# PINEHURST-A, B, C

PLAN ID: 2234 (SLAB) - LEFT HAND NORTH CAROLINA

DATE:	REVISION:
06/09/2017	INITIAL RELEASE OF PLANS
07/21/2017	CLIENT REVISIONS
08/15/2017	REVISED PORCHES TO DROPPED SOFFIT
08.28.17	SPLIT SETS INTO SLAB AND BASEMENT VERSIONS
10/10/2017	CLIENT REVISIONS
10/20/2017	REVISED ROOF PITCH AT FRONT GABLE AT ELEVATIONS 'A' AND 'B'
11/01/2017	REMOVED PORCH RAILING FROM ELEVATION 'C' REMOVED DROPPED HEADER AT DINING
02/07/2018	ELECTRICAL REVISIONS
05/03/2018	ADDED EXTENDED PORCH OPTION
06/11/2018	CLIENT REVISIONS
11/14/2018	CLIENT REVISIONS
01/09/2019	REVISED CODE REFERENCES
06/18/2019	CLIENT REVISIONS
12/13/2019	CLIENT REVISIONS
02/28/2020	CLIENT REVISIONS

# SHEET INDEX:

CS ARCHITECTURALS - COVERSHEET

0 ARCHITECTURALS - QUICK VIEW

1A ARCHITECTURALS - ELEVATIONS A

1B ARCHITECTURALS - ELEVATIONS C

3A ARCHITECTURALS - ELEVATIONS C

3A ARCHITECTURALS - FLOOR PLANS A

3B ARCHITECTURALS - FLOOR PLANS B

3C ARCHITECTURALS - FLOOR PLANS C

4 ELECTRICAL - FLOOR PLANS

REVIEWERS STAMP LOCATION

MODEL 'PINEHURST' SQUARE FOOTAGES					
AREA		ELEV 'C'			
lst FLOOR	A	896 SF			
2nd FLOOR	`.	1311 SF			
TOTAL LIVING		2207 SF			
GARAGE		370 SF			
PORCH		68 SF			

MORGAN NORTH LOT 42

**66 YOUNG FARM DRIVE** 

LILLINGTON, NC 27546
CRAWL FOUNDATION



Express D.R.HORTON America's Builder

PINEHURST (SLAB)'

COVERSHEET

PLAN REV DATE

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SHEET NUMBER

# N.C ATTIC VENT CALCULATION FOR MODEL 'PINEHURST': 1:150 RATIO

(PER NCRC SECTION R806.2)

ROOF AREA I: = 1344 SF

HE NET FREE VENTILATING AREA SHALL NOT DE LESS THAN 150 OF THE AREA OF THE SPACE VISHTILATED, PROVIDED WITH AT LECATE OF PRECISIT AND WITH MADE THAN BO FRECISIT WITH AT LECATE OF THE UPPER PORTION OF THE SPACE OF WITH LATORS LOCATED IN THE UPPER PORTION OF THE SPACE OF WITH LATOR AT LEAST 3 THEFT ADON'T THE EARLY OR CRINICE VISTO SWITH THE BALLANCE OF THE REALE OR ORNICE VISTO SWITH THE BALLANCE OF THE REALE OR REPULLATION FROM THE SPACE OF THE REPULLATION OF THE SPACE OF THE SWITH THE SPACE OF THE SPACE OF THE SWITH THE PRINCIPLE OF THE SWITH THE SWITH THE SWITH THE PRINCIPLE OF THE SWITH THE SWITH THE PRINCIPLE OF THE SWITH THE SWITH THE PRINCIPLE OF THE PRINCIPLE OF THE SWITH THE PRINCIPLE OF THE PRINCIPLE O

EXCEPTIONS

I. EXCLOSED ATTIC/RAFTER SPACES REQUIRING LESS THAN
I. SQ FT OF VENTILATION MAY BE VENTED WITH CONTINUOUS
SOFFIT VENTILATION ONLY.

P. ENCLOSED ATTIC/RAFTER SPACES OVER INCONDITIONED SPACE MAY BE VENTED WITH CONTINUOUS SOFFIT VENT ONL

SPACE PAY BE VERIFY AIR CORNINOS SYMTY VERN C SPERAL CONTRACTOR SHALL VERIFY THE NET PREE WHILLAND OF THE VENT PRODUCT SELECTED BY OWNER WERFIY WHITH MAPPECHIER OF HICH AND LOW VENTS TO BE USED FOR HIMMAN CALCULATED VENTS REGUIRED. THE REGUIRED VENTLANDIOS SHALL BE MAINTAINED. PROVIDE INSULATION STOP SUCH THAT INSULATION DOCUMENT OF THE AIR NOVEMENT AS REGUIRED.

OGS NOT OBSTRUCT FREE AIR MOVEMENT AS REGUIRED YN THE BUILDING PEFICIAL.

LL OVERLAP FRANED ROOF AREAS SHALL HAVE PRINSE BETHENDE STENDEN THE ADJACHN ATTICS IN THE ROOF HEATHING (AS ALLONED BY THE STRUCTURAL ENGINEER) CHARLES AND ATTIC SYNTIALTICS HOMERY OF ALLONED BY THE STRUCTURAL ENGINEERY CHARLES AND ATTIC SYNTIAL ENGINEERY CHARLES AND ATTIC SYNTIAL SYNTIAL STRUCTURE MOTERAL BENEFIL EVENTED MOTERAL TO ATTIC SYNAEL SYNTIAL EVENTED MOTERAL SYNTIAL R DEVELOPER, AT ALL CANTILEVERED FLOORS, NITILEVERED ARCHITECTURAL POP-OUTS, AND ANY DOUBLE ANTILEVERED ARCHITECTURAL POP-CUTS, AND ANY DOU RAMING PROJECTIONS THAT ARE SEPARATED FROM THE ENTING CALCULATIONS SHOWN ABOVE, PROVIDE A ONTINIOUS 2" CORROSION RESISTANT SOFFIT VENT AT NOERSIDE OF FRAMED ELEMENT.

# OTFS.

ALL ROOF DRAINAGE SHALL BE PIPED TO STREET OR APPROVED DRAINAGE FACILITY.

- TRUSS MANUFACTURER SHALL SUBMIT STRUCTURAL CALCS AND SHOP DR TO THE BUILDER'S GENERAL CONTRACTOR AND BUILDING DEPARTMENT FOR REVIEW PRIOR TO FABRICATIONS.

IT SQUARE INCH VENT FOR EVERY ISO SQUARE INCHES OF CEILING

144 SO. N. = 1 SQ. FT.

BLDG. CEILING (SP) X 144 = BLDG (SQ. IN.)

BLDG. (SQ. N.) 150 = 5Q. IN. OF VENT REQUIRED

5Q. IN. OF VENT REQUIRED / 2 = 50% AT HIGH & 50% AT LOW.

545.12 SQ. IN. OF VENT AT HIGH \$ 645.12 SQ. IN. OF VENT AT LOW REQUIRED.

1344 SQ. FT. X |44 = 143536 SQ. IN. 143536 SQ. IN. / 150 = 1240.24 SQ. IN. OF VENT REQ'D 1240.24 SQ. IN. / 2 = 645.12 SQ. IN

-ALL PLIMBING VENTS SHALL BE COMBINED INTO A MINIMUM AMOUNT OF ROOF PENETRATIONS, ALL ROOF PENETRATIONS SHALL OCCUR TO THE REAR OF THE MAIN RIDGE

# N.C ATTIC VENT CALCULATION FOR MODEL 'PINEHURST': 1:300 RATIC

as an alternate to the l/150 ratio listed above, the net free cross-ventilation area may be reduced to 1/300 nema a class 1 of 11 vapor retarder is installed on the warm - in - winter side of the ceiling.

SE VINITUS INSCRIPTION TO THE CONTROLL IN CONTROL INCOLUTION INCOLUTION INCOLUTION INCOLUTION INCOLUTION INCOLUTI

## (PER NCRC SECTION R806.2)

I SQUARE INCH VENT FOR EVERY 300 SQUARE INCHES OF CEILING \*144 SQ. IN. = 1 SQ. FT. BLDG. CEILING (SF) X 144 = BLDG (SQ. IN.) BLDG. (SQ. IN.) / 300 = SQ. IN. OF VENT REQUIRED SQ. IN. OF VENT REQUIRED / 2 = 50% AT HIGH & 50% AT LOW.

ROOF AREA Is = 1344 SF

1344 S.Q. FT. V. 144 = 193536 S.Q. IN. 193536 S.Q. IN. / 300 = 6451.2 S.Q. IN. / FVENT REQ'D 6451.2 SQ. IN. / 2 = 32256 SQ. IN. 32256 SQ. IN. OF VENT AT HIGH & 32256 SQ. IN. OF VENT AT LOW REQUIRE

GRADE CONDITIONS MAY VARY FOR INDIVIDUAL SITE FROM THAT SHOWN. BUILDER SHALL VERIFY AND COORDINATE PER ACTUAL SITE CONDITIONS.

WINDOW HEAD HEIGHTS: IST FLOOR = 6-8" U.N.O. ON ELEVATIONS. 2ND FLOOR = 7'-0" U.N.O. ON ELEVATIONS.

ROOFING: PITCHED SHINGLES PER DEVELOPER

WINDOWS: MANUFACTURER PER DEVELOPER, DIVIDED LITES AS SHOWN ON THE EXTERIOR ELEVATIONS

ENTRY DOOR: AS SELECTED BY DEVELOPER.

GARAGE DOORS: AS SELECTED BY DEVELOPER, RAISED PANEL AS SHOWN.

ALL EXTERIOR MATERIALS TO BE INSTALLED PER MANUFACTURER'S WRITTEN INSTRUCTIONS.

PROTECTION AGAINST DECAY:
(ALL PORTIONS OF A PORCH, SCREEN PORCH OR DECK FROM THE BOTTOM OF
THE HEADER DOWN, INCLIDING POST, RAILS, PICKETS, STEPS AND FLOOR STRUCTURE.)

INGULATION: PER TABLE NIO2.1.2.
EXTERIOR WALLS:
CELING WITH ATTIC ABOVE:
FLOOR OVER GARAGE:
R-49 BATTS MINIMM. VERIFY
R-49 BATTS MINIMM. VERIFY ATTIC KNEEWALL: R-I9 BATTS MINIMUM, VERIFY CRAWL SPACE FLOORING: R-I9 BATTS MINIMUM, VERIFY

# **KEY NOTES:**

## MASONRY:

ADHERED STONE VENEER AS SELECTED BY DEVELOPER, HEIGHT AS NOTED

MASONRY FULL BRICK AS SELECTED BY DEVELOPER, HEIGHT AS NOTED.

MASONRY FULL STONE AS SELECTED BY DEVELOPER, HEIGHT AS NOTED.

8" SOLDIER COURSE. ROWLOCK COURSE

TYPICALS:

CORROSION RESISTANT SCREEN LOUVERED VENTS, SIZE AS NOTED.

CODE APPROVED TERMINATION CHIMNEY CAP.

CORROSION RESISTANT ROOF TO WALL FLASHING. CODE COMPLIANT FLASHING PER NCRC R905.2.8.3

IO STANDING SEAM METAL ROOF, INSTALL PER MANUFCATURER'S WRITTEN INSTRUCTIONS.

DECORATIVE WROUGHT IRON, SEE DETAILS.

SIDING: VINYL SHAKE SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT SHAKE SIDING PER DEVELOPER W/ IX4 CORNER TRIM BOARD.)

3 VINYL LAP SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT LAP SIDING PER DEVELOPER W IX4 CORNER TRIM BOARD.)

4 VINYL WAVY SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS: FIBER CEMENT WAVY SIDING PER DEVELOPER W IX4 CORNER TRIM BOARD.)

VINYL BOARD AND BATT SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER.

(AT SPECIFIED LOCATIONS: FIBER CEMENT PANEL SIDING W IX3 BATTS AT 12° O.C. PER DEVELOPER W IX4 CORNER TRIM BOARD.)

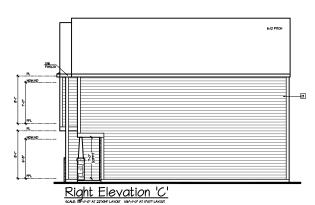
VINYL TRIM SIZE AS NOTED (AT SPECIFIC LOCATIONS: IX FIBER CEMENT TRIM OR EQUAL, UN.O. SIZE AS NOTED

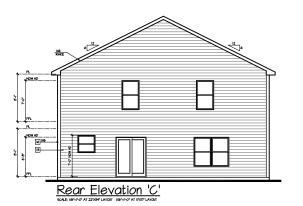
PYPON SHUTTERS, TYPE AS SHOWN, SIZE AS NOTED. (AT SPECIFIC LOCATIONS: FALSE VINYL SHUTTERS, TYPE AS SHOWN, SIZE AS NOTED.)

ALL MINDOWS MIOSE OPENING IS LESS THAN 24" ABOVE THE FINISH FLOOR AND MIOSE OPENING IS GREATER THAN 12" ABOVE THE OUTSIDE WALKING SURFACE MUST HAVE WINDOW OPENING LIMITING DEVICES COMPLYING WITH THE NCRC SECTION RSIZ.21 AND RSIZ.22.2.

AT SINGLE FAMILY DETACHED PLANS: PREFINISHED VENTED SOFFIT AT EAVE PER MANUFACTURER. (VERIFY FIRE SEPARATION DISTANCE FOR SOFFIT PROTECTION PER NCRC SECTION R302.1.1 AND TABLE R302.1)







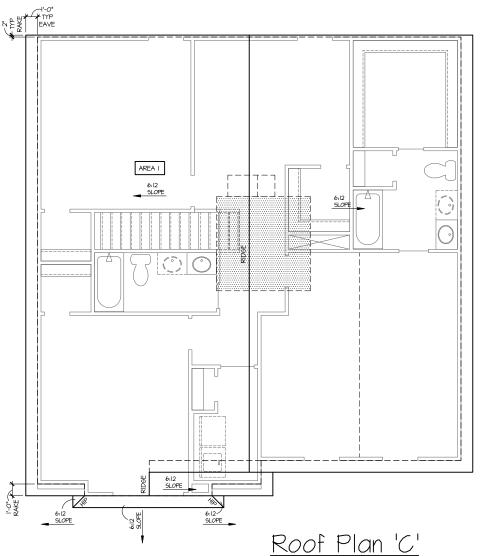
AVAILABLE WITH OPTIONAL 9'-1" FIRST FLOOR PLATE

NOTES AT OPT 9'-1" PLT:

- WDW HT SET AT 7'-6"

- INTERIOR SOFFITS AT 8'-0"

- EXTERIOR SOFFITS AT 8'-0"



SCALE: I/4"=I'-0" AT 22"X34" LAYOUT I/8"=I'-0" AT II"XIT" LAYOUT

TRUSS MANUFACTURE TO VERIFY HEELS PER COMMUNITY STANDARDS, BUILDER TO VERIFY PRIOR TO CONSTRUCTION

ELEVATION

(SF

PINEHURST

20

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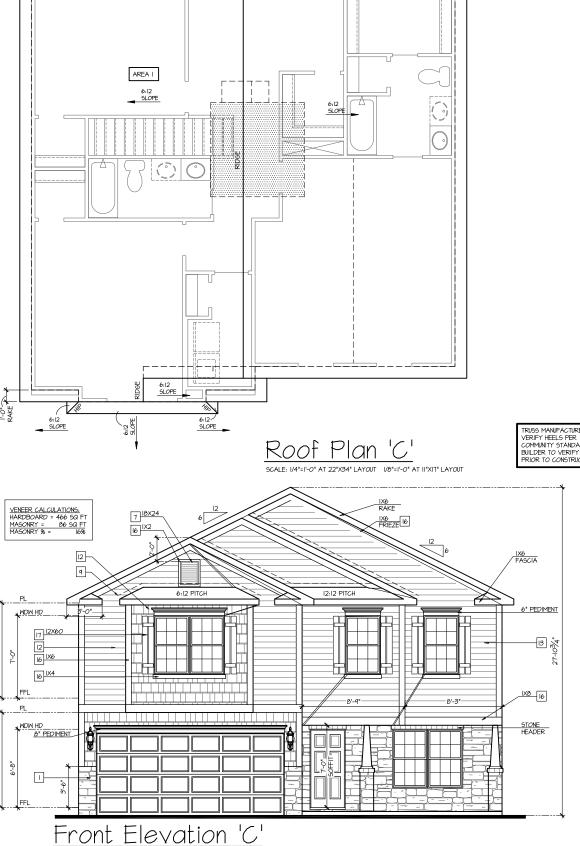
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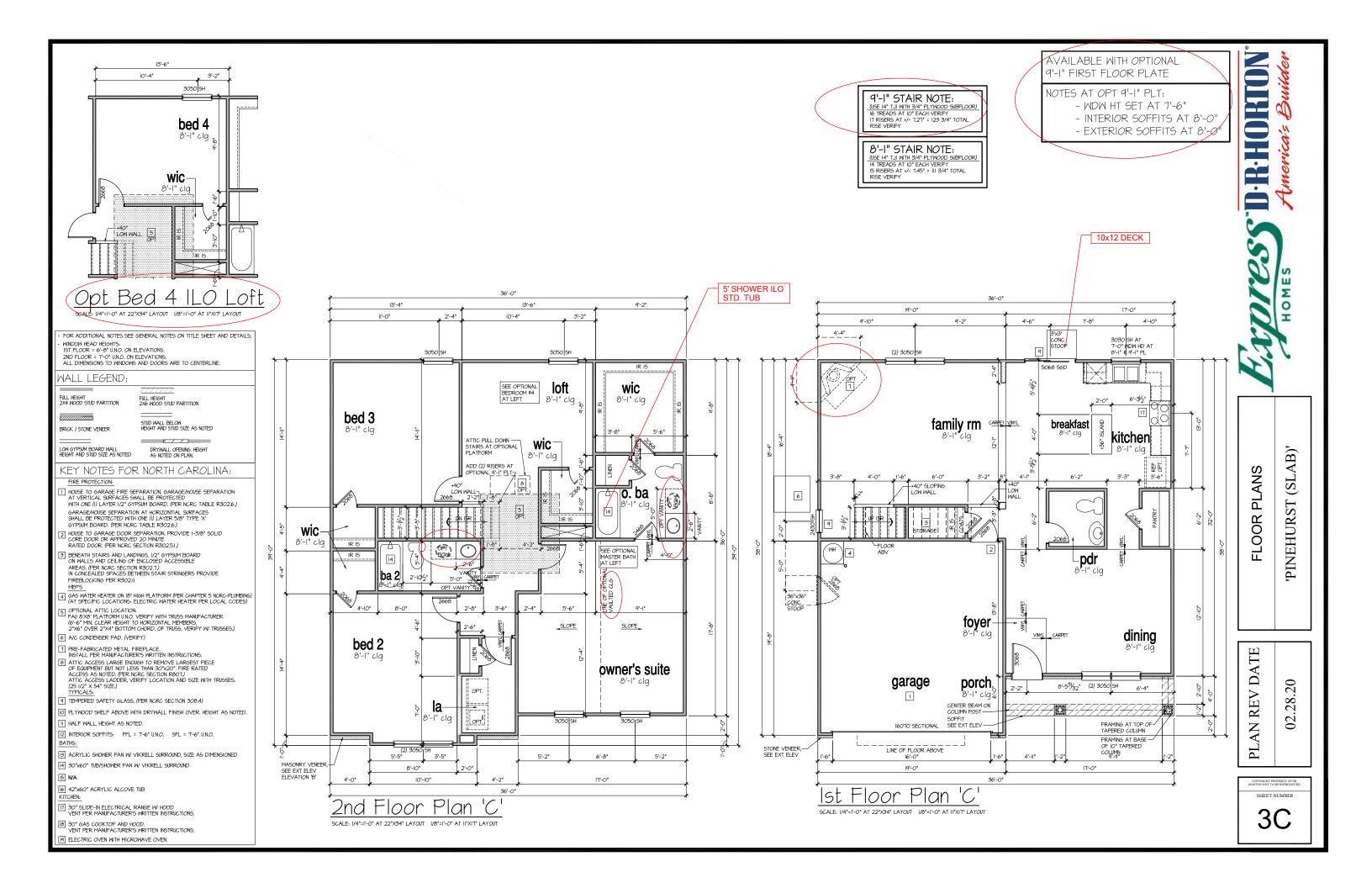
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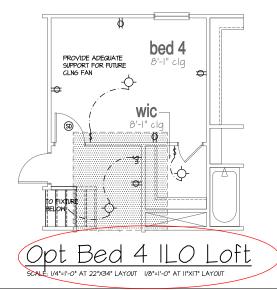
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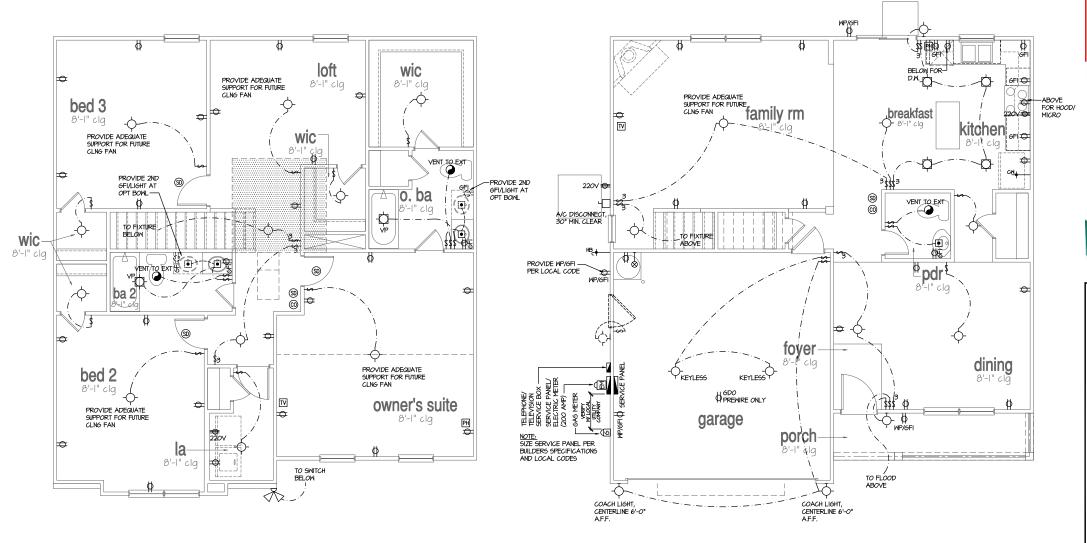


## NOTES

- PROVIDE GROUNDING ELECTRICAL ROD PER LOCAL CODES.
- PROVIDE AND INSTALL ARC FAULT CIRCUIT-INTERRUPTERS (AFCI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
   ALL EXHAUST FANS SHALL HAVE BACKDRAFT DAMPERS.
- FAN/LIGHTS IN WET/DAMP LOCATIONS SHALL BE LABLED "SUITABLE FOR WET OR DAMP LOCATIONS.
- ELECTRICAL SYSTEMS ARE SHOWN FOR INTENT ONLY. THESE SYSTEMS SHALL BE ENGINEERED BY OTHERS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER INSTALLATION AND PLACEMENT.
- PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES,
   PROVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRIPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS.
- HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.
- ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FURNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUMP PITS, DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS.
- PROVIDE POWER, LIGHT AND SWITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S WRITTEN INSTRUCTIONS.

## LEGENIT

LEGI	LEGEND:					
ф	DUPLEX OUTLET	<b>\( \rightarrow \)</b>	CEILING MOUNTED INCANDESCENT LIGHT FIXTURE			
ØwP/GFI	WEATHERPROOF 6FI DUPLEX OUTLET	ф	WALL MOUNTED INCANDESCENT			
∯ 6FI	GROUND-FAULT CIRCUIT-INTERRUPTER DUPLEX OUTLET		LIGHT FIXTURE  RECESSED INCANDESCENT LIGHT FIXTURE			
ф	HALF-SWITCHED DUPLEX OUTLET	Ф	(VP) = VAPOR PROOF			
<b>Ф</b> 220∨	220 VOLT OUTLET	•	CEILING MOUNTED LED LIGHT FIXTURE (VP) = VAPOR PROOF			
0	REINFORCED JUNCTION BOX	•	EXHAUST FAN (VENT TO EXTERIOR)			
\$	WALL SMITCH	-	EXHAUST FAN/LIGHT COMBINATION (VENT TO EXTERIOR)			
\$з	THREE-WAY SMITCH		FLUORESCENT LIGHT FIXTURE			
\$4	FOUR-WAY SWITCH		FLUCKESCENI LIGHT FIXTURE			
CH	CHIMES		TECH HUB SYSTEM			
9	PUSHBUTTON SWITCH	X	CEILING FAN (PROVIDE ADEQUATE SUPPORT)			
<u>so</u>	IIOV SMOKE ALARM W BATTERY BACKUP		CEILING FAN WITH INCANDESCENT			
€	IIOV SMOKE ALARM CO2 DETECTOR COMBO	X	LIGHT FIXTURE (PROVIDE ADEQUATE SUPPORT)			
①	THERMOSTAT	∞	GAS SUPPLY WITH VALVE			
PH	TELEPHONE					
īV	TELEVISION	—+ <sub>HB</sub>	HOSE BIBB			
	ELECTRIC METER	-+ <sub>CM</sub>	I/4" WATER STUB OUT			
ı	ELECTRIC PANEL	Я				
	DISCONNECT SMITCH	<del>-</del> 5	WALL SCONCE			



2nd Floor Plan 'A'
5cale: 1/4"=1"-0" AT 22"x34" LAYOUT 1/8"=1"-0" AT 11"X1" LAYOUT

Ist Floor Plan 'A'
scale: 1/4'=1'-0' at 22'x34' LAYOUT 1/8'=1'-0' AT 11'x17' LAYOUT

ALL ELEVATIONS ARE SIMILAR FLOOR PLANS
'PINEHURST (SLAB)'

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D-R-HORTON

PLAN REV DATE 02.28.20

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SHEET NUMBER

## DESIGN SPECIFICATIONS:

Construction Tupe: Commerical □ Residential □

Applicable Building Codes:

• 2018 North Carolina Residential Building Code with All Local Amendments ASCE 1-10: Minimum Design Loads for Buildings and Other Structures

sign L	oads:		
Ĭ.	Roof	Live Loads	
	1.1.	Conventional 2x	20 PSF
	1.2.	Trues	20 PSF
		12.1. Attic Truss	60 PSF
2.	Roof	Dead Loads	
	2.1.	Conventional 2x	10 PSF
	2.2.	Trues	20 PSF
3.	Snow		15 PSF
	3.1.	importance Factor	10
4.	Floor	Live Loads	
	2.1	Time Discelline	AM DOE

 Typ. Dwelling .......
 Sleeping Areas ... . 3Ø PSF 

5. Floor Dead Loads 5.1. Conventional 2x 52. I-Joist ..... IA PSE 5.3. Floor Truss . 15 PSF 6. Ultimate Design Wind Speed (3 sec. gust) ..... 6.l. Exposure ......

62. Importance Factor..... 63. Wind Base Shear 6.3.1. Vx = 6.32.Vu =

1. Component and Cladding (in PSF)

	7			
MEAN ROOF HT.	UP TO 30°	30'1"-35'	35'1"-40'	40'1"-45'
ZONE I	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-20.2
ZONE 2	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 4	182,-19.0	19.2,-20.0	19.9,-20.7	20.4,-21.3
ZONE 5	182,-240	19.2,-25.2	19.9,-26.1	20.4,-26.9

3.	Seismi	Ċ	
	8.1.	Site Class	D
	82.	Design Category	С
	83.	Importance Factor	10
	8.4.	Seismic Use Group	1
	85.	Spectral Response Acceleration	

8.5.1. Sms = %g 8.5.2. Sml = %g

8.52.5ml = %g 8.6. Seismic Base Shear 861 Vx = 8.62.Vy =

8.1. Basic Structural System (check one) □ Bearing Wall
 □ Building Frame
 □ Moment Frame

□ Dual w/ Special Moment Frame
□ Dual w/ Intermediate R/C or Special Steel ☐ Inverted Pendulum

Wind ⊠ 2000psf

of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95%

polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.

Structural steel shall receive one coat of shop applied

Welding Society's Structural Welding Code AWS DII. Electrodes for shop and field welding shall be class ET@XX. All welding shall be performed by a certified welder per the above

Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.

Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 38: "Building Code Requirements for Reinforced Concrete" and ACI 301:

entrainment amounts (in percent) shall be within -1% to +2% of



# STRUCTURAL PLANS PREPARED FOR:

# PINEHURST - LH

PROJECT ADDRESS:

OWNER: DR Horton, Inc. 8001 Arrowridge Blyd

DESIGNER: GMD Design Group 102 Fountain Brook Circle

Suite C Cary, NC 27511

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify 6LPMIT Engineering, Laboratory 4 Testing, P.C. before construction begins.

# PLAN ABBREVIATIONS:

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	R9	ROOF SUPPORT
CJ	CEILING JOIST	SC.	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
D6P	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
EW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
ОС	ON CENTER	TYP	TYPICAL
PSF	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	ww.	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMI Engineering, Laboratory 4 Testing, P.C. (SUMMI) prior to the initial design. Therefore, trus and joist directions were assumed based on the information provided by <u>DR Horton. Inc.</u> Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately

## SHEET LIST:

REVISION LIST:

Date

5.16.17

5 12.18.18 17837

8.13.18

9 11.30.18 17.837.24

IØ 3.14.19 21851

II 62819 2185IR

6.19.17 125Ø4R2

8.14.17 125Ø4R3

11.17.17 125Ø4R4

7.1Ø18 17837R

10518 17837R3

17837R2

12504R

Revision

Sheet No.	Description
CSI	Cover Sheet, Specifications, Revisions
51.Øm	Monolithic Slab Foundation
SIØs	Stem Wall Foundation
SIØc	Crawl Space Foundation
S1.Øb	Basement Foundation
52.Ø	Basement Plan
53.Ø	First Floor Plan
54.Ø	Second Floor Plan
95.Ø	Roof Framing Plan

Description

Revised garage slab note. Revised roof

overframing. Verified floor joist layouts provided by 84 Lumber dated 3.31.15. Verified roof truss

ayouts provided by 84 Lumber dated 3.28.11.

Added stem wall foundation plan

Revised columns at front porch

Revised SYP and pressure treated member notes

Added crawl space foundation

Revised per new architectural files. Added

Revised dimension at front porch column

included stick framing option at extended porch

Revised NC version only for 2018 NCRC

Revised TN version only for 2018 IRC

Added optional vault

# DR HORTON PROJECT SIGN-OFF:

Manager	Signature	
Operations		
Operations System		
Operations Product Development		

# summit



# GENERAL STRUCTURAL NOTES:

The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory 4 Testing, P.C. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.

The structure is only stable in its completed form. The contractor

hall provide all required temporary bracing during construction to stabilize the structure.

The SER is not responsible for construction sequences, methods,

or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents

should any non-conformities occur.

Any structural elements or details not fully developed on the and state that all the completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions,

is not the responsibility of the SER or SUMMIT.

Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before

accuracy and report any discrepancies to Sull'IIII before construction begins.

6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.

7. This structure and all construction shall conform to all

applicable sections of the international residential code. This structure and all construction shall conform to all

applicable sections of local building codes.

9. All structural assemblies are to meet or exceed to requirements of the current local building code.

The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.

The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade. Any fill shall be placed under the direction or recommendation

maximum dry density.

Excavations of footings shall be lined temporarily with a 6 mil

No concrete shall be placed against any subgrade containing

# STRUCTURAL STEEL:

Structural steel shall be fabricated and erected in accordance with the American institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design latest editions.

rust-irhibitive paint. All steel shall have a minimum yield stress ( $\mathbf{F_y}$ ) of 36 ksi unless

Welding shall conform to the latest edition of the American

"Specifications for Structural Concrete for Buildings".

Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air target values as follows: 3.1. Footings: 5%

32 Exterior Slabs: 5%

No admixtures shall be added to any structural concrete without written permission of the SER

Concrete slabs-on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete 9lab and 9lab Construction".

subgrade modulus of k-250 pcl and a design loading of 200 pst. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.

Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted

Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished

Reinforcing steel may not extend through a control joint.
Reinforcing steel may extend through a saw cut joint.
All welded ulve fabric (WIIIF) for concrete slabs-on-grade shall be placed at mid-depth of slab. The WIIIF, shall be securely

## CONCRETE REINFORCEMENT:

Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered abrasion resistance, and residual strength

Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcemen

manufactured for use as concerns secondary reminorcement. Application of fillbermesh per cubic yard of concrete shall equal a minimum of 01% by volume (15 pounds per cubic yard) Filbermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry

Steel reinforcing bars shall be new billet steel conforming to

ASTM A615, grade 60.
Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures" Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same

size/spacing as the horizontal reinforcement with a class B Lab reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masorry shall be a minimum of 48 bar diameters.

Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.

Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted WOOD FRAMING:

Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be

Southern-Yellow-Pine (GYP) 2.

LVL or PSL engineered wood shall have the following minimum

design values: 2.1. E = 1,900,000 psi 2.2. Fb = 2600 psi 23.Fv = 285 psi

Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-I5. All other moisture exposed wood shall be treated in accordance

with AWPA standard C-2 Nails shall be common uire nails unless otherwise noted Lag screws shall conform to ANSI/ASME standard BI821-1981. Lead holes for lag screws shall be in accordance with NDS

specifications.

All beams shall have full bearing on supporting framing members unless otherwise noted.

Exterior and load bearing stud walls are to be 2x4.57P.2.6 [6" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimu of one king stud shall be placed at each end of the header. King studs shall be continuous.

Individual stude forming a column shall be attached with one 10d nail \*6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer. Multi-ply beams shall have each ply attached with (3) IOd nails a

Four and five ply beams shall be bolted together with (2) rows of 1/2" diameter through bolts staggered @ 16" O.C. unless nated atherwise

The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for

responsibility for the correctness for the structural design for the wood trusses.

The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures."

(ASCE 1-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to

the trusses.
The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."

The truss manufacturer shall provide adequate bracing Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for

the trusses.

Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall

## EXTERIOR WOOD FRAMED DECKS:

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide
"Residential and Commercial," and all other applicable APA standards

All structurally required wood sheathing shall bear the mark of

Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction

perpendicular to framing, unless noted otherwise. Roof sheathing shall be AFA rated sheathing exposure |  $\sigma$  2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nall at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use the long direction perpendicular to framing, Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.

Ubod floor sheathing shall be APA rated sheathing exposure I or 2. Attach sheathing to Its supporting framing with (I)-8d CC ringshark nail at 6"olc at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing Sheathing shall have a span.

applied perpendicular to framing. Sheathing shall have a spain rating consistent with the framing spacing. Use suitable edge support by use of T4G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing Apply building paper over the sheathing as required by the

state Building Code. Sheathing shall have a 1/8" gap at panel ends and edges as mended in accordance with the APA

# STRUCTURAL FIBERBOARD PANELS:

Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.

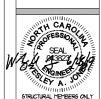
All structurally required fiberboard sheathing shall bear the mark of the AFA.

mians or the AFA.

Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more

Sheathing shall have a 1/8" gap at panel ends and edges are

PROJECT: Pinehurst - LH



SCALE: 22x34 1/4'=1'-@' lk/1 1/8'=1'-@'

PROJECT 4: 528-06R: 21851R DRAWN BY: EMB CHECKED BY: UAL

REFER TO COVERSHEET FOR A COMPLETE LIST OF REVISIONS

## FOUNDATION NOTES:

- FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL
- STRUCTURAL CONCRETE TO BE  $F_c$  = 3000 PSI, PREPARED AND PLACED IN
- ACCORDANCE WITH ACI STANDARD 318.
  FOOTINGS TO BE PLACED ON UNDISTURBED EARTH, BEARING A MINIMUM OF 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE
- CODE ENFORCEMENT OFFICIAL.
  FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 2000 PSF CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING THE
- SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION. FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE ELEMENTS. PROVIDE 2" MINIMUM FOOTING PROJECTION FROM THE FACE OF
- FIRADONINT :

  6. MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN SECTION R404.1 OF THE 2018 NORTH CAROLINA RESIDENTIAL
- PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL.
- PROVIDE FOUNDATION WATERPROPING, AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS.
  PROVIDED PERMETER INSULATION FOR ALL FOUNDATIONS PER 2018 NORTH
- CAROLINA RESIDENTIAL BUILDING CODE. CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK

- VENEREN.
  CRAIUL SPACE TO BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS.
  FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2016 NORTH
  CAROLINA RESIDENTIAL CODE SECTION R403.16. MINIMUM 1/2" DIA. BOLTS SPACED AT 6"-0" ON CENTER WITH A 1" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE. ANCHOR BOLTS SHALL BE 12" FROM THE END OF EACH PLATE SECTION, MINIMUM (2) ANCHOR BOLTS PER PLATE SECTION, ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE. ABBREVIATIONS:

DJ = DOUBLE JOIST GT = GIRDER TRUSS SC = STUD COLUMN 9J = 9INGLE JOI9T FT = FLOOR TRUSS DR = DOUBLE RAFTER EE = EACH END TJ = TRIPLE JOIST OC = ON CENTER CL = CENTER LINE PL = POINT LOAD

- 10. ALL PIERS TO BE 16"x16" MASONRY AND ALL PILASTERS TO BE 8"x16" MASONRY TYPICAL (UNO)
- WALL FOOTINGS TO BE CONTINUOUS CONCRETE, SIZES PER STRUCTURAL PLAN.
- A FOUNDATION EXCAVATION OBSERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER OR HIS QUALIFIED
  REPRESENTATIVE. IF ISOLATED AREAS OF YIELDING MATERIALS AND/OR
  POTENTIALLY EXPANSIVE SOILS ARE OBSERVED IN THE FOOTING EXCAVATIONS AT THE TIME OF CONSTRUCTION, SUMMIT ENGINEERING, LABORATORY 4 TESTING, P.C. MUST BE PROVIDED THE OPPORTUNITY TO REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE PLACEMENT.
- ALL FOOTINGS & SLABS ARE TO BEAR ON UNDISTURBED SOIL OR 95% COMPACTED FILL, VERIFIED BY ENGINEER OR CODE OFFICIAL.

REFER TO BRACED WALL PLAN FOR PANEL LOCATIONS AND ANY REQUIRED HOLDOWNS, ADDITIONAL INFORMATION PER SECTION R602.108 AND FIGURES R602.1065, R602.10.T. R602.10.8(1) AND R602.10.8(2) OF THE 2015 IRC

NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND NOT BRICK VENEER UNO

NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED WHEN SLAB IS INSTALLED ON WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS

REINFORCE GARAGE PORTAL WALLS PER FIGURE R602.10.9 OF THE 2015 IRC.

BEAM POCKETS MAY BE SUBSTITUTED FOR MASONRY PILASTERS AT GIRDER ENDS, BEAM POCKETS SHALL HAVE A MINIMUM 4" SOLID MASONRY BEARING.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS,

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DR HORTON
COMPLETED/REVISED ON 6/12/2018, IT IS THE RESPONSIBILITY OF
THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

# STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

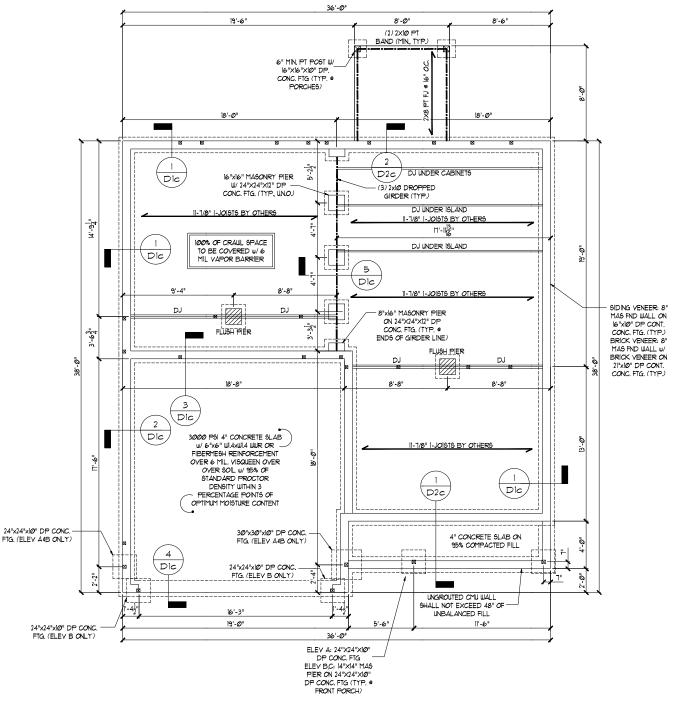
STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

CRAWL SPACE FOUNDATION PLAN

9CALE: 1/4"=1'-@" ON 22"x34" OR 1/8"=1'-@" ON 11"x17"

18"x24" MIN. CRAWL SPACE ACCESS DOOR TO BE LOCATED IN FIELD PER BUILDER PROVIDE MIN (2) 2xIQ HEADER OVER DOOR W/ MIN. 4" BEARING EACH END. AVOID SHOUN POINT LOADS.

DECK FLOOR JOISTS SHALL BE SPACED AT MAX 12" ON CENTER WHEN DECKING INSTALLED DIAGONALLY



CRAWL SPACE FOUNDATION - ALL ELEVATIONS





Space i PROJECT: Pinehurst -Crawl



SCALE: 22x34 1/4"=1"-6" lk/1 1/8"=1"-6" PROJECT 4 528-08R: 21851R DRAWN BY: EMB

CHECKED BY: WAJ

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S1.0c

	REQUIRED	BRACED W	ALL PANEL CONNE	CTIONS
	MATERIAL	MIN. THICKNESS	REQUIRED CONNECTION	
METHOD			@ PANEL EDGES	@ INTERMEDIATE SUPPORTS
C5-W5P	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS # 6" O.C.	6d COMMON NAILS @ 12" O.C.
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** @ T* O.C.	5d COOLER NAILS** @ T" O.C.
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS 9 6" O.C.	6d COMMON NAILS 9 12" O.C.
PF .	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R6/02/10/6.4	PER FIGURE R602.10.6.4
			T DED TADLE DIAGO	

## "OR EQUIVALENT PER TABLE RT02.3.5

## GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AMENDMENTS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS. CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.
- CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED
- O RESIST ALL FORCES ENCOUNTERED DURING ERECTION. PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS:
- MICROLLAM (L.V.L.): F<sub>b</sub> = 2600 PSI, F<sub>v</sub> = 285 PSI, E = 19x10<sup>6</sup> PSI PARALLAM (PSI.): F<sub>b</sub> = 2900 PSI, F<sub>v</sub> = 290 PSI, E = 125x10<sup>6</sup> PSI ALL WOOD MEMBERS SHALL BE <sup>9</sup>2 SYP UNLESS NOTED ON PLAN. ALL STUD
- COLUMNS AND JOISTS SHALL BE 12 SYP (INO).
  ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 12 SYP STUD COLUMN AT EACH
- END UNLESS NOTED OTHERWISE.
- ALL REINFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO ASTM A615 AND SHALL HAVE A MINIMUM COVER OF 3". FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2016 NORTH
- CAROLINA RESIDENTIAL CODE SECTION R403.16. MINIMUM 1/2" DIA. BOLTS SPACED AT 6'-0" ON CENTER WITH A 7" MINIMUM EMBEDMENT INTO MASONRY OR CONCRETE ANCHOR BOLTS SHALL BE 12" FROM THE END OF EACH PLATE SECTION MINIMUM (2) ANCHOR BOL TS PER PLATE SECTION ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.
- CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN
- PERPENDICULAR TO RAFTERS.
  FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH 1/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D31; MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP \*2 DROPPED FOR NON-LOAD BEARING HEADERS EXCEEDING 8-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP 12, DROPPED. (UNLESS NOTED OTHERWISE)

DJ = DOUBLE JOIST GT = GIRDER TRUSS SC = STUD COLUMN SJ = SINGLE JOIST FT = FLOOR TRUSS DR = DOUBLE RAFTER

EE = EACH END TJ = TRIPLE JOIST CL = CENTER LINE

TR = TRIPLE RAFTER PL = POINT LOAD

DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

JOIST & BEAM SIZES SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

INSTALL ANY REQUIRED HOLDOWNS PER SECTION R602.10.8(1) AND R602.10.8(2) OF THE 2015 IRC

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DR HORTON COMPLETED/ACTIVES 1950 ON 61/2008, IT S'THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY ( TESTING, P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

NOTE: MEMBER NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUTBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

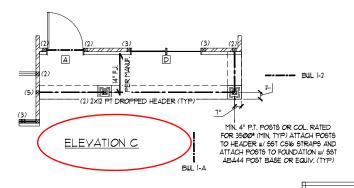
# STRUCTURAL MEMBERS ONLY

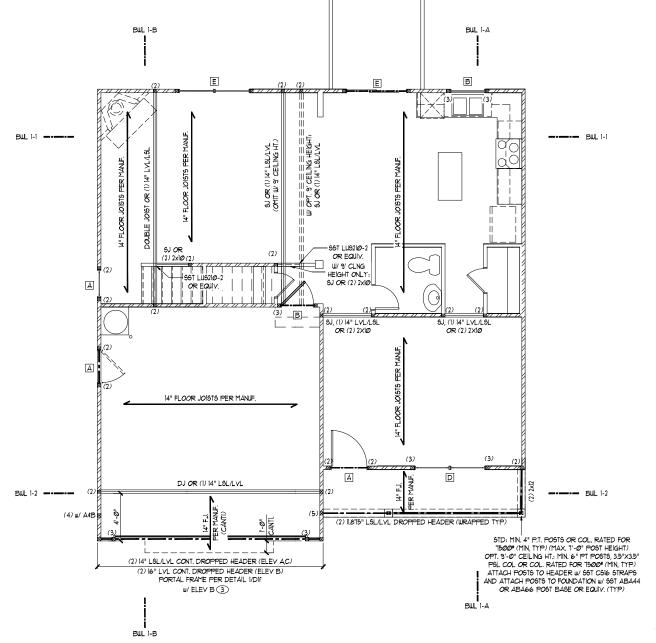
ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC

# FIRST FLOOR FRAMING PLAN

SCALE: 1/4"=1"-0" ON 22"x34" OR 1/8"=1"-0" ON 11"x17"





FIRST FLOOR FRAMING PLAN - ELEVATION A

FIRST FLOOR BRACING (FT)					
CONTINUOUS SHEATHING METHOD					
REQUIRED PROVIDED					
BWL 1-1	11.0	21.5			
BWL 1-2	11.00	11.5			
BWL 1-A	103	32.0			
BWL 1-B 103 35.1					

HE	HEADER SCHEDULE					
TAG	SIZE	JACKS (EACH END)				
А	(2) 2x6	(1)				
В	(2) 2x8	(2)				
С	(2) 2x1Ø	(2)				
D	(2) 2x12	(2)				
E	(2) 9-1/4" L9L/LVL	(3)				
F	(3) 2x6	(1)				
G	(3) 2x8	(2)				
Н	(3) 2xlØ	(2)				
	(3) 2xl2	(2)				

HEADER SIZES SHOWN ON PLANS ARE MINIMUMS. GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE SC NOTED ON PLAN OVERRIDE SC LISTED ABOVE.

L11	LINTEL SCHEDULE			
TAG	TAG: SIZE			
0	L3x3xl/4"	LESS THAN 6'-0"		
2	L5x3x1/4"	6'-0" TO 10'-0"		
3	L5x3-1/2"x5/16"	GREATER THAN 10'-0"		
4	L5x3-1/2"x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS		

SCREWS STAGGERED @ 16" O.C. (TYP FOR (3))

ALL HEADERS WHERE BRICK IS USED, TO BE: (UNO)

## WALL STUD SCHEDULE

1<u>ST & 2ND FLOOR LOAD BEARING STUDS:</u>
2x4 STUDS @ 16" O.C. OR 2x6 STUDS @ 24" O.C. IST FLOOR LOAD BEARING STUDS W/ WALK-UP ATTIC: 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BASEMENT LOAD BEARING STUDS: 2x4 STUDS # 12" O.C. OR 2x6 STUDS # 16" O.C. NON-LOAD BEARING STUDS (ALL FLOORS): 2x4 STUDS @ 24" O.C. TWO STORY WALLS: 2x4 STUDS # 12" O.C. OR 2x6 STUDS # 16" O.C. BALLOON FRAMED W/ CROSS BRACING . 6'-0" O.C. VERTICALLY

KING STUD R	EQUIREMENT:
OPENING WIDTH	KINGS (EACH END
LESS THAN 3'-Ø"	(1)
3'-Ø TO 4'-Ø"	(2)
4'-0" TO 8'-0"	(3)
8'-0" TO 12'-0"	(5)
12'-Ø" TO 16'-Ø"	(6)

KING STUD REQUIREMENTS ABOVE DO NOT APPLY TO PORTAL FRAMED OPENINGS

# BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2015 INTERNATIONAL RESIDENTIAL CODE AS ALLOWED PER SECTION R60210 OF THE 2018 NC RESIDENTIAL CODE WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND
- SPEEDS UP TO 130 MPH. REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING
- 3. BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN
- ACCORDANCE WITH IRC TABLE REØ2.104.

  ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED 10 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.5. THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD (UNO).
- FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL
- OPENINGS, AND ON GABLE END WALLS.
  FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL
- ENGINEERING CALCULATIONS.

  9. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 10 FEET OF
- EACH END OF A BRACED WALL LINE. 10. THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS
- SHALL NOT EXCEED 20 FEET. MASONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R60210.9 OF THE 2015 IRC.
- BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602103
   BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.8.2 AND
- FIGURES R602.10.8(1)4(2)4(3).

  14. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE
- DESIGNED IN ACCORDANCE WITH SECTION R602.10.11 PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.6.4 (UNO)
- 16. ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.

GB = GYP9UM BOARD WSP = WOOD STRUCTURAL PANEL CS-XXX = CONT. SHEATHED ENG = ENGINEERED SOLUTION PF = PORTAL FRAME PF-ENG = ENG. PORTAL FRAME Pinehust -First 1 TH CARO

STRUCTURAL MEMBERS ONL'

Framing

b

summi

TH CAROLA

Testing, C-4361

8CALE: 22x34 1/4'=1'-6'' lbf1 1/8'=1'-6'' PROJECT 4 528-06R: 21851R DRAWN BY: EMB CHECKED BY: WAJ

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S3.Ø

HEADER SCHEDULE				
TAG	TAG SIZE			
Α	(2) 2x6	(1)		
В	(2) 2x8	(2)		
С	(2) 2xlØ	(2)		
Δ	(2) 2x12	(2)		
E	(2) 9-1/4" L5L/LVL	(3)		
F	(3) 2x6	(1)		
G	(3) 2x8	(2)		
H	(3) 2xlØ	(2)		
	(3) 2xl2	(2)		

HEADER SIZES SHOWN ON PLANS ARE MINIMMS, GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. ALL HEADERS TO BE DROPPED UNLESS NOTED OTHERWISE. SC NOTED ON PLAN OVERRIDE SC LISTED ABOVE.

LINTEL SCHEDULE			
TAG	OPENING SIZE		
0	L3x3x1/4"	LESS THAN 6'-0"	
2	L5x3xl/4"	6'-0" TO 10'-0"	
3	3 L5x3-1/2"x5/16"		
4	L5x3-1/2"x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS	

SECURE LINTEL TO HEADER w/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED @ 16" O.C. (TYP FOR (3))

ALL HEADERS WHERE BRICK IS USED, TO BE: (UNO)

## WALL STUD SCHEDULE

15T 4 2ND FLOOR LOAD BEARING STUDS:
2x4 STUDS = 16" O.C. OR 2x6 STUDS = 24" O.C.
15T FLOOR LOAD BEARING STUDS = 10" O.C. OR 2x6 STUDS = 16" O.C.
BASEMENT LOAD BEARING STUDS = 16" O.C.
2x4 STUDS = 0" O.C. OR 2x6 STUDS = 16" O.C.
NON-LOAD BEARING STUDS (ALL FLOORS):
2x4 STUDS = 24" O.C.
TWO STORY WALLS:
2x4 STUDS = 24" O.C.
TWO STORY WALLS:
2x4 STUDS = 0" O.C. OR 2x6 STUDS = 16" O.C. BALLON
PRAMED = 10" O.C. OR 2x6 STUDS = 16" O.C. BALLON
PRAMED = 10" O.C. OR 2x6 STUDS = 16" O.C. BALLON
PRAMED = 10" O.C. OR 2x6 STUDS = 16" O.C. BALLON

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL, PLANS PROVIDED BY DR. HORTON COMPLETED/REVISED ON 6/12/20/8. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PROTOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. CANNOT GLIARANTEE THE ADEQUACY OF THESE STRUCTURAL. PLANS WHEN USED WITH ARCHITECTURAL. PLANS WHEN USED LATED INFERENTLY.

| INSTALL ANY REQUIRED HOLDOWNS PER SECTION | R602.103 AND FIGURES R602.106.5, R602.10.1, R602.108(1) AND R602.108(2) OF THE 2015 IRC

NOTE

WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

JOIST & BEAM SIZES SHOWN ARE MINIMUMS. BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: MEMBER NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

# STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

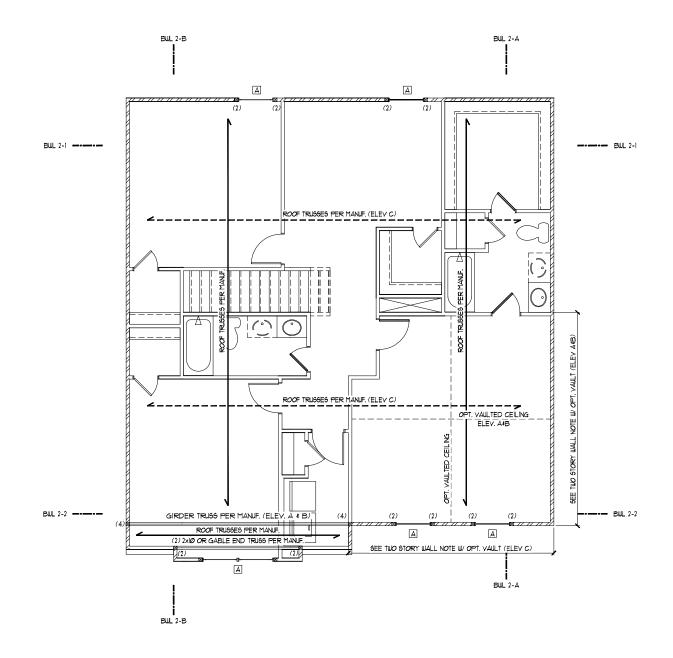
STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

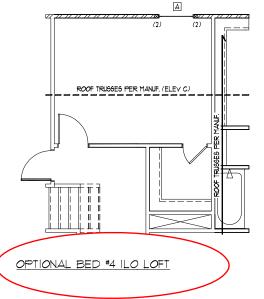
SECOND FLOOR FRAMING PLAN

SCALE: 1/4"=1"-0" ON 22"x34" OR 1/8"=1"-0" ON 11"x17"

## 

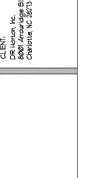
KING STUD REQUIREMENTS ABOVE DO NOT APPLY TO PORTAL FRAMED OPENINGS





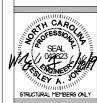
SECOND FLOOR FRAMING PLAN - ALL ELEVATIONS

SECOND FLOOR BRACING (FT)				
CONTINUOUS SHEATHING METHOD				
REQUIRED PROVIDED				
BWL 2-1	5.6	30.0		
BWL 2-2	5.6	23.8		
BWL 2-A	5.1	36.0		
BWL 2-B	5.1	38.0		



su mmit

PROJECT:
Preduct: LH
Second Floor Framing PI



DRAWNG

DATE: 06/28/2019

SCALE: 22:24 1/4\*1\*-0\*

INCT 1/8\*1\*-0\*

PROJECT 1, 528-06R-285R

DRAWN BY: EMB

PROJECT DATE
12504 DIZ1/20/1

REFER TO COVER SHEET FOR A
COMPLETE LIST OF REVISIONS

CHECKED BY: WAJ

CMPLETE LIST OF REVISIO

S4.Ø

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL, PLANS PROVIDED BY DR HORTON. COMPLETED/REVISED ON 6/12/208. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, PC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY 4 TESTING, PC. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

NOTE: 15T PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)

NOTE: ROOF TRUSSES SHALL BE SPACE TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

ROOF FRAMING PLAN

5CALE: 1/4"=1"-0" ON 22"x34" OR 1/8"=1"-0" ON 11"x17"

ROOF TRUSSES PER MANUE.

ROOF TRUSSES PER MANUE.

2x4 RJ = 2x4 OC, W 2x8 HIPS
OR ROOF TRUSSES PER MANUE.

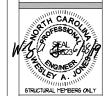
ROOF FRAMING PLAN - ELEVATION C





DR Horton, Inc. 8001 Arrouridge Blvd Charlotte, NC 28273

PROJECT. Priedust - LH Roof Framing Plan



DR4UNG

DATE: 06/28/00%

SCALE: 22/34 | /4\*\*!"-0\*\*
|IMT | /0\*\*:"-0\*\*

INT 1/8":1"-8"
PROJECT 1: 528-988: 2185R
DRAWN BY: EMB
CHECKED BY: WAJ

REFER TO COVER SHEET R

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S5.2

Applicable Building Codes:

• 2018 North Carolina Residential Building Code with All Local Amendments

• ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

9" -	ougos.		
٦.	Roof	Live Loads	
	1.1.	Conventional 2x	2Ø PSF
	1.2.	Trus <b>8</b>	2Ø PSF
		12.1. Attic Truss	60 PSF
2.	Roof	Dead Loads	
	2.1.	Conventional 2x	10 PSF
	2.2.	Truse	2Ø PSF
3.	Snow		15 PSF
	3.1.	Importance Factor	lø
4.	Floor	Live Loads	
	4.1.	Typ. Dwelling	40 PSF
		Sleeping Areas	
		Decks	
	4.4.	Passenger Garage	50 PSF

5. Floor Dead Loads
5.I. Conventional 2x ... 5.2 I-Joist

6.l. Exposure ..... 62. Importance Factor... 63. Wind Base Shear

6.3.l. Vx =

632. Vy = 7. Component and Cladding (in PSF)

MEAN ROOF HT.	UP T <b>Ø</b> 3Ø'	<b>3</b> Ø'I"-35'	35'1"-40'	40'1"-45'
ZONE 1	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-20.2
ZONE 2	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-2 <b>Ø</b> .7	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.1	20.4,-26.9

Seismic Use Group ...

8.5. Spectral Response Acceleration 85.1. Sms = %g 85.2. Sml = %g 8.6. Seismic Base Shear

861.Vx = 862.Vy = 8.1. Basic Structural System (check one)

⊠ Bearing Wall ☐ Building Frame
☐ Moment Frame □ Dual w/ Special Moment Frame

□ Dual w/ Intermediate R/C or Special Steel
□ Inverted Pendulum

8.8. Arch/Mech Components Anchored 8.9. Lateral Design Control: Seismic 🗆 llind 🖂 9. Assumed Soil Bearing Capacity ...

## STRUCTURAL PLANS PREPARED FOR

# STANDARD DETAILS

PROJECT ADDRESS:

OUNER: DR Horton Carolinas Division

ARCHITECT/DESIGNER

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of recoord (SER, Should any cliarcepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory 4 Testing, P.C. before construction begins.

# PLAN ABBREVIATIONS:

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
u	CEILING JOIST	5C	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
EW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
ОC	ON CENTER	TYP	TYPICAL
P <b>S</b> F	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
₽91	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory 4 Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by <u>DR Horton. Inc.</u> Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify **5U**1111 immediately.

# SHEET LIST:

REVISION LIST:

Date

FIII

T |2 |T

3 2.15.18

4 228.18

5 12.19.18

6 2.19.19

8 3.6.19

9 3220

Project No.

Revision

ôheet Nø.	Description
CSI	Cover Sheet, Specifications, Revisions
D1m	Monolithic Slab Foundation Details
Dis	Stem Wall Foundation Details
Dlc	Crawl Space Foundation Details
Dlb	Basement Foundation Details
DIf	Framing Details

# DR HORTON PROJECT SIGN-OFF:

Manager	Signature
Operations	
Operations System	
Operations Product Development	

# SÜMMIT



## GENERAL STRUCTURAL NOTES:

- NERAL STRUCTURAL NOTES:

  The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, after, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For the surposes of these construction documents the SER and SUMMIT. purposes of these construction documents the SER and SUMMIT
- shall be considered the same entity.

  The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction
- to stabilize the structure.

  The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents
- should any non-conformities occur.

  Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions,
- the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or 9UMMIT. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to 9UMMIT before construction begins.

  The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted to the structural drawings.
- noted on the structural drawings.

  This structure and all construction shall conform to all
- applicable sections of the international residential code. This structure and all construction shall conform to all applicable sections of local building codes.
   All structural assemblies are to meet or exceed to requirements.
- of the current local building code.

# FOUNDATIONS:

The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be

- 2. The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- maximum dry density.

  5. Excavations of footings shall be lined temporarily with a 6 mill polyetylene memorane if placement of concrete does not occur within 24 hours of excavation.

- with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design latest editions.
  Structural steel shall receive one coat of shop applied
- rust-inhibitive paint.

  3. All steel shall have a minimum yield stress  $(F_u)$  of 36 kg unless
- otherwise noted

- Number IE.

  Concrete shall have a normal weight aggregate and a minimum compressive strength (fe/ at 28 days of 3000 ps), unless otherwise noted on the plan.

  Concrete shall be proportioned, mixed, and placed in
- Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:
  - 3.1. Footings: 5% 3.2. Exterior Slabs: 5%
- 4. No admixtures shall be added to any structural concrete without written permission of the SER.

- Construction" Any fill shall be placed under the direction or recomme
- of a licensed professional engineer.
  The resulting earl shall be compacted to a minimum of 95%
- No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.

- STRUCTURAL STEEL:

  1. Structural steel shall be fabricated and erected in accordance

- Welding shall conform to the latest edition of the American weraing shall common to the latest edition of the American Welding Society's Structural Welding Code AUS DIJ. Electrodes for shop and field welding shall be class ETØXX. All welding shall be performed by a certified welder per the above

- accordance with the latest editions of ACI 318: "Building Code

- Concrete slabs-on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete Slab and Slab
  - The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from urreported conditions not in accordance with the above assumptions. Control or solu cut joints shall be spaced in interior slabs-on-grade at a maximum of 15-01 O.C. and in exterior
  - slabs-on-grade at a maximum of  $|\mathcal{O}|$  unless otherwise noted. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
  - process within 4 to 12 hours after the state has been has been intered.

    9. Reinforcing steel may extend through a control joint.

    Reinforcing steel may extend through a saw cut joint.

    10. All welded wire fabric (www.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The WWW. shall be securely supported during the concrete pour.

- CONCRETE REINFORCEMENT:

  I. Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 0,1% by volume (15 pounds per cubic yard) Fibermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry
- standard.
  Steel reinforcing bars shall be new billet steel conforming to
- of the inferior of the state of size/spacing as the horizontal reinforcement with a class B
- Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.

- 9. Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters
- into the Footing.

  10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise nated. WOOD FRAMING: Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National"
- Design Specification for Wood Construction" (NDS), Unless otherwise noted, all wood framing members are designed to be Spruce-Yellow-Pise (SYP) 2.

  LVL or PSL engineered wood shall have the following minimum
- sign values: 2.1. E = 1,900,000 psi
  - 2.2. F<sub>b</sub> = 2600 psi 2.3. F<sub>v</sub> = 285 psi
- 2.4.Fc = 100 psi 1.4.1°C incorption blood in contract, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2
- Nails shall be common wire nails unless otherwise noted.

  Lag screws shall confrom to ANSI/ASME standard Bi82.1-1981.

  Lead holes for lag screws shall be in accordance with NDS specification.
- specifications
- All beams shall have full bearing on supporting framing members unless otherwise noted. Exterior and load bearing stud walls are to be 2x4 SYP  $^{\circ}$ 2 = 16"
- O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header.
- of one king stud shall be placed at each end of the header. King stude shall be continuous, individual stude forming a column shall be attached with one lod nail e 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer. Multi-ply beams shall have each ply attached with (3) lod nails e
- 10. Flitch beams, 4-ply beams and 3-ply side loaded beams shall be bolted together with (2) rous of 1/2" diameter through boilts staggered # 16" O.C. unless noted otherwise. Min. edge distance shall be 2" and (2) bolts shall be located a min. 6" from each

# WOOD TRUSSES:

The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.

The wood trusses shall be designed for all required loadings.

dded box bay detail (2/D2f). Added deck

stem wall and crawl space foundations

Revised garage door detail, NC only

Added high-wind foundation details

Revised per Mecklenburg County Comments Revised stem wall deck attachment and roo

Corrected dimensions at perimeter footings

Revised stem wall insulation note

Revised per 2018 NCRC

sheathing on wall sections.

Added tall turndown detail

options with basement. Revised deck options with

- In a wood trusses shall be designed for all required loadings as specified in the local building code, the ACCE Standard "Minimum Design Loads for Buildings and Other Structures."

  (ASCE 1-05), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to
- the trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction" (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings.

  Also, the shop drawings shall show the required attachments for
- the trusses.

  Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer

# EXTERIOR WOOD FRAMED DECKS:

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

- WOOD STRUCTURAL PANELS:

  I. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA
- All structurally required wood sheathing shall bear the mark of

- 3. Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise. Roof sheathing shall be APA rated sheathing exposure I or 2.
- Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use
- have a span rating consistent with the framing spacing, Use suitable edge support by use of plywood clips or limber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code. Wood floor sheathing shall be APA rated sheathing exposure I or 2. Attach sheathing to its supporting framing with (I)-Bd CC ringshark nail at 6°0'c at panel edges and at 12°0'c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing, Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of 14G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- state Building Code.

  Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

- STRUCTURAL FIBERBOARD PANELS:

  1. Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards
- All structurally required fiberboard sheathing shall bear the mark of the AFA. 3. Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more
- Sheathing shall have a 1/8" gap at panel ends and edges are

PROJECT:
Standard Details
Coversheet TH CARO USBA1 4/2

STRUCTURAL MEMBERS ONLY

DATE: 3/2/2 8CALE: 22x34 V4"+1"-8" lbt1 V8"+1"-8" PROJECT 1 P-19Ø1-1Ø DRAWN BY: LAG

CHECKED BY: WAJ

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

CSI

SÜMMIT

SUMMIT Engineering, Laboratory & Testing, P.C.

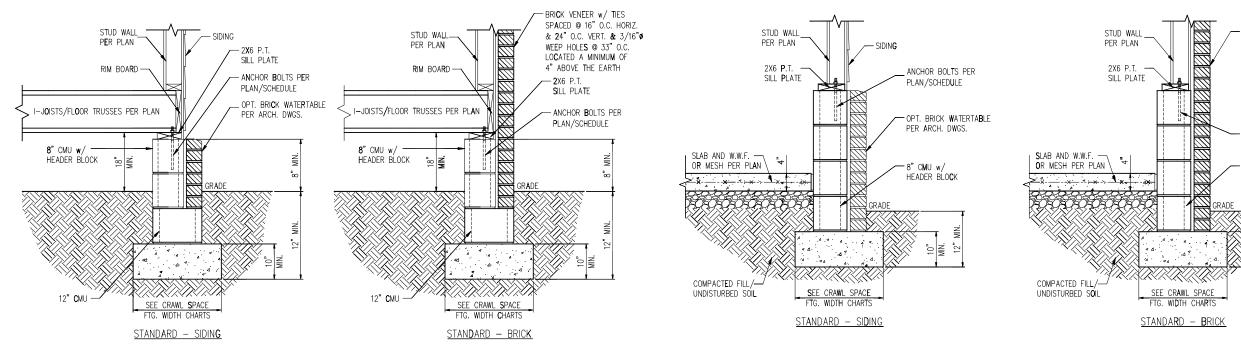


DATE: 3/2/2 8CALE: 22x34 1/4"∗1"-**6"** lbt1 1/8"∗1"-**6"** PROJECT & P-19Ø1-1ØR DRAWN BY: LAG

CHECKED BY: WAJ

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

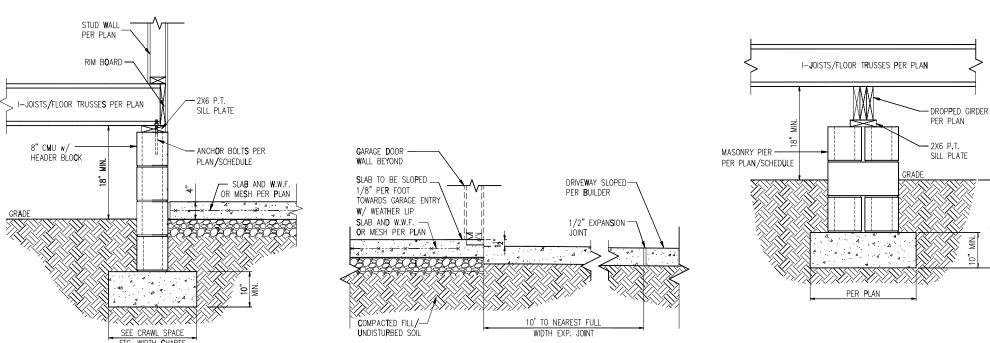




FOUNDATION WALL DETAIL

HOUSE/GARAGE WALL DETAIL

TYP. GARAGE CURB DETAIL



4 SLAB AT GARAGE DOOR TYP. PIER & GI**R**DER DETAIL

# PIER SIZE AND HEIGHT SCHEDULE

SI <b>Z</b> E	HOLLOW	SOLID		
8"X16"	UP TO 32" HEIGHT	UP TO 5'-0" HEIGHT		
1 <b>2</b> "X16"	UP TO 48" HEIGHT	UP TO 9'-0" HEIGHT		
1 <b>6</b> "X16"	UP TO 64" HEIGHT	UP TO 12'-0" HEI <b>G</b> HT*		
24"X24"	UP TO 96" HEIGHT	UP TO 12'-0" HEI <b>C</b> HT*		
*(4) #4 CONT. REBAR w/ #3 STIRRUPS @ 16" O.C.				
AND 24" MIN. LAP JOINTS				

## CRAWL SPACE FOOTING WIDTH

CRAWL SPACE FUUTING	חוטוא		
# OF STO <b>R</b> IES	WIDTH BASED ON SOIL BEARING CAPACITY		
	150 <b>0</b> PSF	2000 PSF	2500 PSF
1 STORY - STD.	16"	16"	16"
1 STORY - BRICK VENEER	21"*	21"*	21"*
2 STORY - STD.	16"	16"	16"
2 STORY - BRICK VENEER	21"*	21"*	21"*
3 STORY - STD.	23"	18"	18"
3 STORY - BRICK VENEER	32"*	24"*	24"*
*5" BRICK LEDGE HAS BEEN		CRAWL SPACE	

FOOTING WIDTH FOR BRICK SUPPORT

## WALL ANOUGH COUEDING

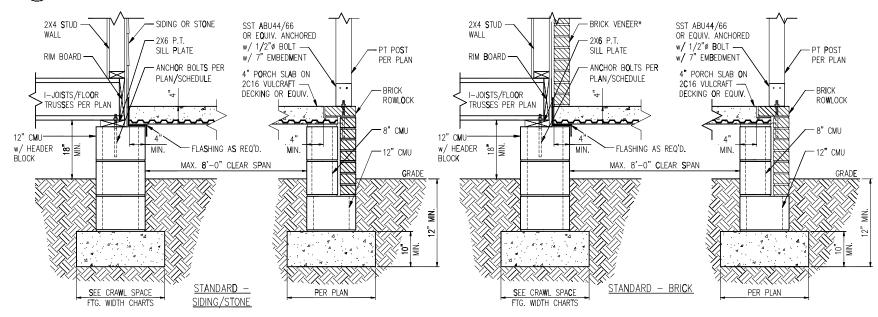
WALL ANCHOR SCHEDULE				
TYPE OF ANCHOR	MIN. CONC.	SPACING	INTERI <b>O</b> R	EXTERIOR
	EMBED <b>M</b> ENT	EMBEDMENT	WALL	WALL
1/2"ø A3 <b>0</b> 7 BOLT <b>S</b> w/	7"	6'-0"	YES	YES
STD. 90° <b>B</b> END				
SST - MAS	4"	5'-0"	NO	YES
HILTI KWIK BOLT KBI 1/2-2-3/4	2-1/4"	6'-0"	YES	NO
1/2"ø HILTI THREADED ROD	7"	6'-0"	YES	YES
w/ HIT HY150 ADHESIVE				

NOTE: INSTALL ALL ANCHORS 12" MAX. FROM ALL BOTTOM PLATE ENDS AND JOINTS.

- NOTES:

  1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET FOR ADDITIONAL INFORMATION.
- PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
   SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
- 4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND CONNECTIONS
- REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN
- 6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC





FRONT PORCH DETAIL W/ SUSPENDED SLAB

# DECK ATTACHMENT SCHEDULE (ALL STRUCTURES EXCEPT BRICK)

FASTENERS	MAX. 8'-0" JOIST	MAX. 16'-0" JOIST
	SPAN	SPAN
5/8" GALV. BOLTS w/ NUT & WASHER b	(1) <b>Q</b> 3'-6" O.C.	(1) @ 1'-8" O.C.
AND	AND	AND
12d COMMON GALV. NAILS°	(2) @ 8" O.C.	(3) @ 6* O.C.

- a. ATTACHMENT INTERPOLATION BETWEEN 8' AND 16' JOIST SPANS IS ALLOWED.
- b. MINIMUM EDGE DISTANCE FOR BOLTS IS  $2\frac{1}{2}$ ".
- c. NAILS MUST PENETRATE THE SUPPORTING STRUCTURE BAND A MINIMUM OF 11/2"

# DECK ATTACHMENT SCHEDULE (BRICK STRUCTURES)

FASTENERS	MAX. 8'-0" JOIST	MAX. 16'-0" JOIST
	SPA <b>N</b>	SPAN
5/8" GALV. BOLTS w/ NUT & WASHER <sup>b</sup>	(1) <b>©</b> 2'-4" O.C.	(1) @ 1'-4" O.C.

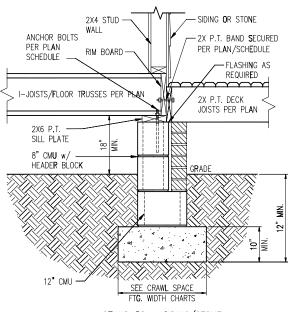
- a. Attachment interpolation between 8' and 16' joist spans is allowed.
- b. MINIMUM EDGE DISTANCE FOR BOLTS IS  $2\frac{1}{2}$ ".

# CRAWL SPACE FOOTING WIDTH

FOOTING WIDTH FOR BRICK SUPPORT

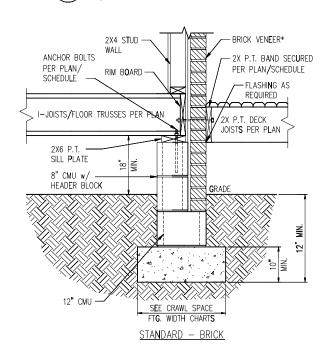
π οι στοικι <b>ε</b> σ	MID IT DAJED ON JOIL DEANING CALACITY			
	1500 PSF	2000 PSF	2500 P <b>\$</b> F	
1 STORY - STD.	16"	16"	16"	
1 STORY - BRICK VENEER	21"*	21"*	21"*	
2 STORY - STD.	16"	16"	16"	
2 Story - Brick Veneer	21"*	21"*	21"*	
3 STORY - STD.	23"	18"	18"	
3 STORY - BRICK VENEER	32"*	24"*	24"*	
*5" BRICK LEDGE HAS BEEN A	ADDED TO THE	CRAWI SPACE		

\*BRICK TIES SPACED @ 16" Q.C. HORIZ. & 24" O.C. VERT. AND 3/16" WEEP HOLES @ 33" O.C. LOCATED A MINIMUM OF 4" ABOVE THE EARTH



STANDARD - SIDING/STONE

# NDECK ATTACHMENT DETAIL



DE**c**k attachment detail w/ brick

- NOTES:

  1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET FOR ADDITIONAL INFORMATION.
- PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
   SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
- 4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND CONNECTIONS
- 5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN
- 6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC





Details undation 8 PROJECT: Standard D Crawl



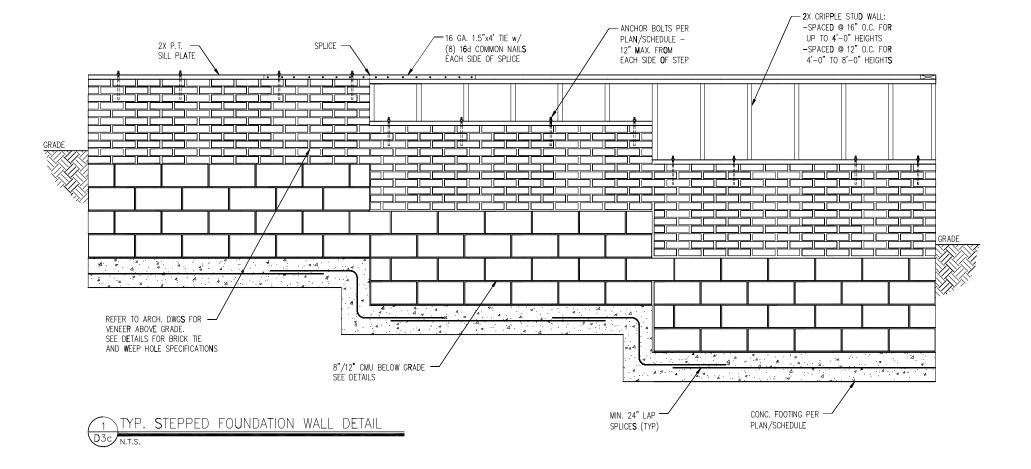
DATE: 3/2/2 8CALE: 22%34 1/4"∗1"-**6"** 16€1 1/8"=1"-**6"** PROJECT 4 P-19Ø1-1Ø DRAWN BY: LAG

CHECKED BY: WAJ

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

D2c





- NOTES:

  1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET FOR ADDITIONAL INFORMATION.
- 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
  3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
- 4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND
- CONNECTIONS
  5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN
- 6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

PROJECT:
Standard Details

Crawl Space 1 STRUCTURAL MEMBERS ONLY

Details

Foundation

DRAUNG DATE: 3/2/20 8CALE: 22%34 1/4"∗1"-**6"** 16€1 1/8"=1"-**6"** PROJECT & P-1967-16R DRAIN BY: LAG

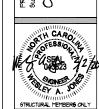
CHECKED BY: WAJ

REFER TO GOVER SHEET FOR A COMPLETE LIST OF REVISIONS

D3c



Details undation Petalls | PROJECT: Standard D Crawl



DATE: 3/2/28

NOTES:

1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET

PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
 SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.

BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND

4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR

5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN 6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

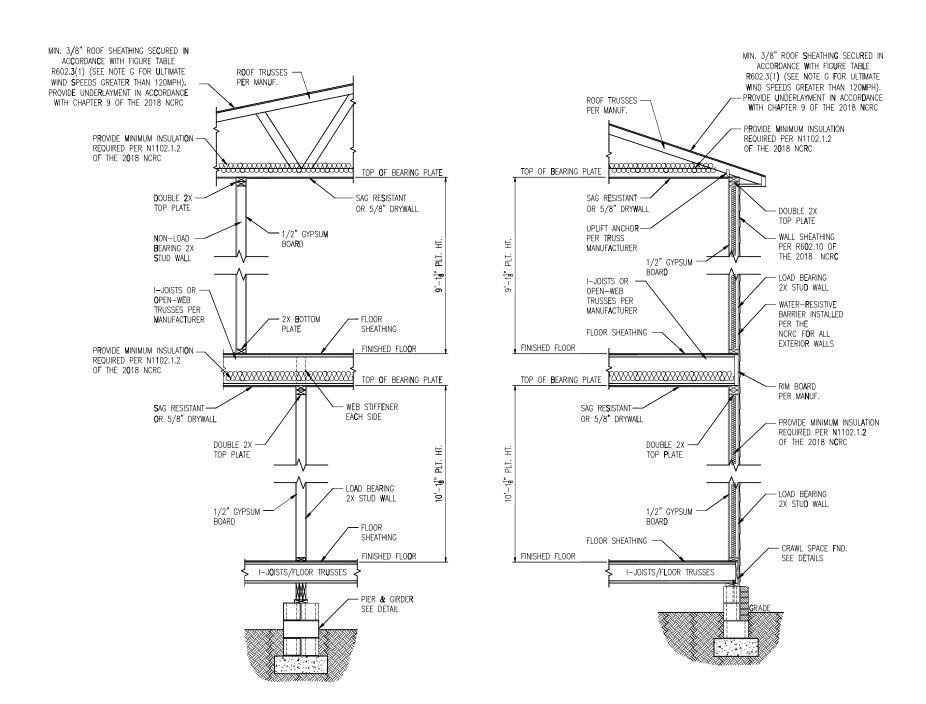
FOR ADDITIONAL INFORMATION.

CONNECTIONS

8CALE: 22%34 1/4"∗1"-**6"** 16€1 1/8"=1"-**6"** PROJECT & P-19Ø1-1ØR DRAIN BY: LAG CHECKED BY: WAJ

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

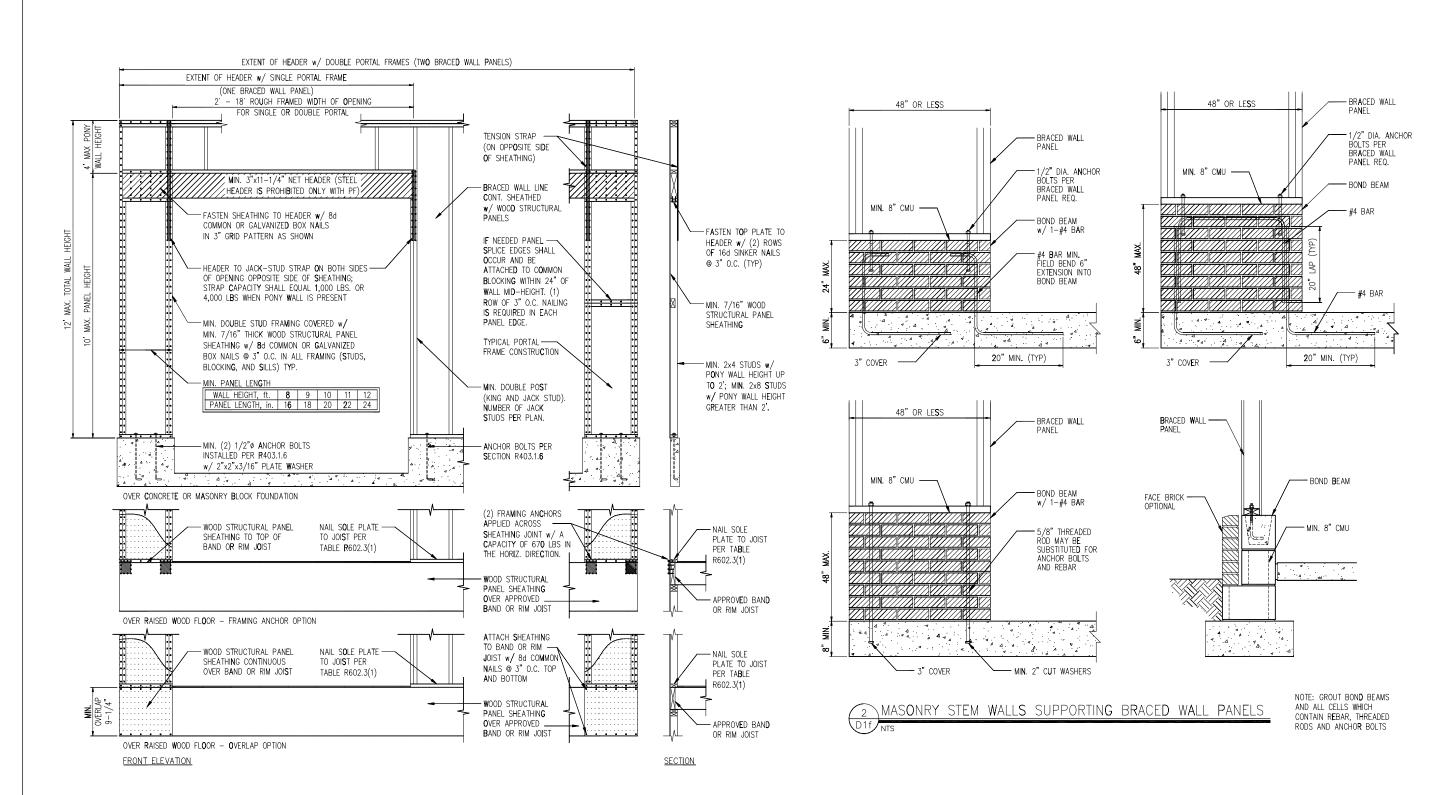
D4c



TYP. INTERIOR LOAD BEARING WALL SECTION

TYP. EXTERIOR LOAD BEARING WALL SECTION

-SIMILAR W/ BRICK AND STONE -BRICK TIES SPACED @ 16" O.C. HORIZ. & 24" O.C. VERT. -MIN. 3/16"0 WEEP HOLES @ 33" O.C.





SÜMMIT

SUMMIT Engineering, Laboratory & Testing, P.C.

CLIENT:
DR Horton Carolina Divi
8001 Arrowridge Blvd.
Charlotte, NC 20213

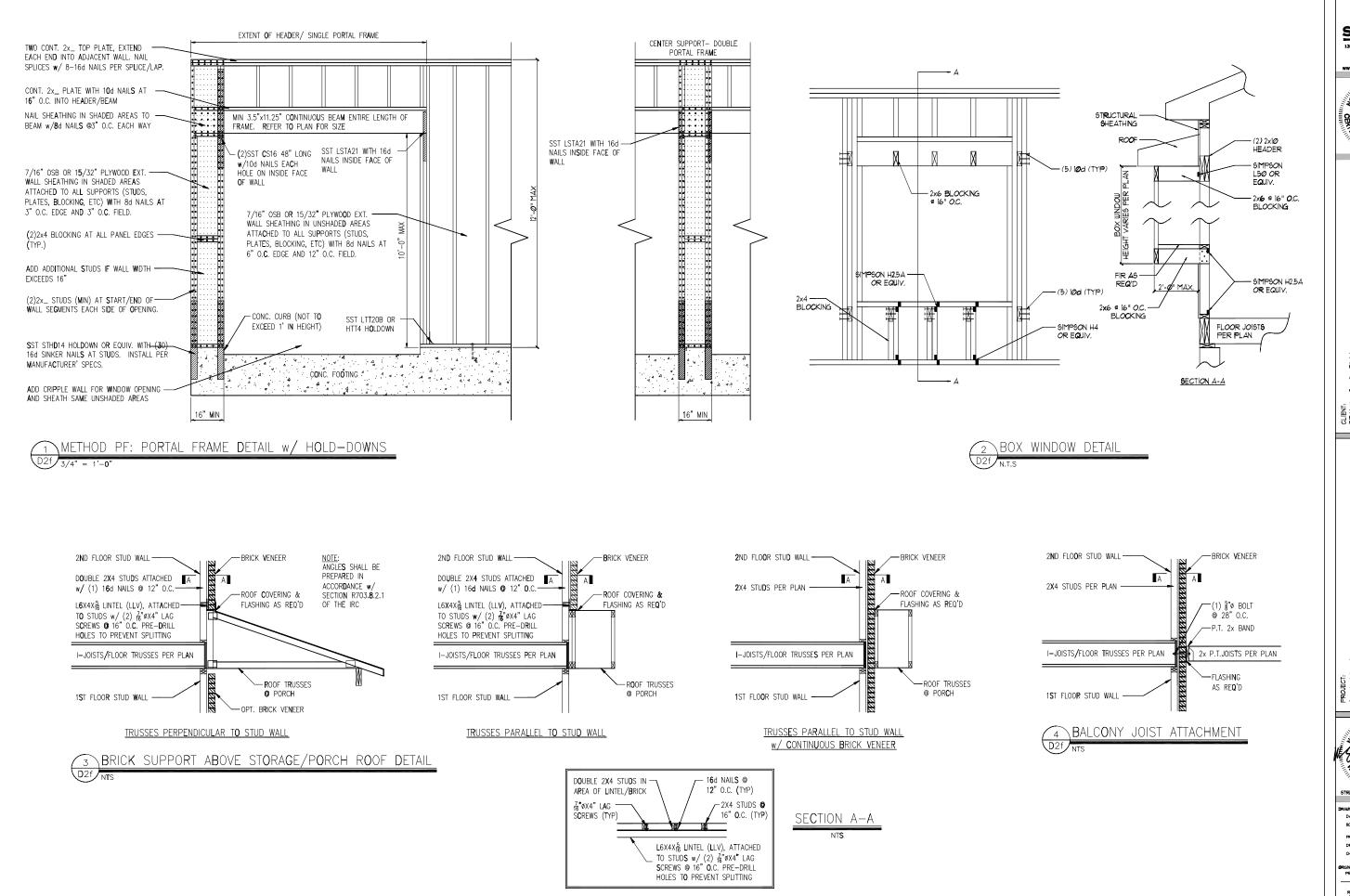


DATE: 3/2/2 8CALE: 22x34 1/4"∗1"-**8"** |bgT 1/8"∗1"-**8"** PROJECT 4 P-19Ø1-1Ø DRAIN BY: LAG CHECKED BY: WAJ

REFER TO GOVER SHEET FOR A COMPLETE LIST OF REVISIONS

D1f

METHOD PF: PORTAL FRAME DETAIL



SUMMIT





Detaí PROJECT: Standard Details Framing

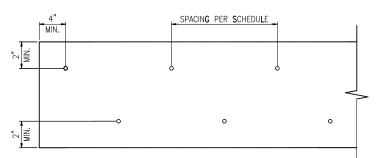


STRUCTURAL MEMBERS ONLY DATE: 3/2/2 8CALE: 22x34 1/4"∗1"-**8"** |bgT 1/8"∗1"-**8"** 

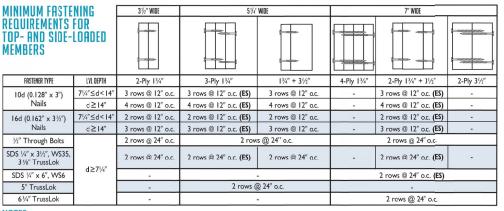
PROJECT & P-19Ø1-1ØR DRAWN BY: LAG CHECKED BY: WAJ

REFER TO GOVER SHEET FOR A COMPLETE LIST OF REVISIONS

D2f



ELEVATION VIEW



- I.All fasteners must meet the minimum requirements in the table above. Side-loaded multiple-ply members must meet the minimum fastening and side-loading capacity
- requirements given on page 48.

  2. Minimum fastening requirements for depths less than 7½" require special consideration. Please contact your technical representative.

L3x3x1/4"x8-1/2" LONG -

STEEL BEAM -

PER PLAN

COPE END OF STEEL

AS REQ'D TO CLEAR

WEB OF STEEL BEAM

LINTEL BOTH SIDES OF WEB

w/ 13/16" # HOLES @ GAGE

side are to be staggered up to one-hall the o.c. spacing, but maintaining the fastene-clearances above and

(3) if "ES" is referenced, then the fastener schedule must be repeated on each side, with the fasteners on the back side offset up to one-half the o.c. spacing of the front side (whether or not it is staggered).

SECTION VIEW

STEEL BEAM

PER PLAN

- STEEL BEAM PER PLAN

(2) 3/4"ø BOLTS

ÈACH ANGLE LEG

NOTES: 3. Three general rules for staggering or offsetting for a certain fastener schedule:

(1) if staggering or offsetting is not referenced, then none is required;

(2) if staggering is referenced, ther fasteners installed in adjacent rows on the front.

MULTI-PLY BEAM CONNECTION DETAIL

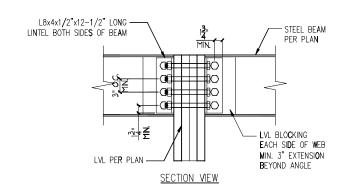
— 10d COMMON NAIL @ 12" O.C.

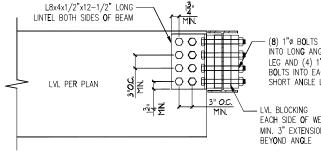
- SIMPSON C\$16 COIL STRAP OR EQUIV. PER MANUF. SPECIFICATIONS

EACH PLY OR PER CODE

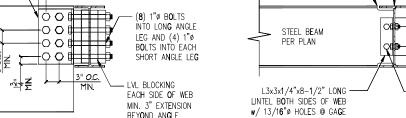
@ 1/3 HEIGHT LOCATIONS

MULTI-PLY STUD CONNECTION DETAIL





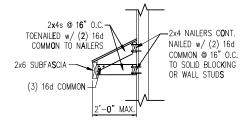
ELEVATION VIEW







**ELEVATION VIEW** 



GABLE ROOF RETURN

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SUMMIT LEDGE OF THE OF T

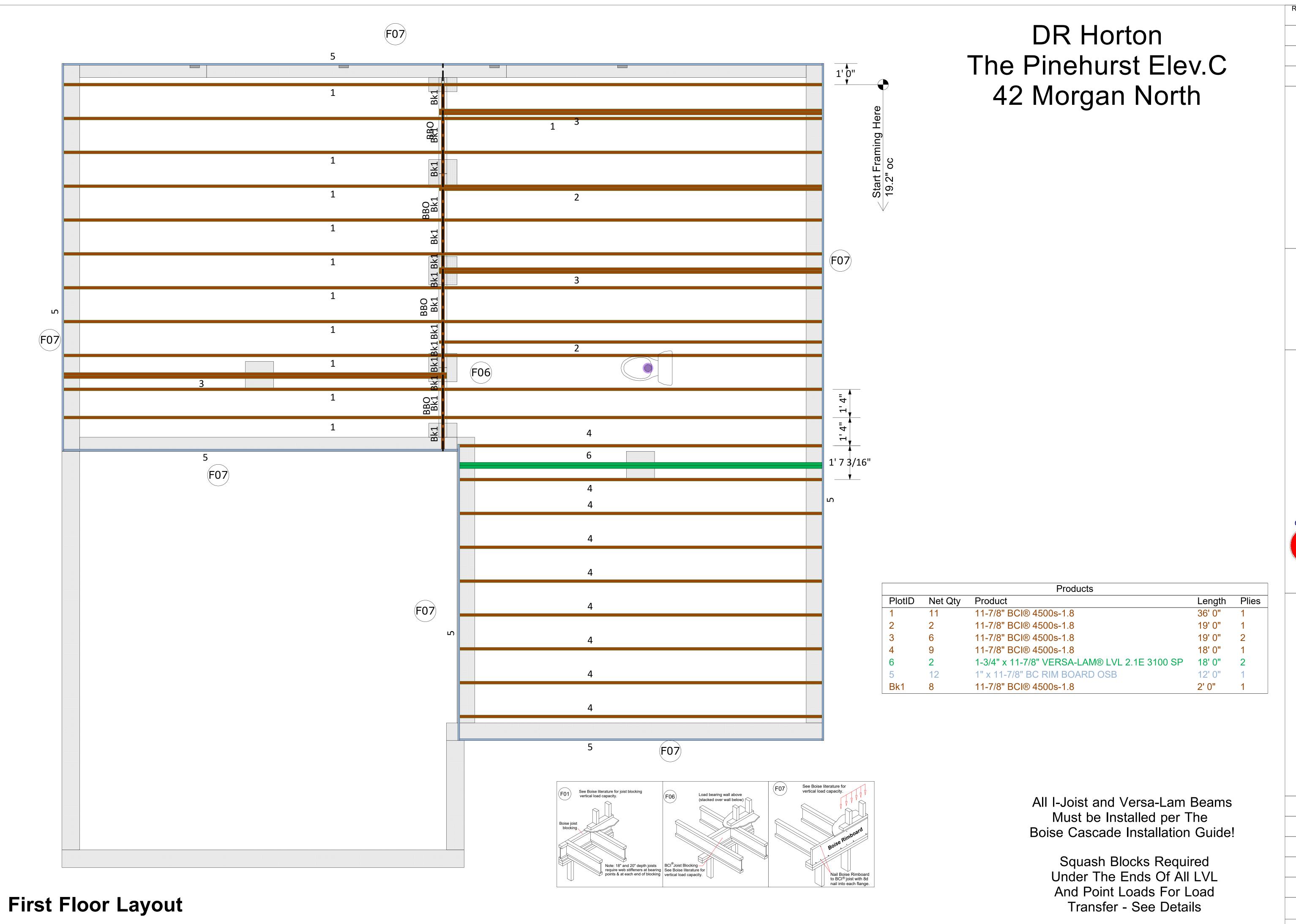
PROJECT: Standard Details Framing Details



DATE: 3/2/2 8CALE: 22x34 1/4"∗1"-**6"** lbt1 1/8"∗1"-**6"** PROJECT 4 P-1907-10R DRAIIN BY: LAG CHECKED BY: WAJ

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

**D**3f



Revisions: BY:

Boise Casca



DR Horton
The Pinehurst Elev.C
42 Morgan North
84 Lumber Charlotte EWI
Charlotte. North Carolina

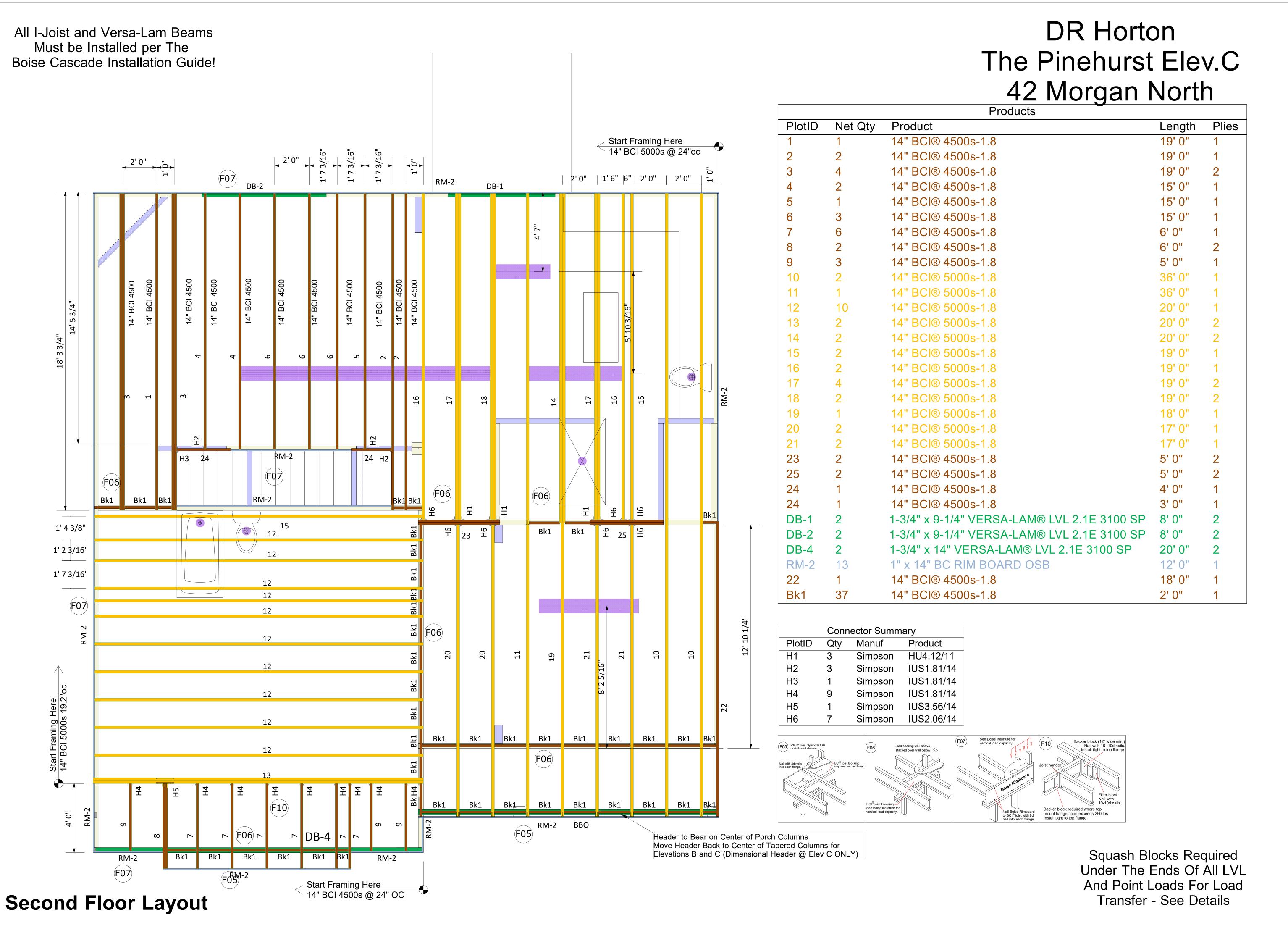
BC FRAMER II

Scale: NTS = 1'-0

Date: 6/28/2019

Designer: GAT
File:The Pinehurst Elev.C

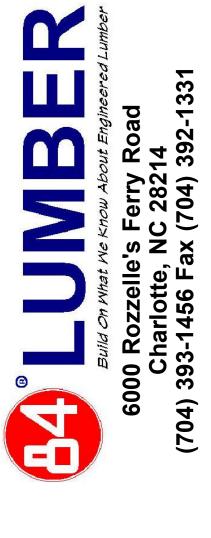
DWG:2/28/2020 Sheet: 2/3



Revisions: BY:

Boise Cascad





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84 Lumber Charlotte EWP
Charlotte. North Carolina

BC FRAMER II

Scale: NTS = 1'-0

Date: 6/28/2019

Designer: GAT

File:The Pinehurst Elev.C

DWG:2/28/2020

Sheet: 2/3

