

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 18030341-MASTER Cali NP Vault Master

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I39464278 thru I39464287

My license renewal date for the state of North Carolina is December 31, 2019.

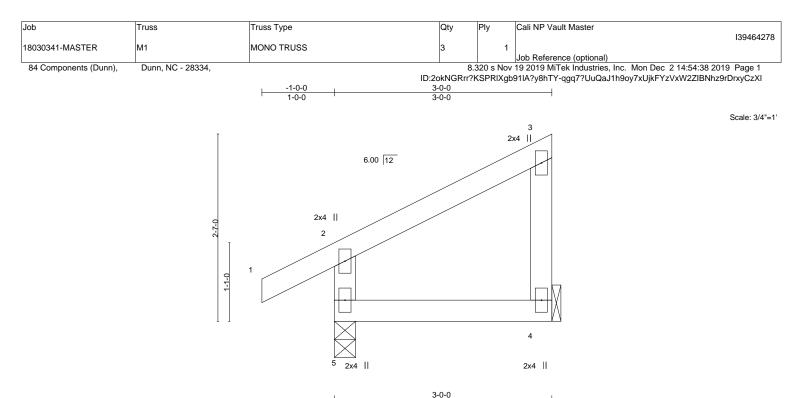
North Carolina COA: C-0844



December 2,2019

# Sevier, Scott

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014 | CSI.<br>TC 0.14<br>BC 0.08<br>WB 0.00<br>Matrix-R | DEFL.         in           Vert(LL)         -0.00           Vert(CT)         -0.00           Horz(CT)         -0.00 | (loc)<br>4-5<br>4-5<br>4 | l/defl<br>>999<br>>999<br>n/a | L/d<br>240<br>180<br>n/a | PLATES<br>MT20<br>Weight: 15 lb | <b>GRIP</b><br>244/190<br>FT = 20% |
|--|---|---|---|--------------------------|-------------------------------|--------------------------|---------------------------------|------------------------------------|

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 3-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing

except end verticals.

### LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD WEBS

2x4 SP No.3

REACTIONS. (lb/size) 5=192/0-3-8, 4=94/Mechanical Max Horz 5=80(LC 9) Max Uplift 5=-22(LC 12), 4=-25(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right

exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

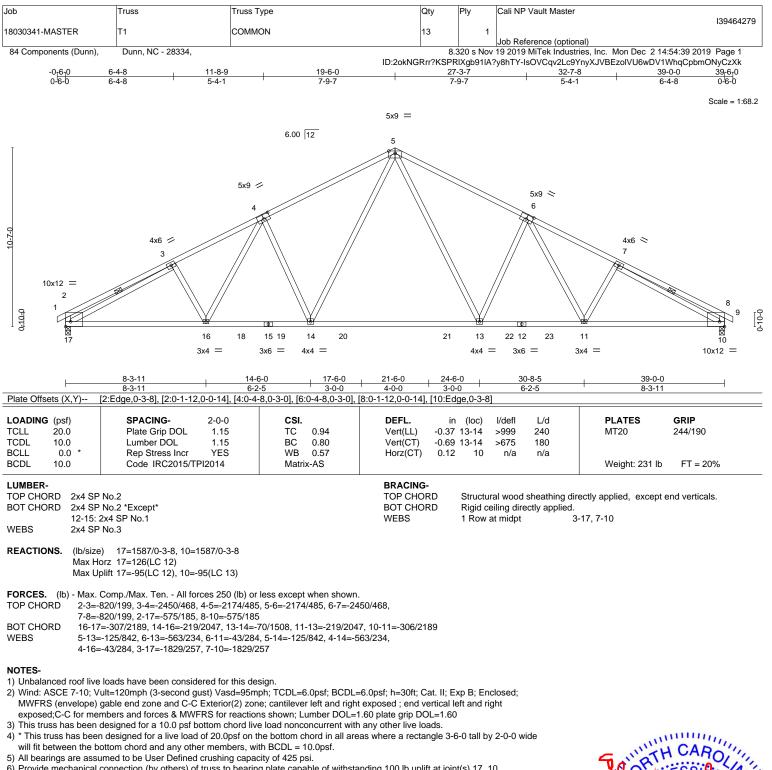
4) Bearings are assumed to be: , Joint 4 User Defined crushing capacity of 425 psi.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.



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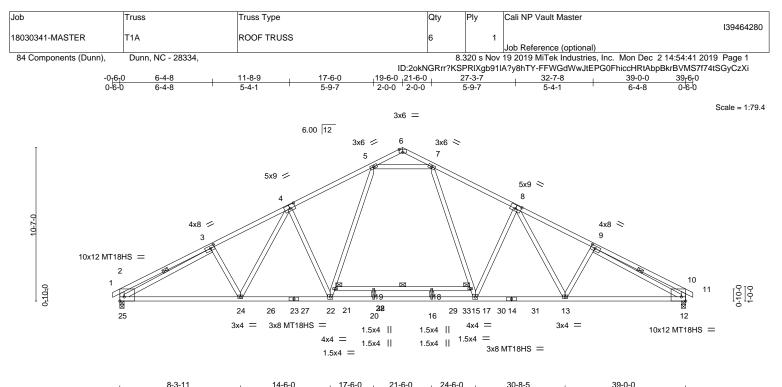


6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17, 10.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.







|  | 8-3-11  | 6-2-5 3-0-0   | 4-0-0 3-0-0                                |   | 8-3-11  |
|--|---|---|--|---|---|
| Plate Offsets (X,Y)  | [2:Edge,0-3-8], [2:0-1-12,0-0-14], [  | 3:0-3-8,0-2-0], [4:0-4-8,0-3-0], [4                       | 6:0-3-0,Edge], [8:0-4-8                    | ,0-3-0], [9:0-3-8,0-2-0], [10   | :0-1-12,0-0-14], [12:Edge,0-3-8]  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014 | <b>CSI.</b><br>TC 0.97<br>BC 0.98<br>WB 0.81<br>Matrix-AS | Vert(LL) -0.52                             | n (loc) l/defl L/d<br>2 16-20 >886 240<br>6 16-20 >436 180<br>4 12 n/a n/a                          | PLATES         GRIP           MT20         244/190           MT18HS         244/190           Weight: 247 lb         FT = 20% |
|  | No.2 *Except*<br>2x4 SP No.1, 14-23: 2x4 SP DSS   |   | BRACING-<br>TOP CHORD<br>BOT CHORD<br>WEBS | Structural wood sheathir<br>Rigid ceiling directly app<br>6-0-0 oc bracing: 17-21<br>1 Row at midpt | ng directly applied, except end verticals.<br>lied. Except:<br>3-25, 9-12   |
| REACTIONS. (Ib/size  | e) 25=1758/0-3-8 12=1758/0-3-8  | 3   |  |   |   |

- REACTIONS. (lb/size) 25=1758/0-3-8, 12=1758/0-3-8 Max Horz 25=126(LC 16) Max Grav 25=1835(LC 2), 12=1835(LC 2)
- FORCES.
   (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

   TOP CHORD
   2-3=-866/142, 3-4=-2994/219, 4-5=-2783/176, 7-8=-2783/176, 8-9=-2994/219, 9-10=-866/142, 2-25=-594/155, 10-12=-594/155

   BOT CHORD
   24-25=-95/2652, 22-24=0/2568, 20-22=0/2172, 16-20=0/2172, 15-16=0/2172, 13-15=0/2568, 12-13=-93/2652

   WEBS
   7-17=0/1064, 15-17=-12/908, 8-15=-552/249, 8-13=-107/257, 21-22=-12/908, 5-21=0/1064, 4-22=-552/249, 4-24=-107/257, 3-25=-2285/66, 9-12=-2285/66, 5-7=-2090/264

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

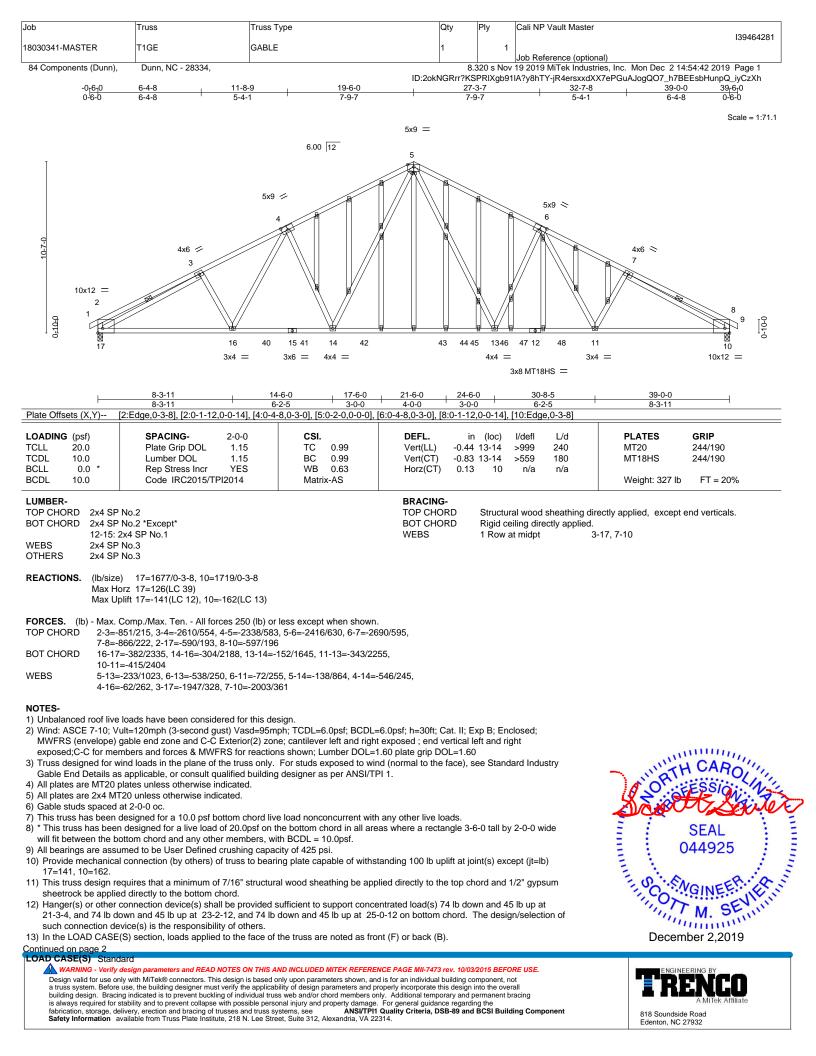
- 3) 150.0lb AC unit load placed on the bottom chord, 19-6-0 from left end, supported at two points, 4-0-0 apart.
- All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

- 7) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

mmn Common with SEAL 44925 "minin December 2,2019





| Truss             | Truss Type | Qty        | Ply          | Cali NP Vault Master  |
|-------------------|------------|------------|--------------|---|
|                   |            |            |              | 139464281   |
| T1GE              | GABLE      | 1          | 1            |   |
|                   |            |            |              | Job Reference (optional)                                      |
| Dunn, NC - 28334, |            | 8.         | 320 s Nov    | 19 2019 MiTek Industries, Inc. Mon Dec 2 14:54:43 2019 Page 2 |
|                   | T1GE       | T1GE GABLE | TIGE GABLE 1 | TIGE GABLE 1 1  |

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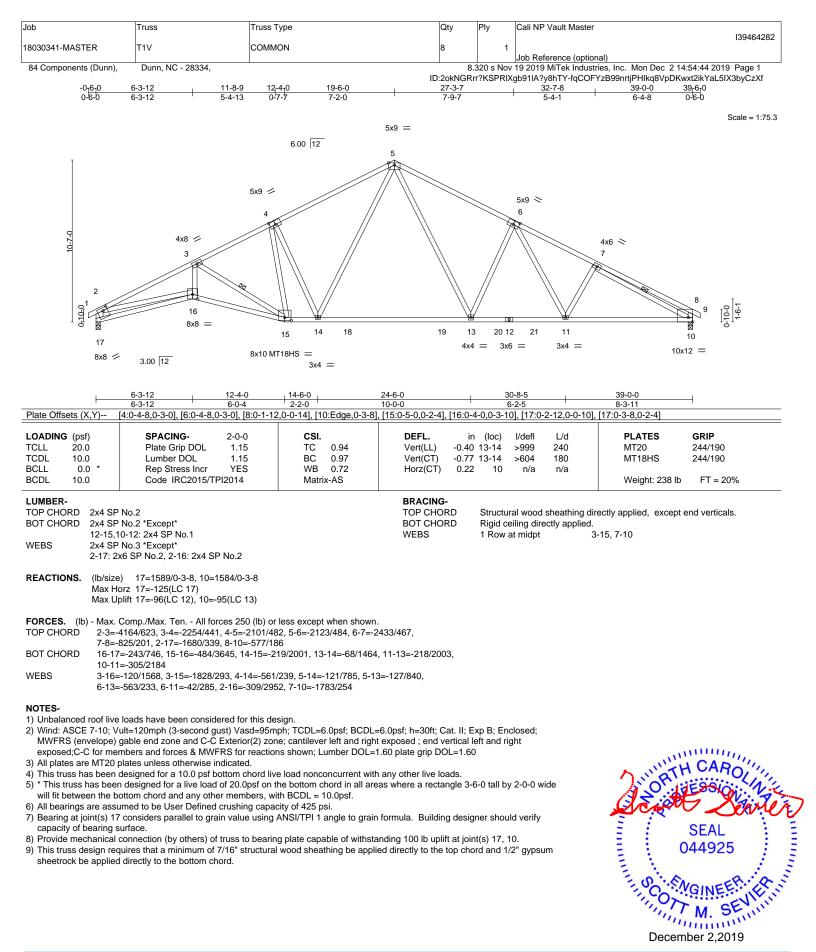
LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-5=-60, 5-8=-60, 8-9=-60, 10-17=-20 Concentrated Loads (lb)

Vert: 43=-74(F) 45=-74(F) 46=-74(F)





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932

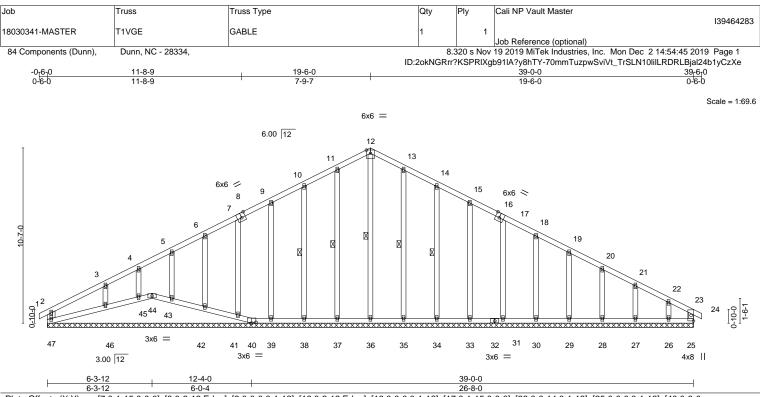


Plate Offsets (X,Y)-- [7:0-1-15,0-0-0], [8:0-2-12,Edge], [8:0-0-0,0-1-12], [16:0-2-12,Edge], [16:0-0-0,0-1-12], [17:0-1-15,0-0-0], [23:0-0-14,0-1-12], [25:0-0-0,0-1-12], [40:0-3-0,0-0-12]

| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0 | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014 | <b>CSI.</b><br>TC 0.10<br>BC 0.08<br>WB 0.12<br>Matrix-R | DEFL. i<br>Vert(LL) -0.0<br>Vert(CT) 0.0<br>Horz(CT) 0.0 | 0 23 n/r                         | L/d<br>120<br>90<br>n/a | PLATES<br>MT20<br>Weight: 269 lb                | <b>GRIP</b><br>244/190<br>FT = 20% |
|--|---|--|--|----------------------------------|-------------------------|---|------------------------------------|
| LUMBER-<br>TOP CHORD 2x4 SP No.2<br>BOT CHORD 2x4 SP No.2  |   |  | BRACING-<br>TOP CHORD                                    | Structural woo<br>except end ver | •                       | rectly applied or 6-0-0                         | oc purlins,                        |
| WEBS 2x6 SP 23-25:   | S 2x6 SP No.2 *Except*<br>23-25: 2x4 SP No.3  |  |  |                                  | rectly applied o        | or 10-0-0 oc bracing.<br>2-36, 11-37, 10-38, 13 | -35, 14-34                         |

## **REACTIONS.** All bearings 39-0-0.

- (lb) Max Horz 47=-125(LC 17)
  - Max Uplift All uplift 100 lb or less at joint(s) 47, 44, 40, 25, 37, 38, 39, 41, 42, 43, 45, 35, 34, 33, 31, 30, 29, 28, 27, 26 except 46=-102(LC 12) Max Grav All reactions 250 lb or less at joint(s) 47, 44, 40, 25, 36, 37, 38, 39, 41, 42, 43, 45, 46, 35, 34, 33, 31, 30, 29, 28, 27, 26

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 10-11=-120/273, 11-12=-133/309, 12-13=-133/309, 13-14=-120/273

### NOTES-

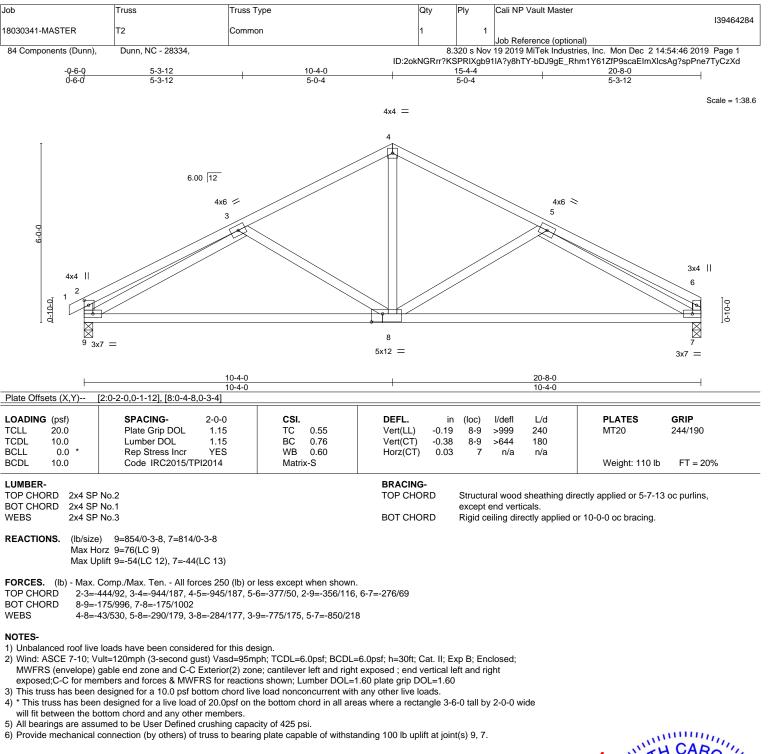
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.

6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 47, 44, 40, 25, 37, 38, 39, 41, 42, 43, 45, 35, 34, 33, 31, 30, 29, 28, 27, 26 except (jt=lb) 46=102.
- 12) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 44, 41, 42, 43, 45, 46.

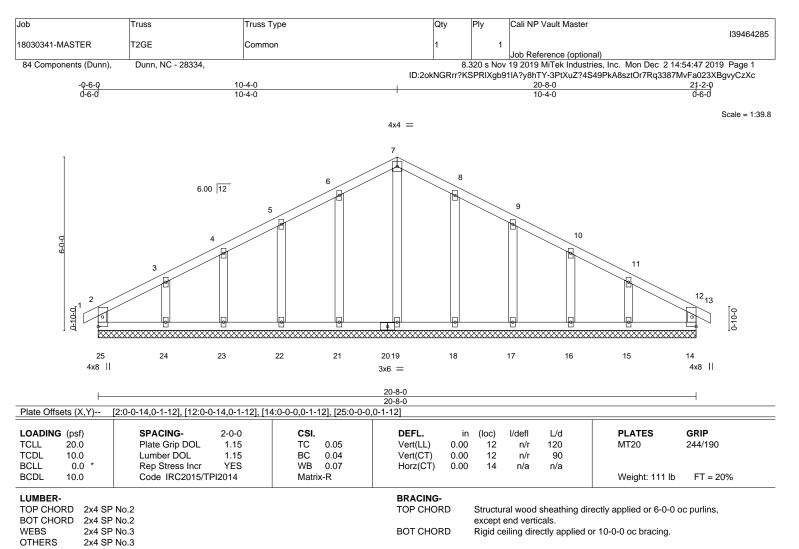












REACTIONS. All bearings 20-8-0.

(lb) - Max Horz 25=75(LC 11)

Max Horz 25=75(LC 11) Max Uplift All uplift 100 lb or less at joint(s) 25, 14, 21, 22, 23, 24, 18, 17, 16, 15 Max Grav All reactions 250 lb or less at joint(s) 25, 14, 19, 21, 22, 23, 24, 18, 17, 16, 15

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

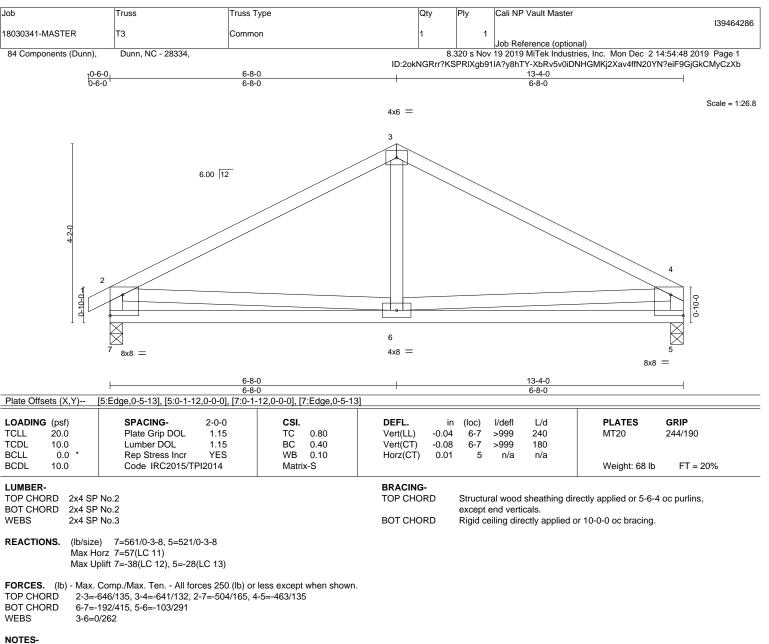
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be User Defined crushing capacity of 425 psi.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 14, 21, 22, 23, 24, 18, 17, 16, 15.



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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

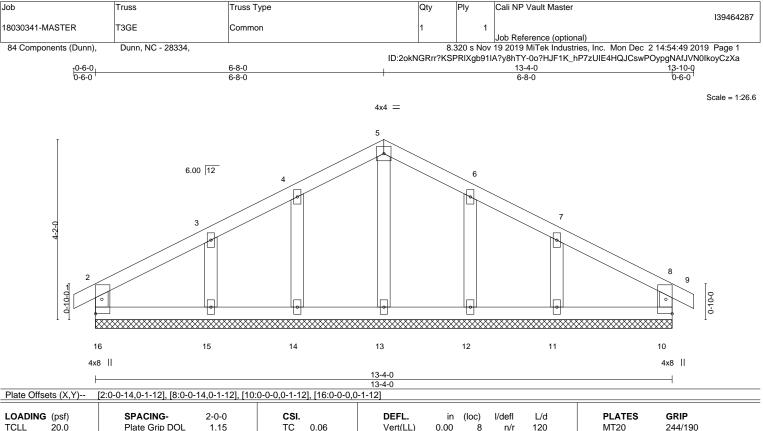
4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 5.







| LUADING (psi)         | SFACING-        | 2-0-0   | 031.   |      | DEFL.    |        | (100)                    | i/uen       | L/u | FLATES        | GRIF     |
|-----------------------|-----------------|---------|--|------|----------|--------|--------------------------|-------------|-----|---------------|----------|
| TCLL 20.0             | Plate Grip DOL  | 1.15    | TC   | 0.06 | Vert(LL) | 0.00   | 8                        | n/r         | 120 | MT20          | 244/190  |
| TCDL 10.0             | Lumber DOL      | 1.15    | BC   | 0.05 | Vert(CT) | 0.00   | 9                        | n/r         | 90  |               |          |
| BCLL 0.0              | Rep Stress Incr | YES     | WB   | 0.03 | Horz(CT) | 0.00   | 10                       | n/a         | n/a |               |          |
| BCDL 10.0             | Code IRC2015/   | TPI2014 | Matri  | x-R  |          |        |                          |             |     | Weight: 63 lb | FT = 20% |
| LUMBER-               |                 |         |  |      | BRACING- |        |                          |             |     |               |          |
| TOP CHORD 2x4 SP No.2 |                 |         | TOP CHORD Structural wood sheathing directly applied o |      |          |        | irectly applied or 6-0-0 | oc purlins, |     |               |          |
| BOT CHORD 2x4 SP No 2 |                 |         |  |      |          | excent | end verti                | cals        |     |               |          |

 BOT CHORD
 2x4 SP No.2
 except end verticals.

 WEBS
 2x4 SP No.3
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

 OTHERS
 2x4 SP No.3
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 13-4-0.

(lb) - Max Horz 16=56(LC 11)

 Max Uplift
 All uplift 100 lb or less at joint(s) 16, 10, 14, 15, 12, 11

 Max Grav
 All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) All bearings are assumed to be User Defined crushing capacity of 425 psi.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 12, 11.



ENGINEERING BY AMITEK Affiliate 818 Soundside Road Edenton, NC 27932

