MEAN ROOF HEIGHT: 19'-9	)"	HEIGHT TO RIDGE: 27'-5"			
CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A		
FENESTRATION U-FACTOR	0.35	0.35	0.35		
SKYLIGHT U-FACTOR	0.55	0.55	0.55		
GLAZED FENESTRATION SHGC	0.30	0.30	0.30		
CEILING R-VALUE	38 or 30ci	38 or 30ci	38 or 30ci		
WALL R-VALUE	15	15	19		
FLOOR R-VALUE	19	19	30		
* BASEMENT WALL R-VALUE	5/13	10/15	10/15		
** SLAB R-VALUE	0	10	10		
* CRAWL SPACE WALL R-VALUE	5/13	10/15	10/19		

\*"10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION
\*\* INSULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF PROTING; INSULITION DEPTH WITH STEM WALL SLAB 24" OR TO BOTTOM OF FOUNDATION WALL
DESIGNED FOR WITHO SPEED OF 120 MPH, 3 SECOND GUST (93 FASTEST MILE) EXPOSURE "B"

COMPONENT & CLADDING DESIGNED FOR THE FOLLOWING LOADS

MEAN ROOF	UP T	O 30'	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45'
ZONE 1	14.2	-15.0	14.9	-15.8	15.5	-16.4	15.9	-16.8
ZONE 2	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	-21.8	17.4	-22.4
DESIGNED FOR WIN	ID SPEED	OF 130 MI	H, 3 SEC	OND GUST	(101 FAS	TEST MILE	E) EXPOSU	IRE "B"
COMPONENT	% CLA	DDING	DESIG	NED FO	R THE	FOLLO	WING	LOADS
MEAN ROOF	UP T	O 30'	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45'
ZONE 1	16.7	-18.0	17.5	-18.9	18.2	-19.6		-20.2
ZONE 2	16.7	-21.0	17.5	-22.1				
ZONE 3	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 4	18.2	-19.0	19.1	-20.0	19.8	-20.7	20.4	-21.3
ZONE 5	18.2	24.0	19.1	-25.2	19.8	-26.2	20.4	26.9

### **GUARD RAIL NOTES**

R312.1 Where required. Guards shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or grade below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect screening shall not be considered as a *guard*.

R312.2 Height. Required quards at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads.

1 Guards on the onen sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the

2. Where the top of the *quard* also serves as a handrail on the open sides of 2. Where the top of the guard shall not be not less than 34 linches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting

the leading edges of the treads.

R312.3 Opening limitations. Required guards shall not have openings from the walking surface to the required *guard* height which allow passage of a sphere 4 inches (102 mm)in diameter.

### Exceptions:

Z:\Builder\Weaver Development Company, Inc\200220B Lauren III\200220B Lauren III.aec

 The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153 mm) in diameter.

Guards on the open sides of stairs shall not have openings which allow passage of a sphere 4 3/8 inches (111 mm) in diameter.

### **ROOF VENTILATION**

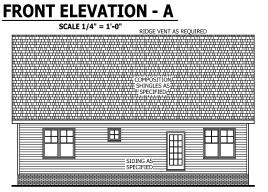
SQUARE FOOTAGE OF ROOF TO BE VENTED = 2,477 SQ.FT. NET FREE CROSS VENTILATION NEEDED:

WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 16.51 SQ.FT. WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE; OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 8.26 SQ.FT.

# COMPOSITION COMPOSITION SININGLES AS SININGLES AS SPECIFIED 12 12 12 (3) 2-0" X 3'-0" + 1 FIXED TOP OF PLATE WINDOW HEIGHT 9'-1 1/2" FIRST FLOOR PLATE H STONE SUB FLOOR RATE AS NEEDED

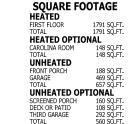
# WINDOWS WITH SIDE LOAD

SCALE 1/8" = 1'-0"



# REAR ELEVATION

SCALE 1/8" = 1'-0"



LOT 1 ATKINS FARM ESTATES

TBD SPRING HILL CHURCH RD

3CG/ COVERED PORCH & PATIO

LILLINGTON, NC 27546

# **AIR LEAKAGE**

Section N1102.4 N1102.4.1 Building thermal envelope. The building thermal envelope shall be durably sealed with an air barrier system to limit infiltration. The sealing methods between dissimilar materials shall allow for differential expansion and contraction. For all homes, where present, the following shall be caulked, gasketed, weather stripped or otherwise sealed with an air barrier material or solid material consistent with Appendix E-2.4 of this code:

1. Blocking and sealing floor/ceiling systems and under knee walls open to unconditioned or exterior space.



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Ξ

⋖ The Lauren **ELEVATION** 

SQUARE FOOTAGE HEATED HEATED OPTIONAL 148 SQ FT 148 SQ FT UNHEATED INHEATED OPTIONAL

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PAGE 1 OF 6

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4/29/2020

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10	10	L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200	-	-
Guardrail in-fill components	50	-	-
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40	-	L/360
Snow	20	-	-

**STRUCTURAL NOTES** 

All construction shall conform to the latest requirements of

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

### ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1,9x106 PSI Paralel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Instal all connections per manufacturers instructions

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" upless noted otherwise 3. 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise.

FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4"

PF: Portal fame per figure R602.10.1

ROOF SHEATHING: OSB or CDX roof sheathing minimum

3/8" thick.

CONCRETE AND SOILS: See foundation notes.

**ROOF TRUSS REQUIREMENTS** 

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins.

KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE, All required anchors for trusses due to uplift or bearing

shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights

### **BRACE WALL PANEL NOTES**

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless

GYPSUM: All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

REQUIRED LENGTH OF BRACING: Required brace wall length

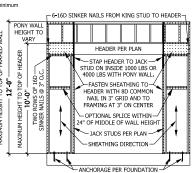
for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PE contributes 1.5 times its actual length

HD: 800 lbs hold down hold down device fastened to the edge of the brace wall panel dosets to the corner.

### Methods Per Table R602.10.1

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d(2 1/2" long x 0.113" diameter). CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing

GB: Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws



### PORTAL FRAME AT OPENING METHOD PF PER FIGURE AND SECTION R602.10.1

# **EXTERIOR HEADERS**

SCALE 1/4" = 1'-0"

(2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE

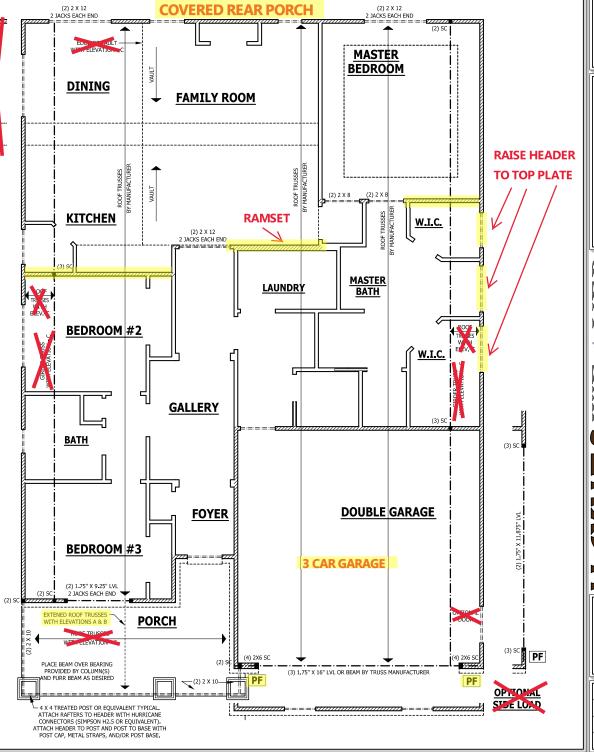
- KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16 KING STUD(S) 1 2 3 5 6

# INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE LADDER FRAMED

# FIRST FLOOR STRUCTURAL

SCALE 1/4" = 1'-0'



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STRUCTURAL Ħ The Lauren FLOOR **FIRST** 

SQUARE FOOTAGE HEATED

HEATED OPTIONAL 148 SQ.FI 148 SQ.FI UNHEATED INHEATED OPTIONAL

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PAGE 4 OF 6

# **ROOF TRUSS REQUIREMENTS**

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Han, Inc., attention before construction begins. KNEE WALL AND CEILING HEIGHTS, All finished knee wall heights and celling height are shown furred down 10° from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

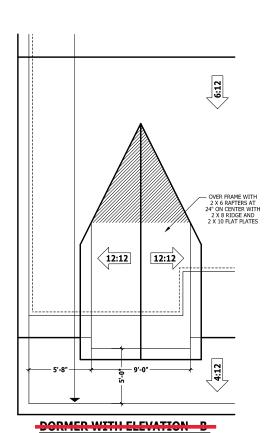
ANCHORAGE, All required anchors for trusses due to uplift or bearing

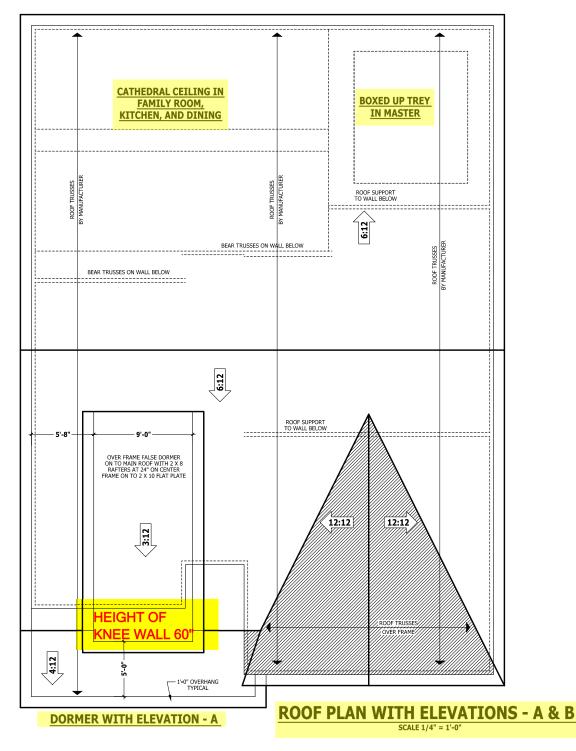
shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

HEEL HEIGHT ABOVE FIRST FLOOR PLATE

HEEL HEIGHT ABOVE SECOND FLOOR PLATE





DIMENSIONS AND CONDITIONS
EFORE CONSTRUCTION BEGIN
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8 ంఠ The Lauren III ROOF PLAN I ELEVATIONS -

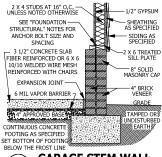
SQUARE FOOTAGE HEATED FIRST FLOOR 1791 SQ.FT TOTAL 1791 SQ.FT HEATED OPTIONAL UNHEATED

INHEATED OPTIONAL

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GARAGE STEM WALL D SCALE 3/4" = 1'-0"

# **DECK STAIR NOTES**

AM110.1 Stairs shall be constructed per Figure AM110. Stringer spans shall be no greater than 7 foot span between supports. Spacing between stringers shall be based upon decking material used per AM107.1. Each Stringer shall have minimum 3 1/2 inches between step cut and back of stringer If used, suspended headers shall shall be attached with 3/8 inch galvanized bolts with nuts and washers to securely support stringers at the top.

### **DECK BRACING**

SECTION AM109

AM109.1 Deck bracing. Decks shall be braced to provide lateral stability. The following are acceptable means to ovide lateral stability.

AM109.1.1. When the deck floor height is less than 4'-0" above finished grade per Figure AM109 and the deck is attached to the structure in accordance with Section AM104, lateral bracing is not required.

AM109.1.2. 4 x 4 wood knee braces may be provided on each column in both directions. The knee braces shall attach to each post at a point not less than 1/3 of the post length from the top of the post, and the braces shall be angled between 45 degrees and 60 degrees from the horizontal. Knee braces shall be bolted to the post and the girder/double band with one 5/8 inch hot dipped galvanized bolt with nut and washer at both ends of the brace per Figure AM109.1

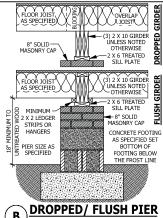
AM109.1.3. For freestanding decks without knee braces or diagonal bracing, lateral stability may be provided by embedding the post in accordance with Figure AM109.2

and the following:									
POST SIZE	MAX TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER					
4 X 4	48 SF	4'-0"	2'-6"	1'-0"					
6 X 6	120 SF	6'-0"	3'-6"	1'-8"					

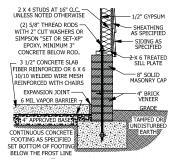
AM109.1.4. 2 x 6 diagonal vertical cross bracing may be provided in two perpendicular directions for freestanding decks or parallel to the structure at the exterior column line for attached decks. The 2 x 6's shall be attached to the posts with one 5/8 inch hot dipped galvanized bolt with nut and washer at each end of each bracing member per Figure AM109 3

AM109.1.5. For embedment of piles in Coastal Regions,

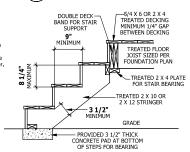
see Chapter 45.



# В SCALE 3/4" = 1'-0"



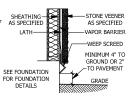
<48" GARAGE WING WALL Ε SCALE 3/4" = 1'-0'



### FIGURE AM110 TYPICAL DECK STAIR DETAIL

SCALE 3/4" = 1'-0"

# WEEP SCREEDS



**WEEP SCREED** 

SCALE 3/4" = 1'-0"

shall be provided at or below the foundation plate line on exterior stud walls in accordance with ASTM C 926. The weep screed shall be placed a minimum of 4 inches (102 mm) above the earth or 2 inches (51 mm) above paved areas and shall be of a type that will allow trapped water to drain to the exterior of the building. The weather-resistant barrier shal Ian the attachment flance. The exterior lath shall cover and terminate on the attachment flange of the weep screed.

All weep screeds and stone veneer to be installed ner manufactures instructions and per the 2012 North Carolina Residential

Building code. R703.6.2.1 - A minimum 0.019-inch (0.5 mm) (No. 26 galvanized sheet gage), corrosion-resistant weep screed or plastic veep screed, with a minimum vertical attachment flange of 31/2 inches (89 mm)

NEDA 72

2 X 4 STUDS AT 16" O.C. — UNLESS NOTED OTHERWISE

SUB FLOOR AS-

SPECIFIED

'AS SPECIFIED

2 X 6 TREATED

SEE "FOUNDATION

STRUCTURAL" NOTES FOR

ANCHOR BOLT SIZE AND

SPACING

CONTINUOUS CONCRETI

FOOTING AS SPECIFIED

BELOW THE FROST LINE

SET BOTTOM OF FOOTIN

C

SILI PLATE

X X X X X X

1/2" GYPSUM SEE ROOF EDGED OR PORCH FLOOR 12 PLAN OR ELEVATION SHINGLES AS SPECIFIED FOR PITCH SHEATHING AS SPECIFIED - 15# BUILDING FELT - 8" SOLID MASONRY CAR ROOF TRUSSES BY 4" CONCRETE BLOCK PORCH HEADER PER -4" BRICK VENEER PLAN INSTALLED OVER - EXPANSION TOTAL CENTER OF COLUMN BASE -VINYL OR HARDIE SOFFIT -6 MIL VAPOR BARRIER INSTALLED PER MANUFACTURERS BLOCKING INSTALLED INSTRUCTIONS ON BOTH SIDES & UNDER 3 1/2" SLAB HEADER AS DESIRED TAPERED COLUMN OVER MASONRY BASE 1 X MATERIAL · TAMPED OR ATTACHED TO HEADER CENTER LINE OF HEADER UNDISTURBED WITH POST CAP AND COLUMN **№ FARTH PORCH HEADER WITH** CRAWL SPACE AT GARGE **TAPERED COLUMN** SCALE 3/4" = 1'-0"

BOLT POST TO GIRDER GALVANIZED BOLTS BOLT BAND TO HOUSE WITH 5/8" HOT-DIPPED GALVANIZED BOLTS AT 1'8" O.C. MINIMUM TREATED HOUSE BAND WITH TREATED SHEATHING BETWEEN HOUSE AND DECK 2 1/2" FROM EDGE OF BAND BANDS FOR THE LENGTH AND NAIL WITH (3) COMMON HOT-DIPPED GALVANIZED NAILS AT 6" O.C. NAILS MUST CORROSTON OF THE DECK OR LISE 5/4 X 6 OR 2 X 4-TREATED DECKING FLASHING TO PROTECT UNTREATED FRAMING FLASHING MINIMUM 1/4" GAP PENETRATE A MINIMUM 1 1/2 INTO SUPPORTING BAND TREATED DECK TREATED FLOOR JOIST SIZED PER GIRDER SIZED PER FOUNDATION PLAN FOUNDATION PLAN ATTACH JOIST TO ATTACH JOIST TO GIRDER WITH HANGER BAND WITH HANGER OR TREATED 2 X 2 LEDGER STRIP OR TREATED 2 X 2 FOOTING SIZED PER LEDGER STRIP FOUNDATION PLAN SET BOTTOM OF -TREATED POST FROST LINE FOUNDATION PLAN

# **DECK ATTACHMENT DETAIL TO FRAMED WALL**

SCALE 3/4" TO 1'-0"

SMOKE ALARMS

SECTION R314 R314.1 Smoke detection and notification. All smoke alarms shall be listed in accordance with UL 217 and installed in accordance with

the provisions of this code and the household fire warning equipment provisions of NFPA 72. R314.2 Smoke detection systems. Household fire alarm systems installed in accordance with NFPA 72 that include smoke alarms, or a combination of smoke detector and audible notification device installed as required by this section for smoke alarms, shall be permitted. The household fire alarm system shall provide the same level of smoke detection and alarm as required by this section for smoke alarms. Where a household fire warning system is installed using a combination of smoke detector and audible notification device(s), it shall become a permanent fixture of the occupancy and owned by the homeowner. The system shall be monitored by an

Exception: Where smoke alarms are provided meeting the requirements of Section R314.4.

approved supervising station and be maintained in accordance with R314.3 Location. Smoke alarms shall be installed in the following

 In each sleeping room. 2. Outside each separate sleeping area in the immediate vicinity of

the bedrooms.

3. On each additional *story* of the *dwelling*, including *basements* and habitable attics (finished) but not including crawl spaces. uninhabitable (unfinished) attics and uninhabitable (unfinished) attic-stories. In dwellings or dwelling units with split levels and without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full *story* 

below the upper level. When more than one smoke alarm is required to be installed within an individual dwelling unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of

the alarms in the individual unit. **R314.4 Power source.** Smoke alarms shall receive their primary power from the building wiring when such wiring is served from a commercial source, and when primary power is interrupted, shall receive power from a battery. Wiring shall be permanent and without a disconnecting switch other than those required for overcurrent protection. Smoke alarms shall be interconnected.

# STAIRWAY NOTES

shall not be less than 6 feet 8 inches (2032 mm) measured vertically from

the sloped line adjoining the tread nosing or from the floor surface of the landing or platform on that portion of the stainway. R311.7.4 Stair treads and risers. Stair treads and risers shall meet the requirements of this section. For the purposes of this section all dimensions and dimensioned surfaces shall be exclusive of carpets, rugs or runners.

R311.7.4.1 Riser height. The maximum riser height shall be 8 1/4 inches (210 mm). The riser shall be measured vertically between leading edges of the adiacent treads. P311 7.4.2 Tread denth. The minimum tread denth shall be 9 inches (229 mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm)

from the side where the treads are narrower. Winder treads shall have a minimum tread depth of 4 inches (102 mm) at any point.

R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greater than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stairways with solid

R311.7.7 Handrails. Handrails shall be provided on at least one side of each continuous run of treads or flight with four or more risers.

R311,7,7,1 Height, Handrail height, measured vertically from the sloped plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm)and not more than 38 inches (965 mm). Exceptions:

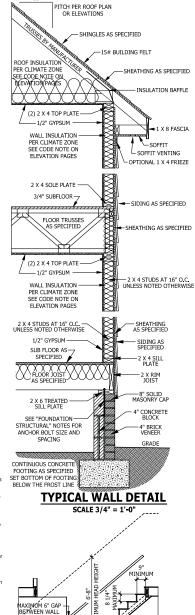
1. The use of a volute, turnout or starting easing shall be allowed over the lowest tread.

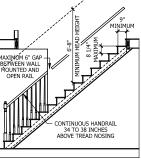
 When handrail fittings or bendings are used to provide continuous transition between flights, the transition from handrall to guardrall, or used at the start of a flight, the handrall height at the fittings or bendings shall be permitted to exceed the maximum height.

R311.7.7.2 Continuity. Handrails for stairways shall be continuous for the full length of the flight, from a point directly above the top riser of the flight to a point directly above the lowest riser of the flight. Handrail ends shall be returned or shall terminate in newel posts or safety terminals. Handrails adjacent to a wall shall have a space of not less than 11/2 inch (38 mm) between the wall and the handrails.

1. Handrails shall be permitted to be interrupted by a newel post.

The use of a volute, turnout, starting easing or starting newel shall be allowed over the lowest tread. 3. Two or more separate rails shall be considered continuous if the termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the wall-mounted rail must return into the wall.





TYPICAL STAIR DETAIL

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NHEATED OPTIONAL

SQUARE FOOTAGE HEATED

148 SQ.FI 148 SQ.FI

HEATED OPTIONAL

JNHEATED

URCHASER MUST VERIFY AL

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Lauren

The

DETAIL

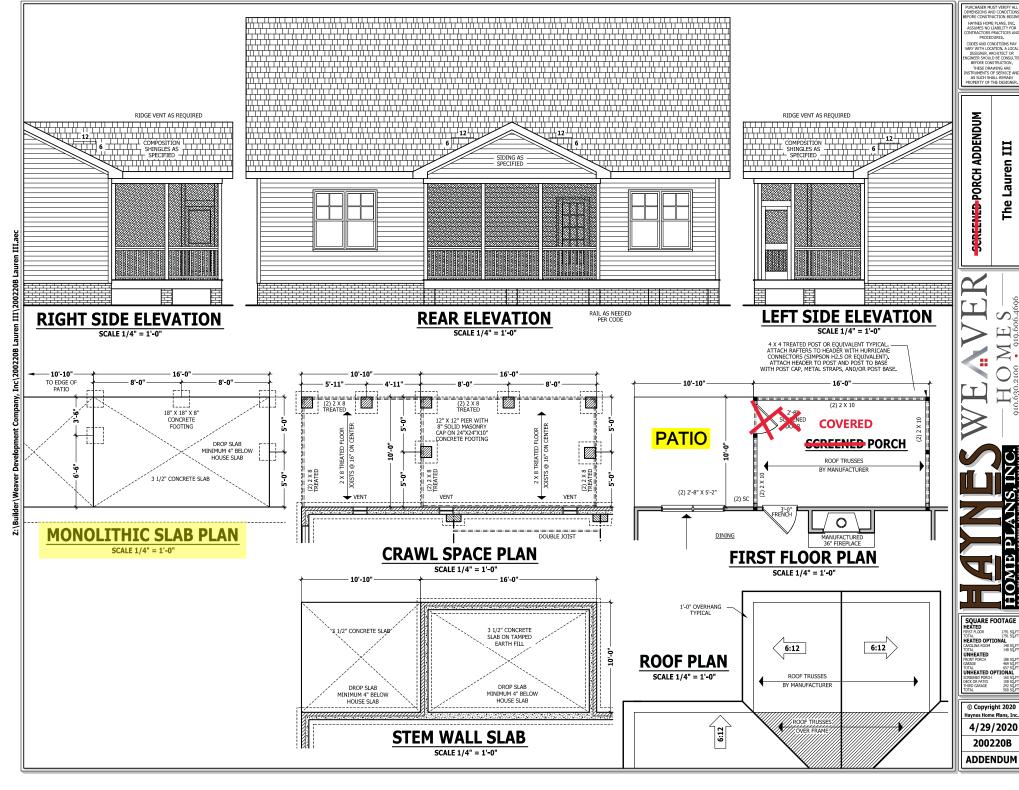
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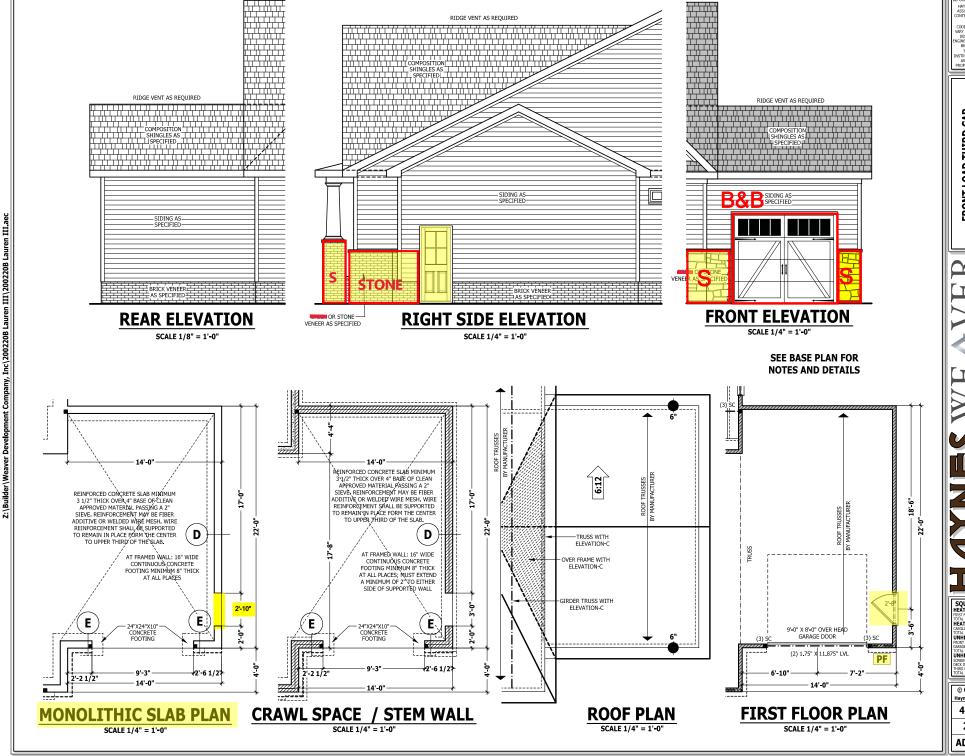
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PAGE 6 OF 6

III aec III\200220B Inc\200220B ï





PURCHASER MUST VERIFY ALL
DIMENSIONS AND CONDITIONS
BEFORE CONSTRUCTION BEGINS
HAYNES HOME PLANS, INC.
ASSUMES NO LIABILITY FOR
CONTRACTORS PRACTICES AND
PROCEDURES.
CODES AND CONDITIONS MAY
VARY WITH LOCATION, A LOCAL
DESIONER, AGOLITECT OR
BEFORE CONSTRUCTION,
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ERTY OF THE DESI

FRONT LOAD THIRD CAR
The Lauren III

WEAVER

- HOMES

910.630.2100 - 919.660.4696

910.630.2100 - 100.960.4696

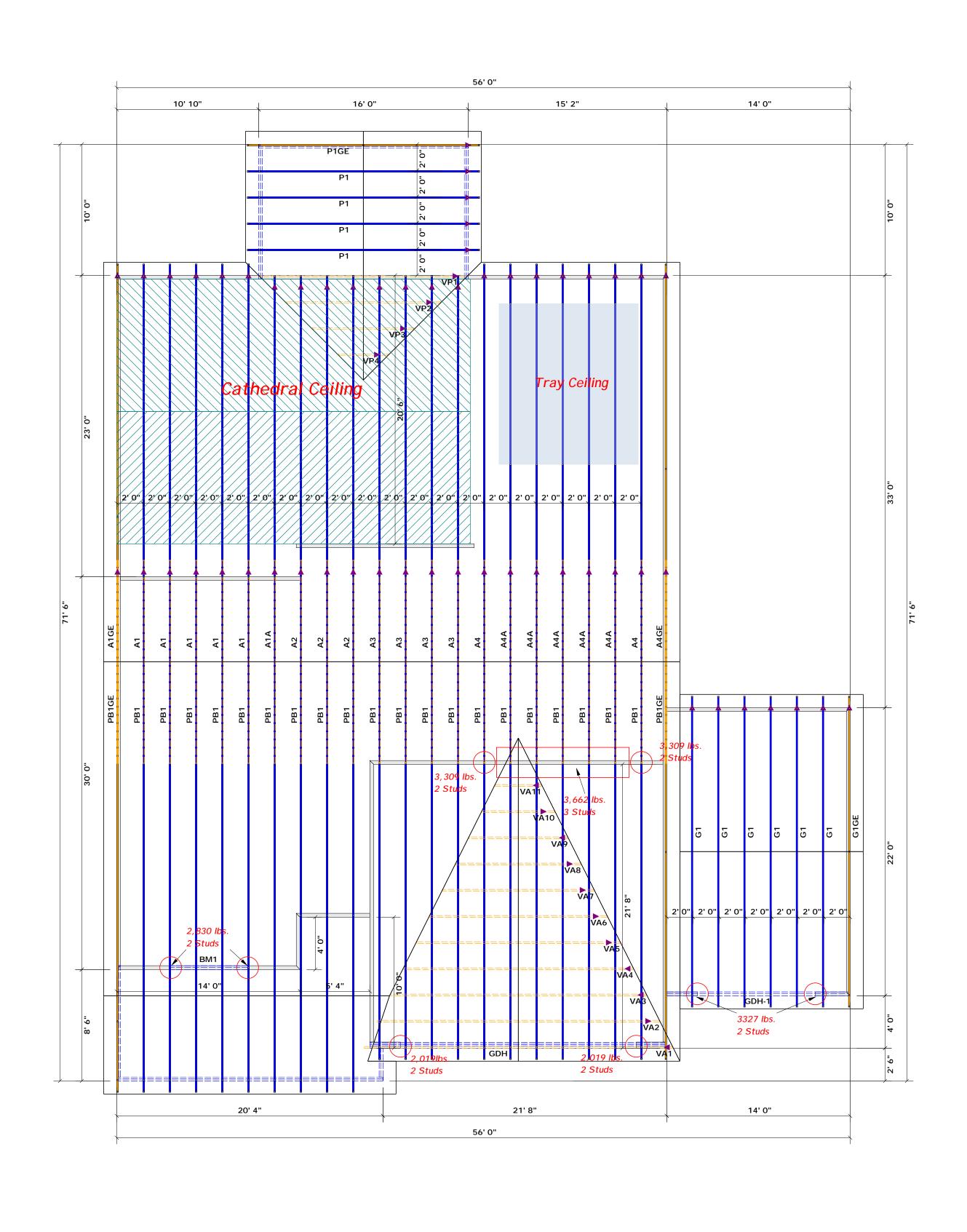
HOMEPIANS, INC

SQUARE FOOTAGE

HEATED
PIRST FLOOR 1791 SQ.FT.
TOTAL 10 1791 SQ.FT.

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ADDENDUM



= Denotes Left End of Truss
 (Reference Engineered Truss Drawing)
 Do Not Erect Trusses Backwards

3400 ! 6600 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(0.45% ON HARDS 85025(), 4-6))

MARICA OF JACK STUDO ACCUME(D. 9) CA CAS OF FEADER 675003

2550 1 5100 2

7650 3

10200 4 12750 5 15300 6

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan SCALE: 3/16" = 1'

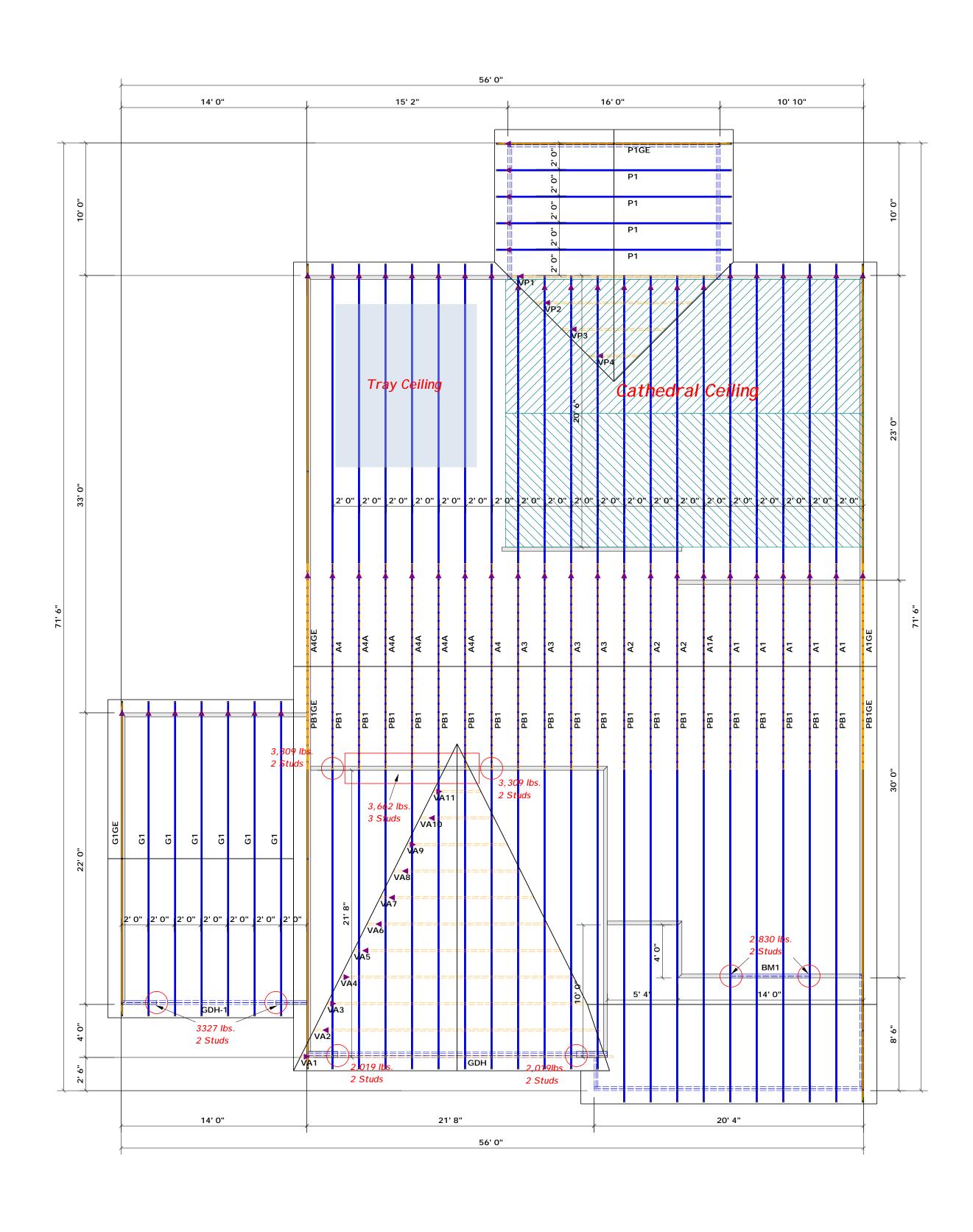
Beam Legend								
PlotID	Length	Product	Plies	Net Qty				
BM1	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2				
GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2				
GDH	23' 0"	1-3/4"x 16" LVL Kerto-S	3	3				

	BUILDER	Weaver Development	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.  These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer
EADER	JOB NAME	Lot 1 Atkins Farm Estates	ADDRESS	Spring Hill Church Rd.	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package
DRYH	PLAN	Lauren III / Elev. A / CP / 3 Car	MODEL	Roof	or online @ sbcindustry.com  Bearing reactions less than or equal to 3000# are deemed to comply with the
<u>ε</u>	1 27 (14	Eddi cii 111 / Elev. 7( / Gi / G odi	WOBLE	Root	prescriptive Code requirements. The contractor shall refer to the attached Tables
3	SEAL DATE	2/18/20	DATE REV.	04/21/21	( derived from the prescriptive Code requirements ) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those
5	QUOTE #	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.
_	JOB #	J0421-2504	SALESMAN	Lenny Norris	Signature Curtis Quick



Fayetteville, N.C. 28309

Phone: (910) 864-8787 Fax: (910) 864-4444



= Denotes Left End of Truss(Reference Engineered Truss Drawing)Do Not Erect Trusses Backwards

3400 ! 6600 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(0.45% ON HARDS 85025(), 4-6))

MARICA OF JACK STUDO ACCUME(D. 9) CA CAS OF FEADER 675003

2550 1 5100 2

7650 3

10200 4 12750 5 15300 6

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan SCALE: 3/16" = 1'

PlotID	Length	Product	Plies	Net Qty
BM1	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
GDH	23' 0"	1-3/4"x 16" LVL Kerto-S	3	3

	BUILDER	Weaver Development	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.  These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer
*B0V	JOB NAME	Lot 1 Atkins Farm Estates	ADDRESS	Spring Hill Church Rd.	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package
1111111	PLAN	Lauren III / Elev. A / CP / 3 Car	MODEL	Roof	or online @ sbcindustry.com  Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables
	SEAL DATE	2/18/20	DATE REV.	04/21/21	( derived from the prescriptive Code requirements ) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those
	QUOTE #	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.
	JOB #	J0421-2504	SALESMAN	Lenny Norris	Signature Curtis Quick



Phone: (910) 864-8787 Fax: (910) 864-4444

Client:

Project: Address: Weaver Development

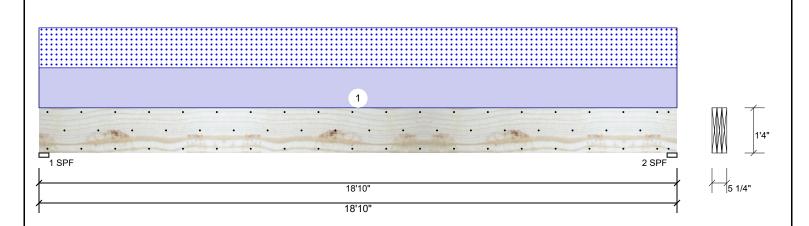
Date: 4/21/2021 Input by:

Curtis Quick Job Name: The Lauren III Beams Page 1 of 6

Project #:

1.750" X 16.000" **Kerto-S LVL** 3-Ply - PASSED **GDH** 

Level: Level



### Member Information Reactions UNPATTERNED Ib (Uplift) Application: Brg Live Wind Type: Floor Dead Snow Const Plies: 3 Design Method: ASD 0 1127 951 0 0 1 Moisture Condition: Dry **Building Code:** IBC 2012 2 0 1127 951 0 0 Deflection LL: 480 Load Sharing: Yes Deflection TL: 360 Deck: Not Checked Importance: Normal Temp <= 100°F Temperature: **Bearings** Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.500" 2078 L D+S 1127 / 951

2 - SPF 3.500"

27%

1127 / 951

2078 L

D+S

### Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9334 ft-lb	9'5"	62010 ft-lb	0.151 (15%)	D+S	L
Unbraced	9334 ft-lb	9'5"	10984 ft-lb	0.850 (85%)	D+S	L
Shear	1735 lb	17'3 3/8"	20608 lb	0.084 (8%)	D+S	L
LL Defl inch	0.078 (L/2813)	9'5 1/16"	0.460 (L/480)	0.170 (17%)	S	L
TL Defl inch	0.171 (L/1288)	9'5 1/16"	0.613 (L/360)	0.280 (28%)	D+S	L

# **Design Notes**

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	101 PLF	0 PLF	101 PLF	0 PLF	0 PLF	A4A	
	Self Weight				19 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Client: Weaver Development

Project: Address:

4/21/2021 Input by: Curtis Quick

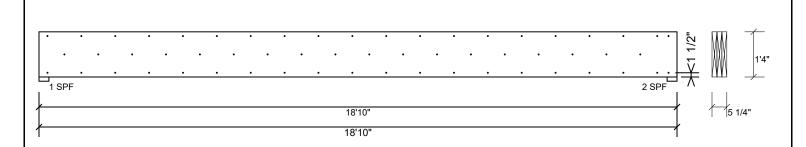
Job Name: The Lauren III Beams

Page 2 of 6

Project #:

**Kerto-S LVL** 1.750" X 16.000" 3-Ply - PASSED **GDH** 

Level: Level



# Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

### Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- Informing & Installation

  I. VIL beams must not be cut or drilled

  Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  Damaged Beams must not be used

  Design assumes top edge is laterally restrained

  Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation

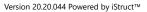
For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





Client: Weaver Development

Project: Address:

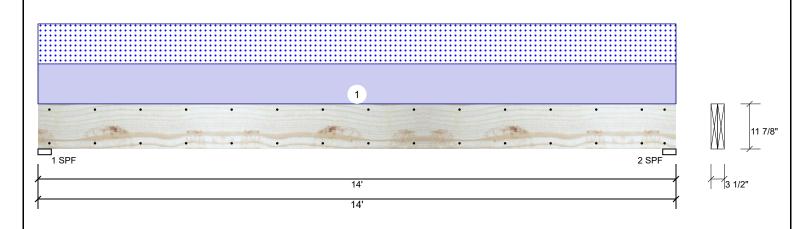
Date: 4/21/2021 Input by: Curtis Quick

Project #:

1.750" X 11.875" 2-Ply - PASSED Kerto-S LVL GDH-1

Level: Level

Job Name: The Lauren III Beams



### **Member Information** Reactions UNPATTERNED Ib (Uplift) Application: Brg Live Wind Type: Floor Dead Snow Const Plies: 2 Design Method: ASD 0 1696 1631 0 0 1 Moisture Condition: Dry **Building Code:** IBC 2012 2 0 1696 1631 0 0 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal Temp <= 100°F Temperature: **Bearings** Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.500" 1696 / 1631 D+S 64% 3327 L 2 - SPF 3.500" 64% 1696 / 1631 3327 L D+S

### **Analysis Results**

_						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10893 ft-lb	7'	22897 ft-lb	0.476 (48%)	D+S	L
Unbraced	10893 ft-lb	7'	10911 ft-lb	0.998 (100%)	D+S	L
Shear	2747 lb	1'2 5/8"	10197 lb	0.269 (27%)	D+S	L
LL Defl inch	0.195 (L/832)	7' 1/16"	0.339 (L/480)	0.580 (58%)	S	L
TL Defl inch	0.398 (L/408)	7' 1/16"	0.451 (L/360)	0.880 (88%)	D+S	L

### **Design Notes**

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 8'2 5/8" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	233 PLF	0 PLF	233 PLF	0 PLF	0 PLF	G1
	Self Weight				9 PLF					

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

### Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

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Page 3 of 6

Client:

Project: Address: Weaver Development

Date: 4/21/2021 Input by: Curtis Quick

Job Name: The Lauren III Beams

Page 4 of 6

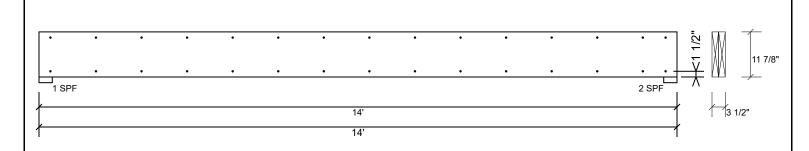
Project #:

**Kerto-S LVL** GDH-1

1.750" X 11.875"

2-Ply - PASSED

Level: Level



# Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

1 3		•	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	163.7 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

### Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

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Client: Weaver Development

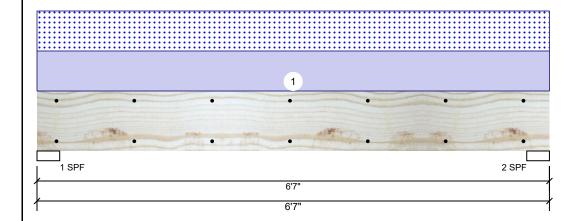
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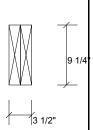
Date: 4/21/2021 Input by: Curtis Quick Job Name: The Lauren III Beams

Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** BM1

Level: Level





Page 5 of 6

### **Member Information**

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal Temperature: Temp <= 100°F

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No

Deck: Not Checked

# Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	1564	1541	0	0
2	0	1564	1541	0	0

# **Bearings**

Bearing Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF 3.500"	60% 1564 / 1541	3105 L	D+S
0 005 0 500"	000/ 4504/4544	0405	D.O

# **Analysis Results**

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4423 ft-lb	3'3 1/2"	14423 ft-lb	0.307 (31%)	D+S	L
Unbraced	4423 ft-lb	3'3 1/2"	10451 ft-lb	0.423 (42%)	D+S	L
Shear	2161 lb	1'	7943 lb	0.272 (27%)	D+S	L
LL Defl inch	0.040 (L/1842)	3'3 1/2"	0.153 (L/480)	0.260 (26%)	S	L
TL Defl inch	0.080 (L/914)	3'3 1/2"	0.204 (L/360)	0.390 (39%)	D+S	L

# **Design Notes**

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	468 PLF	0 PLF	468 PLF	0 PLF	0 PLF	A1
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

### Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- This design is valid until 2/26/2023

6. For flat roofs provide proper drainage to prevent ponding

# Manufacturer Info

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Client: Weaver Development

Project: Address:

Date: 4/21/2021

Input by: Curtis Quick Job Name: The Lauren III Beams

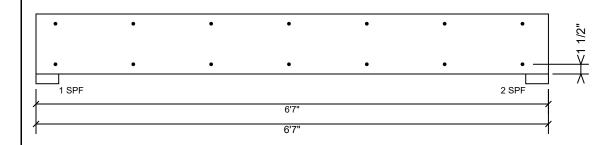
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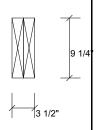
**Kerto-S LVL** BM1

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 6 of 6

# Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

### Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 2/26/2023

Manufacturer Info