

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 19620-19620A Wilmington C Vault

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I35701913 thru I35701919

My license renewal date for the state of North Carolina is December 31, 2019.

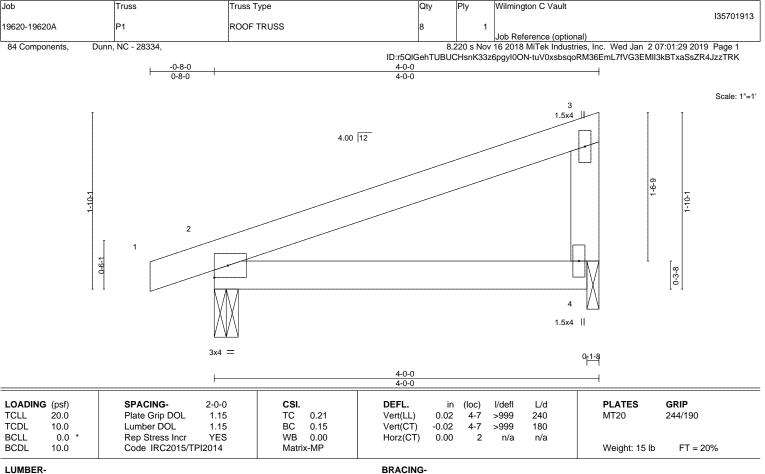
North Carolina COA: C-0844



January 2,2019

Sevier, Scott

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.



TOP CHORD

BOT CHORD

TOP CHORD BOT CHORD

2x4 SP No.2 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. 2=198/0-3-0, 4=151/0-1-8 (lb/size) Max Horz 2=48(LC 6)

Max Uplift 2=-51(LC 6), 4=-47(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 2 and 47 lb uplift at joint 4.



Structural wood sheathing directly applied or 4-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing

except end verticals.

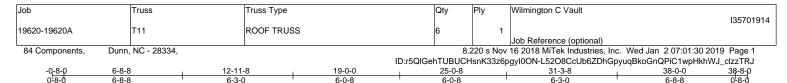
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

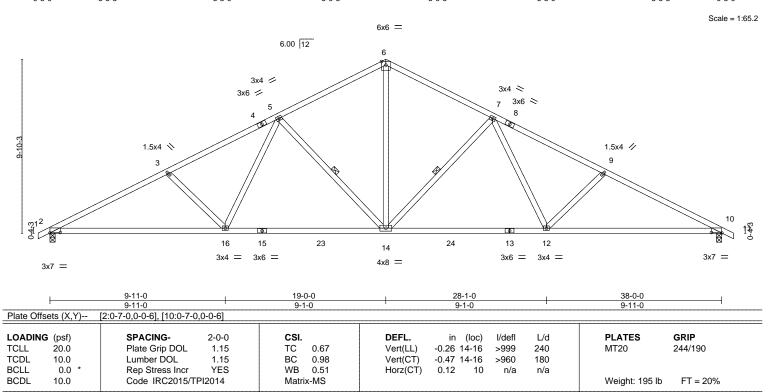
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Sefety Information, available from Truss Plate pictities 218 N. Les Street, Suite 312, Alexanderia, VA 22314. fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Qua
Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.







BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.1 *Except*

13-15: 2x4 SP No.2

WEBS 2x4 SP No.3

(lb/size) 2=1560/0-3-8, 10=1560/0-3-8 REACTIONS.

Max Horz 2=-128(LC 11)

Max Uplift 2=-50(LC 10), 10=-50(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 $2\hbox{-}3\hbox{--}2863/385,\ 3\hbox{-}5\hbox{--}2594/365,\ 5\hbox{-}6\hbox{--}1804/336,\ 6\hbox{-}7\hbox{--}1804/336,\ 7\hbox{-}9\hbox{--}2594/365,}$ TOP CHORD

9-10=-2863/385

BOT CHORD $2\text{-}16\text{=-}253/2517,\ 14\text{-}16\text{=-}127/2014,\ 12\text{-}14\text{=-}127/2014,\ 10\text{-}12\text{=-}253/2517}$

WEBS 3-16=-391/172, 5-16=-11/572, 5-14=-713/190, 6-14=-172/1223, 7-14=-713/190,

7-12=-11/572, 9-12=-391/172

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 2 and 50 lb uplift at joint 10.



Structural wood sheathing directly applied or 2-6-6 oc purlins.

5-14, 7-14

Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Row at midpt

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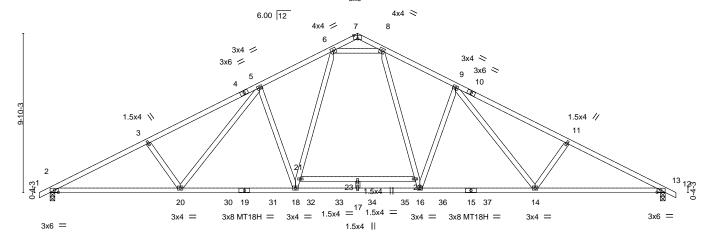
Job Truss Truss Type Wilmington C Vault 135701915 19620-19620A T11A ROOF TRUSS Job Reference (optional) 84 Components, Dunn, NC - 28334,

8.220 s Nov 16 2018 MiTek Industries, Inc. Wed Jan 2 07:01:32 2019 Page 1 ID:r5QlGehTUBUCHsnK33z6pgyl0ON-HTA8Ztdl7jpxwZzL0FDCthskcWtlOhv19qo5gezzTRH

Structural wood sheathing directly applied or 2-6-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

21-0-6 19-0-0 20-6-0 22-10-2 25-0-10 1-6-0 1-6-0 1-9-12 2-2-8 12-11-7 17-6-0 31-11-3 6-0-13 3x6 = 0.6-6Scale = 1:71.2



 	8-0-7	15-1-14 7-1-7	19-0-		29-11-9	38-0-0	
Plate Offsets (X,Y)	8-0-7 [2:0-2-12,0-1-8], [7:0-3-0,E		3-10-	2 3-10-2	7-1-7	8-0-7	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI2	2-0-0 CSI. 1.15 TC 1.15 BC NO WB 2014 Matri	0.81 0.96 0.63 x-MS	Vert(CT) -	in (loc) I/defl L/d 0.31 14-16 >999 240 0.48 14-16 >942 180 0.13 12 n/a n/a	PLATES GRIP MT20 244/190 MT18H 244/190 Weight: 218 lb FT = 2	

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 *Except*

21-22: 2x4 SP No.2

REACTIONS. (lb/size) 2=1560/0-3-8, 12=1560/0-3-8

Max Horz 2=-128(LC 11)

Max Uplift 2=-50(LC 10), 12=-50(LC 11) Max Grav 2=1591(LC 27), 12=1593(LC 28)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2993/386, 3-5=-2812/401, 5-6=-2185/391, 8-9=-2192/391, 9-11=-2818/401,

11-12=-2999/386

BOT CHORD 2-20=-263/2641, 18-20=-125/2079, 17-18=-36/1697, 16-17=-36/1697, 14-16=-125/2085,

> 3-20=-377/174, 5-20=-68/601, 5-18=-678/201, 9-16=-678/201, 9-14=-68/601, 11-14=-376/174, 8-22=-107/831, 16-22=-111/778, 18-21=-111/774, 6-21=-107/791,

6-8=-1617/344

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 2 and 50 lb uplift at ioint 12.
- 7) Load case(s) 25, 26, 27, 28 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss
- 8) MULTIPLE LOADCASES This design is the composite result of multiple load cases.
- 9) User moving load cases exist: Review the load cases for details.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard Except:

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

January 2,2019

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	Wilmington C Vault	
						135701915
19620-19620A	T11A	ROOF TRUSS	6	1		
					Job Reference (optional)	

84 Components,

Dunn, NC - 28334,

8.220 s Nov 16 2018 MiTek Industries, Inc. Wed Jan 2 07:01:32 2019 Page 2 ID:r5QlGehTUBUCHsnK33z6pgyl0ON-HTA8Ztdl7jpxwZzL0FDCthskcWtlOhv19qo5gezzTRH

LOAD CASE(S) Standard Except:

Uniform Loads (plf)

Vert: 1-7=-60, 7-13=-60, 24-27=-20

25) User defined: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-7=-60(F), 7-13=-60(F), 24-27=-20(F)

26) 1st User Defined Moving Load - User defined: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-7=-60(F), 7-13=-60(F), 24-32=-20(F), 32-33=-50(F=-20), 27-33=-20(F)

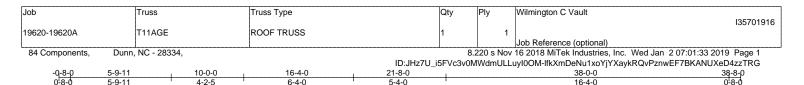
27) 2nd User Defined Moving Load - User defined: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-7=-60(F), 7-13=-60(F), 24-33=-20(F), 33-34=-50(F=-20), 27-34=-20(F)

28) 3rd User Defined Moving Load - User defined: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

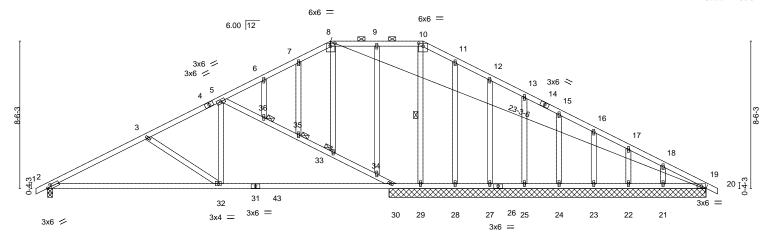
Vert: 1-7=-60(F), 7-13=-60(F), 24-34=-20(F), 34-35=-50(F=-20), 27-35=-20(F)





5-4-0

Scale = 1:66.5



	10-0-0	ı	6-4-0	l l	5-4-0			16-4-0	0	
ets (X,Y)	[2:0-2-9,0-1-8], [8:0-3-0,0	0-2-0], [10:0-3-	-0,0-2-0]							
(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
20.0	Plate Grip DOL	1.15	TC	0.53	Vert(LL)	-0.17 32-39	>999	240	MT20	244/190
10.0	Lumber DOL	1.15	BC	0.95	Vert(CT)	-0.38 32-39	>638	180		
0.0 *	Rep Stress Incr	YES	WB	0.42	Horz(CT)	0.06 40	n/a	n/a		
10.0	Code IRC2015/TI	PI2014	Matrix-	MS					Weight: 236 lb	FT = 20%
	(psf) 20.0 10.0 0.0 *	ets (X,Y) [2:0-2-9,0-1-8], [8:0-3-0,0] 6 (psf) SPACING- 20.0 Plate Grip DOL 10.0 Lumber DOL 0.0 * Rep Stress Incr	ets (X,Y) [2:0-2-9,0-1-8], [8:0-3-0,0-2-0], [10:0-3-6 (psf)	ets (X,Y) [2:0-2-9,0-1-8], [8:0-3-0,0-2-0], [10:0-3-0,0-2-0] 6 (psf) SPACING- 2-0-0 CSI. 20.0 Plate Grip DOL 1.15 TC 10.0 Lumber DOL 1.15 BC 0.0 * Rep Stress Incr YES WB	ets (X,Y) [2:0-2-9,0-1-8], [8:0-3-0,0-2-0], [10:0-3-0,0-2-0] 5 (psf) SPACING- 2-0-0 CSI. 20.0 Plate Grip DOL 1.15 TC 0.53 10.0 Lumber DOL 1.15 BC 0.95 0.0 * Rep Stress Incr YES WB 0.42	ets (X,Y) [2:0-2-9,0-1-8], [8:0-3-0,0-2-0], [10:0-3-0,0-2-0] 5 (psf) SPACING- 2-0-0 CSI. DEFL. 20.0 Plate Grip DOL 1.15 TC 0.53 Vert(LL) 10.0 Lumber DOL 1.15 BC 0.95 Vert(CT) 0.0 * Rep Stress Incr YES WB 0.42 Horz(CT)	ets (X,Y) [2:0-2-9,0-1-8], [8:0-3-0,0-2-0], [10:0-3-0,0-2-0] 6 (psf) SPACING- 2-0-0 CSI. DEFL. in (loc) 20.0 Plate Grip DOL 1.15 TC 0.53 Vert(LL) -0.17 32-39 10.0 Lumber DOL 1.15 BC 0.95 Vert(CT) -0.38 32-39 0.0 * Rep Stress Incr YES WB 0.42 Horz(CT) 0.06 40	ets (X,Y) [2:0-2-9,0-1-8], [8:0-3-0,0-2-0], [10:0-3-0,0-2-0] 5 (psf) SPACING- 2-0-0 CSI. DEFL. in (loc) I/defl 20.0 Plate Grip DOL 1.15 TC 0.53 Vert(LL) -0.17 32-39 >999 10.0 Lumber DOL 1.15 BC 0.95 Vert(CT) -0.38 32-39 >638 0.0 * Rep Stress Incr YES WB 0.42 Horz(CT) 0.06 40 n/a	ets (X,Y) [2:0-2-9,0-1-8], [8:0-3-0,0-2-0], [10:0-3-0,0-2-0] 6 (psf)	ets (X,Y) [2:0-2-9,0-1-8], [8:0-3-0,0-2-0], [10:0-3-0,0-2-0] 5 (psf)

21-8-0

LUMBER-

10-0-0

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS

2x4 SP No.3 *Except* 5-30: 2x4 SP No.2

5-9-11

4-2-5

6-4-0

16-4-0

BRACING-TOP CHORD

Structural wood sheathing directly applied or 3-6-12 oc purlins,

2-0-0 oc purlins (6-0-0 max.): 8-10.

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

38-0-0

16-4-0

2-2-0 oc bracing: 2-32,30-32. 10-29

WEBS JOINTS

BOT CHORD

1 Row at midpt 1 Brace at Jt(s): 33, 35, 36

REACTIONS. All bearings 18-3-8 except (jt=length) 2=0-3-8.

(lb) -Max Horz 2=111(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 28, 27, 25, 24, 23, 22, 21

except 29=-233(LC 3)

Max Grav All reactions 250 lb or less at joint(s) 29, 28, 27, 25, 24, 23, 22, 21

except 2=1050(LC 21), 19=399(LC 1), 30=828(LC 3), 19=399(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

2-3=-1769/330, 3-5=-1487/290, 5-6=-670/236, 6-7=-606/251, 7-8=-586/278,

8-9=-525/272, 9-10=-525/272, 10-11=-593/287, 11-12=-616/255, 12-13=-611/216, 13-15=-612/179, 15-16=-611/142, 16-17=-611/113, 17-18=-613/103, 18-19=-600/95

2-32=-219/1547, 30-32=-129/1284, 29-30=-66/525, 28-29=-65/523, 27-28=-65/523,

25-27=-65/523, 24-25=-65/523, 23-24=-65/523, 22-23=-65/523, 21-22=-65/523,

19-21=-65/523

5-36=-837/133, 35-36=-845/138, 33-35=-873/154, 33-34=-828/129, 30-34=-905/150,

5-32=0/513, 3-32=-316/127

NOTES-

WFBS

BOT CHORD

1) Unbalanced roof live loads have been considered for this design.

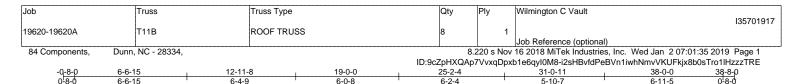
- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 19, 28, 27, 25, 24, 23, 22, 21, 19 except (jt=lb) 29=233.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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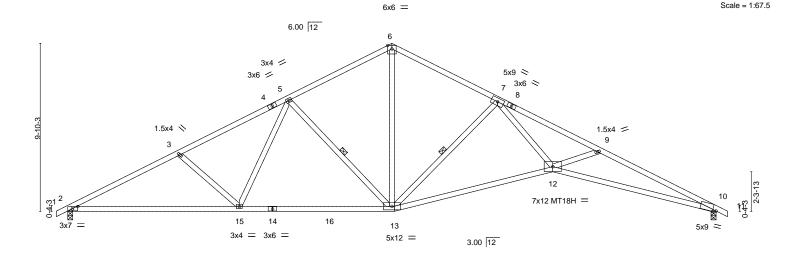


Plate Offsets (X,Y) [2	2:0-3-12,0-1-8], [10:0-2-6,Edge]	9-1-1	3-3-0	9-0-14
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.82 BC 0.86 WB 0.68 Matrix-S	DEFL. in (loc) l/defl L/d Vert(LL) -0.38 13-15 >999 240 Vert(CT) -0.87 12-13 >522 180 Horz(CT) 0.35 10 n/a n/a	PLATES GRIP MT20 244/190 MT18H 244/190 Weight: 192 lb FT = 20%

BRACING-TOP CHORD

BOT CHORD

WEBS

28-5-2

1 Row at midpt

Structural wood sheathing directly applied or 2-2-0 oc purlins.

5-13, 7-13

Rigid ceiling directly applied or 9-6-4 oc bracing.

19-1-12

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

1-4,8-11: 2x4 SP DSS

BOT CHORD 2x4 SP No.1 *Except*

10-12: 2x4 SP DSS, 13-14: 2x4 SP No.2

WEBS 2x4 SP No.3 *Except* 7-12: 2x4 SP No.2

REACTIONS. (lb/size) 2=1557/0-3-8, 10=1557/0-3-8

Max Horz 2=-129(LC 11)

Max Uplift 2=-52(LC 10), 10=-52(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2821/381, 3-5=-2535/351, 5-6=-1764/334, 6-7=-1785/336, 7-9=-4821/517,

9-10=-5223/628

BOT CHORD 2-15=-255/2458, 13-15=-121/1979, 12-13=-189/2683, 10-12=-498/4739 **WEBS** 3-15=-372/183, 5-15=0/566, 5-13=-709/179, 6-13=-167/1167, 7-13=-1556/248,

7-12=-159/2687, 9-12=-376/215

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.



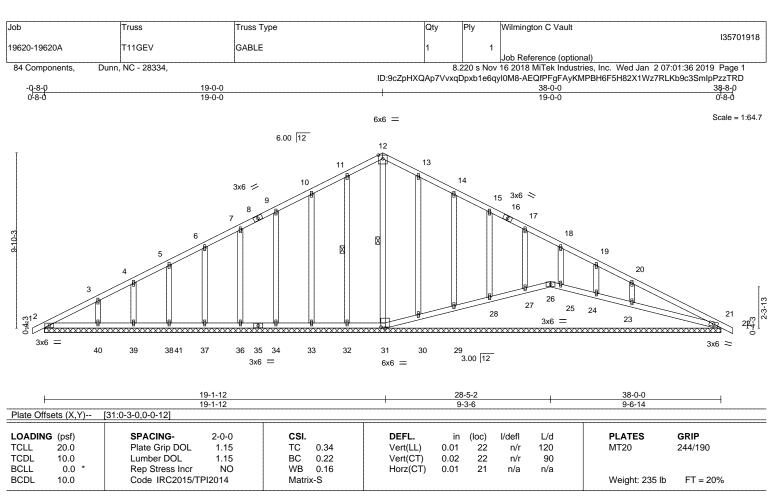
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Sefety Information, available from Truss Plate pictities 218 N. Les Street, Suite 312, Alexanderia, VA 22314. fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Qua
Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD **OTHERS** 2x4 SP No.3 **BRACING-**

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 25-26,23-24.

WEBS

1 Row at midpt

REACTIONS. All bearings 38-0-0.

(lb) -Max Horz 2=-129(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 26, 21, 32, 33, 34, 36, 37, 38, 39, 40, 30, 29, 28, 27, 25,

24, 23

All reactions 250 lb or less at joint(s) 2, 31, 26, 21, 32, 33, 34, 36, 37, 38, 39, 40, 30, 29, 28, Max Grav

27, 25, 24 except 23=415(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WFBS 20-23=-293/139

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 26, 21, 32, 33, 34, 36, 37, 38, 39, 40, 30, 29, 28, 27, 25, 24, 23.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 26, 30, 29, 28, 27, 25, 24, 23.

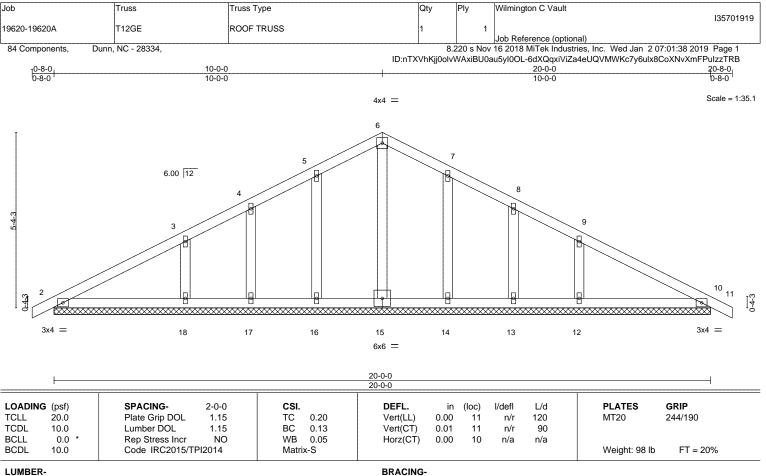


MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.2 BOT CHORD **OTHERS** 2x4 SP No.3

REACTIONS. All bearings 20-0-0.

(lb) -Max Horz 2=-70(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 16, 17, 18, 14, 13, 12, 10

All reactions 250 lb or less at joint(s) 2, 15, 16, 17, 14, 13, 10 except 18=320(LC 21), 12=320(LC Max Grav

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 16, 17, 18, 14, 13, 12, 10.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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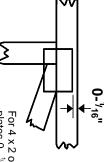


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- ¹/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

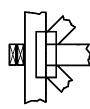
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



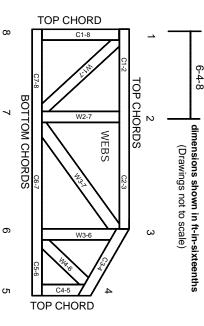
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only

Industry Standards:

ANSI/TPI1: DSB-89:

National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.
Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
 Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves
- may require bracing, or alternative Tor I bracing should be considered.

 Never exceed the design loading shown and never
- stack materials on inadequately braced trusses.

 Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.

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- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

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- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Do not cut or alter truss member or plate without prior

15. Connections not shown are the responsibility of others.

- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.