

T	ROORUS	DF 8 SES	<b>Te</b> & FL & B		∎ ≀ ∕\S				
Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444									
Bearing deeme require attache Code r founda require but noi profess suppor those s registe design exceed	g reaction d to comp ments. Ti d Tables equireme tion size d to supp d to supp d to supp d greater t sional sha t system specified the supp 1 5000#.	hs less the oly with the contra ( derivec nts ) to d and num bort react than 1500 all be reta for any r in the att or syste	han or equ he prescr ictor shall I from the letermine ber of woo ber of woo b	al to 3000 iptive Coc refer to t prescript the minim od studs ter than 3 istered de esign the easign the nat exceer oles. A ill be retal reactions	D# are le he ive num 0000# osign ds ined to that				
		Davi	d Lar	ndry					
	AD CH/ (BASED NEER OF DA	ART FO	OR JA( As recession recovered	CK STU 0 & (6)) 9 8 EA END	DS				
2 3 4 1700 3400 5100 6800 8500 10200 11900 13600	80 F 1 2 3 4 5 6 7 8	255x 5100 765x 1020 1275	Solution         Solution           Solution         Solution<	NOLLY 24 (N) 340 1020 1360	001 SQUES 0.057 001 SQUES 0.057 00 1 2 30 00 3 00 4 00 5				
UNTY Harnett	DRESS Spring Hill Church Rd.	DEL Roof	TE REV. 04/21/21	AWN BY David Landry	LESMAN Lenny Norris				
Weaver Development CO	Lot 4 Atkins Farm Estates AD	Halifax / 3GRF, 4BR	Seal Date DA	DR	J0421-2508 SA				
BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #				
THIS IS These the component design a See indi- identified designe permanent for the o	A TRUSS russes are ents to b at the spe ividual de d on the r is respo ent bracin overall str	PLACEM e designé e incorpo cification sign she placemen onsible fo ng of the ructure. T	MENT DIAG ed as indi- prated into of the bu ets for each the drawing r tempora roof and f The design	GRAM ON vidual bui the build uilding de ch truss c the build ry and floor syste of the tr	LY. Iding signer. Iesign Iding em and uss				

support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



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TI R	RO RUS teilly R Favet	OF & SES	& FL & B	OOF EAN ial Par 28309	₹ //S <sup>∙k</sup>				
	Phon Fax:	e: (91) : (910)	0) 864 864-4	-8787 1444					
Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables ( derived from the prescriptive Code requirements ) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.									
LO	AD CH	Davi ART FO	d Lar	ndry CKISTU	IDS				
NUN	(BASED NBER OF DA	CN TABL	és rooze() Réquirée Voirder 8 - «	() & (b)) ) & EA END	of 8 a				
20 20 20 20 20 20 20 20 20 20	939001474(2) 1 2 3 4 5 6 7 8	2550 5100 7650 10200 15300	1 1 1 1 1 1 1 1 1 1 1 1 1 1	NOTEN A NOTENA A NOTEN	1014-0014 (F) 14-9014-90-24 1014-00-24 1010-00-1 100-00-1 100-00-5 100				
	Church Rd.			,	S				
Harnett	Spring Hill C	Roof	04/21/21	David Landr	Lenny Norris				
COUNTY	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALESMAN				
Weaver Development	Lot 4 Atkins Farm Estates	Halifax / 3GRF, 4BR	E Seal Date	Quote #	J0421-2508				
<b>BUILDER</b>	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #				
THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss									

support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.cor

	/	С	lient:	Weaver Dev	velopment			Date:	4/21/	2021				Pa	ge 1 of 8
	•Destars	P	roject:					Input by:	David	d Landry					
L L'	spesign	A	ddress:					Job Nam	e: Lot 4	Atkins Fa	rm Estates				
								Project #:	JU42	1-2508					
BM1	S-P-F #2	2.00	0" X 1	10.000	)" 2-P	'Iy - P	ASSE	=D	LOVOI. LO						
			1												,
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	C. C. Then		-	_	att The		-							IAM	9 1/4"
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1 SPI	F						2 SPF	1							
			5'6"					1						<b>∕</b> 3"	
/			5'6"					4							
Member I	nformation						React	ions I IN	ΡΔΤΤΓ		b (Unlift	)			
Туре:	Girder		Applicati	on:	Floor		Bra	Liv	e	Dead	Snow	,	Wind	Const	
Plies:	2		Design N	lethod:	ASD		1		0	919	919		0	0	
Moisture Co	ndition: Dry		Building	Code:	IBC/IRC 2015		2		0	919	919		0	0	
Deflection L	L: 480		Load Sha	aring:	No Not Ob a should										
Deflection I	L: 360 Normal		Деск:		Not Checked										
Temperature	e: Temp <= 100°F														
	•						Bearin	ngs							
							Beari	ng Lengt	h (	Cap. Re	act D/L lb	Total	Ld. Case	Ld. Co	mb.
							1 - SF	PF 3.500"		41%	919 / 919	1837	L	D+S	
Analysis R	esults						2 - SF	PF 3.500"		41%	919 / 919	1837	L	D+S	
Analysis	Actual L	ocation A	llowed	Capacity	Comb.	Case									
Moment	2122 ft <b>-l</b> b	2'9" 39	946 ft <b>-l</b> b	0.538 (54	%) D+S	L									
Unbraced	2122 ft <b>-l</b> b	2'9" 36	654 ft <b>-i</b> b	0.581 (58	%) D+S	L									
Shear	1169 <b>b</b>	1' 28	872 lb	0.407 (41	%) D+S	L									
LL Defl incl	h 0.018 (L/3452)	2'9" 0.	.126 (L/480)	0.140 (14	%) S	L									
TL Defl inc	h 0.035 (L/1726)	2'9" 0.	.168 (L/360)	0.210 (21	%) D+S	L									
Design No	otes	d Day pails	( 109,20) 0	+ 10" o o M	avimum and di	atonao nat									
to exceed	l plies using 2 rows of 10 I 6".	id Box nails	: (.128x3") a	t 12" o.c. M	aximum end di	stance not									
2 Refer to l	ast page of calculations t	for fastener	s required for	or specified	loads.										
4 Top loads	re designed to be suppo	ally by all pl	lies.	e only.											
5 Top brace	ed at bearings.														
6 Bottom bi 7 Lateral sl	raced at bearings. enderness ratio based o	n sinale plv	width.												
ID	Load Type	L	ocation 1	rib Width	Side	Dead 0.9	) Li	ve 1 Sno	ow 1.15	Wind	1.6 Cons	t. 1.25	Commen	ts	
1	Uniform				Тор	334 PLF	= 0	PLF :	334 PLF	0	PLF	0 PLF	A4		
									Manufac	turer Info		Co	omtech, Inc.		
													001 S. Reilly Road ayetteville, NC	l, Suite #639	
												28	314 0-864-TRUS		
													Loop		
					This	design is val	id unti <b>l</b> 2/26/	2023					con	пте	CH
Version 20.20.04	4 Powered by iStruct™												CDISS.		

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	-	Client:	Weaver Developr	nent		Date:	4/21/2021	Page 2 of 8
1		Project:				Input by:	David Landry	C C
	isDesign	Address:				Job Name:	Lot 4 Atkins Farm Estates	
I +						Project #:	J0421-2508	
DM4	S D E #2	2 000" V	10 000"	2 Dby	DASSE		evel: Level	
	3-F-F #Z	2.000 A	10.000	2-Fiy -	- PASSE	וט		
	•		•	•		1		$\overline{1}$
	•	÷	•	•	÷	/2"		N/N/I I
								Å Å   9 1/4
•	•	•	•	•	• -	<u> </u>		
						1 <del></del>		
1 S	SPF				2 SPF	1		
∤		5'6"				ł		
						<b>,</b>		1 10
1		5'6"				1		
Multi-Ph	v Analysis							
		(10) 5 "	(100 0)					
Fasten al	l plies using 2 rov	vs of 10d Box nails	(.128x3") at 12	" o.c Maxin	num end dis	tance no	t to exceed 6"	
Capacity		0.0 %						
Load Viold Limit n	or Foot	0.0 PLF						
Yield Limit p	er Fastener	78.7 lb						
Yield Mode		IV						
Edge Distan	ice	1 1/2"						
Min. End Dis	stance	3"						
Load Combi	ination	4.00						
Duration Fac	ctor	1.00						
						Г	Manufacturer Info	Comtech, Inc.
						F		1001 S. Relly Road, Suite #639 Fayetteville, NC
								USA 28314
								910-864-TRUS
								Comtech
1				This design i	is valid until 2/26/2	2023		

1	isDesign	Client: Projec Addre	Weaver t: ss:	Development		Da In Jo Pr	ate: put by: bb Name: oject #:	4/21/2021 David Landı Lot 4 Atkins J0421-2508	y Farm Estates		Page 3 (	of 8
BM2	Kerto-S LV	L 1.7	50" X 9.	250"	2-Ply -	PASSI	ED	evel: Level				
		1										
	·				•							9 1/4
	F End Grain	4'8"		2 SPF ENd G	rain						3 1/2"	
ł		4'8"										
Member I	nformation					Reaction	s UNP	ATTERNE	) lb (Uplift)	)		
Type: Plies: Moisture Co Deflection L Deflection T Importance: Temperature	Girder 2 ondition: Dry L: 480 L: 360 Normal	Aj Di Bi Lo Di	oplication: esign Method: uilding Code: oad Sharing: eck:	Floor ASD IBC/IRC 207 No Not Checker	15 d	Brg 1 2	Live 0 0	Dead 1526 1526	Snow 1510 1510	Wind 0 0	Const 0 0	
remperatur	e: 1emp <= 100 F					Bearing	5					
						Bearing 1 - SPF	Length 3.500"	Cap. 1 28%	React D/L lb 1526 / 1510	Total Ld. Ca 3036 L	D+S	
Analysis R Analysis Moment Unbraced Shear	Actual         Lc           2881 ft-lb         2881 ft-lb           1735 lb         1735 lb	cation Allow 2'4" 14423 2'4" 12555 1' 7943 I	ed Capao ft-lb 0.200 ft-lb 0.229 b 0.218	city Comb. (20%) D+S (23%) D+S (22%) D+S	Case L L L	Grain 2 - SPF End Grain	3.500"	28%	1526 / 1510	3036 L	D+S	
LL Defl inc	h 0.015 (L/3370) 2'	4 1/16" 0.105	(L/480) 0.140	(14%) S	L							
<ul> <li>Design Na</li> <li>1 Fasten al to exceed</li> <li>2 Refer to 1</li> <li>3 Girders a</li> <li>4 Top loads</li> <li>5 Top brace</li> <li>6 Bottom b</li> <li>7 Lateral sl</li> </ul>	I plies using 2 rows of 100 d 6". ast page of calculations for re designed to be suppor s must be supported equa ed at bearings. raced at bearings. enderness ratio based on	d Box nails (.12 or fasteners req ted on the botto Ily by all plies. single ply widtl	Bx3") at 12" o.c uired for specif m edge only.	. Maximum end	distance not							
ID 1	Load Type Uniform Self Weight	Locati	on Trib Wid	th Side Top	Dead 0.9 647 PLF 7 PLF	Live <sup>-</sup> 0 PLF	1 Snow <del>-</del> 64	/ 1.15 Wi 7 PLF	nd 1.6 Const 0 PLF	t. 1.25 Comm 0 PLF A3	nents	
Notes		chemicals		6, For	flat roofs provide p	roper drainage to	prevent	Manufacturer II	ſſo	Comtech, Inc. 1004 ⊂ Date.	Road Suite #830	
Calculated Structu structural adequad design criteria responsibility of th ensure the com application, and to Lumber	red Designs is responsible only of the y of this component based on the and loadings shown. It is the e customer and/or the contractor to conent. suitability of the intended verify the dimensions and loads.	Handling & Ins 1. LVL beams must 2. Refer to ma regarding inst fastening details approvals 3. Damaged Beam 4. Design assumes	tallation not be cut or drilled nufacturer's product allation requirement , beam strength value s must not be used top edge is laterally re	information s, multi-ply is, and code strained	ding		N 55 1 1 1 1 1	Metsä Wood 301 Merritt 7 Bu Norwalk, CT 068 800) 622-5850 vww.metsawoo CC-ES: ESR-38	ilding, 2nd Floor 351 d.com/us 333	Fayetteville, N USA 28314 910-864-TRUS	Road, Suite #639 C	
2. LVL not to be t Version 20.20.04	reated with fire retardant or corrosive	5. Provide lateral : lateral displacem	support at bearing po ent and rotation	ints to avoid Th	is design is valid	until 2/26/202	3				тесн	1

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	/	Client:	Weaver Developmen	t	Date:	4/21/2021	Page 4 of 8
1		Project:			Input by:	David Landry	
	isDesign	Address:			Job Nam	e: Lot 4 Atkins Farm Estates	
					Project #	: J0421-2508	
BM2	Kerto-S L	/L 1.750'	' X 9.250"	2-Ply -	- PASSED	Level: Level	
•	•	•	•	•	-		
					1/2		
					$\overline{\Sigma}$		9 1/-
•	•	•	•	•	<u> </u>		
	PE End Grain		2 SPE End	l Grain	Λ		, 
		4101	2 0.11 2.10	, orami			
		48					3 1/2
1		4'8"		1			
Multi-Plv	v Analysis						
Eacton all	plios using 2 rows	of 10d Boy pails (	128v2") at 12" o	c Mavimur	m and distance n	et to overad 6"	
Capacity	plies using 2 rows						
Load		0.0 PLF					
Yield Limit pe	er Foot	163.7 PLF					
Yield Limit pe	er Fastener	81.9 lb.					
Edge Distand	се	1 1/2"					
Min. End Dis	stance	3"					
Load Combin	nation	1.00					
Duration 1 ac		1.00					
				For Art of Contract		Manufacturer Info	Comtech, Inc.
Calculated Struct	tured Designs is responsible only of	the Handling & Installati	6. on	ponding	e proper grainage to prevent	Metsä Wood	1001 S. Reilly Road, Suite #639 Fayetteville, NC
structural adequi design criteria	and loadings shown. It is	the 1. LVL beams must not be c the 2. Refer to manufacture	ut or dri <b>ll</b> ed er's product information			301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-864-TRUS
ensure the con application and the	une customer and/or the contracto emponent suitability of the inten to verify the dimensions and loade	ded fastening details, beam	requirements, mu <b>l</b> ti-ply strength values, and code			(800) 622-5850	310-004-11/03
Lumber		approvals 3. Damaged Beams must no 4. Design assumes ton edge	ot be used a is laterally restrained			ICC-ES: ESR-3633	
<ol> <li>Dry service c</li> <li>LVL not to be</li> </ol>	conditions, unless noted otherwise e treated with fire retardant or corros	5. Provide lateral support lateral displacement and	at bearing points to avoid rotation	This design is ve	lid until 2/26/2023		соттесн
Varsian 20.20		•		inia uosiyii is Va		I	

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Version 20.20.044 Powered by iStruct<sup>T</sup>

	-	C	Client:	Weaver De	velopment		Da	ate:	4/21/20	021			Page 5 of 8
Tis	Design	F م	Project: Address:				Inj Jo	put by: bb Name	David I e: Lot 4 A	_andry .tkins Farm E:	states		
							Pr	roject #:	J0421-	2508			
GDH	Kerto-S L	VL 1.	<b>750''</b> 2	X 11.87	75'' 3-	Ply - P	ASSE	כ	Level: Lev	el			
									2				
		3											
					1								,
	- Then	1.164		·	all's m	•	·	in	The		·		
	d Oscia					•				• • •	•		
1 SPF End	d Grain										2 SPF End	Grain	
					18'10"								<b>5 1/4</b> "
1					18'10"							1	
Member In	formation						Reaction			NED Ib (I	Inlift)		
Туре:	Girder		Applicat	tion:	Floor		Brg	Live	e [	Dead	Snow	Wind	Const
Plies:	3		Design	Method:	ASD		1		0	2720	188	0	0
Moisture Cond	dition: Dry 480		Building	Code:	IBC/IRC 2015		2		0	2720	188	0	0
Deflection TL:	360		Deck:	lanng.	Not Checked								
Importance:	Normal												
Temperature:	Temp <= 100	)°F											
							Bearings	5					
							Bearing	Lengt	h Ca 1	ap. React D	/LIb To	otal Ld. Case	Ld. Comb.
							End	3.500	1	0 /0 2720	/ 100 2	900 L	D+3
Analysis Re	sults						Grain	2 500"	1	90/ 2720	1100 0	000 1	D+S
Analysis	Actual	Location A		Capacity	Comb.	Case	End	5.500		070 2720	/100 2	300 L	0.3
Moment	12191 π⊣p 13035 ftJb	9'5" Z 9'5" 1	3056 ft_lb	0.436 (44	%) D D+S	Uniform	Grain						
Unbraced	10000 1140	00 1	000011-10	(100%)	5.0	L							
Shear	2368 lb	1'2 5/8" 1	1970 <b> </b> b	0.198 (20	%) D	Uniform							
LL Defl inch	0.037 (L/6029)	9'5 1/16" 0	0.459 (L/480	0) 0.080 (8%	6) S	L							
TL Defl inch	0.565 (L/390)	9'5 1/16" 0	0.612 (L/360	)) 0.920 (92	%) D+S	L	4						
1 Easten all r	t <b>es</b> blies using 2 rows o	f 10d Box nails	s ( 128x3")	at 12" o.c. M	aximum end dis	stance not	4						
to exceed 6	5". 5".		.120,00 )	at 12 0.0. W									
2 Refer to las 3 Girders are	st page of calculation designed to be sur	ns for fastener	rs required bottom edu	for specified	loads.								
4 Top loads r	nust be supported e	equally by all p	lies.	jo onij.									
5 Top must b	e laterally braced a	t a maximum c	of 10'11 5/8'	" o.c.									
7 Lateral sler	iderness ratio base	d on single ply	width.										
ID	Load Type	L	ocation	Trib Width	Side	Dead 0.9	Live ?	1 Sno	ow 1.15	Wind 1.6	Const. 1.	25 Comment	S
1	Uniform				Тор	60 PLF	0 PLF	F	0 PLF	0 PLF	0 P	LF Wall	
2	Tie-In	0-0-0 to	18-10-0	1-0-0	Тор	20 PSF	0 PSF	F	20 PSF	0 PSF	0 P	SF Roof	
3	Uniform				Тор	195 PLF	0 PLF	F	0 PLF	0 PLF	0 P	LF B1GE	
	Self Weight					14 PLF							
									Manufact	urer Info		Comtech, Inc.	
Notes Calculated Structured	Designs is responsible only	chemical of the <b>Handling</b>	⊧s   & Installati	on	<ol> <li>For flat ponding</li> </ol>	t roofs provide p J	roper drainage to	prevent	Metsä Wo	od		1001 S. Reilly Road Fayetteville, NC	, Suite #639
design criteria and responsibility of the	of this component based of loadings shown. It is customer and/or the control	the 1 LVL beam the 2 Refer t	ms must not be c to manufacture	utordri <b>ll</b> ed er's product inf	ormation		301 Merritt 7 Building, 2nd Floor USA Norwalk, CT 06851 USA 910-964-TRUS						
ensure the compon application, and to ver	ent suitability of the inte ify the dimensions and loads	ended fastening approval	y msta∎ation ;details,beam s	strength values, a	nd code				(800) 622- www.mets	5850 awood.com/us			
Lumber 1. Dry service conditi	ions, unless noted otherwise	3. Damager 4. Design a 5. Provide	d Beams must no ssumes top edge lateral support	ot be used a is laterally restrain at bearing points	ted				ICC-ES: E	SR-3633		Incom	TOCUL
2. LVL not to be trea	ted with fire retardant or cor	rosive lateral dis	splacement and i	rotation	This c	design is va <b>l</b> id	until 2/26/2023	3				con	песн
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	/		Client:	Weaver Developn	nent	Date:	4/21/2021	Page 6 of 8
1	to Decentary		Project:			Input I	by: David Landry	
L 1	Ispesign		Address:			Job N	ame: Lot 4 Atkins Farm Estates	
	1/	1.1/1	4 7501	V 44 075	0 DL.		t #: J0421-2508	
GDH	Kerto-S	LVL	1.750"	X 11.875"	3-Ply	- PASSED		
								Ξ.
· ·	• • •	• •	• •	• •	• •	• • •	• • • •	
								<u>V</u>   11 7/8"
1 SPF	End Grain						2 SPF E	nd Grain
					18'10"			· · · · · · · · · · · · · · · · · · ·
1					18'10"			1
Multi-Ply	Analysis							
Fasten all	plies using 2 i	rows of	10d Box nails	s (.128x3") at 12'	' o.c Nail fro	om both sides. N	laximum end distance not t	to exceed
6"								
Capacity		0.0	% DI F					
Load Yield Limit pe	er Foot	163	B.7 PLF					
Yield Limit pe	er Fastener	81.	9 lb.					
Yield Mode Edge Distanc	e.	IV 1.1	/2"					
Min. End Dist	tance	3"	-					
Load Combin	nation tor	1.0	0					
	101	1.0	0					
							Manufacturer Info	Comtech, Inc.
Notes Calculated Struct	ured Designs is responsible	e on <b>l</b> y of the	chemicals Handling & Installa	ation	<ol><li>For flat roofs pro ponding</li></ol>	wide proper drainage to preve	Metsä Wood	1001 S. Reilly Road, Suite #639 Fayetteville, NC
structural adequa design criteria	acy of this component ba and loadings shown.	t is the	1. LVL beams must not b 2. Refer to manufact	e cut or dri <b>ll</b> ed turer's product information			301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-964-TRUS
ensure the con application, and to	use customer and/or the c mponent suitability of th overify the dimensions and	intended	regarding installatio fastening details, bea approvals	n requirements, multi-ply m strength values, and code			(800) 622-5850	
Lumber	anditione unless noted -#	nuice	<ol> <li>Damaged Beams mus</li> <li>Design assumes top e</li> </ol>	t not be used dge is laterally restrained			ICC-ES: ESR-3633	
2. LVL not to be	treated with fire retardant	or corrosive	5. Provide lateral suppo lateral displacement ar	rt at bearing points to avoid nd rotation	This design is	valid until 2/26/2023		соттесн
Version 20.20.0	044 Powered by iStru	ict™			-			CSDI

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-	-	Client: Weaver De	evelopment	Date:	4/21/2021	Page 7 of 8
Tie	sDesign	Project: Address:		Input by Job Nan	: David Landry ne: Lot 4 Atkins Farm Estates	
				Project #	#: J0421-2508	
GDH2	Kerto-S LVL	1.750" X 11.	875" 2-Ply	- PASSED		
	2		1			
•			-			M
	C. The co		atter a			11 7/8"
	End Grain				2 SPF End Grain	
/			9'10"			3 1/2"
∤			9'10"			4
Member In	nformation	Application	Floor	Reactions UN	NPATTERNED Ib (Uplift)	) Mind Const
Plies:	2	Design Method:	ASD	1 Erg	0 1653 1313	
Moisture Con	ndition: Dry	Building Code:	IBC/IRC 2015	2	0 1653 1313	0 0
Deflection TL	.: 360	Deck:	Not Checked			
Importance:	Normal					
lemperature:	: Iemp <= 100°⊦			Bearings		
				Bearing Leng	th Cap. React D/L lb	Total Ld. Case Ld. Comb.
				1 - SPF 3.500	)" 28% 1653 / 1313	2966 L D+S
Analysis Re	esults			Grain		
Analysis	Actual Locat	tion Allowed Capacit	y Comb. Cas	e 2 - SPF 3.500 End	)" 28% 1653 / 1313	2966 L D+S
Moment	6627 ft-lb 4	'11" 22897 ft-lb 0.289 (2 '11" 9857 ft-lb 0.672 (6	9%) D+S L 7%) D+S L	Grain		
Shear	2231 lb 8'7	3/8" 10197 lb 0.219 (2	2%) D+S L			
LL Defl inch	0.056 (L/2022) 4	'11" 0.234 (L/480) 0.240 (2	4%) S L			
TL Defl inch	0.126 (L/895) 4	'11" 0.312 (L/360) 0.400 (4	0%) D+S L			
Design No 1 Easten all	tes plies using 2 rows of 10d Br	ox nails (128x3") at 12" o.c. N	Aaximum end distance	not		
to exceed	6".	astonors required for specified	i loode			
3 Girders ar	e designed to be supported	on the bottom edge only.	loaus.			
4 Top loads 5 Top brace	must be supported equally d at bearings.	by all plies.				
6 Bottom bra	aced at bearings.	ala al width				
	Load Type	Location Trib Width	Side Dead	0.9 Live 1 Sr	now 1.15 Wind 1.6 Const	t. 1.25 Comments
1	Uniform		Тор 60	PLF 0 PLF	0 PLF 0 PLF	0 PLF Wall Above
2	Uniform		Тор 267	PLF 0 PLF	267 PLF 0 PLF	0 PLF G1
	Self Weight		9	PLF		
					Manufacturer Info	Comtech, Inc.
Notes Calculated Structure	d Designs is responsible only of the H	chemicals andling & Installation	<ol><li>For flat roofs proportion</li></ol>	ovide proper drainage to prevent	Metsä Wood	1001 S. Reilly Road, Suite #639 Fayetteville, NC
design criteria an responsibility of the	of this component based on the 1 nd loadings shown. It is the 2 customer and/or the contractor to	LVL beams must not be cut or drilled Refer to manufacturer's product i regarding installation requirements,	nformation multi-ply		301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	28314 910-864-TRUS
ensure the compo application, and to ve Lumber	onent suitability of the intended erify the dimensions and loads. 3.	fastening details, beam strength values, approvals Damaged Beams must not be used	and code		www.metsawood.com/us	
1. Dry service condi 2. LVL not to be tre	itions, unless noted otherwise 5 sated with fire retardant or corrosive	Design assumes top edge is laterally restra Provide lateral support at bearing point lateral displacement and rotation	ined to avoid This design in	valid until 2/26/2022		соттесн
	I Denne de la Comentation	,	i nis design is	valiu unui 2/26/2023	ļ	

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isDesign	Client: Project: Address:	Weaver Developmen	nt	Date: Input by: Job Name Proiect #:	4/21/2021 David Landry : Lot 4 Atkins Farm Estates J0421-2508	Page 8 of 8
GDH2 Kerto-S L	VL 1.750	" X 11.875"	2-Ply - F	PASSED	Level: Level	
•••	•	•	• •	•	•••	
• • •	•	•	• •	•	• • • • • • • • • • • • • • • • • • •	
		9'10"				3 1/2"
1		9'10"			1	
Multi-Ply Analysis Fasten all plies using 2 rows Capacity Load Yield Limit per Foot Yield Mode Edge Distance Min. End Distance Load Combination Duration Factor	of 10d Box nails 0.0 % 0.0 PLF 163.7 PLF 81.9 lb. IV 1 1/2" 3" 1.00	(.128x3") at 12" o	o.c Maximum e	end distance nc	ot to exceed 6"	
Notes Calculated Structured Designs is responsible only of t structural adequacy of this component based on t design criteria and loadings shown. It is t responsibility of the customer and/or the contractor	chemicals he <b>Handling &amp; Installat</b> he 1. LVL beams must not be he 2. Refer to manufactu to regarding installation	e tion cut or drilled rer's product information requirements multi-alu	<ol> <li>For flat roofs provide pro ponding</li> </ol>	per drainage to prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	Comtech, Inc. 1001 S. Relly Road, Suite #639 Fayetterville, NC USA 28314 910-864-TRUS
ensure the component suitability of the intend application, and to wrify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosi	ed fastening details, bear approvals 3. Damaged Beams must 4. Design assumes top ed 5. Provide lateral support lateral displacement and	a strength values, and code not be used ge is laterally restrained a thearing points to avoid d rotation	This design is va <b>l</b> id u	ntil 2/26/2023	(800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	соттесн
Version 20.20.044 Powered by iStruct™						CSDI

CSD 🗱



RE: J0421-2508 818 Soundside Rd Lot 4 Atkins Farm Estates Edenton, NC 27932 Site Information: Customer: Lot/Block: Project Name: J0421-2508 Model: Address: Subdivision: City: State: General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions): Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.3 Wind Code: ASCE 7-10 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf This package includes 29 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	E15058952	A1	4/21/2021	21	E15058972	G1-GR	4/21/2021
2	E15058953	A1GE	4/21/2021	22	E15058973	G1SG	4/21/2021
3	E15058954	A2	4/21/2021	23	E15058974	H1	4/21/2021
4	E15058955	A3	4/21/2021	24	E15058975	H1-GR	4/21/2021
5	E15058956	A3A	4/21/2021	25	E15058976	H1GE	4/21/2021
6	E15058957	A4	4/21/2021	26	E15058977	V1	4/21/2021
7	E15058958	A5	4/21/2021	27	E15058978	V2	4/21/2021
8	E15058959	A5-GR	4/21/2021	28	E15058979	V3	4/21/2021
9	E15058960	A6	4/21/2021	29	E15058980	V4	4/21/2021
10	E15058961	A6-GR	4/21/2021				
11	E15058962	A6GE	4/21/2021				
12	E15058963	B1	4/21/2021				
13	E15058964	B1GE	4/21/2021				
14	E15058965	B2	4/21/2021				
15	E15058966	C1GE	4/21/2021				
16	E15058967	D1	4/21/2021				
17	E15058968	D1GE	4/21/2021				
18	E15058969	D2	4/21/2021				
19	E15058970	D3	4/21/2021				

4/21/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

G1

Truss Design Engineer's Name: Gilbert, Eric

E15058971

20

My license renewal date for the state of North Carolina is December 31, 2021 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

Trenco



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818 Soundside Road Edenton, NC 27932



TOP CHORD 1-2=-1613/0, 2-3=-1516/0, 3-4=-1471/166, 4-5=-655/111, 5-6=-453/132, 6-7=-432/188, 7-8=-344/181, 8-9=-398/132, 9-10=-400/114, 10-11=-1295/156, 11-12=-1927/44, 12-14=-1885/9, 14-15=-1742/0, 15-16=-2233/131, 16-17=-2288/79, 17-18=-2455/0, 1-27=-3566/0 BOT CHORD 26-27=-326/435, 24-26=0/1410, 23-24=0/1781, 22-23=0/1781, 21-22=0/1752, 24-26-0/1420, 24-26-0/14

 
 20-21=0/1752, 18-20=0/1752

 WEBS
 3-26=-407/223, 11-24=0/1078, 24-35=-997/480, 35-36=-811/423, 15-36=-811/415, 4-31=-1088/103, 30-31=-1078/104, 29-30=-1079/104, 29-33=-1079/104, 33-34=-1079/104, 10-34=-1074/102, 1-32=-11/3047, 26-32=-19/3105, 5-31=-9/358, 12-35=-265/82, 14-36=-405/35, 23-36=-405/25, 15-22=-248/666

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are MT20 plates unless otherwise indicated.

5) All plates are 2x6 MT20 unless otherwise indicated.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Ceiling dead load (10.0 psf) on member(s). 3-4, 10-11, 4-31, 30-31, 29-30, 29-33, 33-34, 10-34; Wall dead load (5.0psf) on member(s).3-26, 11-24

Control weight Strip adjecting design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building design parameters use the page system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Claulity Criteria, DSB-89 and BCSI Building Component Safety information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Job	Truss	Truss Type	Qty	Ply	Lot 4 Atkins Farm Estates		
10421-2508	A1GE	GABLE	1	1	E15058953		
2000		O, DEE			Job Reference (optional)		
Comtech, Inc, Fayettev	ille, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Wed Nov 4 15:10:24 2020 Page 2		
		ID:G?Mgu2wAOefhMIzVCCS4xvzzRiE-ENta1OYu6H0vJ9ffGOa5HenqyG40eP1XgOgzQUyMX0z					

NOTES-

11) Refer to girder(s) for truss to truss connections.

12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18.

13) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclidual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, terction and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





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November 4,2020

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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 4 Atkins Farm Estates
					E15058956
J0421-2508	A3A	ATTIC	1	2	
				<b>_</b>	Job Reference (optional)
Comtech, Inc, Fayettev	rille, NC - 28314,		8	3.330 s Oct	7 2020 MiTek Industries, Inc. Wed Nov 4 15:10:28 2020 Page 2

ID:G?Mgu2wAOefhMIzVCCS4xvzzRiE-6875tlbPAWWLonzQVEe1SUxZ6uNOaCm7b0eAZFyMX0v

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-5=-60, 5-6=-80, 6-7=-60, 7-8=-60, 8-9=-80, 9-12=-60, 2-16=-20, 13-16=-40, 12-13=-20, 6-8=-20 Drag: 5-16=-10, 9-13=-10

Concentrated Loads (lb)

Vert: 23=-1837(F)

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🛕 WARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design values for use only with MTek® connectors. This should be used only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracking indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Compon Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Design valid for use only with MTER® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Componen Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

# B18 Soundside Road Edenton, NC 27932



6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-13

7) Attic room checked for L/360 deflection.



🛕 WARNING - Verify design pa meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTek® connectors. This skip included only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual Truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







BCDL 10	.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.20	) 23-25	>999	240	Weight: 362 lb	FT = 20%
LUMBER-	2x6 SF	'No.1		BRACING- TOP CHORD	Structu	ral wood	sheathing di	rectly applied or 4-11-2 o	c purlins,
WEBS	2x10 S 16-24: 2x6 SF 2-30 8-	P No.1 Except 2x8 SP No.1 'No.1 *Except* 25 8-21 11-21: 2x4 SP No.2		BOT CHORD	Rigid c 7-6-8 o	eiling dire c bracing	ectly applied o g: 26-27 g: 25-26	or 10-0-0 oc bracing, Ex	cept:
OTHERS	2x4 SF	No.2		WEBS JOINTS	1 Row 1 Brace	at midpt at Jt(s):	228, 35, 36, 3	27-28, 8-25 39	
REACTIONS.	(size Max H	e) 27=0-3-8, 16=0-3-8 orz 27=-568(LC 13)				.,			

Vert(CT)

 $\dot{-}$ 

-0.41 23-25

10

0.03

>823

240

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 1-2=-104/679, 2-3=-119/689, 3-4=-162/618, 4-5=-231/544, 5-6=-836/27, 6-7=-1459/0, 7-8=-1525/0, 8-9=-2182/299, 9-10=-2217/254, 10-11=-2248/233, 11-13=-2334/261, 13-14=-2405/257, 14-15=-2402/206, 15-16=-2448/168, 27-29=-186/761, 1-29=-57/438 BOT CHORD 26-27=0/1080, 25-26=0/1123, 23-25=0/1661, 22-23=0/1661, 21-22=0/1661, 20-21=-99/2197, 19-20=-99/2197, 18-19=-99/2197, 16-18=-99/2197

1.15

VES

вС

\//R

0.91

0 67

WEBS 26-28=-90/1727, 6-25=-7/928, 28-29=-551/132, 28-30=-1674/279, 30-35=-1478/229, 35-36=-1478/229, 5-36=-1478/229, 27-28=-3394/386, 2-30=-1089/257, 25-37=-1602/470, 8-37=-1636/489, 8-38=-505/1236, 38-39=-317/683, 21-39=-337/745, 11-21=-400/246, 23-38=-214/625

# NOTES-

TCLL

TCDL

BOU

10.0

00

Lumber DOL

Ctropp In

Max Uplift 27=-64(LC 13), 16=-133(LC 13) Max Grav 27=1799(LC 21), 16=1291(LC 1)

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Ceiling dead load (10.0 psf) on member(s). 5-6, 28-29, 28-30, 30-35, 35-36, 5-36; Wall dead load (5.0psf) on member(s).26-28, 6-25

9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 25-26 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 27 except (it=lb) 16=133.

🛕 WARNING - Verify design p neters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid of use only with MTek® connectors. This skot into CLODED will be REFERENCE FAGE MIL-14/3 feV, 319/2020 BEFORE DSE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Braching indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Compore Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







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FOND CONT







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Plate Offsets (X,Y)	[1:0-1-4,0-2-0], [7:0-1-4,0-2-0], [9:0-4-0,	0-4-12], [11:0-4-0,0-4-12]						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.78 BC 0.75 WB 0.20 Matrix-S	DEFL.         ii           Vert(LL)         -0.23           Vert(CT)         -0.38           Horz(CT)         0.07           Wind(LL)         0.07	n (loc) 3 9-11 9 9-11 1 8 7 9-11	l/defl >999 >653 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 223 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-       BRACING-         TOP CHORD       2x6 SP No.1         BOT CHORD       2x10 SP No.1         WEBS       2x6 SP No.1 *Except*         4-13,1-11,7-9: 2x4 SP No.2       BOT CHORD         Structural wood sheathing directly applied or 4-2-11 oc purlins, except end verticals.         BOT CHORD       Structural wood sheathing directly applied or 9-6-8 oc bracing.         JOINTS       1 Brace at Jt(s): 13					oc purlins,			
REACTIONS. (size) 12=0-3-8, 8=Mechanical Max Horz 12=313(LC 11) Max Grav 12=1446(LC 21), 8=1446(LC 20)								
FORCES.       (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       1-2=-1600/0, 2-3=-984/147, 5-6=-984/147, 6-7=-1600/0, 1-12=-1600/0, 7-8=-1601/0         BOT CHORD       11-12=-303/406, 9-11=-0/997         WEBS       6-9=-6/678, 2-11=-7/678, 3-13=-1036/187, 5-13=-1036/187, 1-11=0/915, 7-9=0/919								
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads have been considered for this design.</li> <li>2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-2-12 to 4-8-12, Interior(1) 4-8-12 to 10-9-8, Exterior(2) 10-9-8 to 15-2-5, Interior(1) 15-2-5 to 21-4-4 zone; end vertical left and right exposed; C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60</li> <li>3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</li> <li>4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.</li> </ul>								

b) Ceiling dead load (10.0 psf) on member(s). 2-3, 5-6, 3-13, 5-13; Wall dead load (5.0psf) on member(s).6-9, 2-11
c) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 9-11
c) Refer to girder(s) for truss to truss connections.

8) Attic room checked for L/360 deflection.



November 4,2020

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SI IDER Left 2x4 SP No.2 - 2-6-0. Right 2x4 SP No.2 -x 2-6-0

REACTIONS. All bearings 13-5-0.

(lb) - Max Horz 2=-224(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 15, 13 except 16=-256(LC 12), 12=-251(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 10, 14, 15, 13 except 16=270(LC 19), 12=265(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 4-16=-280/263, 8-12=-280/260

NOTES-1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 15, 13 except (jt=lb) 16=256, 12=251.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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818 Soundside Road Edenton, NC 27932



WEBS 4-14=-25/552, 8-12=-26/554, 5-15=-863/391, 7-15=-863/391

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0 to 4-4-13, Interior(1) 4-4-13 to 12-1-8, Exterior(2) 12-1-8 to 16-6-7, Interior(1) 16-6-7 to 24-3-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11.



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# BIS Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 4 Atkins Farm Estates
					E15058972
J0421-2508	G1-GR	COMMON GIRDER	1	2	
				<b>J</b>	Job Reference (optional)
Comtech, Inc, Fa	yetteville, NC - 28314,			3.330 s Oct	7 2020 MiTek Industries, Inc. Wed Nov 4 15:10:48 2020 Page 2

ID:G?Mgu2wAOefhMlzVCCS4xvzzRiE-X?Kf3bryTf1VBrVGgQ?jFimrqyLsGKO3B7UEG5yMX0b

# LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 13=-1547(B) 12=-1645(B) 15=-1547(B) 16=-1547(B) 17=-1547(B) 18=-1547(B) 19=-1547(B) 20=-1547(B) 21=-1645(B) 22=-1645(B) 23=-1645(B) 23=-1645(B)

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## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-2 to 3-7-11, Interior(1) 3-7-11 to 10-11-8, Exterior(2) 10-11-8 to 15-4-5, Interior(1) 15-4-5 to 22-8-2 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



November 4,20

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Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

n/a

n/a

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 154 lb

FT = 20%

LUMBER-
TOP CHORD

BCDL

2x6 SP No.1 T CHORD 2x4 SP No.2 OTHERS

10.0

REACTIONS. All bearings 21-11-0.

2x6 SP No.1

Max Horz 2=119(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 20, 21, 22, 17, 16, 15, 12 except 23=-109(LC 12), 14=-106(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 19, 20, 21, 22, 23, 17, 16, 15, 14, 12

Matrix-S

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code IRC2015/TPI2014

### NOTES-

- Unbalanced roof live loads have been considered for this design.
   Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 21, 22, 17, 16, 15, 12 except (jt=lb) 23=109, 14=106.



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TOP CHORD

BOT CHORD

### LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. All bearings 12-8-3.

(lb) - Max Horz 1=-144(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-161(LC 12), 6=-161(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=342(LC 19), 6=342(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 2-8=-355/291, 4-6=-355/291

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 6-4-1, Exterior(2) 6-4-1 to 10-8-14, Interior(1) 10-8-14 to 12-3-15 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=161, 6=161.

6) Non Standard bearing condition. Review required.



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Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



Max Horz 1=-108(LC 8)

Max Uplift 1=-27(LC 13), 3=-27(LC 13)

Max Grav 1=204(LC 1), 3=204(LC 1), 4=311(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate orip DOL=1.60

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



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3x4 1/

BRACING-

TOP CHORD

BOT CHORD

3x4 📎

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

	6-8-3	1
LOADING         (psf)         SPACING-         2-0-0           TCLL         20.0         Plate Grip DOL         1.15           TCDL         10.0         Lumber DOL         1.15           BCLL         0.0 *         Rep Stress Incr         YES           Code         IRC2015/TPI2014         Code         IRC2015/TPI2014	CSI.         DEFL.         in         (loc)         l/defl         L/d           TC         0.15         Vert(LL)         n/a         -         n/a         999           BC         0.07         Vert(CT)         n/a         -         n/a         999           WB         0.02         Horz(CT)         0.00         3         n/a         n/a	PLATES         GRIP           MT20         244/190           Weight: 27 lb         FT = 20%

2x4 || 6-8-3

# LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=6-8-3, 3=6-8-3, 4=6-8-3

Max Horz 1=-72(LC 8)

Max Uplift 1=-26(LC 13), 3=-26(LC 13)

Max Grav 1=146(LC 1), 3=146(LC 1), 4=187(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate arip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



November 4,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.03 BC 0.02 WB 0.01 Matrix-P	<b>DEFL.</b> in (loc) I/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 3 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 14 lb         FT = 20%
LUMBER-			BRACING-	

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=3-8-3, 3=3-8-3, 4=3-8-3

Max Horz 1=-36(LC 8) Max Uplift 1=-13(LC 13), 3=-13(LC 13)

Max Grav 1=72(LC 1), 3=73(LC 1), 4=93(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

arip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



Structural wood sheathing directly applied or 3-8-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

November 4,2020

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