

RE: B0419-1974 Topsail A Trenco 818 Soundside Rd Edenton, NC 27932

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.1 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 13 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E12828932	a01	3/21/2019
2	E12828933	a04	3/21/2019
3	E12828934	a05	3/21/2019
4	E12828935	a06	3/21/2019
5	E12828936	a09	3/21/2019
6	E12828937	a10	3/21/2019
7	E12828938	b01	3/21/2019
8	E12828939	b02	3/21/2019
9	E12828940	b03	3/21/2019
10	E12828941	m01	3/21/2019
11	E12828942	m02	3/21/2019
12	E12828943	m03	3/21/2019
13	E12828944	v01	3/21/2019

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2019

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



Plate Offsets (X,Y) [2:0-0-0,0-1-6], [8:0-0-0,0-1-6]								
LOADING	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP				
TCLL	20.0	Plate Grip DOL 1.15	TC 0.58	Vert(LL) -0.17 10-12 >999 360 MT20 244/190				
TCDL	10.0	Lumber DOL 1.15	BC 0.40	Vert(CT) -0.23 10-12 >999 240				
BCLL	0.0 *	Rep Stress Incr YES	WB 0.54	Horz(CT) 0.03 8 n/a n/a				
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.06 2-12 >999 240 Weight: 181 lb FT = 2	0%			

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x4 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x4 SP No.2 *Except*

7-10,3-12: 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (lb/size) 2=1268/0-5-8, 8=1268/0-5-8 Max Horz 2=287(LC 9) Max Uplift 2=-159(LC 10), 8=-159(LC 11) Max Grav 2=1336(LC 17), 8=1336(LC 18)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1832/513, 3-5=-1681/617, 5-7=-1681/617, 7-8=-1833/513

- BOT CHORD 2-12=-251/1590, 10-12=-9/1042, 8-10=-251/1398
- WEBS 5-10=-230/860, 7-10=-500/332, 5-12=-230/860, 3-12=-500/332

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 10-10-3, Exterior(2) 10-10-3 to 15-3-0, Interior(1) 19-7-13 to 26-11-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0

(1) This truss has been designed for a live load of 20.0ps) on the bottom chick between the bottom chord and any other members, with BCDL = 10.0psf.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=159. 8=159.



Structural wood sheathing directly applied or 3-11-6 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





NOTES-

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2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 10-10-3, Exterior(2) 10-10-3 to 15-3-0, Interior(1) 19-7-13 to 26-11-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0

between the bottom chord and any other members, with BCDL = 10.0psf.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=159. 8=159.



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LOADING (psf) TCLL 20.0 TCDL 10.0 PCLL 0.0 *	SPACING- 2-1-8 Plate Grip DOL 1.15 Lumber DOL 1.15 Bob Strass Lar	CSI. TC 0.75 BC 0.90	DEFL. in (loc) l/defl L/d Vert(LL) -0.23 8-10 >999 360 Vert(CT) -0.41 8-10 >569 240 Vert(CT) 0.01 - p/p p/p	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.16 8-10 >999 240	Weight: 156 lb FT = 20%

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LOWIDER	
TOP CHORD	2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.3 *Except*
	5-10,5-8: 2x4 SP No.2, 11-12: 2x6 SP No.1
WEDGE	

BRACING-TOP CHORD BOT CHORD WEBS

2-0-0 oc purlins (4-8-10 max.), except end verticals (Switched from sheeted: Spacing > 2-0-0). Rigid ceiling directly applied or 8-0-15 oc bracing. 2x4 SPF No.2 - 5-8 T-Brace: Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

Left: 2x4 SP No.3

REACTIONS. (lb/size) 8=1271/0-5-8, 2=1047/0-5-8 Max Horz 2=339(LC 10) Max Uplift 8=-241(LC 10), 2=-113(LC 10) Max Grav 8=1492(LC 17), 2=1088(LC 17)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1458/322, 3-5=-1318/450

BOT CHORD 2-10=-455/1223, 8-10=-134/417

3-10=-556/367, 5-10=-395/1396, 5-8=-994/324 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 10-10-3, Exterior(2) 10-10-3 to 15-3-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=241 2=113

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

7) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-64, 5-6=-64, 2-10=-21, 8-10=-81, 7-8=-21





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Plate Off	sets (X,Y)	[2:0-0-0,0-0-6]		1		1						
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.57	Vert(LL)	-0.16	8-10	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.41	Vert(CT)	-0.22	8-10	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.60	Horz(CT)	0.01	8	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matrix	k-S	Wind(LL)	0.05	2-10	>999	240	Weight: 137 lb	FT = 20%
LUMBER	र-	·				BRACING-					·	
TOP CH	ORD 2x4 SF	P No.1				TOP CHOP	RD.	Structu	ral wood	sheathing di	rectly applied or 5-7-3 of	oc purlins,

BOT CHORD

WEBS

except end verticals.

1 Row at midpt

Rigid ceiling directly applied or 10-0-0 oc bracing.

5-8

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x4 SP No.3 *Except*

 5-10,5-8: 2x4 SP No.2

WEDGE

Left: 2x4 SP No.3

REACTIONS. (lb/size) 8=784/0-5-8, 2=848/0-5-8 Max Horz 2=319(LC 10) Max Uplift 8=-153(LC 10), 2=-82(LC 10) Max Grav 8=980(LC 17), 2=883(LC 17)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1076/205, 3-5=-948/326

BOT CHORD 2-10=-349/914, 8-10=-100/314

WEBS 3-10=-541/351, 5-10=-250/947, 5-8=-766/249

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 10-10-3, Exterior(2) 10-10-3 to 15-3-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 8=153.



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Fiale Olisels (A, I)-	[11.0-2-0,Euge]			
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. TC 0.08 BC 0.02	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) 0.00 1 n/r 120 MT20 244/190 Vert(CT) -0.00 1 n/r 120 MT20 244/190	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Weight: 177 lb FT = 20%	
LUMBER- TOP CHORD 2x4	SP No.1		BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.	

BOT CHORD

WEBS

TOP CHORD	2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.3
OTHERS	2x4 SP No 3

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SPF No.2 - 10-19, 9-20, 12-17, 13-16 T-Brace: Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance.

Brace must cover 90% of web length.

REACTIONS. All bearings 20-0-0.

- Max Horz 2=470(LC 10) (lb) -
 - Max Uplift All uplift 100 lb or less at joint(s) 15, 19, 2 except 20=-124(LC 10),
 - 21=-104(LC 10), 22=-107(LC 10), 23=-107(LC 10), 24=-104(LC 10), 25=-171(LC 10), 16=-130(LC 11)
 - Max Grav All reactions 250 lb or less at joint(s) 15, 20, 21, 22, 23, 24, 25, 17, 16 except 19=283(LC 17), 2=270(LC 10)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- 2-3=-531/360, 3-4=-408/261, 4-5=-325/216 TOP CHORD

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 10-10-3, Corner(3) 10-10-3 to 15-3-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 19, 2 except (jt=lb) 20=124, 21=104, 22=107, 23=107, 24=104, 25=171, 16=130.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



818 Soundside Road Edenton, NC 27932

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GI munum March 21,2019

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2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Bearing at joint(s) 2, 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-2-12 to 4-7-9, Interior(1) 4-7-9 to 5-8-3, Exterior(2) 5-8-3 to 10-1-0, Interior(1) 14-5-13 to 15-6-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Bearing at joint(s) 1, 3 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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Vert: 1-4=-60, 2-5=-115(F=-95)





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	<mark>0-4-0</mark> 		6-6-0 6-2-0				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.64 BC 0.82 WB 0.00 Matrix-P	DEFL. Vert(LL) - Vert(CT) - Horz(CT) Wind(LL)	in (loc) 0.08 2-4 0.16 2-4 0.00 4 0.19 2-4	l/defl L/d >946 360 >473 240 n/a n/a >385 240	PLATES MT20 Weight: 22 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1WEBS2x4 SP No.3

BRACING-TOP CHORD

RD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=247/0-1-8, 2=280/0-3-0 Max Horz 2=62(LC 6) Max Uplift 4=-128(LC 6), 2=-136(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=128, 2=136.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

C Variation MALLIN HILL SEAL 036322 G١ munum March 21,2019

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BRACING-

TOP CHORD

BOT CHORD

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LUMBER-
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TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. All bearings 2-5-0.

(lb) - Max Horz 2=37(LC 6)

Max Uplift All uplift 100 lb or less at joint(s) 3, 2 Max Grav All reactions 250 lb or less at joint(s) 3, 3, 2, 4

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 2-4-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-281/189, 8-9=-265/189

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0

- between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 13, 12 except (jt=lb) 14=144, 15=150, 11=147, 10=150.



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