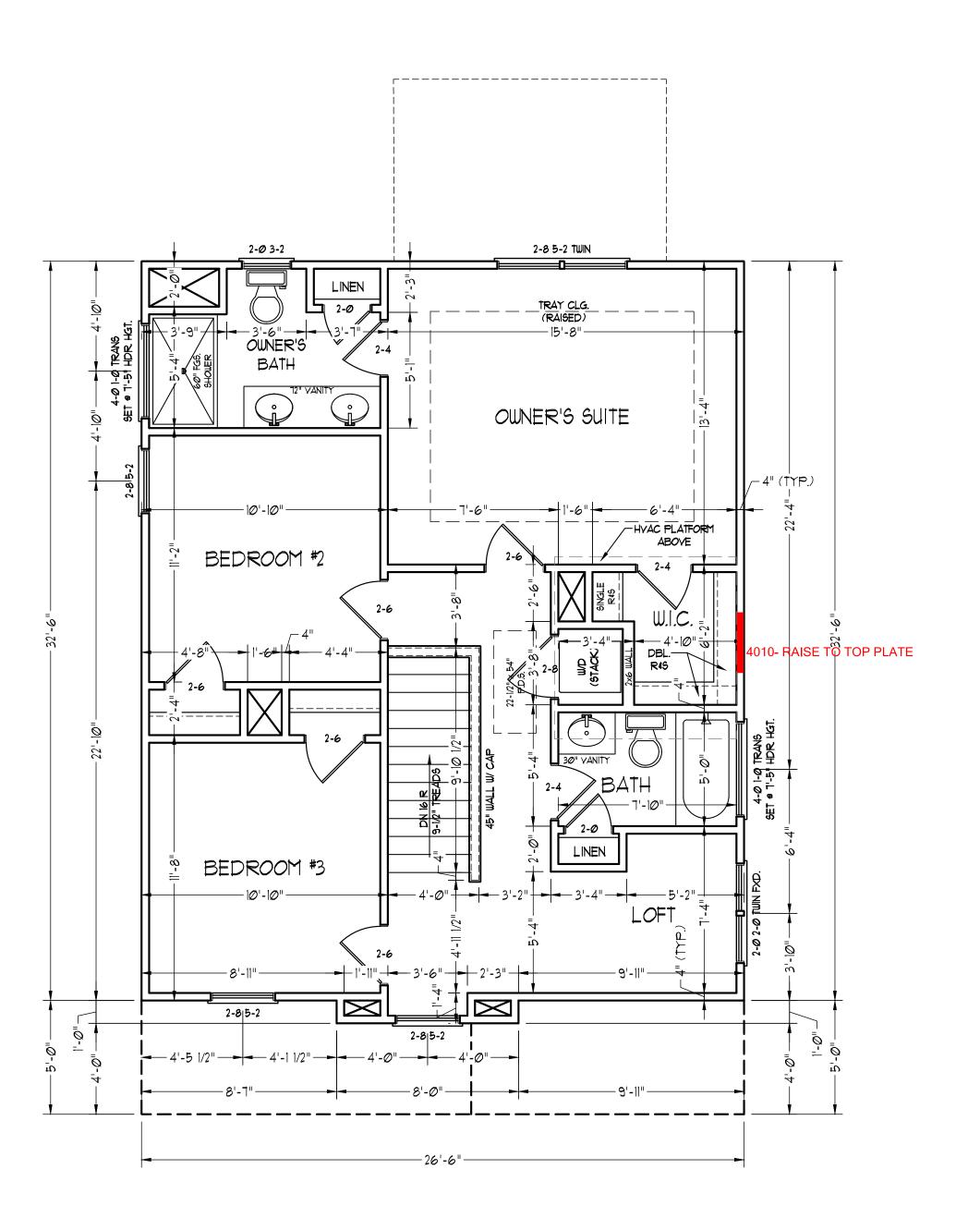
REVIEWED BY:

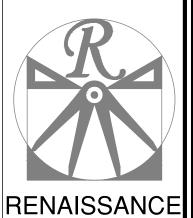
FIRST FLOOR

PLAN
A-4

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.

11x17 PRINTS ARE NOT TO SCALE





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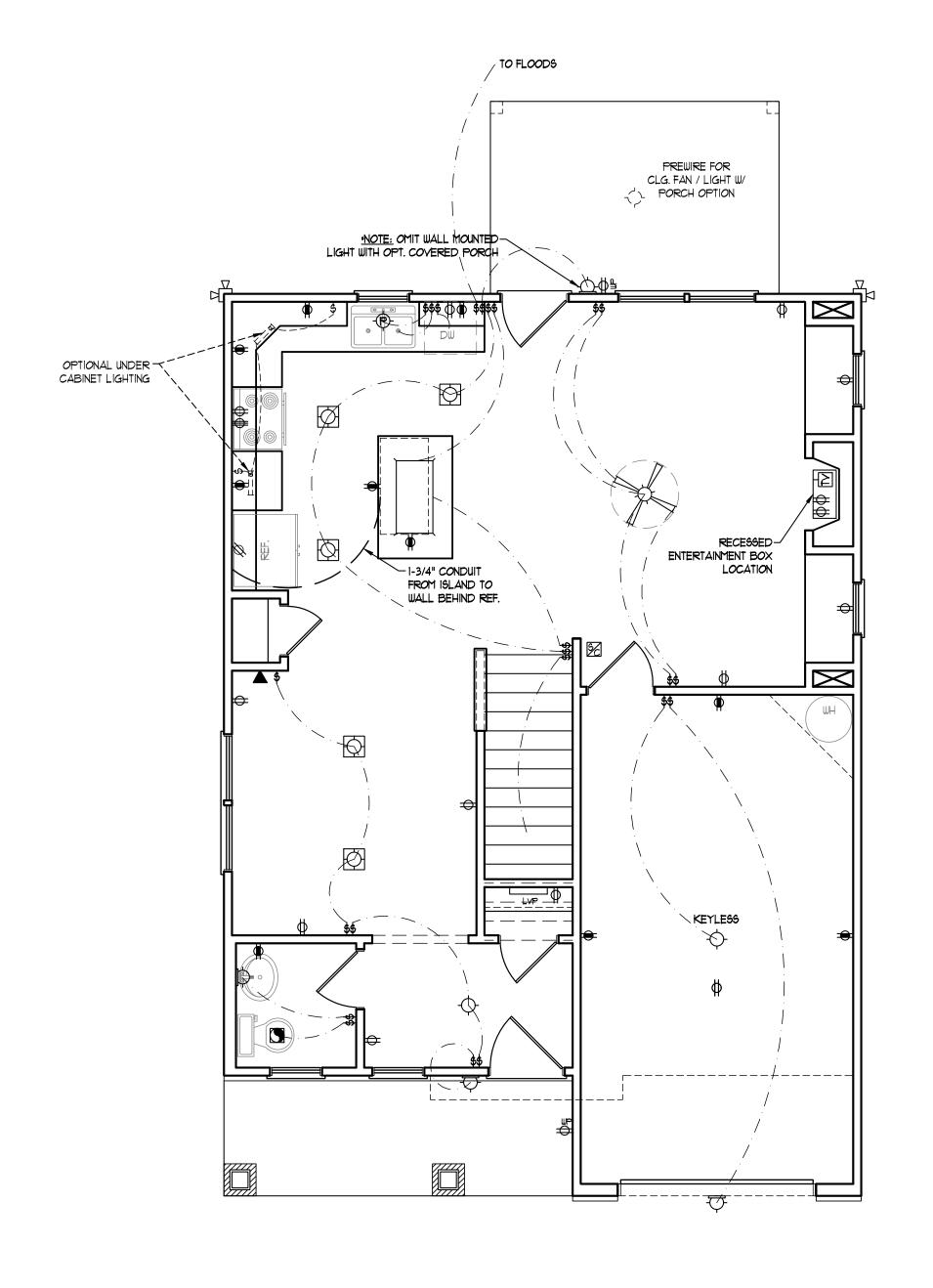
DRAWN BY: WG

ENGINEERED BY:
REVIEWED BY:

SECOND FLOOR

PLAN
A-5

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE



ELECTRICAL LAYOUT NOTES:

- 1.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.
- 2.) VANITY LIGHTS TO BE SET @ 90" AFF. (TYP.)
- 3.) ADDITIONAL EXTERIOR OUTLETS
 REQUIRED BY CODE TO BE
 LOCATED BY ELECTRICIAN.
- 4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

ELECTRICAL LEGEND

- →
 IIØ ∨ OUTLET
- = 110 V GFI OUTLET
- = 110 Y SWITCHED OUTLET
- BB 110 V BASEBOARD OUTLET
- 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110/ GF1
- ₩EATHERPROOF
- **⇒** 220 ∨ OUTLET
- Ø 110 V DEDICATED CIRCUIT
- # 220 Y DEDICATED CIRCUIT
- SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- -CEILING MOUNT LIGHT
- -P- PENDANT LIGHT
- RECESSED CAN LIGHT
- MINI CAN LIGHT
- EYEBALL LIGHT
- FLUORESCENT LIGHT
- UNDERCABINET LIGHT
- FLOOD LIGHT
- \$ SWITCH
- \$D DIMMER SWITCH
- ▲ TELEPHONE
- △ DATA
- TELEPHONE AND DATA
- TY- TY CONNECTION
- TV/ DATA
- CD- CONDUIT FOR COMPONENT WIRING:

 6P SPEAKER
- MOV SMOKE/ CM DETECTOR
- 5D 110 Y SMOKE DETECTOR

 EXHAUST FAN

LOW VOLTAGE PANEL

ALARM PANEL

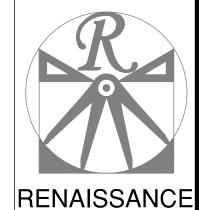




CEILING FAN W/ LIGHT

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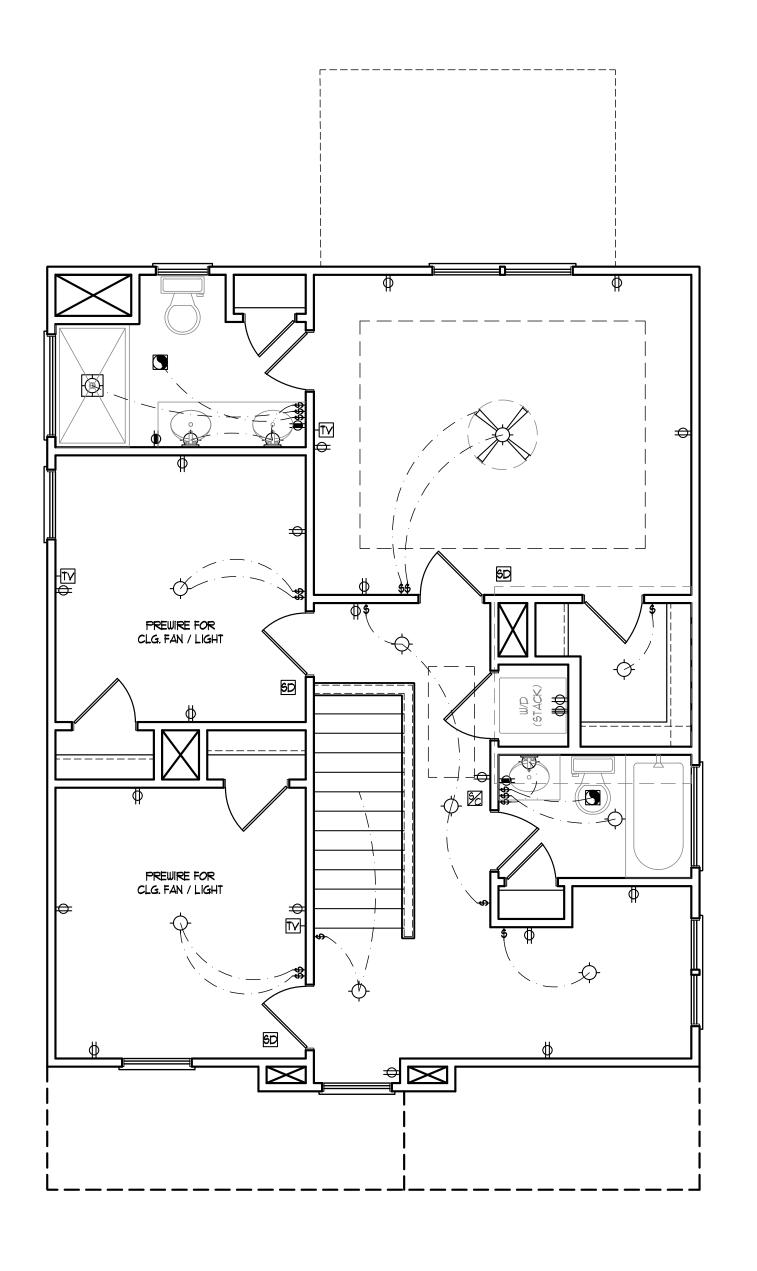
SCALE: 1/4" = 1'-0"

DRAWN BY: WG

ENGINEERED BY:
REVIEWED BY:

FIRST FLOOR ELECTRICAL PLAN

E-1



ELECTRICAL LAYOUT NOTES:

1.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.

2.) VANITY LIGHTS TO BE SET @ 90" AFF. (TYP.)

3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.

4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

ELECTRICAL LEGEND

- → 110 ∨ OUTLET
- € 110 Y GFI OUTLET
- = 110 Y SWITCHED OUTLET

BB - 110 V BASEBOARD OUTLET

- 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110V GF1
- ₩EATHERPROOF
- **⇒** 22Ø ∨ OUTLET
- Ø 110 V DEDICATED CIRCUIT
- # 220 Y DEDICATED CIRCUIT
- ► SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- -CEILING MOUNT LIGHT
- -P- PENDANT LIGHT
- RECESSED CAN LIGHT
- MINI CAN LIGHT
- EYEBALL LIGHT
- FLUORESCENT LIGHT

undercabinet light

- FLOOD LIGHT
- SWITCH
- \$D DIMMER SWITCH
- ▲ TELEPHONE
- △ DATA
- TV- TY CONNECTION
- TV/ DATA
- CD- CONDUIT FOR COMPONENT WIRING
- 110 V SMOKE/ CO DETECTOR

SPEAKER

110 Y SMOKE DETECTOR EXHAUST FAN

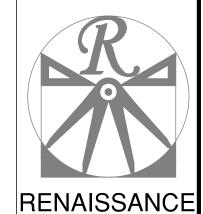
LOW VOLTAGE PANEL







SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE



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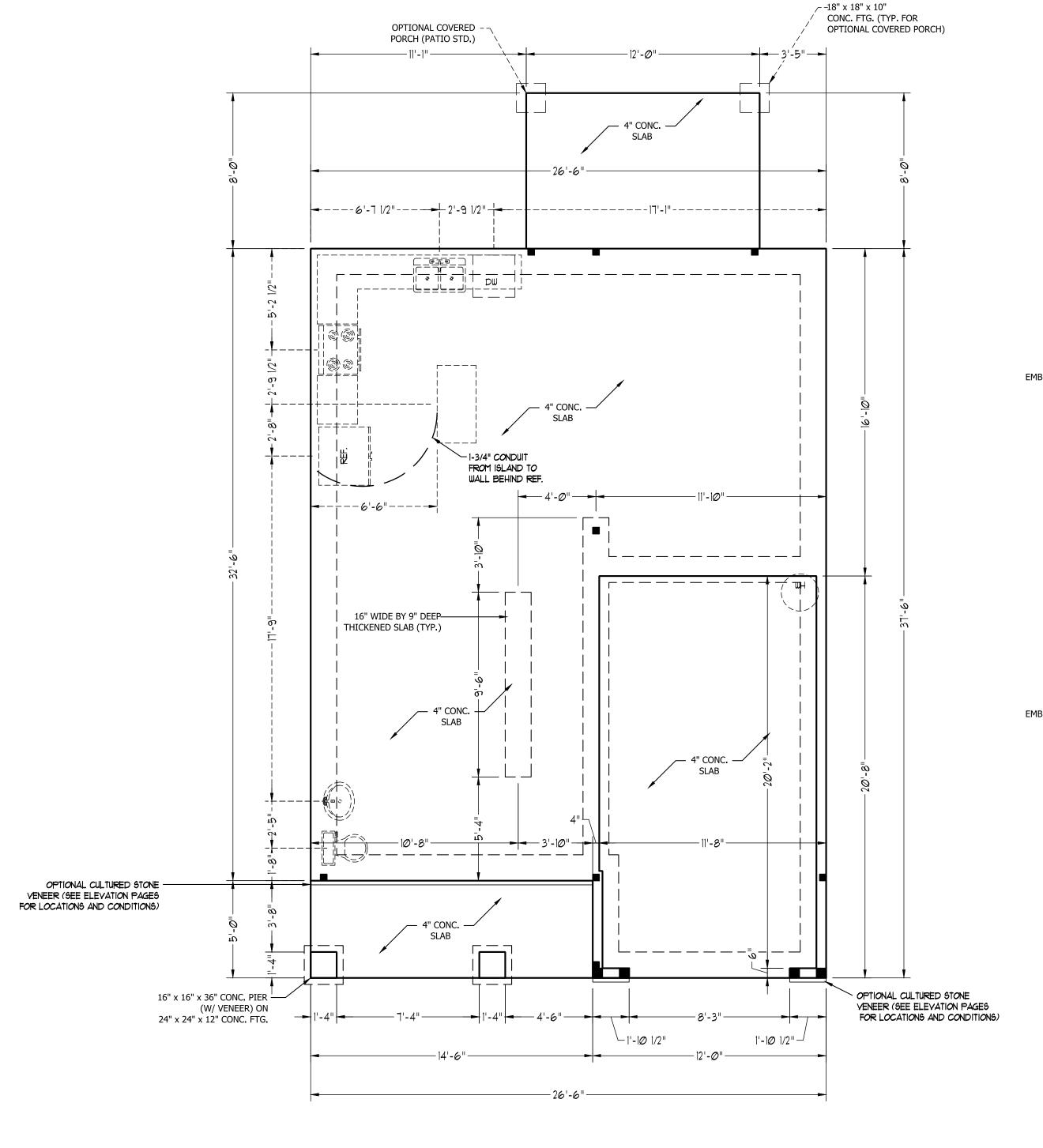
DATE: AUGUST 25, 2020

SCALE: 1/4" = 1'-0" DRAWN BY: WG

ENGINEERED BY: REVIEWED BY:

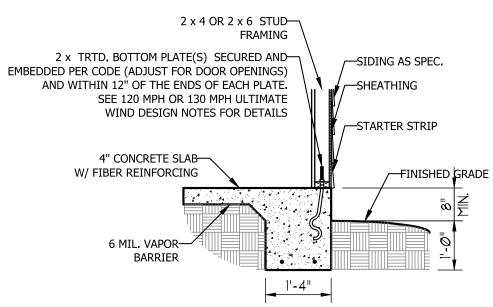
SECOND FLOOR ELCTRICAL PLAN

E-2

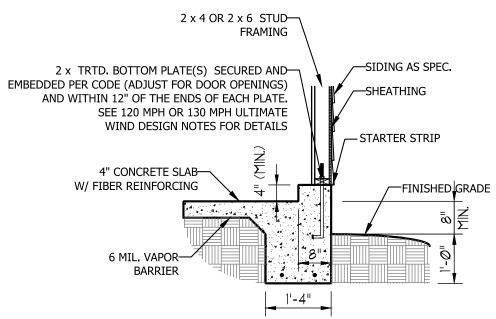


ULTIMATE DESIGN WIND SPEED NOTES FOR LESS THAN 30' MEAN ROOF HEIGHT:

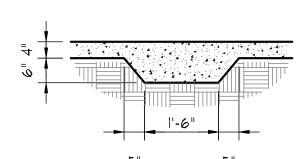
- 1. STRUCTURAL DESIGN PER NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION.
- 2. FOR 120 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 6'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
- 3. FOR 130 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 4'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
- 4. MEAN ROOF HEIGHT IS LESS THAN 30 FEET.
- 5. EXTERIOR WALLS DESIGNED FOR 120 OR 130 MPH WINDS.
- 6. INSTALL 7/16" OSB SHEATHING ON ALL EXTERIOR WALLS OF ALL STORIES IN ACCORDANCE WITH SECTION R602.10.3 OF THE NCRC, 2018 EDITION.
- 7. ENERGY EFFICIENCY COMPLIANCE AND INSULATION VALUES OF THE BUILDING TO BE IN ACCORDANCE WITH CHAPTER 11 OF THE NCRC, 2018 EDITION.



TYPICAL SLAB DETAIL



GARAGE CURB DETAIL



THICKENED SLAB DETAIL

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.

11x17 PRINTS ARE NOT TO SCALE



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REV.: SCALE: 1/4" = 1'-0"

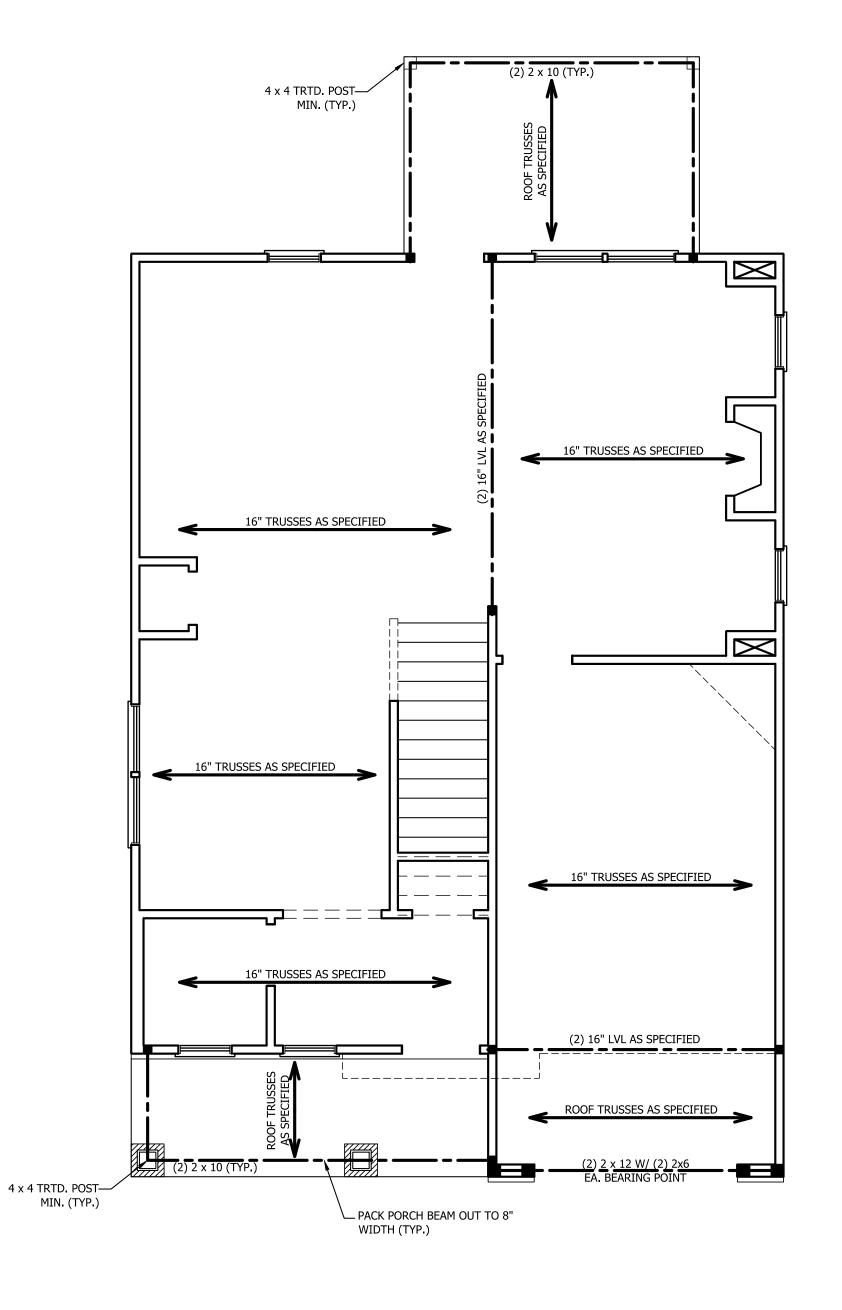
DRAWN BY: WG

REVIEWED BY:

ENGINEERED BY:

MONO SLAB FOUNDATION PLAN

S-1



STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 4 (UNO).
- 3. INSTALL AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
- 4. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD
- 5. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)
- 6. ALL 4 X 4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES (OR EQUAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4 AND 6 X 6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS AT TOP (UNO.)
- FOR FIBERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/
 (2) METAL ANGLES USING 2" CONC. SCREWS. FASTEN ANGLES TO COLUMNS W/
 1/4" THROUGH BOLTS W/ NUTS AND WASHERS. LOCATE ANGLES ON OPPOSITE
 SIDES OF COLUMN. THROUGH BOLTS MUST BE INSTALLED PRIOR TO SETTING
 COLUMN.

BRACE WALL PANEL NOTES:

EXTERIOR WALLS: ALL EXTERIOR WALLS TO BE SHEALTHED WITH CS-WSP OR CS-SFB IN ACCORDANCE WITH SECTION R602.10.3 UNLESS NOTED OTHERWISE.

REQUIRED LENGTH OF BRACING: REQUIRED BRACE WALL LENGTH FOR EACH SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE INTERPOLATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB CONTRIBUTE THIER ACTUAL LENGTH. METHOD GB CONTRIBUTES 0.5 ITS ACTUAL LENGTH. METHOD PF CONTRIBUTES 1.5 TIMES ITS ACTUAL LENGTH.

GYPSUM: ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS TO HAVE 1/2" GYPSUM INSTALLED. WHEN NOT USING METHOD GB GYPSUM TO BE FASTENED PER TABLE R702.3.5. METHOD GB TO BE FASTENED PER TABLE R602.10.1.

HD: 800 LBS HOLD DOWN DEVICE FASTENED TO THE EDGE OF THE BRACE WALL PANEL NEAREST TO THE CORNER

METHODS: PER TABLE R602.10.1

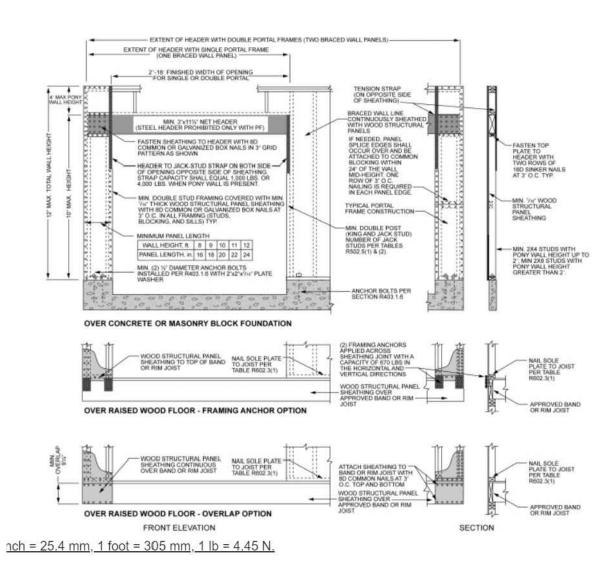


FIGURE R602.10.1
METHOD PF—PORTAL FRAME CONSTRUCTION

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WEAVER HOMES CAROLINA COLLECTIC HICKORY

DATE: AUGUST 25, 2020

REV.:

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DRAWN BY: WG

ENGINEERED BY:

REVIEWED BY:

SECOND FLOOR FRAMING PLAN

S-2

SCALE NOTE: 18x24 PRINTS ARE
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11x17 PRINTS ARE NOT TO SCALE

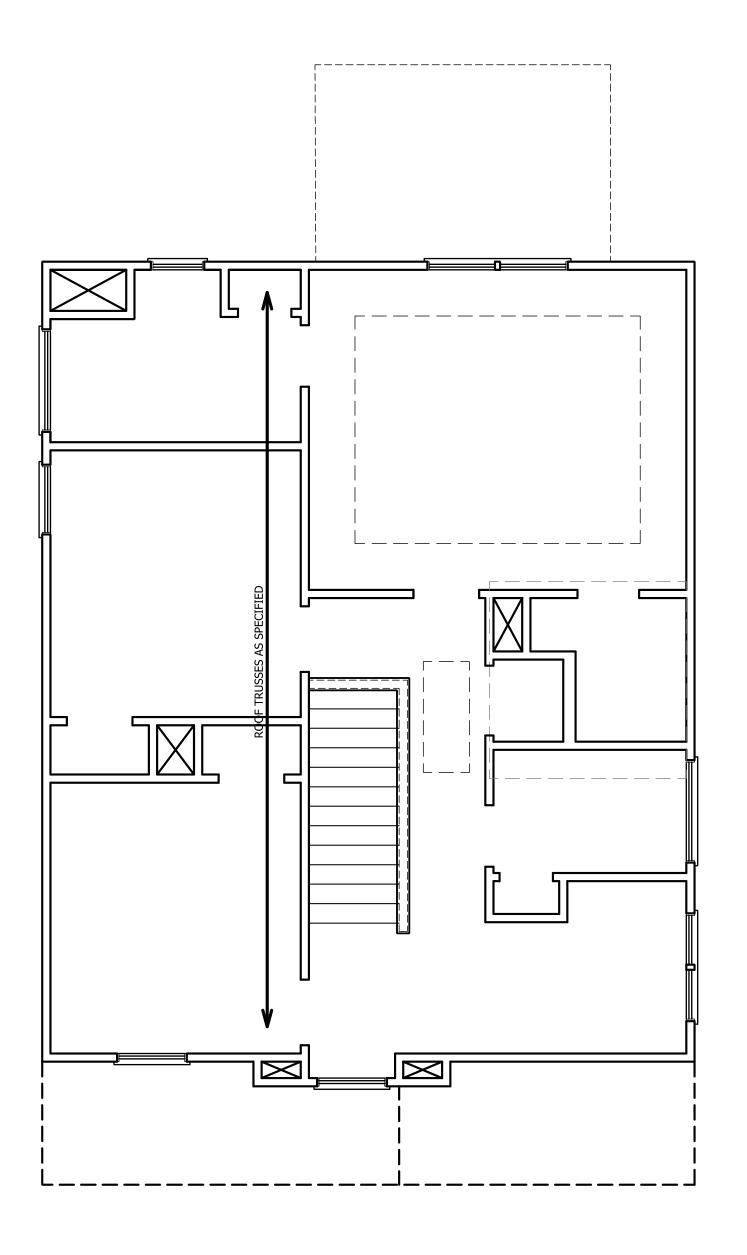


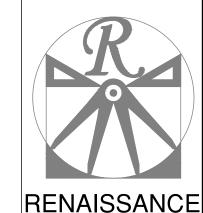
TABLE R602.7.5
MINIMUM NUMBER OF FULL HEIGHT STUDS
AT EACH END OF HEADERS IN EXTERIOR WALLS

HEADER SPAN (FEET)	MAXIMUM STUD SPACING (INCHES) (PER TABLE R602.3(5)				
(, == ,)	16	24			
UP TO 3'	1	1			
4'	2	1			
8'	3	2			
12'	5	3			
16'	6	4			

STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)

DSP - DOUBLE STUD POCKET TSP - TRIPLE STUD POCKET



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WEAVER HOMES CAROLINA COLLECTIO HICKORY

DATE: AUGUST 25, 2020

REV.:

SCALE: 1/4" = 1'-0"

DRAWN BY: WG

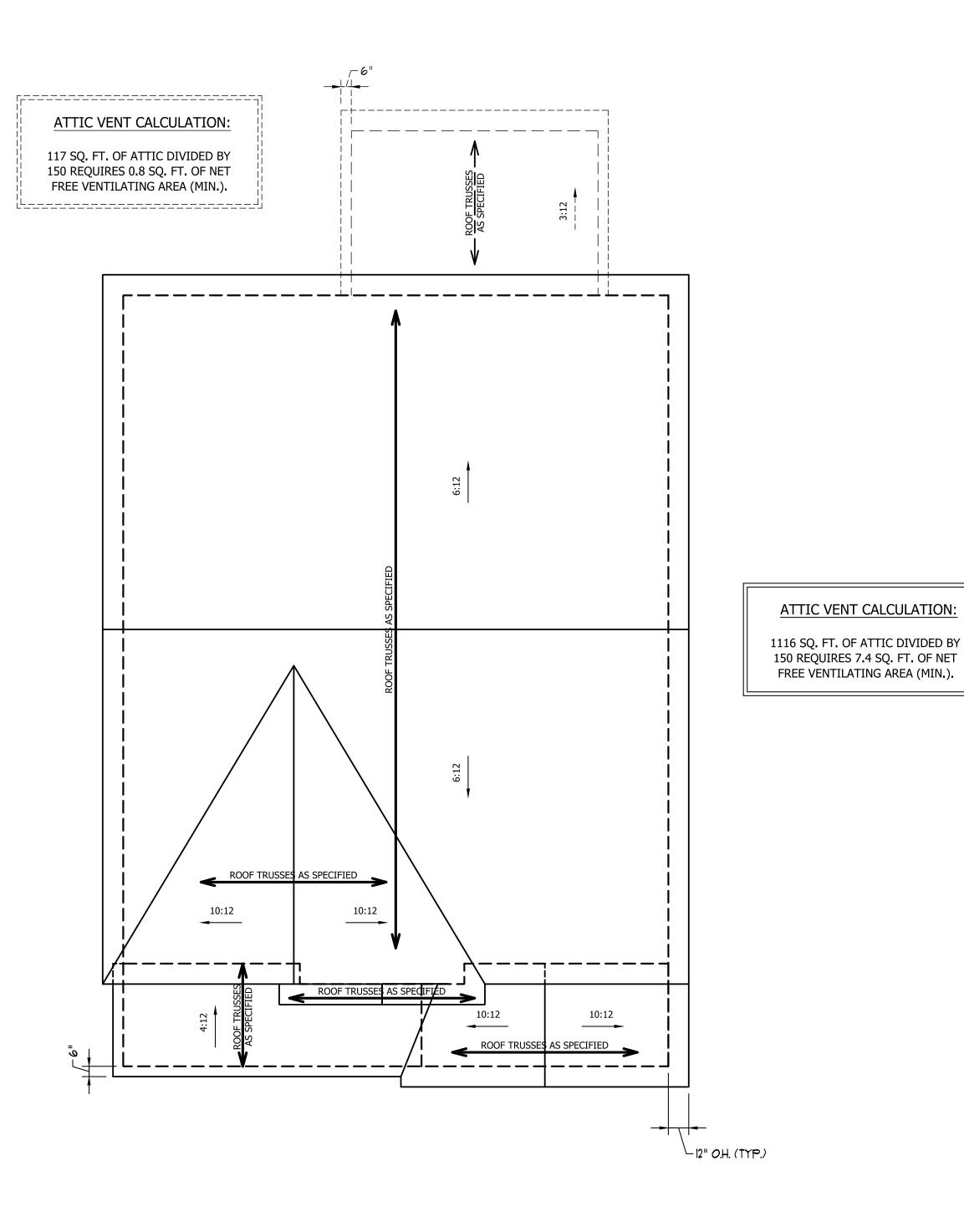
ENGINEERED BY:
REVIEWED BY:

ATTIC FLOOR FRAMING PLAN

S-3

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.

11x17 PRINTS ARE NOT TO SCALE



ATTIC VENT CALCULATION:

1040 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 6.9 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE #2 SPF (UNO).
- 2. HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
- 3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2×8 RIDGES, 2×6 RAFTERS @ 16" O.C. AND FLAT 2×10 VALLEYS OR USE VALLEY TRUSSES.
- 4. FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
- 5. REFER TO SECTION R802.11 OF THE 2018 NCRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.

200

RENAISSAN

RALEIGH, NC 27612 (919) 649-4128 WWW.RRDCAROLINA.COM

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IENSIONS ARE SUBLECT TO CHANGE WITHOUT NOTICE.

JARE FOOTAGE AND DIMENSIONS ARE ESTIMATED AND

VARY IN ACTUAL CONSTRUCTION. ACTUAL POSITION OF

SEG ON LOT WILL BE DETERMINED BY THE SITE PLAND

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ACHIBITED. SEE NEW HOME SALES CONSULTANT FOR

STRING TO STAND.

WEAVER HOMES CAROLINA COLLECTIC HICKORY

DATE: AUGUST 25, 2020

REV.:

SCALE: 1/4" = 1'-0"

DRAWN BY: WG

ENGINEERED BY:

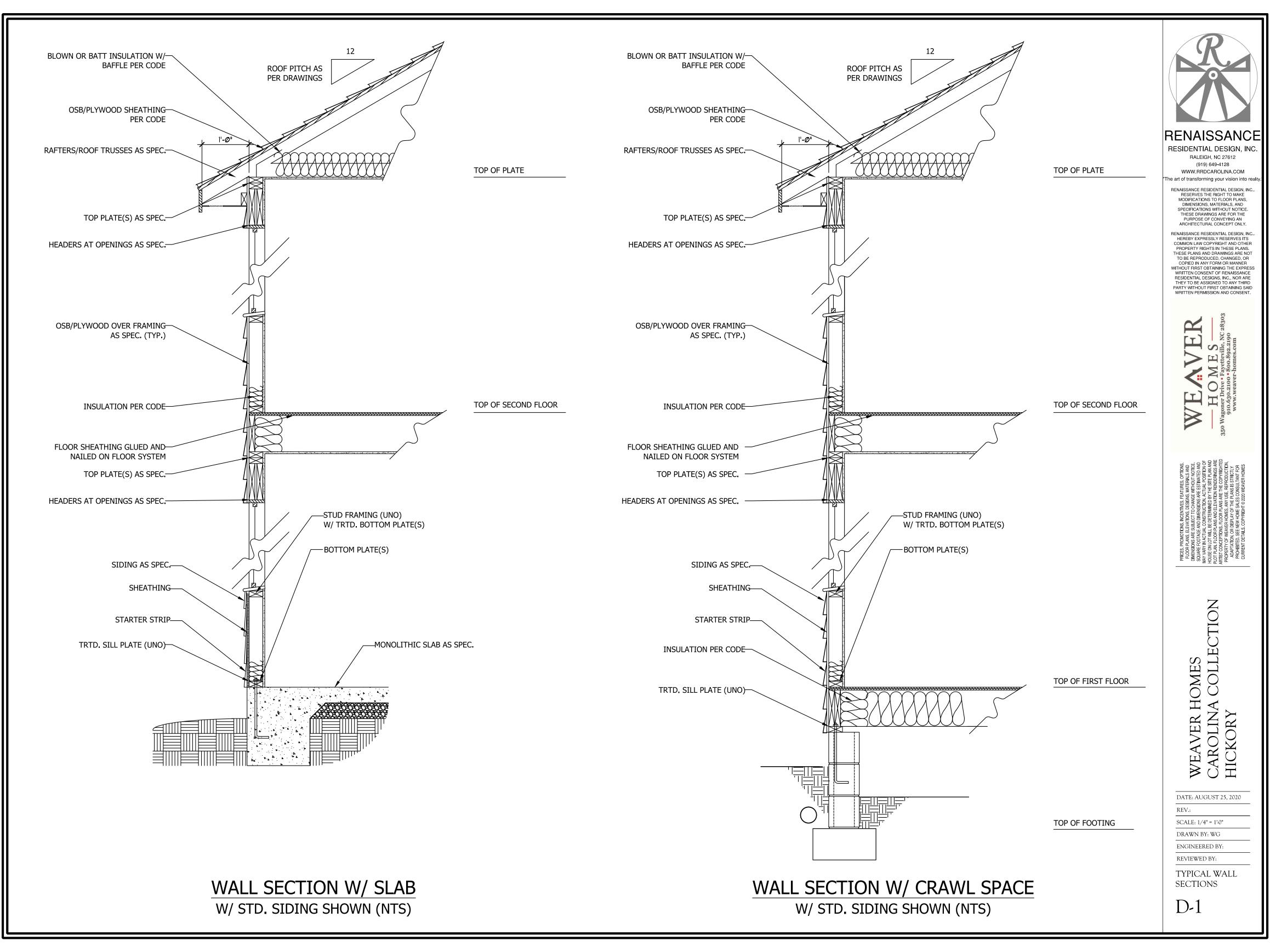
REVIEWED BY:

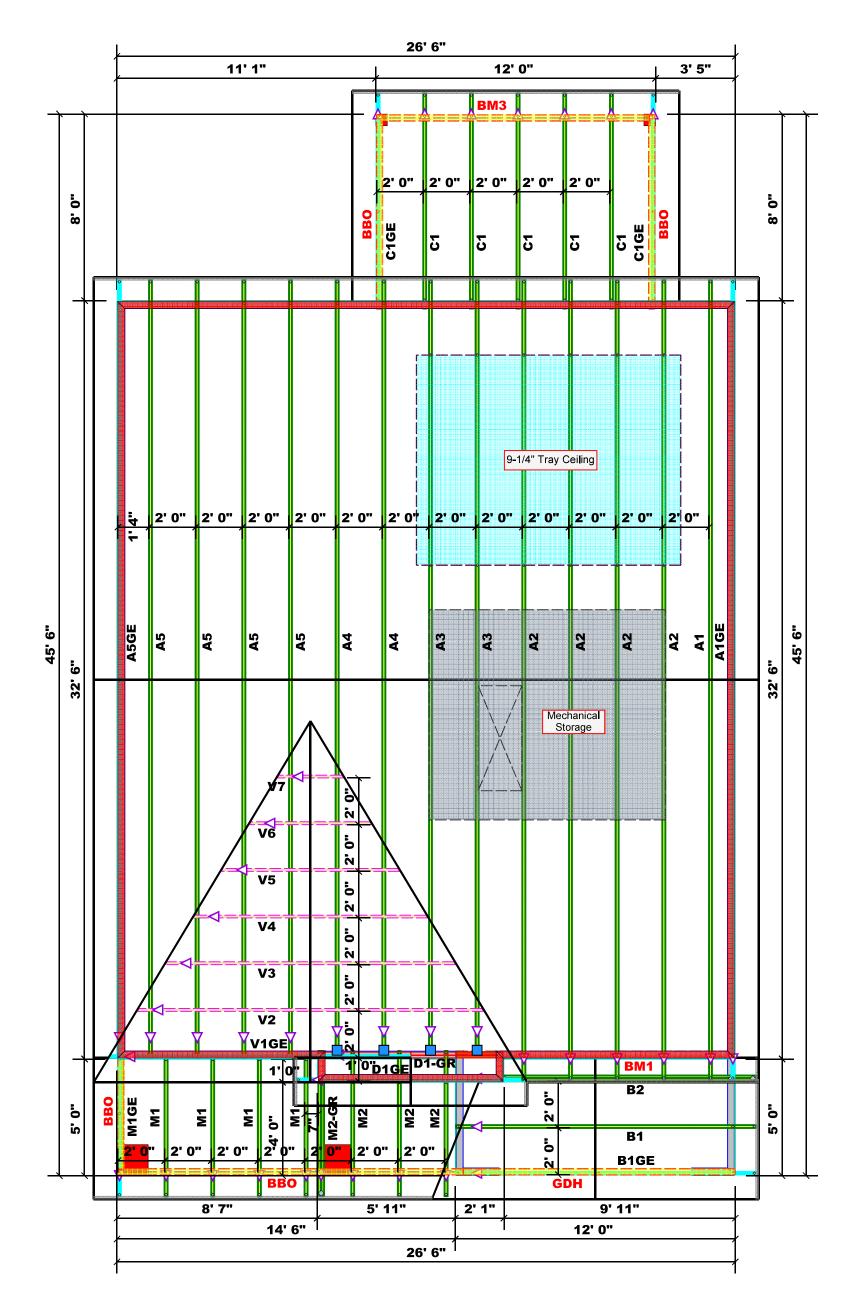
ROOF PLAN

TO SCALE AS NOTED.

11x17 PRINTS ARE NOT TO SCALE

SCALE NOTE: 18x24 PRINTS ARE





Dimension Notes

1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise

2. All interior wall dimensions are to face of frame wall unless noted otherwise

3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Roof Area = 1468.1 sq.ft.
Ridge Line = 52.07 ft.
Hip Line = 0 ft.
Horiz. OH = 98.57 ft.
Raked OH = 159.04 ft.
Decking = 50 sheets

Hatch Legend
Padded HVAC
2nd Floor Walls
Tray Ceiling
Drop Beam

Sym Product Manuf Qty Supported Header Truss HUS26 USP 4 NA 16d/3-1/2" 16d/3-1/2'		Conne	Nail Information				
HUS26 USP 4 NA 16d/3-1/2" 16d/3-1/2'	Sym	Product	Manuf	Qty	Supported Member	Header	Truss
		HUS26	USP	4	NA	16d/3-1/2"	16d/3-1/2"

		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM3	12' 0"	2x10 SPF No.2	2	2
GDH	12' 0"	2x12 SPF No.2	2	2

Truss Placement Plan
Scale: 1/4"=1'



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Cod requirements) to determine the minimum foundatio size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attache Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

nature David Landry

David Landry

LOAD CHART FOR JACK STUDS

(BASED ON TABLES RE025(I) & (b))

NUMBER OF JACK STUDS REQUIRED & EA END OF HEADER/STREER

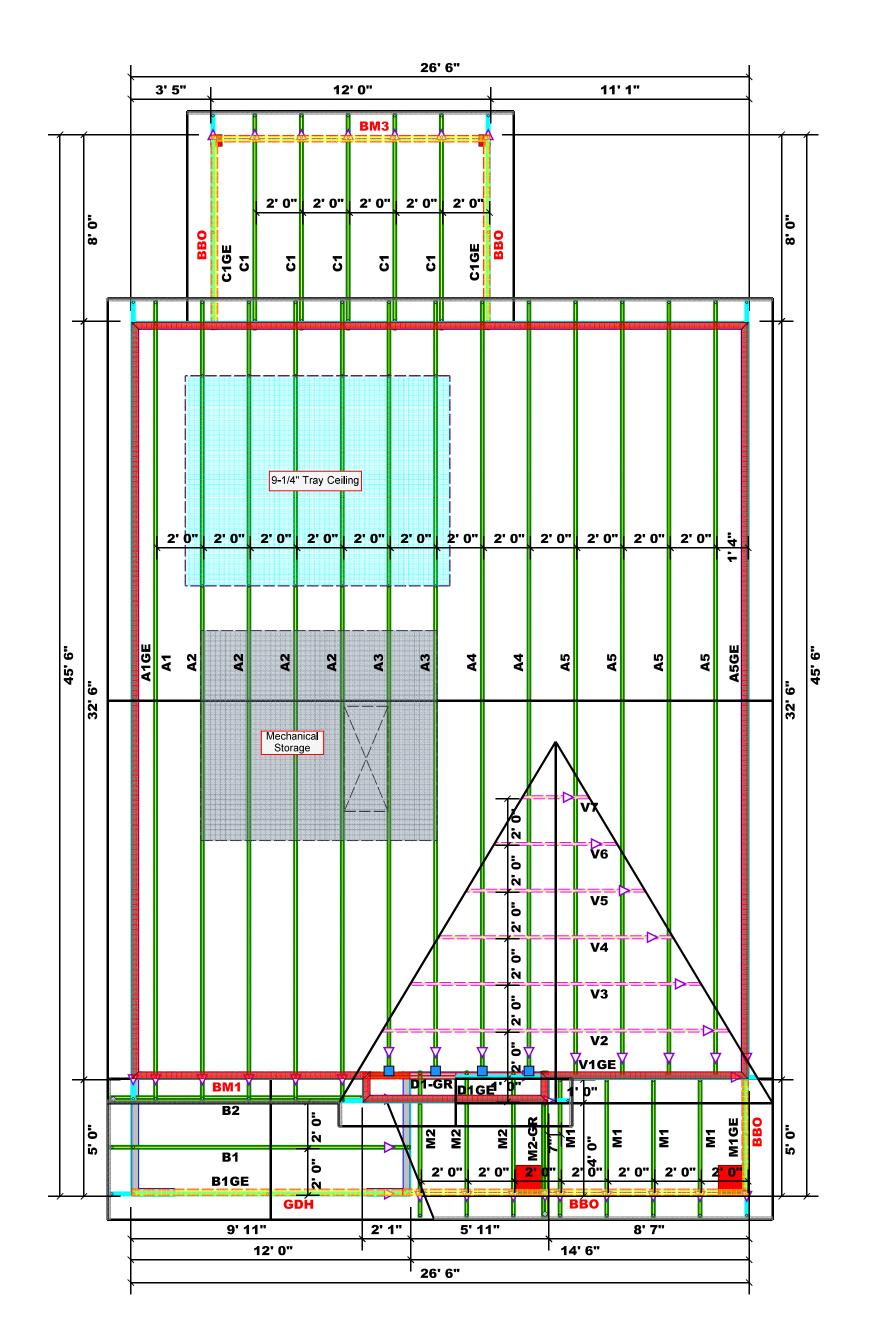
		HE	4DBR/:	SIRDER		
END REACTION (UP TO)	REQ10 STUDS FOR (2) PLY HEADER		FNIS KEACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER	FNN RFACTION (UP TO)	RCQ'D STUDS FOR
1700	1	2	550	1	3400	1
3400	2	5	100	2	6800	a) 3
5100	3	7	650	3	10200) 3
6800	4	10	200	4	13600) 4
8500	5	12	750	5	17000) 5
10200	6	15	5300	6		
11900	7					
13600	8					
15300	9					

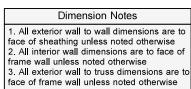
BUILDER	Weaver Development Co. Inc.	CITY / CO.	CITY / CO. Sanford / Harnett	10200 11900 13600 15300
JOB NAME	JOB NAME Lot 3 Barbecue Church Rd.	ADDRESS	4903 Barbecue Church Rd.	6 7 8 9
PLAN	Hickory "C"	MODEL	Roof	15300
SEAL DATE Seal Date	Seal Date	DATE REV.	05/13/21	0 6
QUOTE#	Quote #	DRAWN BY	DRAWN BY David Landry	
JOB #	J0521-2896	SALES REP.	SALES REP. Lenny Norris	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building design at the specification of the building design at the specification of the building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

= Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards





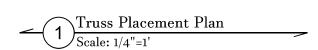
All Walls Shown Are Considered Load Bearing

Roof Area	= 1468.1 sq.ft.
Ridge Line	= 52.07 ft.
Hip Line	= 0 ft.
	= 98.57 ft.
Raked OH	= 159.04 ft.
Decking	= 50 sheets

Hatch Legend				
Padded HVAC				
2nd Floor Walls				
Tray Ceiling				
Drop Beam				

	Conne	Nail Information				
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	4	NA	16d/3-1/2"	16d/3-1/2"

Products						
PlotID	Length	Product	Plies	Net Qty		
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2		
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2		
BM3	12' 0"	2x10 SPF No.2	2	2		
GDH	12' 0"	2x12 SPF No.2	2	2		
GDH	12 0"	2X12 SPF N0.2				





Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Cod requirements) to determine the minimum foundatio size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attache Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

ignature David Landry

David Landry

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R5025(I) & (b))

NUMBER OF JACK STUDS REQUIRED ® EA END OF MEADER/STADER

(But to the control of the control of

1700 1 2550 1 3400 1 3400 2 6800 2 5100 2 5100 3 10200 3 7650 3 13600 4 6800 4 10200 4 8500 5 12750 5 17000 5 10200 6 15300 6 11900 7 13600 8 15300 9

Weaver Development Co. Inc.CITY / CO.Sanford / HarnettSanford / HarnettLot 3 Barbecue Church Rd.ADDRESS4903 Barbecue Church Rd.Hickory "C"MODELRoofSeal DateDATE REV.05/13/21Quote #DRAWN BYDavid LandryJ0521-2896SALES REP.Lenny Norris

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

SEAL

JOB NAME

BUILDER

= Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0521-2896

Lot 3 Barbecue Church Rd.

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15725374 thru E15725398

My license renewal date for the state of North Carolina is December 31, 2021.

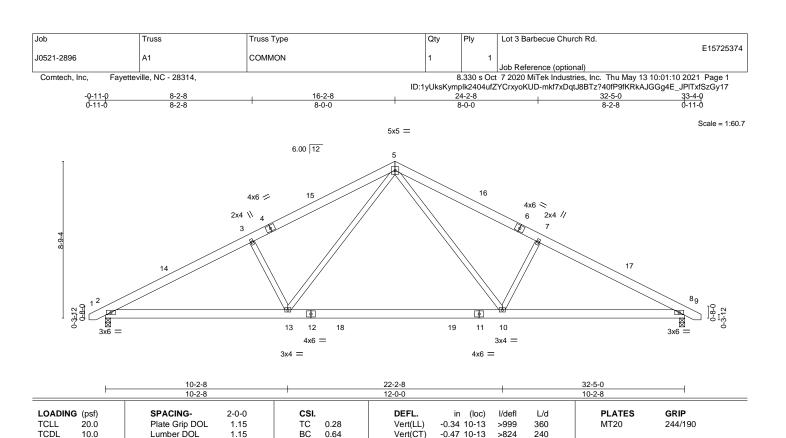
North Carolina COA: C-0844



May 13,2021

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



LUMBER-

BCLL

BCDL

TOP CHORD BOT CHORD 2x6 SP No.1 2x6 SP No.1 2x4 SP No.2 **WEBS**

10.0

0.0

Wind(LL) BRACING-

Horz(CT)

0.05

0.05 2-13

8

n/a

>999

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 4-11-7 oc purlins.

Weight: 208 lb

FT = 20%

Rigid ceiling directly applied or 10-0-0 oc bracing.

n/a

240

REACTIONS. (size) 2=0-3-8, 8=0-3-8

Max Horz 2=-110(LC 10)

Max Uplift 2=-89(LC 12), 8=-89(LC 13) Max Grav 2=1337(LC 1), 8=1337(LC 1)

Rep Stress Incr

Code IRC2015/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2307/486, 3-5=-2125/534, 5-7=-2125/534, 7-8=-2307/486

BOT CHORD 2-13=-316/2007, 10-13=-106/1303, 8-10=-320/1964

WEBS $5\text{-}10\text{=-}147/921,\ 7\text{-}10\text{=-}454/288,\ 5\text{-}13\text{=-}147/921,\ 3\text{-}13\text{=-}454/288}$

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 3) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB

Matrix-S

0.27

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

YES

- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2 and 89 lb uplift at
- 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



May 13,2021



Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
					E15725375
J0521-2896	A1GE	COMMON SUPPORTED GAB	1	1	
					Job Reference (optional)
Comtech, Inc, Fayette	ville, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 10:01:14 2021 Page 1

ID:1yUksKymplk2404ufZYCrxyoKUD-fWvenbtONMiuSdOnuFDbVHuw5unP04fuKNR9oDzGy13 0-11-0

Scale = 1:58.6

5x5 = 6.00 12 12 10 13 4x6 / 4x6 <> 14 15 16 17 18 19 • 3x4 = 33 32 38 37 36 35 30 29 28 27 26 25 24 23

-0-11-0 0-11-0		33-4-0 32-5-0		34-3-0 0-11-0
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. DEFL. TC 0.04 Vert(LL) BC 0.02 Vert(CT) WB 0.16 Horz(CT) Matrix-S Horz(CT)	in (loc) l/defl L/d 0.00 20 n/r 120 0.00 20 n/r 120 0.00 20 n/a n/a	PLATES GRIP MT20 244/190 Weight: 258 lb FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 **BOT CHORD** 2x4 SP No.2 **OTHERS**

BRACING-

TOP CHORD **BOT CHORD**

4x6 =

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 32-5-0.

Max Horz 2=-171(LC 17) (lb) -

Max Uplift All uplift 100 b or less at joint(s) 2, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22 Max Grav All reactions 250 lb or less at joint(s) 2, 30, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23,

4x6 =

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 10-11=-114/284, 11-12=-114/284

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

16-2-8

- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22,
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



May 13,2021



Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
			1 -	•	E15725376
J0521-2896	A2	ROOF SPECIAL	4	1	
	. –				Job Reference (optional)
Comtech. Inc. Favette	ville. NC - 28314.				7 2020 MiTek Industries, Inc. Thu May 13 10:01:17 2021 Page 1

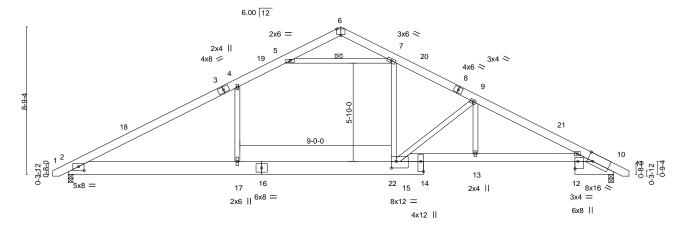
4x6 = Scale: 3/16"=1'

Structural wood sheathing directly applied or 4-0-8 oc purlins.

5-7

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt



	1	10-2-8		1	16-2-8	19-2-8	21-1-8	3 ₁ 2	4-2-8		30-1-8	32-5-0	1
	Г	10-2-8		1	6-0-0	3-0-0	1-11-0) (3-1-0		5-11-0	2-3-8	<u> </u>
Plate Offse	ets (X,Y)	[2:0-4-0,0-2-14], [6:0-3-0	,Edge], [10:0-4	-0,Edge], [1:	2:Edge,0-2-0]	, [14:Edge,0-2-0]	, [15:0-3	-8,0-4-1	[2]				
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLA	TES	GRIP
TCLL	20.Ó	Plate Grip DOL	1.15	TC	0.75	Vert(LL)	-0.21	17	>999	360	MT2	0	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.67	Vert(CT)	-0.38	17	>999	240			
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.70	Horz(CT)	0.09	10	n/a	n/a			
BCDL	10.0	Code IRC2015/Ti	PI2014	Matri	x-S	Wind(LL)	0.17	2-17	>999	240	Weig	ht: 247 lb	FT = 20%

BRACING-

WFBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1

BOT CHORD 2x10 SP No.1 *Except*

10-15: 2x6 SP 2400F 2.0E

WEBS 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 10=0-3-8

Max Horz 2=110(LC 11)

Max Uplift 2=-90(LC 12), 10=-90(LC 13) Max Grav 2=1393(LC 2), 10=1353(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-2217/403, 4-5=-1870/483, 7-9=-2258/519, 9-10=-2889/551 BOT CHORD 2-17=-193/1848, 15-17=-195/1860, 13-15=-371/2525, 10-13=-380/2525 WEBS 4-17=-29/402, 7-15=-114/967, 9-15=-1075/232, 9-13=0/61-6, 5-7=-1955/459

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.
- 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

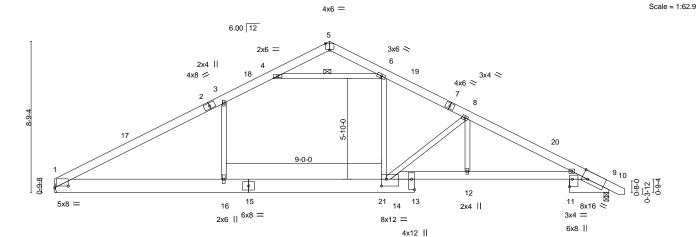


May 13,2021









	1	9-11-8		1	18-11-8	1	20-10-8	23-1	1-8 ₁	29-10-8	₁ 32-2-0 ₁	
		9-11-8		1	9-0-0		1-11-0	3-1	-0	5-11-0	2-3-8	
Plate Off	sets (X,Y)	[1:0-9-6,0-1-2], [5:0-3-0,E	Edge], [9:0-4-0	Edge], [11:E	dge,0-2-0], [13:Edge,0-2-0], [1	4:0-3-8,	0-4-12]				
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.76	Vert(LL)	-0.21	16	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.66	Vert(CT)	-0.36	16	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.70	Horz(CT)	0.09	9	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-S	Wind(LL)	0.16	1-16	>999	240	Weight: 243 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WFBS

LUMBER-

TOP CHORD 2x6 SP No.1

BOT CHORD 2x10 SP No.1 *Except* 9-14: 2x6 SP 2400F 2.0E

WEBS 2x4 SP No.2

REACTIONS. (size) 1=Mechanical, 9=0-3-8

Max Horz 1=-111(LC 8)

Max Uplift 1=-76(LC 12), 9=-90(LC 13) Max Grav 1=1345(LC 2), 9=1347(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD
1-3=-2189/401, 3-4=-1853/491, 6-8=-2237/517, 8-9=-2874/549

BOT CHORD
1-16=-198/1827, 14-16=-200/1839, 12-14=-375/2511, 9-12=-384/2511

WEBS
6-14=-1117/966, 3-16=-53/392, 4-6=-1931/474, 8-14=-1081/228, 8-12=0/620

NOTES-

- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.



Structural wood sheathing directly applied or 3-10-0 oc purlins.

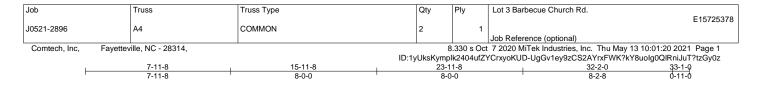
4-6

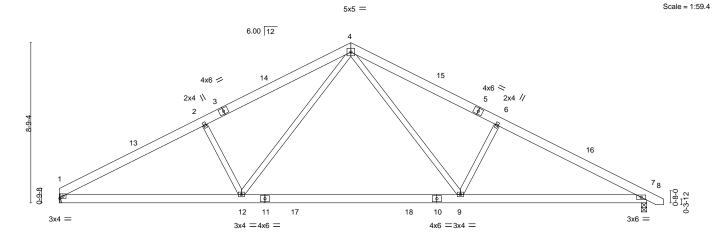
Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midnt

May 13,2021







	9-11-8 9-11-8	-	21-11-8 12-0-0		+	32-2-0 10-2-8	——
Plate Offsets (X,Y) [1:0-1-14	,0-1-8]						
TCLL 20.0 Pla TCDL 10.0 Lu BCLL 0.0 * Re	PACING- 2-0-0 ate Grip DOL 1.15 umber DOL 1.15 ep Stress Incr YES ode IRC2015/TPI2014	CSI. TC 0.28 BC 0.64 WB 0.27 Matrix-S	DEFL. in Vert(LL) -0.34 Vert(CT) -0.47 Horz(CT) 0.05 Wind(LL) 0.05	9-12	l/defl L/d >999 360 >822 240 n/a n/a >999 240	PLATES MT20 Weight: 204 lb	GRIP 244/190 FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

2x4 SP No 2 WFBS

REACTIONS. 1=Mechanical, 7=0-3-8 (size)

Max Horz 1=-111(LC 8) Max Uplift 1=-76(LC 12), 7=-89(LC 13) Max Grav 1=1278(LC 1), 7=1331(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2276/496, 2-4=-2096/546, 4-6=-2113/532, 6-7=-2294/484

BOT CHORD 1-12=-319/1973, 9-12=-109/1291, 7-9=-324/1953

WEBS 4-9=-147/922, 6-9=-454/288, 4-12=-144/897, 2-12=-437/286

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-11-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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lob	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Chu	urch Rd.	E.==
10521-2896	A5	COMMON	4	1			E15725379
10321-2696	AS	COMMON	4	'	Job Reference (option	onal)	
Comtech, Inc, Fa	yetteville, NC - 28314,		<u> </u>		ct 7 2020 MiTek Indus	stries, Inc. Thu May 13	10:01:24 2021 Page 1
	8-2-8	16-2-8		lk2404ufZY 24-2-8	CrxyoKUD-MRWQt0?f	0RyUf99iULPxuOJajw2 32-5-0	2wMZNNdwsh8ezGy0v
-	8-2-8	8-0-0		8-0-0		8-2-8	33-4-0 0-11-0
			5x5 =				Scale = 1:59
		6.00 12	4				
		4×6 - 14		15			
	0	4,0			4x6 ≈ 5 2x4 //		
		4 \\ 3			5 2x4 // 6		
8-9-4					19		
	13					16	
1 /	//						78
] -8 ×		A		<u> </u>	¥		0-3-12
Ճ		12 11 17	18		9		1918
3x6 =		3x4 =4x6 =		4x6 =:			3x6 =
	10-2-8	1	22-2-8		1	32-5-0	
	10-2-8	<u> </u>	12-0-0			10-2-8	
OADING (psf)	SPACING-	2-0-0 CSI .	DEFL.	in (loc)	I/defl L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.15 TC 0.29	Vert(LL) -0.	34 9-12	>999 360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15 BC 0.65	Vert(CT) -0.	47 9-12	>822 240		

LUMBER-

BCLL

BCDL

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 **WEBS**

0.0 *

Wind(LL) BRACING-

Horz(CT)

0.05

0.05

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-10-13 oc purlins.

Weight: 206 lb

FT = 20%

Rigid ceiling directly applied or 10-0-0 oc bracing.

n/a

240

n/a

>999

REACTIONS.

(size) 1=0-3-8, 7=0-3-8 Max Horz 1=-111(LC 10) Max Uplift 1=-77(LC 12), 7=-89(LC 13) Max Grav 1=1284(LC 1), 7=1337(LC 1)

Rep Stress Incr

Code IRC2015/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD BOT CHORD 1-2=-2310/503, 2-4=-2129/551, 4-6=-2126/535, 6-7=-2308/487

1-12=-327/2012, 9-12=-111/1304, 7-9=-326/1966

WEBS $4-9 = -147/921, \, 6-9 = -454/288, \, 4-12 = -149/924, \, 2-12 = -458/292$

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 1) Unidad ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB

0.27

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

YES

- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



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Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
					E15725380
J0521-2896	A5GE	COMMON SUPPORTED GAB	1	1	
					Job Reference (optional)
Comtech, Inc, Fayettev	rille, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 10:01:26 2021 Page 1

ID:1yUksKymplk2404ufZYCrxyoKUD-JpdAlh1wY2DBuTJ5bmRP_pOz3jtCqVef5ELnDXzGy0t

Scale = 1:57.6

32-5-0 16-2-8 16-2-8

5x5 = 6.00 12 10 11 9 12 8 4x6 / 13 4x6 ≥ 14 15 5 16 17 18 0-3-12 0-3-12 0-3-15 0-8-0 3x4 = 3x4 = 37 36 35 34 32 31 30 29 28 27 26 25 24 23 22 21 4x6 =4x6 =

			32-5-0	ı ı
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.04	DEFL. in (loc) I/defl L/d Vert(LL) 0.00 19 n/r 120	PLATES GRIP MT20 244/190
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	BC 0.02 WB 0.16 Matrix-S	Vert(CT) 0.00 19 n/r 120 Horz(CT) 0.00 19 n/a n/a	Weight: 256 lb FT = 20%

32-5-0

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 **BOT CHORD** 2x4 SP No.2 **OTHERS**

BRACING-

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 32-5-0

Max Horz 1=-175(LC 17) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except

37=-101(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 1, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22, 21, 19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 9-10=-114/284, 10-11=-114/284

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=101.



May 13,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Compor Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



lob	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.	
0521-2896	B1	COMMON	1	1		E1572538 ⁻
Comtech, Inc,	Fayetteville, NC - 28314,	5-11-8 5-11-8	ID:1yUksKyr		Job Reference (optional) tt 7 2020 MiTek Industries, Inc. Thu May 13 !YCrxyoKUD-FClxjN2A4gTv8mSTjBTt3ETH2 12-10-0 0-11-0	
			5x5 =			Scale = 1:36
			3			
		10.00 12 9		10	4x4 ◇	
	6-0-15	4x4 1/2		4		
	1	8		X	5]	
	3x6		7 2x4		3×6	
		5-11-8 5-11-8	2X4	11-11-0 5-11-8		

1-7

1-7

-0.01

-0.02

0.00

0.01

Vert(LL)

Vert(CT)

Horz(CT)

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

>999

>999

>999

n/a

360

240

n/a

240

LUMBER-

TCLL

TCDL

BCLL

BCDL

TOP CHORD BOT CHORD 2x6 SP No.1 2x6 SP No.1 2x4 SP No.2 WFBS

20.0

10.0

0.0

SLIDER Left 2x4 SP No.2 - 3-9-13, Right 2x4 SP No.2 -x 3-9-13

Plate Grip DOL

Rep Stress Incr

Code IRC2015/TPI2014

Lumber DOL

REACTIONS. (size) 1=0-3-8, 5=0-3-8

Max Horz 1=-134(LC 10) Max Uplift 1=-18(LC 12), 5=-29(LC 13) Max Grav 1=475(LC 1), 5=524(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-3=-491/156, 3-5=-517/155 TOP CHORD BOT CHORD 1-7=0/307, 5-7=0/307

WEBS 3-7=0/277

NOTES-

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 5-11-8, Exterior(2) 5-11-8 to 10-4-5, Interior(1) 10-4-5 to 12-8-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

TC

ВС

WB

0.14

0.12

0.06

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.15

1.15

YES

- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.



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244/190

FT = 20%

MT20

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 87 lb





Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
				'	E15725382
J0521-2896	B1GE	COMMON SUPPORTED GAB	1	1	
					Job Reference (optional)
Comtech, Inc, Fayettevil	lle, NC - 28314,			8.330 s Oct	t 7 2020 MiTek Industries, Inc. Thu May 13 10:01:30 2021 Page 1
			ID:1yUksKymp	lk2404ufZY	CrxyoKUD-Bbth834QcHjdN4csqcVL8fZfHKF4mLMF?sJ?MlzGy0p
		5-11-8		11-11-0	12-10-0
		5-11-8		5-11-8	0-11-0

Scale = 1:36.3 5x5 = 5 10.00 12 4x4 / 4x4 🚿 8 3x6 II 3x6 II 12 15 14 13 11 11-11-0 SPACING-2-0-0 CSI. DEFL L/d **PLATES GRIP** in (loc) I/defl Plate Grip DOL TC Vert(LL) 244/190 1.15 0.03 -0.00 120 MT20 9 n/r ВС Lumber DOL 1.15 0.02 Vert(CT) 0.00 9 n/r 120

0.00

9

n/a

n/a

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TCLL

TCDL

BCLL

BCDL

LOADING (psf)

2x6 SP No.1 2x6 SP No.1 TOP CHORD BOT CHORD 2x4 SP No.2 **OTHERS**

20.0

10.0

0.0

SLIDER Left 2x4 SP No.2 -x 1-6-11, Right 2x4 SP No.2 -x 1-6-11

Rep Stress Incr

Code IRC2015/TPI2014

REACTIONS. All bearings 11-11-0.

(lb) - Max Horz 1=-167(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 14, 12 except 15=-181(LC 12), 11=-169(LC 13)

WB

0.05

Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 14, 15, 12, 11

YES

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 14, 12 except (it=lb) 15=181, 11=169,
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 9.



Weight: 96 lb

FT = 20%

Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
J0521-2896	B2	ROOF SPECIAL	1		E15725383
				2	Job Reference (optional)

Comtech, Inc. Fayetteville, NC - 28314, 8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:31 2021 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-fnR3LP42NarU?EB2OK1aht5ookY?VmUOEW3YukzGy0o

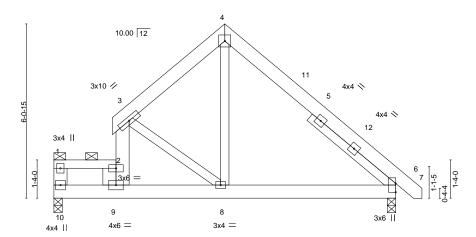
5-11-8 3-11-0

Scale = 1:37.8 5x5 =

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 1-2.

Rigid ceiling directly applied or 10-0-0 oc bracing.



	2-0-8	3-11-0	3-11-6	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.17	Vert(LL) -0.01 8-9 >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.23	Vert(CT) -0.04 8-9 >999 240	
BCLL 0.0 *	Rep Stress Incr NO	WB 0.18	Horz(CT) 0.00 6 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.01 8-9 >999 240	Weight: 184 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 *Except*

1-2: 2x4 SP No.1

BOT CHORD 2x6 SP No.1 WFBS 2x6 SP No 1 *Except*

4-8.3-8: 2x4 SP No.2 SLIDER Right 2x4 SP No.2 -x 3-9-13

REACTIONS. (size) 10=0-3-8, 6=0-3-8

Max Horz 10=-131(LC 8)

Max Grav 10=859(LC 1), 6=555(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-10=-363/0, 1-2=-630/0, 3-4=-478/103, 4-6=-578/69 TOP CHORD

BOT CHORD 9-10=0/631, 8-9=0/325, 6-8=0/334 **WEBS** 4-8=0/359, 2-9=-497/2, 2-3=-404/132

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-2-12 to 2-4-12, Interior(1) 2-4-12 to 5-11-8, Exterior(2) 5-11-8 to 10-4-5, Interior(1) 10-4-5 to 12-8-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Provide adequate drainage to prevent water ponding.6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-260, 3-4=-60, 4-7=-60, 6-10=-20



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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Compon Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
J0521-2896	B2	ROOF SPECIAL	1	2	E15725383
					Job Reference (optional)

Comtech, Inc. Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:32 2021 Page 2 $ID:1yUksKymplk2404ufZYCrxyoKUD-7z_SZl5g8uzLcOmFy1YpD4ezY8tEEDkYTAo6QAzGy0n$

LOAD CASE(S) Standard

2) Dead + 0.75 Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-250, 3-4=-50, 4-7=-50, 6-10=-20

3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-2=-220, 3-4=-20, 4-7=-20, 6-10=-40

4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-156, 3-4=27, 4-12=35, 6-12=27, 6-7=20, 6-10=-12

Horz: 3-4=-39, 4-12=47, 6-12=39, 6-7=32

5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-170, 3-4=35, 4-11=27, 6-11=35, 6-7=58, 6-10=-12

Horz: 3-4=-47, 4-11=39, 6-11=47, 6-7=70

6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-235, 3-4=-58, 4-6=-58, 6-7=-51, 6-10=-20

Horz: 3-4=38, 4-6=-38, 6-7=-31

7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-235, 3-4=-58, 4-6=-58, 6-7=11, 6-10=-20

Horz: 3-4=38, 4-6=-38, 6-7=31

8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-179, 3-4=-13, 4-6=11, 6-7=4, 6-10=-12

Horz: 3-4=1, 4-6=23, 6-7=16

9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-191, 3-4=11, 4-6=-13, 6-7=2, 6-10=-12

Horz: 3-4=-23, 4-6=-1, 6-7=14

10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-201, 3-4=-35, 4-6=-11, 6-7=-4, 6-10=-20

Horz: 3-4=15, 4-6=9, 6-7=16

11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-213, 3-4=-11, 4-6=-35, 6-7=-28, 6-10=-20

Horz: 3-4=-9, 4-6=-15, 6-7=-8

12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-179, 3-4=21, 4-6=9, 6-7=2, 6-10=-12

Horz: 3-4=-33, 4-6=21, 6-7=14

13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-191, 3-4=9, 4-6=21, 6-7=14, 6-10=-12

Horz: 3-4=-21, 4-6=33, 6-7=26

14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-179, 3-4=21, 4-6=9, 6-7=2, 6-10=-12

Horz: 3-4=-33, 4-6=21, 6-7=14

15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-191, 3-4=9, 4-6=21, 6-7=14, 6-10=-12

Horz: 3-4=-21, 4-6=33, 6-7=26

16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-201, 3-4=-1, 4-6=-13, 6-7=-6, 6-10=-20

Horz: 3-4=-19, 4-6=7, 6-7=14

17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-213, 3-4=-13, 4-6=-1, 6-7=6, 6-10=-20

Horz: 3-4=-7, 4-6=19, 6-7=26

18) Dead: Lumber Increase=0.90. Plate Increase=0.90 Plt. metal=0.90

Uniform Loads (plf)

Vert: 1-2=-220, 3-4=-20, 4-7=-20, 6-10=-20

19) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-236, 3-4=-61, 4-6=-43, 6-7=-38, 6-10=-20 Horz: 3-4=11, 4-6=7, 6-7=12

20) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-245, 3-4=-43, 4-6=-61, 6-7=-56, 6-10=-20

Horz: 3-4=-7, 4-6=-11, 6-7=-6

21) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60

meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE



818 Soundside Road

Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
J0521-2896	B2	ROOF SPECIAL	1	_	E15725383
30321-2090	DZ	ROOF SPECIAL	'	2	Job Reference (optional)

Comtech, Inc. Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:32 2021 Page 3 $ID:1yUksKymplk2404ufZYCrxyoKUD-7z_SZI5g8uzLcOmFy1YpD4ezY8tEEDkYTAo6QAzGy0n$

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-2=-236, 3-4=-36, 4-6=-45, 6-7=-40, 6-10=-20

Horz: 3-4=-14, 4-6=5, 6-7=10

22) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-245, 3-4=-45, 4-6=-36, 6-7=-31, 6-10=-20 Horz: 3-4=-5, 4-6=14, 6-7=19

23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-260, 3-4=-60, 4-7=-20, 6-10=-20

24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-220, 3-4=-20, 4-7=-60, 6-10=-20

25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-250, 3-4=-50, 4-7=-20, 6-10=-20

26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-220, 3-4=-20, 4-7=-50, 6-10=-20

Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
					E15725384
J0521-2896	C1	Monopitch	5	1	
					Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,			3.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 10:01:33 2021 Page 1
		ID:1	UksKympll	2404ufZY0	CrxyoKUD-bAYqm56JvC5CEXLRVk32mlBzkYDJziqhiqYfydzGy0m
-0-11-	0 1	8-0-)		
0-11-0) '	8-0-)		

Scale = 1:17.0 3x4 || 3 3.00 12 6 4x4 = 0-6-1 3x6 = 6x6 =

			8-3-8									
Plate Offsets	(X,Y) [2:	:0-2-14,0-0-6], [6:0-2-0,0	0-1-8]									
LOADING (p	osf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20	o.ó	Plate Grip DOL	1.15	TC	0.84	Vert(LL)	-0.05	`2-Ś	>999	360	MT20	244/190
TCDL 10	0.0	Lumber DOL	1.15	BC	0.24	Vert(CT)	-0.10	2-5	>969	240		
BCLL (0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	5	n/a	n/a		
BCDL 10	0.0	Code IRC2015/TP	12014	Matrix	c-P	Wind(LL)	0.10	2-5	>886	240	Weight: 37 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD BOT CHORD 2x4 SP No.1 2x6 SP No.1 2x4 SP No.2

WFBS 2x6 SP No.1 **OTHERS**

REACTIONS. (size) 2=0-3-0, 5=0-3-8 Max Horz 2=74(LC 8)

Max Uplift 2=-150(LC 8), 5=-127(LC 8) Max Grav 2=375(LC 1), 5=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 8-0-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 5=127.



Structural wood sheathing directly applied or 5-3-4 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

May 13,2021



Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.			
						E15725385		
J0521-2896	C1GE	GABLE	2	1				
					Job Reference (optional)			
Comtech, Inc,	Fayetteville, NC - 28314,			8.330 s Oc	t 7 2020 MiTek Industries, Inc. Thu May 13 10:0	01:34 2021 Page 1		
		ID:1vUksKvmplk2404ufZYCrxvoKUD-4M6C_Q7xqVD3shwd3SaHIViGDvYKi9srwUHDV3zGv0l						

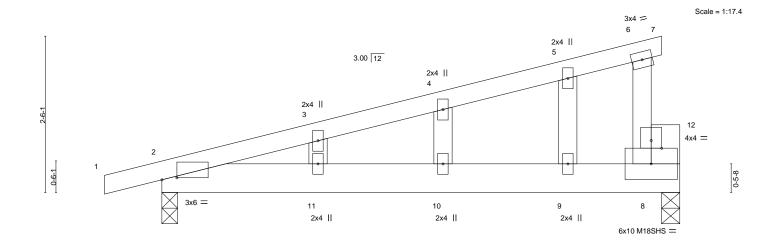


Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [12:0-2-0,0-1-8]		8-3-8	<u> </u>		
LOADING (psf)		CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP		
(1 - /			()			
TCLL 20.0	Plate Grip DOL 1.15	TC 0.35	Vert(LL) 0.09 10-11 >999 240	MT20 244/190		
TCDL 10.0	Lumber DOL 1.15	BC 0.26	Vert(CT) -0.08 10-11 >999 240	M18SHS 244/190		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.01	Horz(CT) -0.00 8 n/a n/a			
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 41 lb FT = 20%		

8-3-8

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2

OTHERS 2x4 SP No.2 *Except* 8-12: 2x6 SP No.1

-0-11-0 0-11-0

REACTIONS. (size) 2=0-3-0, 8=0-3-8

Max Horz 2=105(LC 8)

Max Uplift 2=-216(LC 8), 8=-188(LC 8) Max Grav 2=375(LC 1), 8=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

BOT CHORD 2-11=-284/207, 10-11=-284/207, 9-10=-284/207, 8-9=-284/207

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216, 8=188.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

May 13,2021



Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
					E15725386
J0521-2896	D1-GR	Common Girder	1	2	
					Job Reference (optional)
Comtech Inc Favettevi	III. NC - 2831/	•		330 c Oct	7 2020 MiTek Industries Inc. Thu May 13 10:01:38 2021 Page 1

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Scale = 1:27.4 4x4

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

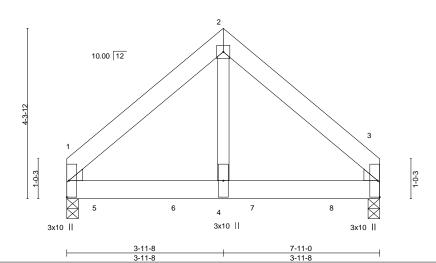


Plate Offsets (X,Y)-- [1:0-0-4,0-0-5], [1:0-0-8,0-3-6], [3:0-0-4,0-0-5], [3:0-0-8,0-3-6]

			. , ,, ,		
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.38	Vert(LL) -0.02 3-4 >999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.57	Vert(CT) -0.04 3-4 >999 240	
BCLL	0.0 *	Rep Stress Incr NO	WB 0.39	Horz(CT) 0.01 3 n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.01 3-4 >999 240	Weight: 100 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WFBS 2x4 SP No 2

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 3=0-3-8

Max Horz 1=91(LC 26)

Max Uplift 1=-191(LC 8), 3=-180(LC 9) Max Grav 1=2919(LC 1), 3=2779(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2418/177, 2-3=-2418/177 BOT CHORD

1-4=-100/1678, 3-4=-100/1678

2-4=-154/3142 WEBS

NOTES-

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=191, 3=180.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1261 lb down and 93 lb up at 0-9-12, 1258 lb down and 96 lb up at 2-9-12, and 1325 lb down and 96 lb up at 4-9-12, and 1325 lb down and 96 lb up at 6-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 1-3=-20



May 13,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/ITPI Quality Criteria, DSB-89 and BCSI Building Compor Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
10504 2000	D4 CB	Common Girdon			E15725386
J0521-2896	D1-GR	Common Girder	1	2	Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:38 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-y7MjpoARjkjUKJDOIIfDTLuyiZrSes3Qr6FQeqzGy0h

LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 5=-1261(B) 6=-1258(B) 7=-1258(B) 8=-1258(B)



Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.			
					E15725387			
J0521-2896	D1GE	COMMON SUPPORTED GAB	1	1				
					Job Reference (optional)			
Comtech, Inc, Fayettev	ille, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Thu May 13 10:01:36 2021 Page 1			
		ID:1vUksKymplk2404ufZYCrxvoKUD-0IEvO68BC7Tn5?30BtclOwphplleA248OomJZxzGv0i						

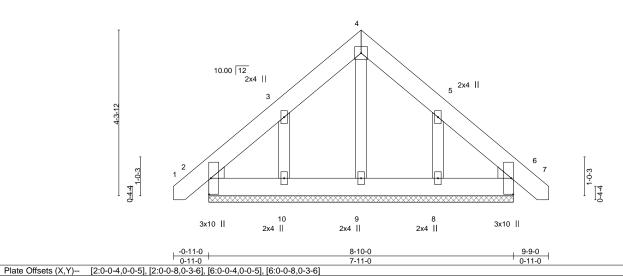
4-10-8

Scale = 1:28.2 4x4 =

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

8-10-0



1 1010 011	0010 (71,17	[2.0 0 1,0 0 0]; [2.0 0 0,0	0 0], [0.0 0 1,	0 0 0], [0.0	0 0,0 0 0]							
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.02	Vert(LL)	0.00	6	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.01	Vert(CT)	0.00	6	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	6	n/a	n/a		
BCDL	10.0	Code IRC2015/TP	I2014	Matri	x-P						Weight: 60 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2

OTHERS

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. All bearings 7-11-0.

(lb) - Max Horz 2=-118(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-152(LC 12), 8=-148(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

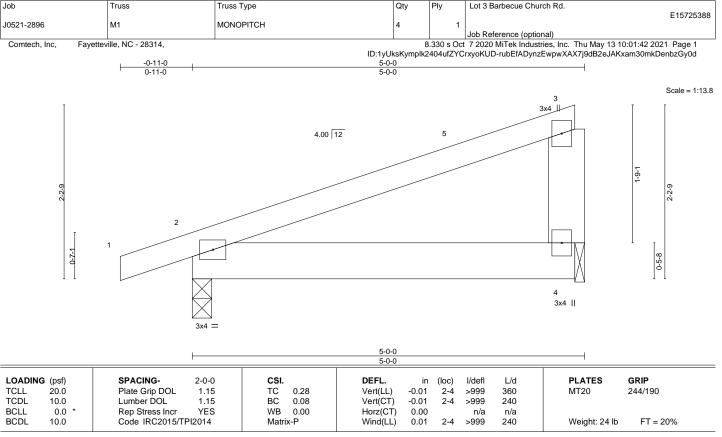
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=152, 8=148,
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



May 13,2021





LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x6 SP No.1 BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 2=0-3-0, 4=0-1-8

Max Horz 2=63(LC 8)

Max Uplift 2=-102(LC 8), 4=-79(LC 8) Max Grav 2=255(LC 1), 4=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=102.



May 13,2021



Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Chur	ch Rd.	
J0521-2896	M1GE	GABLE	1	1			E15725389
30321-2090	WIGE	GABLE	'	'		nal)	
Comtech, Inc. Fa	yetteville, NC - 28314, -0-11-0 0-11-0		5-0-0 5-0-0		yoKUD-FTHMHBFq4uc	ries, Inc. Thu May 13 VgNGkCGHsFqgCTI	10:01:45 2021 Page 1 NLTn7aSSiSIOwzGy0a Scale = 1:13.8
LOADING (psf) TCLL 20.0	SPACING- 2 Plate Grip DOL	-0-0 CSI. 1.15 TC 0.09	5-0-0 5-0-0 DEFL. Vert(LL) 0.0		l/defl L/d >999 240	PLATES MT20	GRIP 244/190
TCLL 20.0 TCDL 10.0 BCLL 0.0 *	Lumber DOL	1.15 TC 0.09 1.15 BC 0.09 YES WB 0.02	Vert(LL) 0.0 Vert(CT) -0.0 Horz(CT) -0.0	1 8	>999 240 >999 240 n/a n/a	MT20	244/190

LUMBER-

BCDL

TOP CHORD BOT CHORD 2x4 SP No.1 2x6 SP No.1 2x6 SP No.1 **WEBS** 2x4 SP No.2 OTHERS

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 6=0-1-8

Max Horz 2=90(LC 8)

Max Uplift 2=-147(LC 8), 6=-115(LC 8) Max Grav 2=255(LC 1), 6=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code IRC2015/TPI2014

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

Matrix-S

- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=147, 6=115.



Weight: 27 lb

FT = 20%

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design, Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Compon Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty Ply Lot 3 Barbecue Church Rd. F15725390 J0521-2896 M2 Half Hip Job Reference (optional) 8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:47 2021 Page 1 Comtech, Inc. Fayetteville, NC - 28314, ID:1yUksKymplk2404ufZYCrxyoKUD-BsO6itH5cVsDvhP7KhJKKFmWdB?hF1llw?xPSpzGy0Y Scale = 1:12.8 3x4 || 4.00 12 -10-1510 4x6 = 4x4 || 3x4 =5-3-8 1-4-0 LOADING (psf) SPACING-CSI. DEFL **PLATES** GRIP 2-0-0 in (loc) I/defl L/d Plate Grip DOL Vert(LL) **TCLL** 20.0 1.15 TC 0.24 -0.00 >999 360 MT20 244/190 8 TCDL 10.0 Lumber DOL 1.15 вс 0.24 Vert(CT) -0.01 8 >999 240 WB **BCLL** 0.0 Rep Stress Incr NO 0.00 Horz(CT) -0.00 n/a n/a **BCDL** Code IRC2015/TPI2014 Wind(LL) 8 >999 240 Weight: 28 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1 2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 WFBS

REACTIONS. (size) 7=0-3-8, 2=0-3-0

Max Horz 2=69(LC 12)

Max Uplift 7=-173(LC 8), 2=-138(LC 8) Max Grav 7=561(LC 19), 2=349(LC 1)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-425/505, 5-8=-279/265, 5-6=-233/338, 6-7=-292/309

BOT CHORD 2-8=-546/359. 7-8=-338/233

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-7-4, Interior(1) 3-7-4 to 5-0-12 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=173, 2=138,
- 7) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s). The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 5-9=-40, 6-9=-80, 2-7=-20

Concentrated Loads (lb)

Vert: 9=-400

2) Dead + 0.75 Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-50, 3-4=-50, 5-9=-100, 6-9=-130, 2-7=-20



Structural wood sheathing directly applied or 5-3-8 oc purlins,

except end verticals, and 2-0-0 oc purlins: 3-8, 5-6. Except:

Rigid ceiling directly applied or 10-0-0 oc bracing.

10-0-0 oc bracing: 3-5

May 13,2021

meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <a href="https://www.normanent.org/nor



Job Truss	s Tr	russ Type	Qty	Ply	Lot 3 Barbecue Church Rd.
	l.,				E15725390
J0521-2896 M2	Hi	Half Hip	3	1	Job Reference (optional)

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:47 2021 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-BsO6itH5cVsDvhP7KhJKKFmWdB?hF1llw?xPSpzGy0Y

Comtech, Inc. Fayetteville, NC - 28314, LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 9=-350 3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-6=-40, 2-7=-40 Concentrated Loads (lb) Vert: 9=-300 4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=12, 2-8=52, 8-10=115, 7-10=52 Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55 Concentrated Loads (lb) Vert: 9=548 5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=42, 2-8=52, 8-10=115, 7-10=52 Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55 Concentrated Loads (lb) Vert: 9=566 6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9 Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51 Concentrated Loads (lb) Vert: 9=-420 7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9 Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51 Concentrated Loads (lb) Vert: 9=-420 8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-11, 2-8=10, 8-10=33, 7-10=10 Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7 Concentrated Loads (lb) Vert: 9=154 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert; 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=43 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Concentrated Loads (lb) Vert: 9=43

14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12

Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39

Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27

Concentrated Loads (lb)

Vert: 9=43

15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60

meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

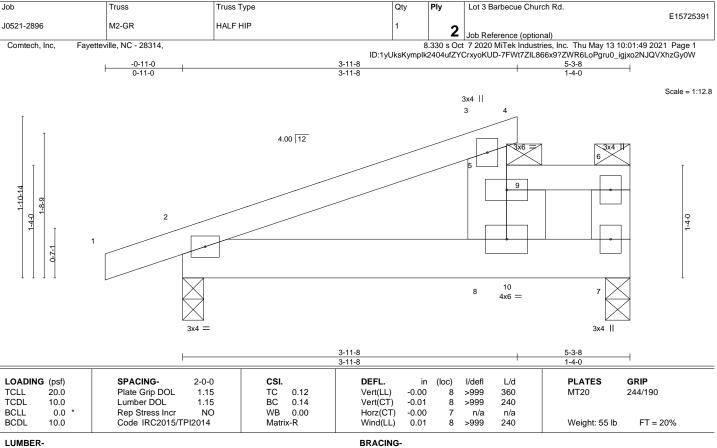


Job Truss	s Ti	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
1,550,0000					E15725390
J0521-2896 M2	l _H	Half Hip	3	1	Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

Concentrated Loads (lb) Vert: 9=-350 8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:47 2021 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-BsO6itH5cVsDvhP7KhJKKFmWdB?hF1llw?xPSpzGy0Y

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LOAD CASE(S) Standard
    Uniform Loads (plf)
            Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12
            Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
    Concentrated Loads (lb)
            Vert: 9=43
16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
    Uniform Loads (plf)
            Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-7=-20
            Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12
    Concentrated Loads (lb)
            Vert: 9=-234
17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
    Uniform Loads (plf)
            Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-21, 2-7=-20
            Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0
    Concentrated Loads (lb)
            Vert: 9=-234
18) Dead: Lumber Increase=0.90. Plate Increase=0.90 Plt. metal=0.90
    Uniform Loads (plf)
            Vert: 1-3=-20, 3-4=-20, 5-6=-120, 2-7=-20
    Concentrated Loads (lb)
            Vert: 9=-200
19) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60
    Uniform Loads (plf)
            Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-8=-3, 8-10=13, 7-10=-3
            Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26
    Concentrated Loads (lb)
            Vert: 9=-454
20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
    Uniform Loads (plf)
            Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-86, 6-9=-116, 2-7=-20
            Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0
    Concentrated Loads (lb)
            Vert: 9=-375
21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60
    Uniform Loads (plf)
            Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-7=-20
            Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9
    Concentrated Loads (lb)
            Vert: 9=-375
22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60
    Uniform Loads (plf)
            Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-86, 6-9=-116, 2-7=-20
            Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0
    Concentrated Loads (lb)
            Vert: 9=-375
23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
    Uniform Loads (plf)
            Vert: 1-3=-60, 3-4=-60, 5-6=-40, 2-7=-20
    Concentrated Loads (lb)
            Vert: 9=-400
24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
    Uniform Loads (plf)
            Vert: 1-3=-20, 3-4=-20, 5-9=-40, 6-9=-80, 2-7=-20
    Concentrated Loads (lb)
            Vert: 9=-400
25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
    Uniform Loads (plf)
            Vert: 1-3=-50, 3-4=-50, 5-6=-100, 2-7=-20
    Concentrated Loads (lb)
            Vert: 9=-350
26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
    Uniform Loads (plf)
            Vert: 1-3=-20, 3-4=-20, 5-9=-100, 6-9=-130, 2-7=-20
```



TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1 2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 WFBS

REACTIONS. (size) 7=0-3-8, 2=0-3-0

Max Horz 2=69(LC 12)

Max Uplift 7=-24(LC 8), 2=-112(LC 8) Max Grav 7=710(LC 19), 2=375(LC 1)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-484/446, 5-8=-334/210, 5-6=-280/291, 6-7=-390/210

BOT CHORD 2-8=-491/415. 7-8=-291/280

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-7-4, Interior(1) 3-7-4 to 5-0-12 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (jt=lb)
- 9) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s). The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 5-9=-160, 6-9=-200, 2-7=-20



Structural wood sheathing directly applied or 5-3-8 oc purlins,

except end verticals, and 2-0-0 oc purlins: 3-8, 5-6. Except:

Rigid ceiling directly applied or 10-0-0 oc bracing.

10-0-0 oc bracing: 3-5

May 13,2021

meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see MSI-89 and BCSI Building Comport Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
					E15725391
J0521-2896	M2-GR	HALF HIP	1	2	Joh Reference (entional)
				_	Job Reference (optional)

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:49 2021 Page 2 $ID:1yUksKymplk2404ufZYCrxyoKUD-7FWt7ZIL866x9?ZWR6LoPgru0_igjxo2NJQVXhzGy0W$

Comtech, Inc. Fayetteville, NC - 28314, LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 9=-400 2) Dead + 0.75 Roof Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-50, 3-4=-50, 5-9=-220, 6-9=-250, 2-7=-20 Concentrated Loads (lb) Vert: 9=-350 3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-6=-160, 2-7=-40 Concentrated Loads (lb) Vert: 9=-300 4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=-108, 2-8=52, 8-10=115, 7-10=52 Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55 Concentrated Loads (lb) Vert: 9=548 5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=-78, 2-8=52, 8-10=115, 7-10=52 Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55 Concentrated Loads (lb) Vert: 9=566 6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9 Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51

Concentrated Loads (lb) Vert: 9=-420

7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9

Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51

Concentrated Loads (lb)

Vert: 9=-420

8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-131, 2-8=10, 8-10=33, 7-10=10

Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7

Concentrated Loads (lb)

Vert: 9=154

9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12

Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27

Concentrated Loads (lb)

Vert: 9=43

10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2

Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34

Concentrated Loads (lb)

Vert: 9=-339

11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20

Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0

Concentrated Loads (lb)

Vert: 9=-234

12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12

Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39

Concentrated Loads (lb)

Vert: 9=43

13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27

Concentrated Loads (lb)

Vert: 9=43

14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60

meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTE(®) connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see MSI-89 and BCSI Building Components And Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road

Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
J0521-2896	M2-GR	HALF HIP	1	2	E15725391
					Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

Vert: 9=-350

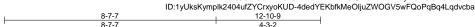
8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:50 2021 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-cR4FKvJzvQEnm98i?ps1ytO3mO2vSO2Bcz9338zGy0V

```
LOAD CASE(S) Standard
    Uniform Loads (plf)
            Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12
            Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
    Concentrated Loads (lb)
            Vert: 9=43
15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60
    Uniform Loads (plf)
            Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12
            Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
    Concentrated Loads (lb)
            Vert: 9=43
16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
    Uniform Loads (plf)
             Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20
            Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12
    Concentrated Loads (lb)
            Vert: 9=-234
17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
    Uniform Loads (plf)
            Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-141, 2-7=-20
            Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0
    Concentrated Loads (lb)
            Vert: 9=-234
18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
    Uniform Loads (plf)
            Vert: 1-3=-20, 3-4=-20, 5-6=-240, 2-7=-20
    Concentrated Loads (lb)
            Vert: 9=-200
19) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60
    Uniform Loads (plf)
            Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-8=-3, 8-10=13, 7-10=-3
            Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26
    Concentrated Loads (lb)
            Vert: 9=-454
20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
    Uniform Loads (plf)
            Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-206, 6-9=-236, 2-7=-20
            Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0
    Concentrated Loads (lb)
            Vert: 9=-375
21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60
    Uniform Loads (plf)
            Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-7=-20
            Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9
    Concentrated Loads (lb)
            Vert: 9=-375
22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60
    Uniform Loads (plf)
            Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-206, 6-9=-236, 2-7=-20
            Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0
    Concentrated Loads (lb)
            Vert: 9=-375
23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
    Uniform Loads (plf)
            Vert: 1-3=-60, 3-4=-60, 5-6=-160, 2-7=-20
    Concentrated Loads (lb)
            Vert: 9=-400
24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
    Uniform Loads (plf)
            Vert: 1-3=-20, 3-4=-20, 5-9=-160, 6-9=-200, 2-7=-20
    Concentrated Loads (lb)
            Vert: 9=-400
25) \ \ {\rm 3rd\ Dead} + 0.75\ {\rm Roof\ Live\ (unbalanced)} + 0.75\ {\rm Attic\ Floor:\ Lumber\ Increase=1.15}, \ {\rm Plate\ Increase=1.15}
    Uniform Loads (plf)
            Vert: 1-3=-50, 3-4=-50, 5-6=-220, 2-7=-20
    Concentrated Loads (lb)
            Vert: 9=-350
26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15
    Uniform Loads (plf)
            Vert: 1-3=-20, 3-4=-20, 5-9=-220, 6-9=-250, 2-7=-20
    Concentrated Loads (lb)
```

Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
J0521-2896	V1GE	ROOF SPECIAL STRUCTU	1	1	E15725392
30321-2090	VIGE	ROOF SPECIAL STRUCTU	'	'	Job Reference (optional)

Comtech, Inc. Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:51 2021 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-4dedYEKbfkMeOljuZWOGV5wFQoPqBq4LqdvcbazGy0U



4x4 =

Scale = 1:44.2

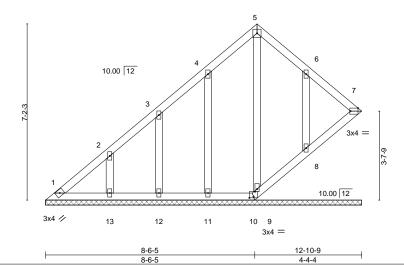


Plate Offsets (X,Y)-- [7:0-3-11,Edge], [9:0-1-6,0-1-0], [10:0-2-0,0-0-10]

LOADING (psf) SPACING-DEFL 2-0-0 CSI. I/defl L/d (loc) **TCLL** 20.0 Plate Grip DOL 1.15 TC 0.06 Vert(LL) n/a n/a 999 TCDL 10.0 Lumber DOL 1.15 ВС 0.03 Vert(CT) n/a 999 n/a **BCLL** 0.0 Rep Stress Incr YES WB 0.08 Horz(CT) 0.00 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S

PLATES GRIP MT20 244/190

Weight: 75 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No 1 OTHERS 2x4 SP No 2 **BRACING-**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 12-10-9.

(lb) - Max Horz 1=231(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10 except 11=-112(LC 12), 12=-107(LC 12), 13=-133(LC 12),

8=-126(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 9, 11, 12, 13, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-295/189

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 7, 9, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10 except (jt=lb) 11=112, 12=107, 13=133, 8=126.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 7, 9, 8.



May 13,2021



Job	Truss	Truss Type		Qty	Ply	Lot 3 Barbecue Church Rd.
						E15725393
J0521-2896	V2	VALLEY		1	1	
						Job Reference (optional)
Comtech, Inc, Fa	ayetteville, NC - 28314,				8.330 s Oc	et 7 2020 MiTek Industries, Inc. Thu May 13 10:01:53 2021 Page 1
			ID:1	yUksKym	plk2404ufZ\	CrxyoKUD-00mOzwLsBLcMectHgxQkaW0Zjc3QfjMeIxOjgSzGy0S
		7-5-0	1			14-10-0
	1	7-5-0	'			7-5-0
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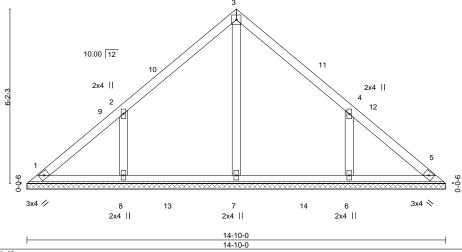


Plate Off	sets (X,Y)	[4:0-0-0,0-0-0]											
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.15	TC	0.14	Vert(LL)	n/a	` -	n/a	999	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.15	Vert(CT)	n/a	-	n/a	999			
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.09	Horz(CT)	0.00	5	n/a	n/a			
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-S						Weight: 64 lb	FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

OTHERS 2x4 SP No.2

OTHERS 2x4 SP No.2

REACTIONS. All bearings 14-10-0.

(lb) - Max Horz 1=-140(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-135(LC 12), 6=-135(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=400(LC 19), 8=393(LC 19), 6=393(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-338/247, 4-6=-338/247

NOTES-

- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-5-0, Exterior(2) 7-5-0 to 11-9-13, Interior(1) 11-9-13 to 14-5-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=135, 6=135.



May 13,2021

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
J0521-2896	V3	VALLEY	1	1	E15725394
00021 2000		7,122			Job Reference (optional)
Occupied to Ferrit				000 - 0-1	7 0000 MT-1 lash strikes has The May 40 40 04 FE 0004 Days 4

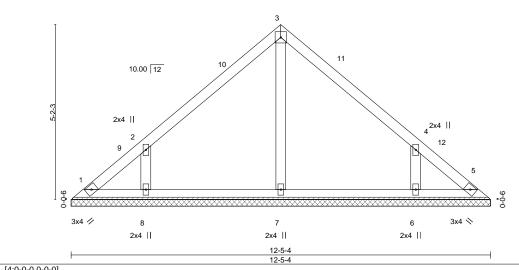
6-2-10

Comtech, Inc, Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:01:55 2021 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-yPt8OcN6jys4tw0foMSCfx5vLPms7dHxlFtqkLzGy0Q 12-5-4

6-2-10

4x4 = Scale: 3/8"=1'



_ Flate Olisets (A, I)	[4.0-0-0,0-0-0]			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL 20.0	Plate Grip DOL 1.15	TC 0.13	Vert(LL) n/a - n/a 999 MT20 244/190	
TCDL 10.0	Lumber DOL 1.15	BC 0.09	Vert(CT) n/a - n/a 999	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.06	Horz(CT) 0.00 5 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Weight: 52 lb FT = 20%	

LUMBER-

Dieta Offesta (V V)

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

OTHERS 2x4 SP No.1

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 12-5-4.

(lb) - Max Horz 1=-116(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-123(LC 12), 6=-123(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=326(LC 19), 6=326(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-312/241, 4-6=-312/241

- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-2-10, Exterior(2) 6-2-10 to 10-7-7, Interior(1) 10-7-7 to 12-0-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123, 6=123.



May 13,2021





Job	Truss	Truss Type	Q	Qty	Ply	Lot 3 Barbecue Chur	ch Rd.	
		l						E15725395
J0521-2896	V4	VALLEY	1		1			
Comton Inc. Fou	attacilla NC 20244					Job Reference (option		0.04.FC 2024 Page 4
Comtech, Inc, Fay	etteville, NC - 28314,		ID:1vl lkcKvr				ries, Inc. Thu May 13 1 3_xV4bsM4zRB8e3dp5	
		5-0-3	iD. TyOKSKyi	IIPIKZ40	4ui2 i Ci xy 1	0.00-QDRVVDYOROG 0-0-7	_xv4b3ivi42l\b0e3up3	izs5i14_vcivGi1zGy0F
		5-0-3				0-0-7 5-0-4		
			4x4 =					Scale = 1:26.4
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	I		10-0-7				I	
LOADING (psf)	SPACING- 2-0	O CSI.	DEFL.	in	(loc)	I/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.1		Vert(LL)	n/a		n/a 999	MT20	244/190
TCDL 10.0	Lumber DOL 1.1		Vert(CT)	n/a		n/a 999	20	
BCLL 0.0 *	Rep Stress Incr YE		Horz(CT)	0.00		n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014		(31)	3.30	Ü		Weight: 38 lb	FT = 20%
LUMBER-			BRACING-	-				
TOD CHORD 2v4 CE	No. 1		TOD CHOI		Ctructure	al wood oboothing di	rooth, applied or C O C) oo purling

TOP CHORD

BOT CHORD

2x4 SP No.2 REACTIONS.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

BOT CHORD

OTHERS

(size) 1=10-0-7, 3=10-0-7, 4=10-0-7 Max Horz 1=-92(LC 8)

Max Uplift 1=-22(LC 13), 3=-30(LC 13)

Max Grav 1=197(LC 1), 3=197(LC 1), 4=344(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



May 13,2021



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



Job	Truss	Truss T	уре	C	ity	Ply	Lot 3 Barbecue Churc	h Rd.	
J0521-2896	V5	VALLE	Y	1		1	Job Reference (options	al)	E15725396
Comtech, Inc,	Fayetteville, NC - 28314,	· ·					t 7 2020 MiTek Industrie	es, Inc. Thu May 13 1	
			3-9-13	ID:1yUk	sKymplk2	2404ufZY 7-7-	CrxyoKUD-NzZH0eP_0 10	tFfkNIETU0vHZjPzdn	dK?fNRD5ULgzGy0N
	F		3-9-13	-		3-9-	13	—	
				4x4 =					Scale = 1:21.4
			,	2					
		10.00	<u> 12</u>						
	3-2-3	,							
		1					3		
	9-0-0							9-0-0	
		3x4 //	2x	4 4			3x4 ≫		
	F			7-7-10 7-7-10					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Inc	1.15	CSI. TC 0.17 BC 0.09 WB 0.02	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015		Matrix-P	поід(СТ)	0.00	ა	11/d 11/d	Weight: 28 lb	FT = 20%

LUMBER-BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 **OTHERS**

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=7-7-10, 3=7-7-10, 4=7-7-10

Max Horz 1=68(LC 9)

Max Uplift 1=-24(LC 13), 3=-30(LC 13)

Max Grav 1=158(LC 1), 3=158(LC 1), 4=230(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



May 13,2021





Job	Truss	Truss	Гуре		Qty	Ply	Lot 3 Barbecue Chur	ch Rd.	
									E15725397
J0521-2896	V6	VALLE	Υ		1	1			
							Job Reference (option		24.52.224.5
Comtech, Inc, Fa	ayetteville, NC - 28314,			ID:1vLlka				ies, Inc. Thu May 13 10 WMXKR1CX8pnGbJ08	
			2-7-7	ib. Tyoks	rtyllipikz-	5-2-1		VIVIARIA I CAOPII GOSOO	d333vvgti 1to2Gyotvi
			2-7-7 2-7-7	-		2-7-6	5	+	
				4x4 =					Scale: 3/4"=1'
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				2					
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				/ \	\				
	2-2-3								
	2-5				`	/ /			
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	9-0-0				/////////////////////////////////////	******		9-0-0	
	0		***************************************	************	*****	******	***************************************	3	
				4					
		3x4 //	2	2x4			3x4 📏		
				5040					
		-		5-2-13 5-2-13				4	
				J-Z=13					
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	ir	ı (loc)	I/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.07	Vert(LL)	n/a	ı `-	n/a 999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.04	Vert(CT)			n/a 999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.01	Horz(CT	0.00	3	n/a n/a		
BCDL 10.0	Code IRC2015/TF	PI2014	Matrix-P					Weight: 19 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-2-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=5-2-13, 3=5-2-13, 4=5-2-13

Max Horz 1=-44(LC 8)

Max Uplift 1=-15(LC 13), 3=-19(LC 13)

Max Grav 1=102(LC 1), 3=102(LC 1), 4=149(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



May 13,2021





Job Truss Truss Type Qty Ply Lot 3 Barbecue Church Rd. F15725398 J0521-2896 V7 VALLEY Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 10:02:00 2021 Page 1 $ID:1yUksKymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKUD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKuD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKuD-JMh1RJRFYVVNzhvdbv2NM_onuQUvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKuD-JMh1RJRFYVVNZhvdbv2NM_onuQuvovWgvXabPYzGy0Linderskymplk2404ufZYCrxyoKuD-JMh1RJRFYVVNZhvVD-JMh1RJRFYVVNZhvVD-JMh1RJRFYVVNZhvVD-JMh1RJRFYVVNZhvVD-JMh1RJRFYVVNZhvVD-JMh1RJRFYVVNZhvVD-JMh1RJRFYVVNZhvVNZhvVD-JMh1RJRFYVVNZhVD-JMh1RJRFYVV$ 1-5-0 1-5-0 Scale = 1:8.7 3x4 = 2 10.00 12 3 0-0-6 9-0-0 3x4 // 3x4 📏 2-10-0 2-10-0 Plate Offsets (X,Y)-- [2:0-2-0,Edge] LOADING (psf) SPACING-CSI. DEFL. **PLATES** GRIP 2-0-0 I/defI L/d 244/190 **TCLL** 20.0 Plate Grip DOL 1.15 TC 0.01 Vert(LL) n/a n/a 999 MT20 TCDL 10.0 Lumber DOL 1.15 ВС 0.03 Vert(CT) n/a 999 n/a BCLL 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-P Weight: 8 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 **BRACING-**TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 2-10-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=2-10-0, 3=2-10-0

Max Horz 1=-20(LC 8)

Max Uplift 1=-4(LC 12), 3=-4(LC 13)

Max Grav 1=81(LC 1), 3=81(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate arip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



May 13,2021

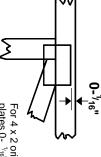


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. and fully embed teeth. Apply plates to both sides of truss Dimensions are in ft-in-sixteenths.



For 4×2 orientation, locate plates 0- $\frac{1}{16}$ from outside edge of truss.

ω

O

S

required direction of slots in This symbol indicates the

connector plates

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE



to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



output. Use T or I bracing Indicated by symbol shown and/or if indicated. by text in the bracing section of the

BEARING



number where bearings occur.
Min size shown is for crushing only reaction section indicates joint Indicates location where bearings (supports) occur. Icons vary but

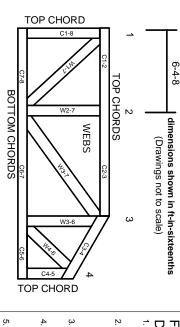
Industry Standards:

ANSI/TPI1:

DSB-89:

National Design Specification for Metal Plate Connected Wood Truss Construction. Installing & Bracing of Metal Plate Connected Wood Trusses. Guide to Good Practice for Handling, Building Component Safety Information Design Standard for Bracing.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values established by others. Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

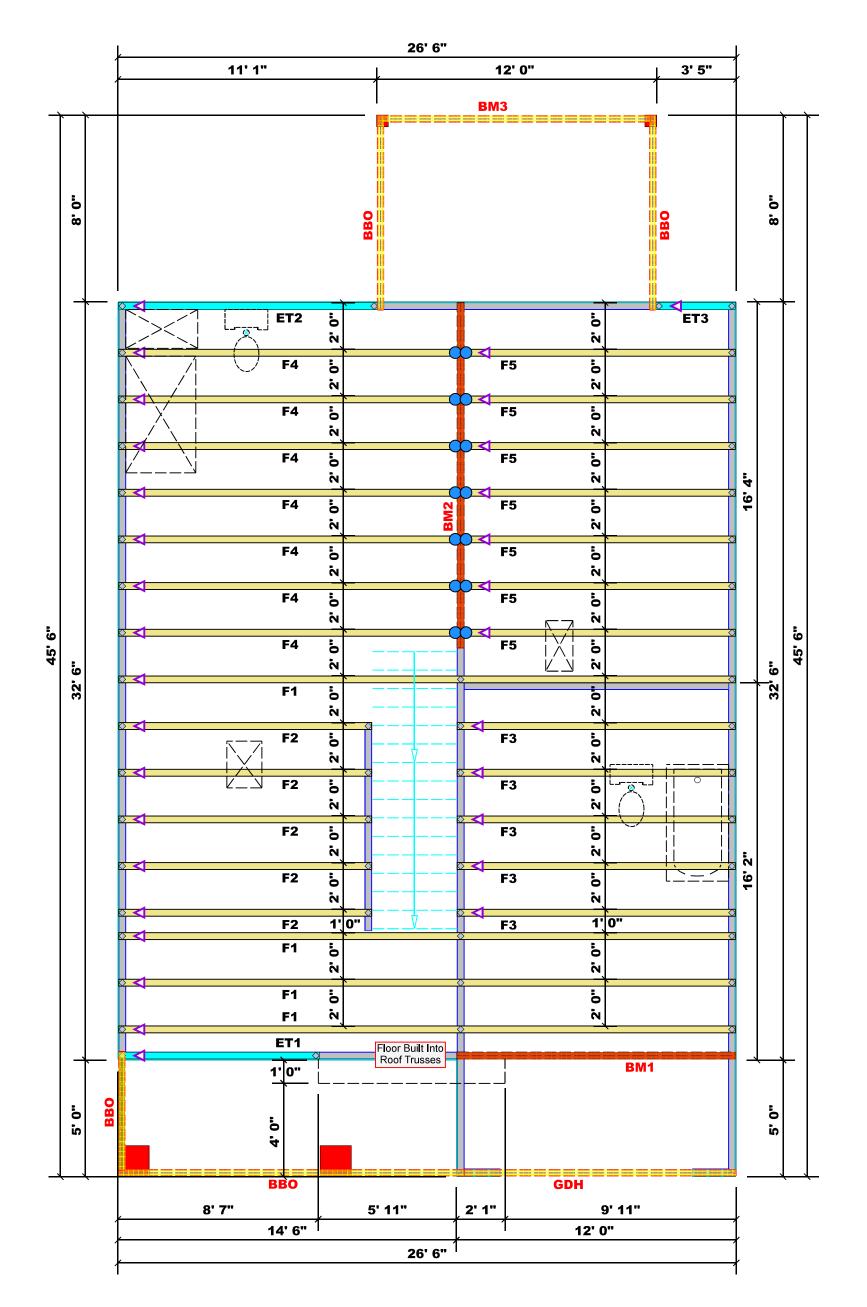
General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- 10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to
- 11. Plate type, size, orientation and location dimensions camber for dead load deflection.
- 12. Lumber used shall be of the species and size, and

indicated are minimum plating requirements.

- in all respects, equal to or better than that
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.



Dimension Notes

1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
2. All interior wall dimensions are to face of frame wall unless noted otherwise
3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes

1. Plumbing drop locations shown are NOT exact.
2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
3. Adjust spacing as needed not to exceed 24"oc.

	Conne	ctor Info	rmat	ion	Nail Information		
Sym	Product	Manuf	Qty	Supported Member	Header	Truss	
	HUS410	USP	14	NA	16d/3-1/2"	16d/3-1/2"	

Products								
PlotID	Length	Product	Plies	Net Qty				
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2				
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2				
BM3	12' 0"	2x10 SPF No.2	2	2				
GDH	12' 0"	2x12 SPF No.2	2	2				

Truss Placement Plan
Scale: 1/4"=1'



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Cod requirements) to determine the minimum foundatio size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attache Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

nature David Landry

David Landry

LOAD CH	ART FOR JACK	STUDS
(BASE	DION TABLES R5025(I) 3	(b));
NUMBER OF J.	ASK STUDS REQUIRED ® HEADER/SIRDER	EA END OF
ND REACTION (UP TO) 316 STUBS FOR 3) PLY HEABER	ID RFACTION (UP TO) Q'D STUDS FOR () PLY HLADLR	NN REACTION (UP TO) Q'D GTUDG FOR

END REACTION (UP TO)	REGID STUDS FOR (2) PLY HEADER	FNI) RFACTION (UP TO)	REQ'D STUDS FOR (3) MLY HLADLR	END REACTION	(UP TO) RCQ'D STUDS FOR
1700	1	255/	0 1	340	00 1
3400	2	5100	2	680	00 2
5100	3	765/	0 3	102	00 3
6800	4	1020	0 4	136	00 4
8500	5	1275	0 5	170	00 5
10200	6	1530	0 6		
11900	7				
13600	8				
15300	9				

Inc.	CITY / CO.	CITY / CO. Sanford / Harnett	13600 15300
	ADDRESS	4903 Barbecue Church Rd.	8 9
	MODEL	Floor	
	DATE REV . 05/13/21	05/13/21	
	DRAWN BY	DRAWN BY David Landry	
	SALES REP.	SALES REP. Lenny Norris	

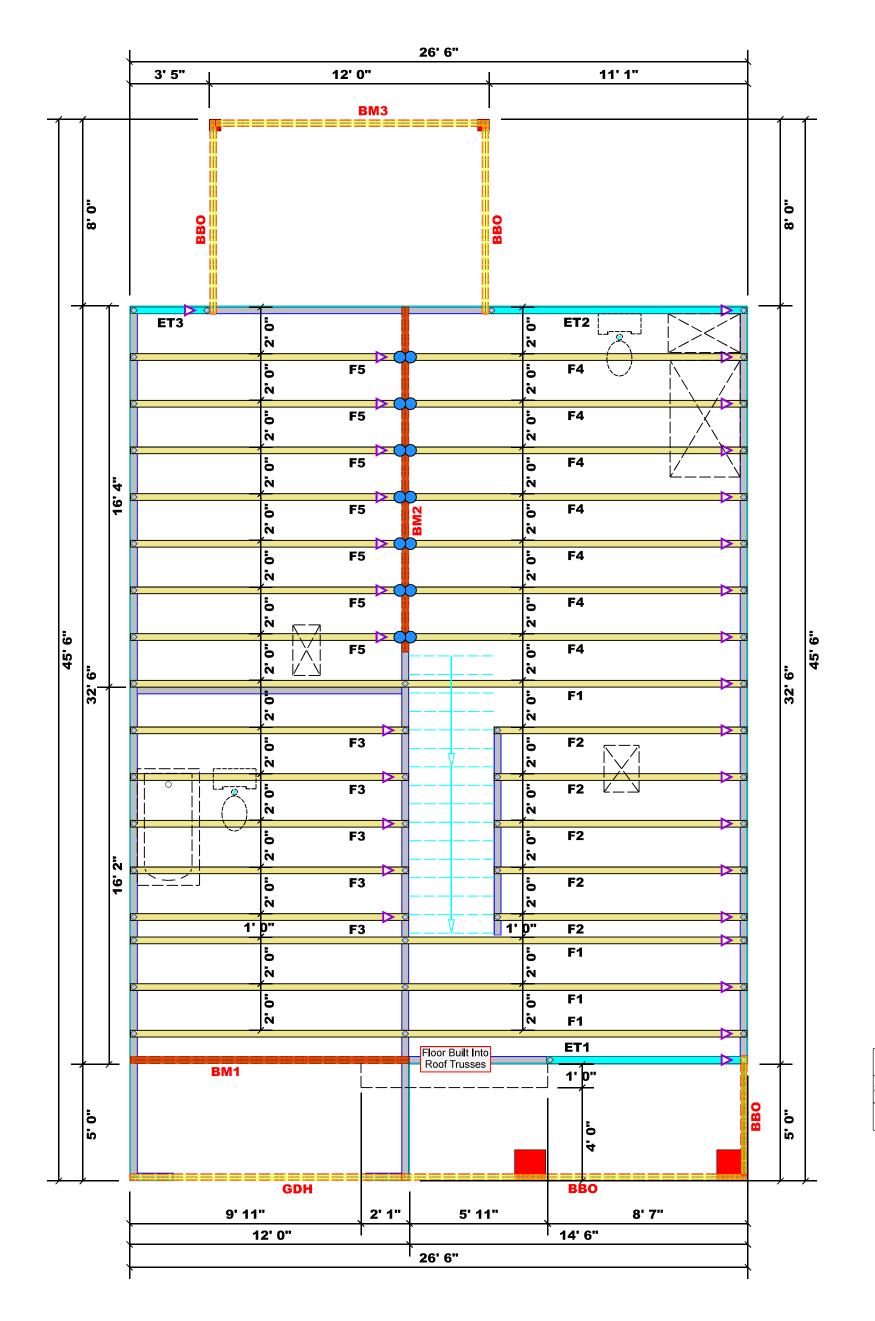
THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Lot 3 Barbecue

JOB NAME

BUILDER

= Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards



Dimension Notes

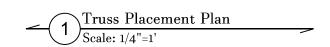
1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
2. All interior wall dimensions are to face of frame wall unless noted otherwise
3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes
Plumbing drop locations shown are NOT exact Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses. Adjust spacing as needed not to exceed 24"oc

	Conne	Nail Info	ormation			
Sym	Product Manuf Qty Supported Member		Header	Truss		
	HUS410	USP	14	NA	16d/3-1/2"	16d/3-1/2"

		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM3	12' 0"	2x10 SPF No.2	2	2
GDH	12' 0"	2x12 SPF No.2	2	2



= Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Cod requirements) to determine the minimum foundatio size and number of wood studs required to support reactions greater than 3000# but not greater than 1500#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attache Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 1500#.

Signature David Landry

David Landry

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R5025(I) & (b))

NUMBER OF JACK STUDS REQUIRED & EALEND OF HEADER/STROER

ı			MEADERA	SIRDER	ě.		
	END REACTION (UP TO)	REQ15 STUDS FOR (2) PLY HEADER	FND RFACTION (UP TO)	REQ'D STUDS FOR (3) PLY HUADUR		FNh RFACTION (UP TO)	RCQ'D STUDS FOR
I	1700	1	2550	1		3400	1
ı	3400	2	5100	2		6800	3
	5100	3	7650	3		10200	2
	6800	4	10200	4		13600	4
	8500	5	12750	5		17000	Ş
	10200	6	15300	6			
ı	11900	7_					
ı	13600	8					
	15300	9					
۱		_					

Weaver Development Co. Inc.	CITY / CO.	CITY / CO. Sanford / Harnett
Lot 3 Barbecue Church Rd.	ADDRESS	4903 Barbecue Church Rd.
Hickory "C"	MODEL	Floor
Seal Date	DATE REV.	05/13/21
Quote #	DRAWN BY	DRAWN BY David Landry
J0521-2897	SALES REP.	SALES REP. Lenny Norris

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

BUILDER

Client: Weaver Development

Project:

5/13/2021 Date: Input by: David Landry

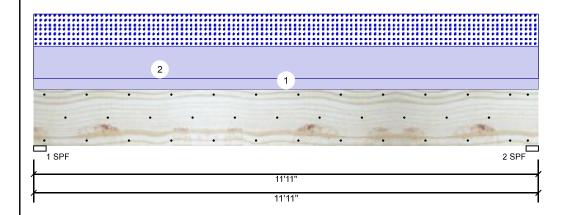
Job Name: Lot 3 Barbecue Church Road

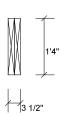
J0521-2897 Project #:

Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED BM₁

Address:

Level: Level





Page 1 of 8

Member Information

Girder Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temp <= 100°F Temperature:

Floor Application: Design Method: ASD **Building Code:** IBC/IRC 2015

Load Sharing: Deck:

Not Checked Ceiling: Gypsum 1/2"

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	2869	2079	0	0
2	Vertical	0	2869	2079	0	0

Bearings

Bearing Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 3.500"	Vert	95%	2869 / 2079	4948	L	D+S
2-SPF 3.500"	Vert	95%	2869 / 2079	4948	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	13679 ft-Ib	5'11 1/2"	39750 ft-lb	0.344 (34%)	D+S	L
Unbraced	13679 ft-Ib	5'11 1/2"	13699 ft-lb	0.999 (100%)	D+S	L
Shear	3615 lb	1'7 1/2"	13739 l b	0.263 (26%)	D+S	L
LL Defl inch	0.069 (L/2000)	5'11 1/2"	0.287 (L/480)	0.240 (24%)	S	L
TL Defl inch	0.164 (L/840)	5'11 1/2"	0.383 (L/360)	0.428 (43%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 8'8 3/8" o.c.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall	
2	Uniform			Тор	349 PLF	0 PLF	349 PLF	0 PLF	0 PLF	A2	
	Self Weight				12 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemica**l**s

Handling & Installation

- Handling & Installation

 1. IVL beam must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-obj fastering details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/7/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info





Client: Project:

Address:

Weaver Development

Date: 5/13/2021

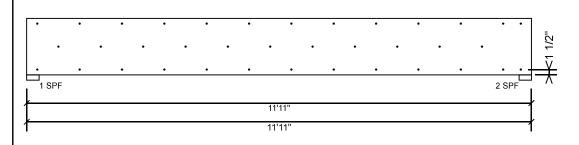
Input by: David Landry

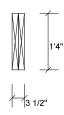
Job Name: Lot 3 Barbecue Church Road

J0521-2897 Project #:

2-Ply - PASSED **Kerto-S LVL** 1.750" X 16.000" BM1

Level: Level





Page 2 of 8

Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity 0.0 % 0.0 PLF Load Yield Limit per Foot 245.6 PLF Yield Limit per Fastener 81.9 lb. Yield Mode IV Edge Distance 1 1/2" Min. End Distance 3" Load Combination Duration Factor 1.00

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemica**l**s

Handling & Installation

Handling & Installation

1. IVL beam must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-obj fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is valid until 4/7/2024

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851

Manufacturer Info

(800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633





Client: Project:

Address:

Weaver Development

5/13/2021

Page 3 of 8

Date: Input by: David Landry

Job Name: Lot 3 Barbecue Church Road

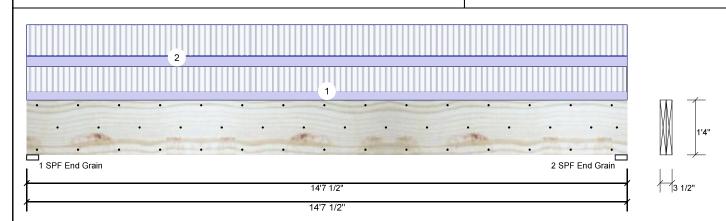
Level: Level

J0521-2897 Project #:

Kerto-S LVL BM₂

1.750" X 16.000"

2-Ply - PASSED



Girder Floor Application: Type: Plies: Design Method: ASD Moisture Condition: Dry **Building Code:** IBC/IRC 2015 Deflection LL: 480 Load Sharing: Deflection TL: 360 Deck: Not Checked Importance: Normal - II Ceiling: Gypsum 1/2" Temp <= 100°F Temperature:

Rea	ctions UNP/	ATTERNED	b lb (Uplift))	
Dra	Direction	Livo	Doad	Snow	\\/in

ыy	Direction	LIVE	Deau	SHOW	vviilu	Const
1	Vertical	3868	1385	0	0	0
2	Vertical	3868	1385	0	0	0

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	18077 ft-Ib	7'3 3/4"	34565 ft-lb	0.523 (52%)	D+L	L
Unbraced	18077 ft -l b	7'3 3/4"	18085 ft-lb	1.000 (100%)	D+L	L
Shear	5044 lb	1'7 1/2"	11947 l b	0.422 (42%)	D+L	L
LL Defl inch	0.229 (L/743)	7'3 13/16"	0.355 (L/480)	0.646 (65%)	L	L
TL Defl inch	0.311 (L/547)	7'3 13/16"	0.473 (L/360)	0.658 (66%)	D+L	L

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	49%	1385 / 3868	5254	L	D+L
2 - SPF End Grain	3.500"	Vert	49%	1385 / 3868	5254	L	D+L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top must be laterally braced at a maximum of 6'5 1/8" o.c.
- 6 Lateral slenderness ratio based on single ply width.

I D	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Near Face	79 PLF	235 PLF	0 PLF	0 PLF	0 PLF	F5
2	Uniform			Far Face	98 PLF	294 PLF	0 PLF	0 PLF	0 PLF	F4
	Self Weight				12 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemica**l**s Handling & Installation

Handling & Installation

1. IVL beam must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-obj fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/7/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info





Client:

Weaver Development

Date: 5/13/2021

Input by: David Landry

Job Name: Lot 3 Barbecue Church Road

Page 4 of 8

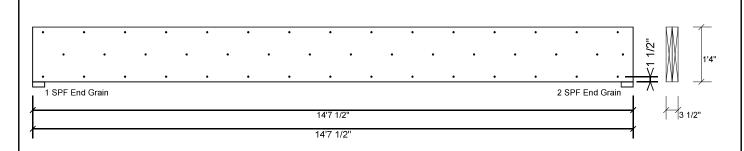
J0521-2897 Project #:

2-Ply - PASSED **Kerto-S LVL** 1.750" X 16.000" BM₂

Project:

Address:

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity 79.8 % 196.0 PLF Load Yield Limit per Foot 245.6 PLF Yield Limit per Fastener 81.9 lb. Yield Mode IV Edge Distance 1 1/2" Min. End Distance 3" D+L Load Combination Duration Factor 1.00

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemica**l**s

Handling & Installation

Handling & Installation

1. IVL beam must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-obj fastering details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is valid until 4/7/2024

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info





Client: 5/13/2021 Weaver Development Date: Page 5 of 8 Project: Input by: David Landry isDesign Address: Job Name: Lot 3 Barbecue Church Road J0521-2897 Project #: Level: Level 2.000" X 10.000" 2-Ply - PASSED S-P-F #2 **BM3** 1 1 SPF End Grain 2 SPF End Grain 12 **Member Information** Reactions UNPATTERNED lb (Uplift) Girder Application: Floor Live Dead Wind Brg Direction Snow Const Type: Plies: Design Method: ASD 564 Vertical 0 564 0 0 Moisture Condition: Dry **Building Code:** IBC/IRC 2015 0 564 564 0 0 2 Vertical Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Normal - II Importance: Ceiling: Gypsum 1/2" Temp <= 100°F Temperature: Bearings Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.500" Vert 25% 564 / 564 1128 L D+S Fnd Grain Analysis Results 2 - SPF 3.500" Vert 25% 564 / 564 1128 L D+S Analysis Actual Location Allowed Capacity Comb. Case End Moment 3130 ft-lb 6' 3946 ft-lb 0.793 (79%) D+S L Grain 3130 ft-lb 6' 3131 ft-lb 1.000 Unbraced (100%)10'11 1/4" 2872 lb Shear 928 lb 0.323 (32%) D+S L LL Defl inch 0.135 (L/1022) 6' 0.289 (L/480) 0.470 (47%) S L TL Defl inch 0.271 (L/511) 6' 0.385 (L/360) 0.704 (70%) D+S **Design Notes** 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code. 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6". 3 Refer to last page of calculations for fasteners required for specified loads. 4 Girders are designed to be supported on the bottom edge only. 5 Top loads must be supported equally by all plies. 6 Top must be laterally braced at a maximum of 8'8 11/16" o.c. 7 Lateral slenderness ratio based on single ply width. ID Load Type Trib Width Side Dead 0.9 Snow 1.15 Wind 1.6 Const. 1.25 Comments Location Live 1 Uniform 94 PLF 0 PLF 94 PLF 0 PLF 0 PLF C1 1 Тор Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA Manufacturer Info 28314 910-864-TRUS соттесн

This design is valid until 4/7/2024

Client: Weaver Development Date: 5/13/2021 Page 6 of 8 Project: Input by: David Landry isDesign Address: Job Name: Lot 3 Barbecue Church Road Project #: J0521-2897 Level: Level S-P-F #2 2.000" X 10.000" 2-Ply - PASSED **BM3** □ 1 SPF End Grain 2 SPF End Grain

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

12'

Capacity 0.0 % 0.0 PLF Load Yield Limit per Foot 157.4 PLF Yield Limit per Fastener 78.7 lb. Yield Mode IV Edge Distance 1 1/2" Min. End Distance 3" Load Combination 1.00 Duration Factor

Manufacturer info

Comtech, Inc.
1001 S. Reifly Road, Suite #639
Fayetteville, NC
USA
28314
910-864-TRUS

This design is valid until 4/7/2024





Client: Project:

Address:

Weaver Development

5/13/2021

Date: Input by: David Landry

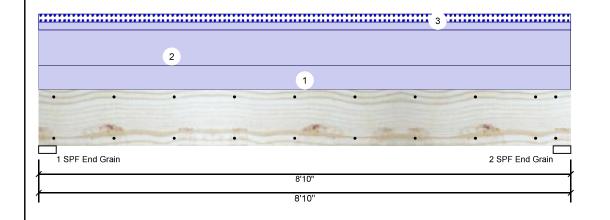
Job Name: Lot 3 Barbecue Church Road J0521-2897 Project #:

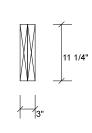
S-P-F #2

Member Information

2.000" X 12.000" 2-Ply - PASSED

Level: Level





Page 7 of 8

Girder Floor Application: Type: Plies: Design Method: ASD Moisture Condition: Dry **Building Code:** IBC/IRC 2015 Deflection LL: 480 Load Sharing: 360 Not Checked Deflection TL: Deck: Normal - II Importance: Ceiling: Gypsum 1/2" Temp <= 100°F Temperature:

Read	ctions UNP/	ATTERNED	lb (Uplift)		
Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	751	88	0	0
2	Vertical	0	751	88	0	0

Analysis Re	sults					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1490 ft-Ib	4'5"	4153 ft-Ib	0.359 (36%)	D	Uniform
Unbraced	1490 ft- l b	4'5"	3539 ft-Ib	0.421 (42%)	D	Uniform
Shear	542 lb	1'2 3/4"	2734 lb	0.198 (20%)	D	Uniform
LL Defl inch	0.004 (L/22622)	4'5 1/16"	0.209 (L/480)	0.021 (2%)	S	L
TL Defl inch	0.042 (L/2381)	4'5 1/16"	0.279 (L/360)	0.151 (15%)	D+S	L

	Bearings	s						
Γ	Bearing	Length	Dir.	Сар	React D/L lb	Total	Ld. Case	Ld. Comb.
	1 - SPF End Grain	3.500"	Vert	19%	751 / 88	839	L	D+S
	2 - SPF End Grain	3.500"	Vert	19%	751 / 88	839	L	D+S

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

I D	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
2	Uniform			Тор	90 PLF	0 PLF	0 PLF	0 PLF	0 PLF	B1GE
3	Tie-In	0-0-0 to 8-10-0	1-0-0	Тор	20 PSF	0 PSF	20 PSF	0 PSF	0 PSF	Roof Load

Manufacturer Info Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS соттесн

CSD

Client: Weaver Development

Date: 5/13/2021

Input by: David Landry Job Name: Lot 3 Barbecue Church Road

Project #: J0521-2897

GDH S-P-F #2 2.000" X 12.000"

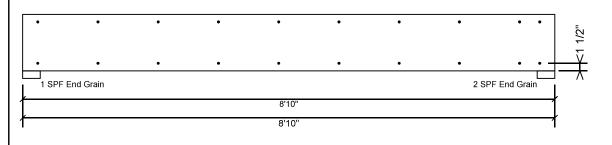
Project:

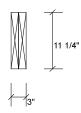
Address:

2-Ply - PASSED

This design is valid until 4/7/2024

Level: Level





Page 8 of 8

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity 0.0 % 0.0 PLF Load Yield Limit per Foot 157.4 PLF Yield Limit per Fastener 78.7 lb. Yield Mode IV Edge Distance 1 1/2" Min. End Distance 3" Load Combination Duration Factor 1.00

> Manufacturer Info Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS соттесн



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0521-2897

Lot 3 Barbecue Church Rd.

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15725976 thru E15725983

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



May 13,2021

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Jo	ob	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
١.,			5	١.	١.	E15725976
Ju	0521-2897	EI1	Floor Supported Gable	1	1	Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 11:30:14 2021 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-IrZNXWV_o4UD5Gd5jl1rwO3UpTJyBDPuGA?jDizGwjd

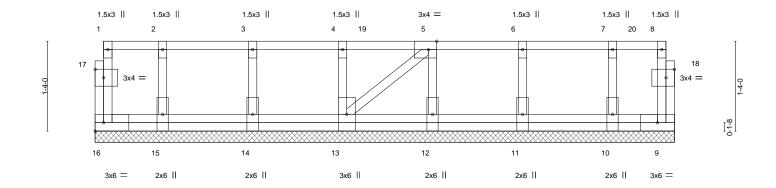
Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

0-1-8

Scale: 3/4"=1'



						0-1-0						
						8-7-0						1
Plate Offse	ets (X,Y)	[5:0-1-8,Edge], [17:0-1-8,6	0-1-8], [18:0-1	-8,0-1-8]								
				1								
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.08	Vert(LL)	n/a	` -	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.00	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	9	n/a	n/a		
BCDL	5.0	Code IRC2015/TP	12014	Matrix	<- Ρ						Weight: 54 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

8-7-0

LUMBER-

TOP CHORD 2x4 SP No.1(flat)

BOT CHORD 2x4 SP No.1(flat)

WEBS 2x4 SP No.3(flat)

OTHERS 2x4 SP No.3(flat)

REACTIONS. All bearings 8-7-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Plates checked for a plus or minus 1 degree rotation about its center.
- 2) Gable requires continuous bottom chord bearing.
- 3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 4) Gable studs spaced at 1-4-0 oc.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 9-16=-10, 1-8=-100

Concentrated Loads (lb) Vert: 3=-71 6=-71 19=-71 20=-77



May 13,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road

Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
10504 0007	СТО	Floor Owner and A Ook In			E15725977
J0521-2897	EIZ	Floor Supported Gable	1	1	Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 11:30:15 2021 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-n17mksWcZOc3jQCHHSY4Sbceitf5wgo2UqkGl8zGwjc

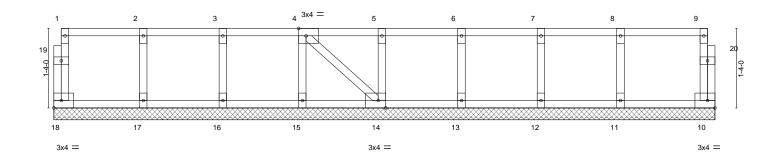
Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

0118

 0_{1}^{1} 8 Scale = 1:18.2



<u> </u>						11-1-0 11-1-0						
Plate Offs	sets (X,Y)	[4:0-1-8,Edge], [14:0-1-8	Edge]									
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	40.ó	Plate Grip DOL	1.00	TC	0.07	Vert(LL)	n/a	` -	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.01	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	10	n/a	n/a		
BCDL	5.0	Code IRC2015/TF	PI2014	Matri	x-S						Weight: 52 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1(flat)

BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)

OTHERS 2x4 SP No.3(flat)

REACTIONS. All bearings 11-1-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 18, 10, 17, 16, 15, 14, 13, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



May 13,2021





Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
J0521-2897	ET2	Floor Supported Gable	1	1	E15725978
30321-2097	LIS	Thor Supported Gable	!	'	Job Reference (optional)

2 1.5x3 II

Comtech, Inc. Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 11:30:16 2021 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-FDh8yCXEKhkwLZmTqA3J?p8qqH?Nf79BjUUplbzGwjb

Structural wood sheathing directly applied or 3-5-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

3x4 = 0-1-8 4 1.5x3 || 3

Scale = 1:9.4

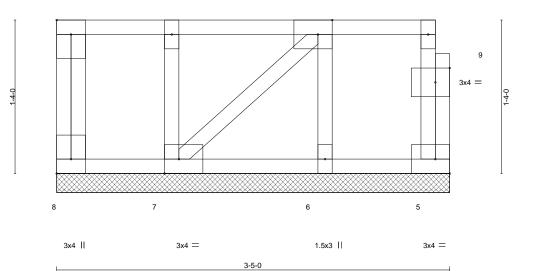


Plate Offs	Plate Offsets (X,Y) [1:Edge,0-1-8], [3:0-1-8,Edge], [7:0-1-8,Edge], [8:Edge,0-1-8], [9:0-1-8,0-1-8]											
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.05	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.01	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	5	n/a	n/a		
BCDL	5.0	Code IRC2015/T	PI2014	Matri	x-P						Weight: 22 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1(flat)

BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.3(flat) WFBS

2x4 SP No.3(flat) OTHERS

All bearings 3-5-0. (lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1 3x4 II

- 1) Plates checked for a plus or minus 1 degree rotation about its center.
- 2) Gable requires continuous bottom chord bearing.
 3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 4) Gable studs spaced at 1-4-0 oc.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.



May 13,2021



Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
J0521-2897	E1	Floor	1	1	E15725979
30321-2097		11001	-	'	Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 11:30:18 2021 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-BcpuNuYUsJ_eatwsyb5n4EE2Z4YI7vIUAozwMTzGwjZ

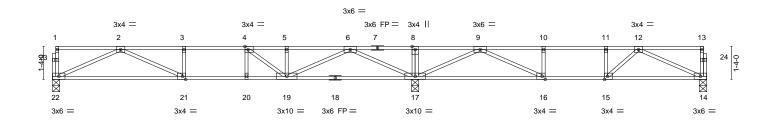
Structural wood sheathing directly applied or 6-0-0 oc purlins,

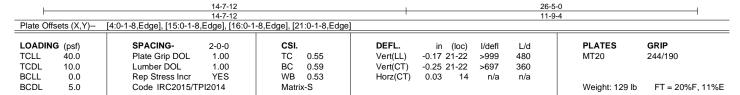
Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

except end verticals.

6-0-0 oc bracing: 17-19,16-17.







BOT CHORD

 LUMBER BRACING

 TOP CHORD
 2x4 SP No.1(flat)
 TOP CHORD

TOP CHORD 2x4 SP No.1(flat)
BOT CHORD 2x4 SP No.1(flat)
WEBS 2x4 SP No.3(flat)

WEBS 2x4 SP No.3(flat)

(size) 22=0-3-8, 17=0-3-8, 14=0-3-8 Max Grav 22=728(LC 10), 17=1669(LC 1), 14=562(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1987/0, 3-4=-1987/0, 4-5=-1740/0, 5-6=-1740/0, 6-8=0/1282, 8-9=0/1282,

9-10=-1183/0, 10-11=-1183/0, 11-12=-1183/0

BOT CHORD 21-22=0/1314, 20-21=0/1987, 19-20=0/1987, 17-19=-191/818, 16-17=-366/574,

15-16=0/1183, 14-15=0/954

WEBS 8-17=-284/0, 2-22=-1440/0, 2-21=0/745, 3-21=-260/0, 6-17=-1781/0, 6-19=0/1122,

 $4\textbf{-}19\textbf{=-}646/0,\,9\textbf{-}17\textbf{=-}1465/0,\,9\textbf{-}16\textbf{=-}0/917,\,10\textbf{-}16\textbf{=-}329/0,\,12\textbf{-}14\textbf{=-}1044/0,\,12\textbf{-}15\textbf{=-}86/311$

NOTES-

REACTIONS.

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 1.5x3 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.



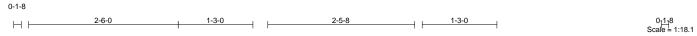
May 13,2021

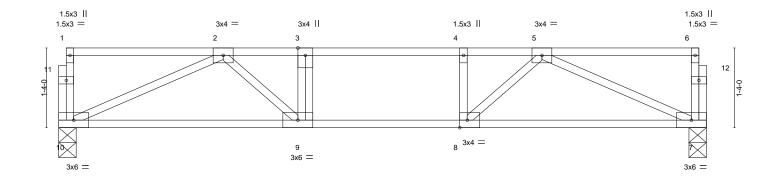


Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
J0521-2897	E2	Elect	_	_	E15725980
JU521-2897	FZ	Floor	5	'	Job Reference (optional)

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1		TC 0.42 Vert(LL) -0.08 9-10 >999 480 MT20 244/190 BC 0.35 Vert(CT) -0.11 9-10 >999 360 WB 0.30 Horz(CT) 0.02 7 n/a n/a Matrix-S Weight: 56 lb FT = 20%F 11%F		
Plate Offsets (X	') [8:0-1-8,Edge]			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.42	Vert(LL) -0.08 9-10 >999 480	MT20 244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.35	Vert(CT) -0.11 9-10 >999 360	
BCLL 0.0	Rep Stress Incr YES	WB 0.30	Horz(CT) 0.02 7 n/a n/a	
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S		Weight: 56 lb FT = 20%F, 11%E

BRACING-

10-10-0

LUMBER-

TOP CHORD 2x4 SP No.1(flat)

BOT CHORD 2x4 SP No.1(flat)

2x4 SP No.3(flat) WFBS

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 10=0-3-8, 7=0-3-8

Max Grav 10=576(LC 1), 7=576(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1234/0, 3-4=-1234/0, 4-5=-1234/0

BOT CHORD 9-10=0/981, 8-9=0/1234, 7-8=0/982

WEBS 2-10=-1073/0, 5-7=-1075/0, 5-8=0/485, 2-9=0/478, 3-9=-255/0, 4-8=-266/0

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



May 13,2021





Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
J0521-2897	F3	Floor	5	1	E15725981
00021-2007		1 1001		'	Job Reference (optional)

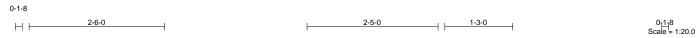
Comtech, Inc, Fayetteville, NC - 28314,

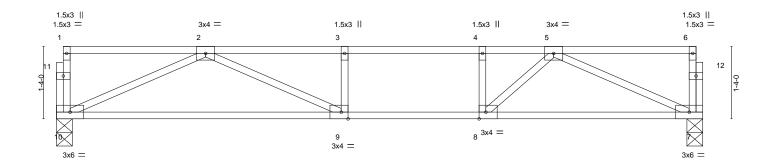
8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 11:30:20 2021 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-7?xfnaalOwEMpB4E3?7F9fJMxuEgbsHne6S1RMzGwjX

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





11-11-0 Plate Offsets (X,Y)--[8:0-1-8,Edge], [9:0-1-8,Edge] LOADING (psf) SPACING-CSI. DEFL. **PLATES** GRIP 2-0-0 I/defl L/d (loc) Plate Grip DOL 1.00 244/190 **TCLL** 40.0 TC 0.68 Vert(LL) -0.19 9-10 >740 480 MT20 TCDL Lumber DOL 10.0 1.00 ВС 0.56 Vert(CT) -0.29 9-10 >490 360 BCLL 0.0 Rep Stress Incr YES WB 0.34 Horz(CT) 0.02 n/a n/a BCDL Code IRC2015/TPI2014 Matrix-S Weight: 59 lb FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

2x4 SP No.3(flat) WFBS

REACTIONS. (size) 10=0-3-8, 7=0-3-8

Max Grav 10=635(LC 1), 7=635(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1508/0, 3-4=-1508/0, 4-5=-1508/0

BOT CHORD 9-10=0/1112, 8-9=0/1508, 7-8=0/1121

WEBS 2-10=-1219/0, 2-9=0/558, 5-7=-1228/0, 5-8=0/655, 4-8=-353/0

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



May 13,2021





Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.
J0521-2897	FA	Floor	7	1	E15725982
30321-2037	7	11001	l'		Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

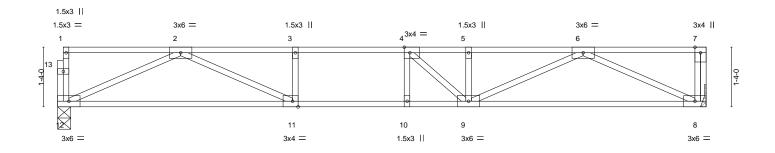
8.330 s Oct 7 2020 MiTek Industries, Inc. Thu May 13 11:30:21 2021 Page 1

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





L						14 0 0						
						14-6-0						1
Plate Of	fsets (X,Y)	[4:0-1-8,Edge], [11:0-1-8,	Edael									
		T	- 3-1								T.	
LOADIN	IG (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.61	Vert(LL)	-0.20	9-10	>838	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.84	Vert(CT)	-0.25	9-10	>684	360		
BCLL	0.0	Rep Stress Incr	YES	WB	0.46	Horz(CT)	0.03	8	n/a	n/a		
BCDL	5.0	Code IRC2015/TF	PI2014	Matri	x-S						Weight: 73 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

14-6-0

LUMBER-

TOP CHORD 2x4 SP No.1(flat)

2x4 SP No.1(flat) BOT CHORD

2x4 SP No.3(flat) WFBS

REACTIONS. (size) 12=0-3-8, 8=Mechanical

Max Grav 12=778(LC 1), 8=784(LC 1)

TOP CHORD 2-3=-2243/0, 3-4=-2243/0, 4-5=-2186/0, 5-6=-2186/0

BOT CHORD 11-12=0/1424, 10-11=0/2243, 9-10=0/2243, 8-9=0/1429

WEBS 2-12=-1561/0, 2-11=0/958, 3-11=-303/0, 6-8=-1573/0, 6-9=0/836, 5-9=-271/41,

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

4-9=-428/186

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.

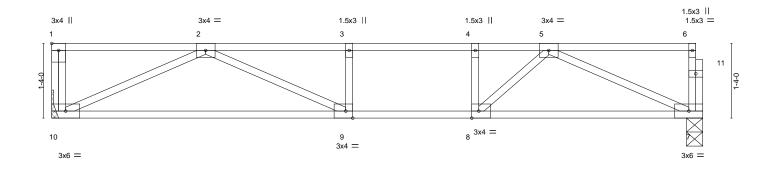


May 13,2021



Job	Truss	Truss Type	Qty	Ply	Lot 3 Barbecue Church Rd.	
						E15725983
J0521-2897	F5	Floor	7	1		
					Job Reference (optional)	
Comtech, Inc,	Fayetteville, NC - 28314,			8.330 s Oc	t 7 2020 MiTek Industries, Inc. Thu M	lay 13 11:30:22 2021 Page 1
			ID:1yUksKymp	k2404ufZYCr	xyoKUD-3N2PCFb?wXU43UEdBQAjF	4Ojkhxu3mx45Qx8VFzGwjV
	2-6-0		2-1-8	1.1	1-3-0	0 ₁ 1 ₁ 8

Scale = 1:19.4



11-7-8 [1:Edge,0-1-8], [8:0-1-8,Edge], [9:0-1-8,Edge] Plate Offsets (X,Y)--LOADING (psf) SPACING-CSI. DEFL. **PLATES** GRIP 2-0-0 (loc) I/defl L/d TCLL 1.00 TC 244/190 40.0 Plate Grip DOL 0.60 Vert(LL) -0.16 9-10 >846 480 MT20 TCDL ВС Vert(CT) 10.0 Lumber DOL 1.00 0.51 -0.25 9-10 >540 360 BCLL 0.0 Rep Stress Incr YES WB 0.33 Horz(CT) 0.02 n/a BCDL Code IRC2015/TPI2014 Matrix-S Weight: 58 lb FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.1(flat)

2x4 SP No.3(flat) WFBS

REACTIONS. (size) 10=Mechanical, 7=0-3-8 Max Grav 10=626(LC 1), 7=619(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1441/0, 3-4=-1441/0, 4-5=-1441/0 9-10=0/1081, 8-9=0/1441, 7-8=0/1087 TOP CHORD

BOT CHORD

WEBS 2-10=-1190/0, 2-9=0/515, 5-7=-1190/0, 5-8=0/606, 4-8=-323/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

May 13,2021



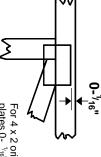


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. and fully embed teeth. Apply plates to both sides of truss Dimensions are in ft-in-sixteenths.



For 4×2 orientation, locate plates 0- $\frac{1}{16}$ from outside edge of truss.

ω

O

S

required direction of slots in This symbol indicates the

connector plates

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE



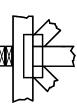
to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



output. Use T or I bracing Indicated by symbol shown and/or if indicated. by text in the bracing section of the

BEARING



number where bearings occur.
Min size shown is for crushing only reaction section indicates joint Indicates location where bearings (supports) occur. Icons vary but

Industry Standards:

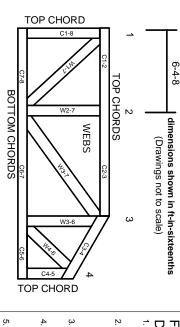
ANSI/TPI1:

National Design Specification for Metal Plate Connected Wood Truss Construction. Building Component Safety Information Design Standard for Bracing.

DSB-89:

Installing & Bracing of Metal Plate Connected Wood Trusses. Guide to Good Practice for Handling,

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values established by others. Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- 10. Camber is a non-structural consideration and is the camber for dead load deflection. responsibility of truss fabricator. General practice is to
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.