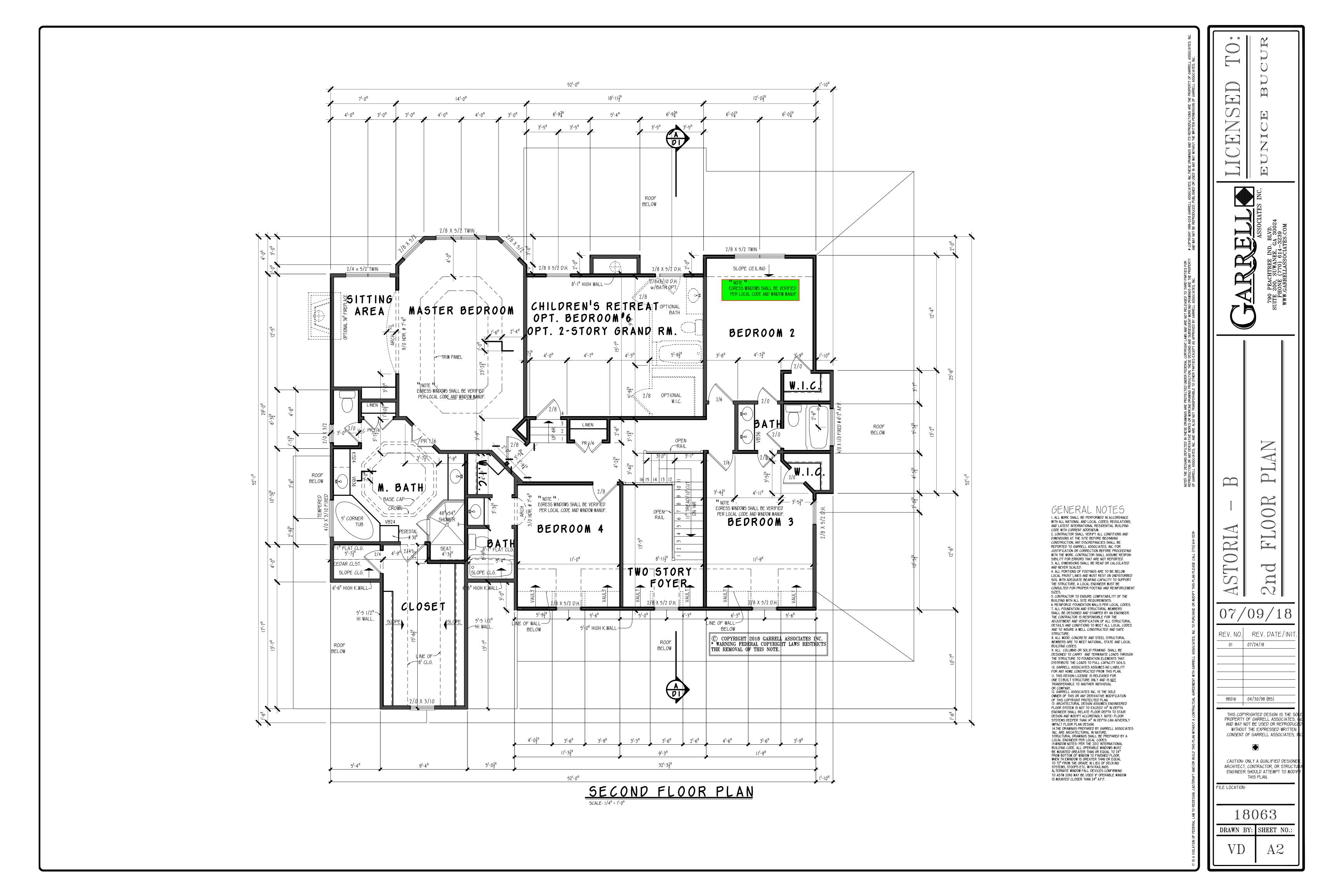
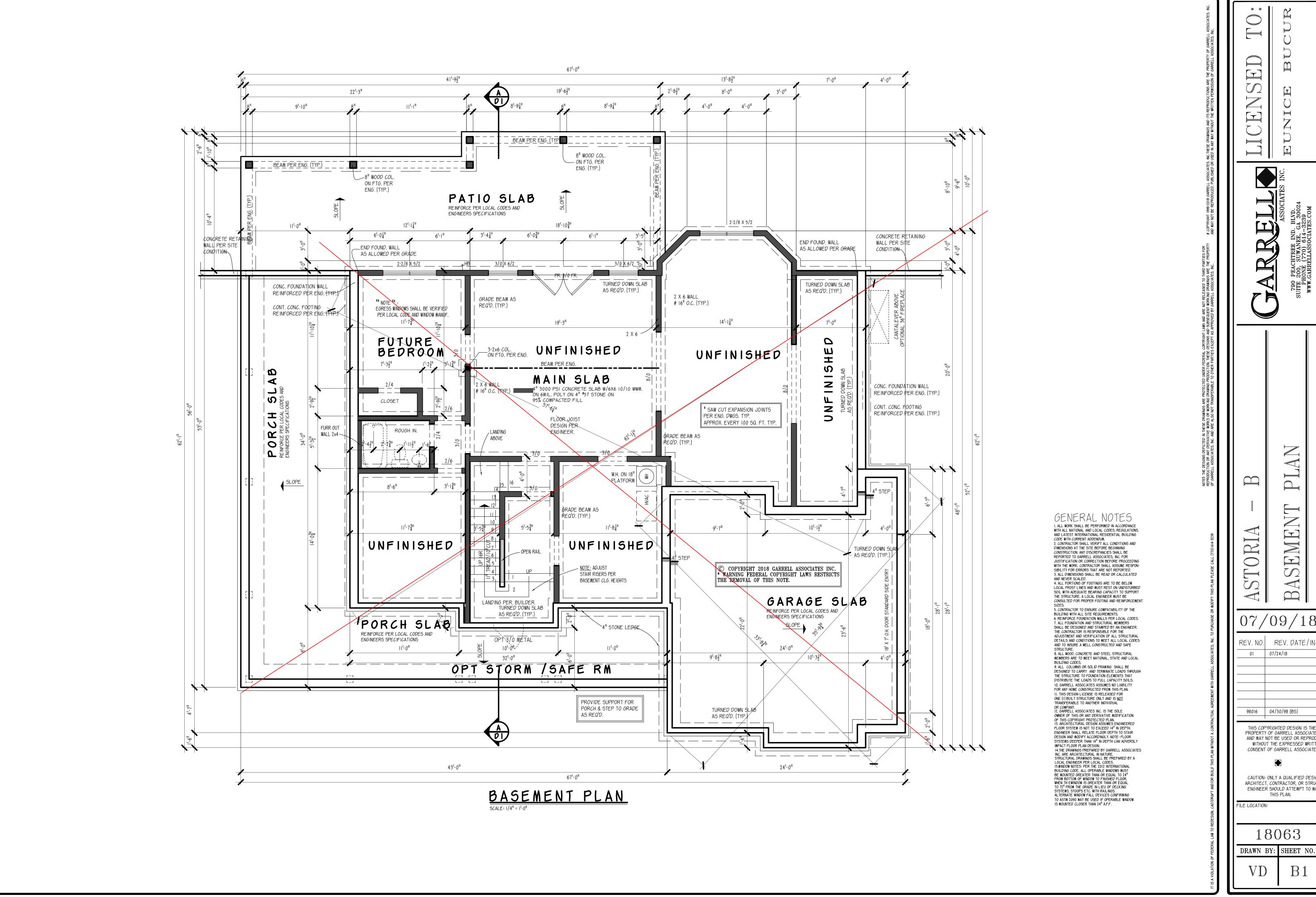


 \bigcap STORI \mathcal{U} REV. NO. REV. DATE/INI 07/24/18 01 98016 04/30/98 (BS) THIS COPYRIGHTED DESIGN IS THE SO PROPERTY OF GARRELL ASSOCIATES, AND MAY NOT BE USED OR REPRODUC WITHOUT THE EXPRESSED WRITTEN CONSENT OF GARRELL ASSOCIATES CAUTION: ONLY A QUALIFIED DESIGNER, ARCHITECT, CONTRACTOR, OR STRUCTUR ENGINEER SHOULD ATTEMPT TO MODI THIS PLAN. FILE LOCATION:

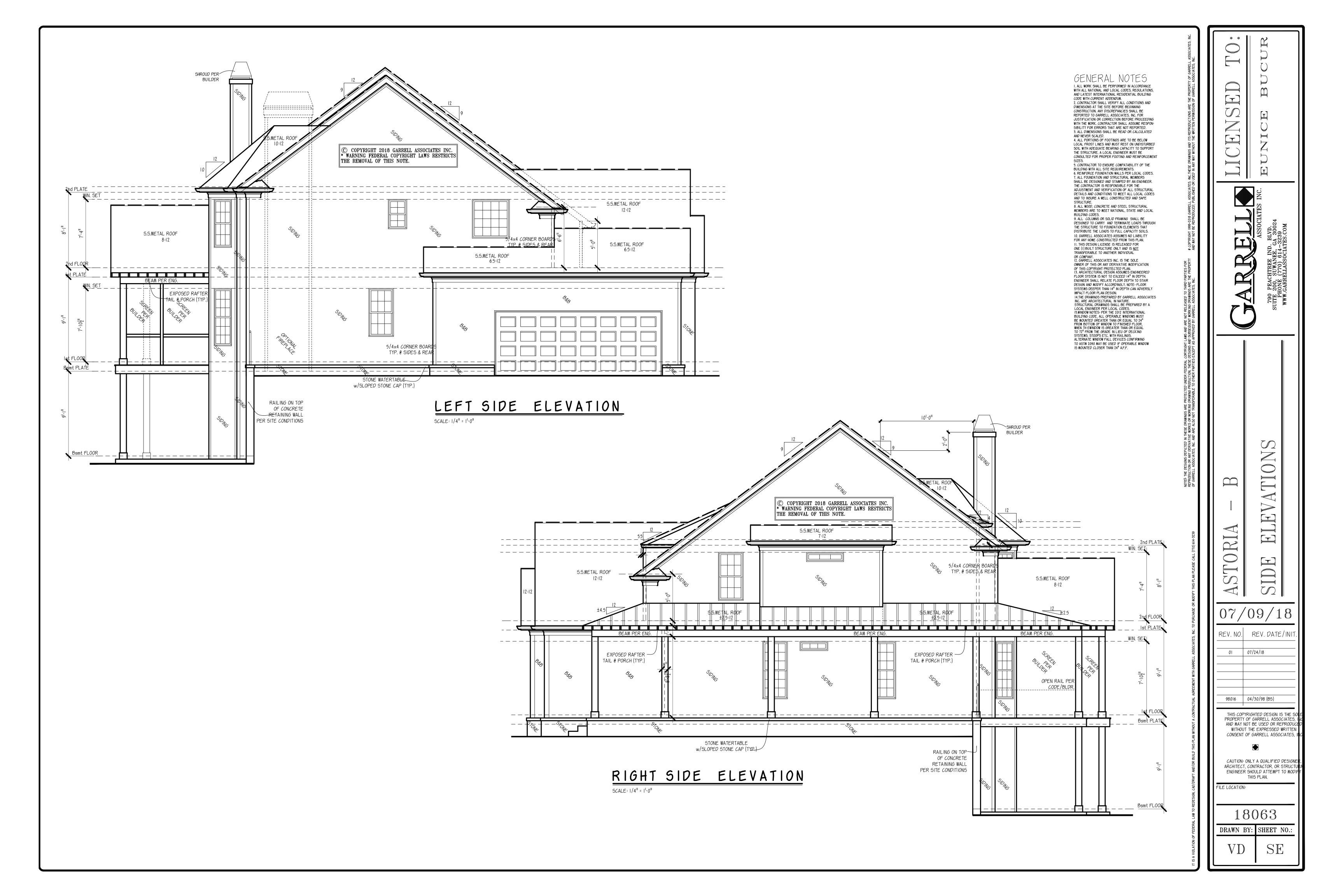
18063

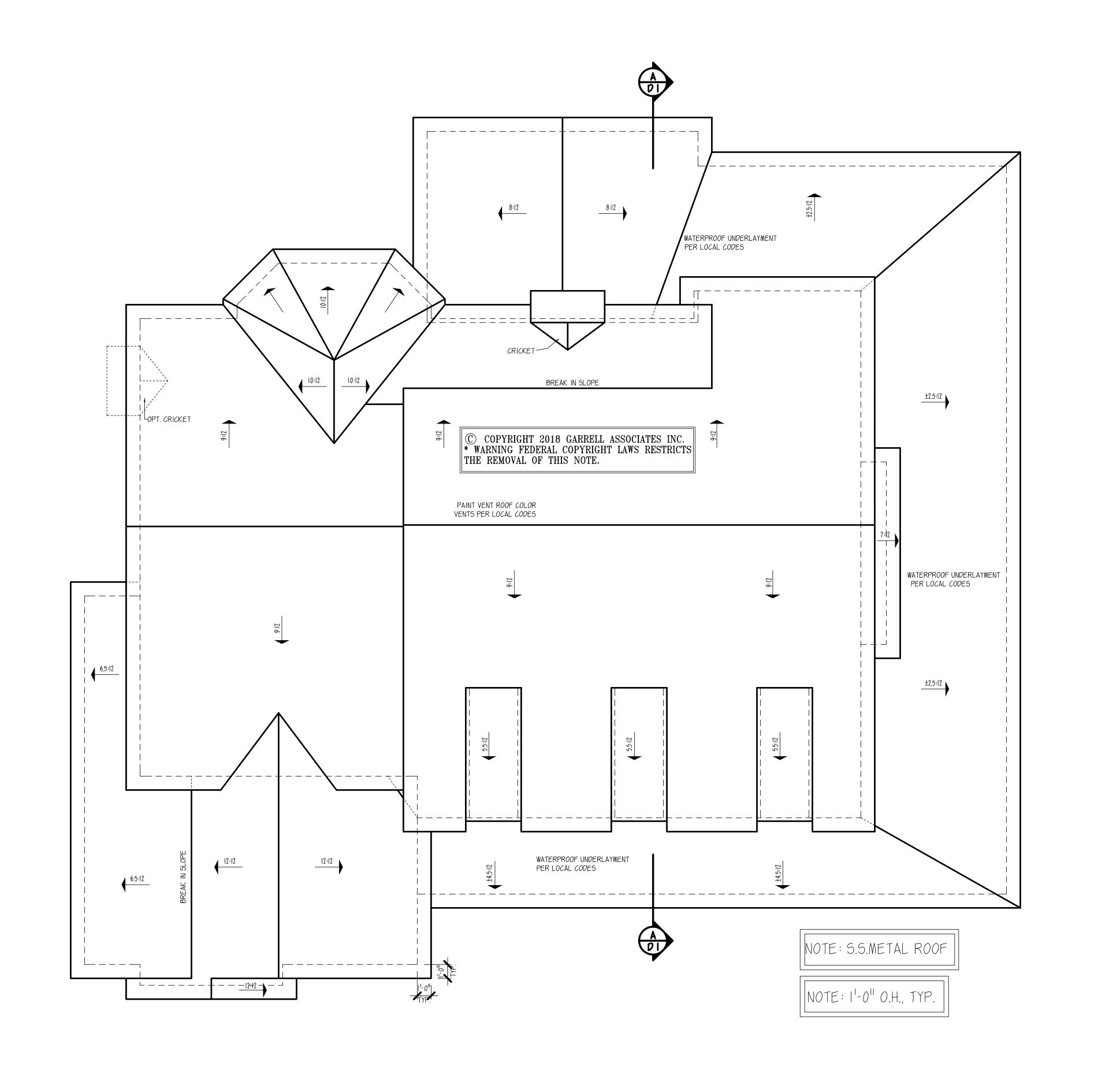
DRAWN BY: SHEET NO. A1





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ROOF PLAN

GENERAL NOTES I. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL NATIONAL AND LOCAL CODES, REGULATIONS, AND LATEST INTERNATIONAL RESIDENTIAL BUILDING CODE WITH CURRENT ADDENDUM.

2. CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE BEFORE BEGINNING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO GARRELL ASSOCIATES, INC. FOR JUSTIFICATION OR CORRECTION BEFORE PROCEEDING WITH THE WORK. CONTRACTOR SHALL ASSUME RESPON-SIBILITY FOR ERRORS THAT ARE NOT REPORTED. 3. ALL DIMENSIONS SHALL BE READ OR CALCULATED
AND NEVER SCALED.

4. ALL PORTIONS OF FOOTINGS ARE TO BE BELOW
LOCAL FROST LINES AND MUST REST ON UNDISTURBED
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DESIGNED TO CARRY AND TERMINATE LOADS THROUGH
THE STRUCTURE TO FOUNDATION ELEMENTS THAT DISTRIBUTE THE LOADS TO FULL CAPACITY SOILS.

10. GARRELL ASSOCIATES ASSUMES NO LIABILITY
FOR ANY HOME CONSTRUCTED FROM THIS PLAN. II. THIS DESIGN LICENSE IS RELEASED FOR ONE (I) BUILT STRUCTURE ONLY AND IS <u>NOT</u> TRANSFERABLE TO ANOTHER INDIVIDUAL TRANSFERABLE TO ANOTHER INDIVIDUAL
OR COMPANY.
12. GARRELL ASSOCIATES INC. IS THE SOLE
OWNER OF THIS OR ANY DERIVATIVE MODIFICATION
OF THIS COPYRIGHT PROTECTED PLAN.
13. ARCHITECTURAL DESIGN ASSUMES ENGINEERED
FLOOR SYSTEM IS NOT TO EXCEED 14" IN DEPTH.
ENGINEER SHALL RELATE FLOOR DEPTH TO STAIR DESIGN AND MODIFY ACCORDINGLY, NOTE: FLOOR SYSTEMS DEEPER THAN 14" IN DEPTH CAN ADVERSLY IMPACT FLOOR PLAN DESIGN. IMPACT FLOOR PLAN DESIGN.

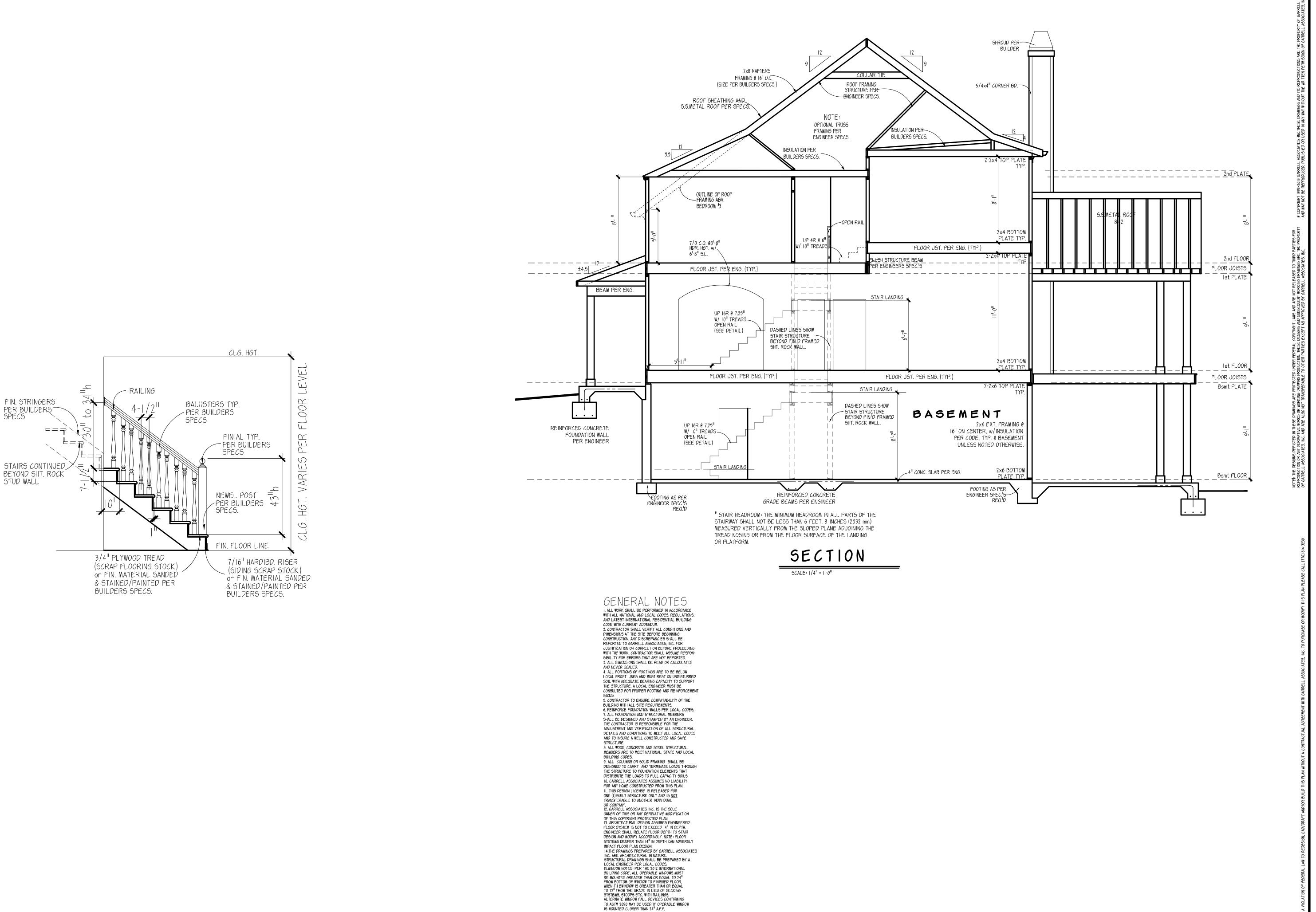
14.THE DRAWINGS PREPARED BY GARRELL ASSOCIATES INC. ARE ARCHITECTURAL IN NATURE.

STRUCTURAL DRAWINGS SHALL BE PREPARED BY A LOCAL ENGINEER PER LOCAL CODES.

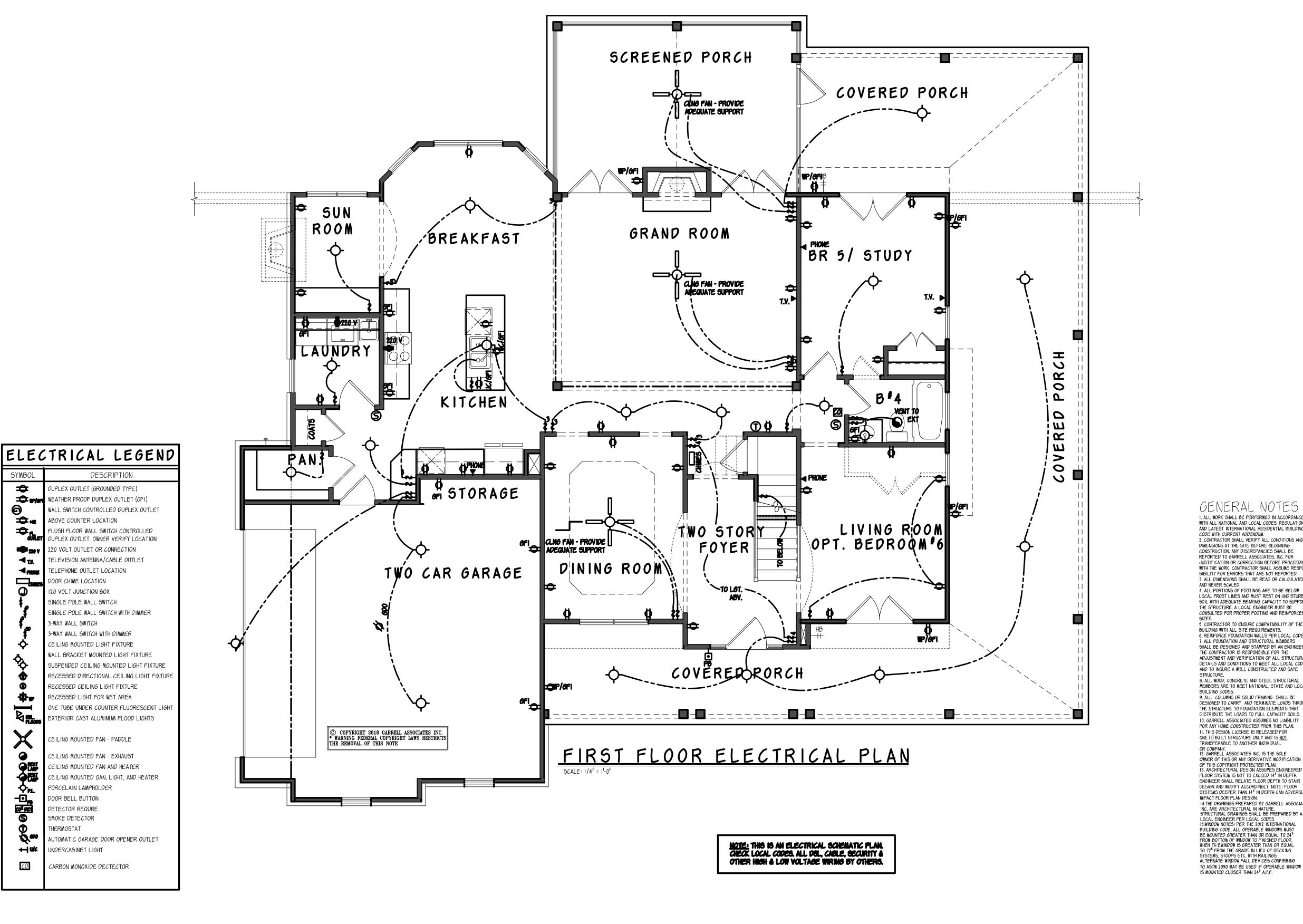
15.WINDOW NOTES: PER THE 2012 INTERNATIONAL BUILDING CODE, ALL OPERABLE WINDOWS MUST BE MOUNTED GREATER THAN OR EQUAL TO 24" FROM BOTTOM OF WINDOW TO FINISHED FLOOR, WHEN THE WINDOW IS GREATER THAN OR EQUAL TO 72" FROM THE GRADE IN LIEU OF DECKING SYSTEMS, STOOPS ETC. WITH RAILINGS.

ALTERNATE WINDOW FALL DEVICES CONFIRMING TO ASTM 2090 MAY BE USED IF OPERABLE WINDOW TO ASTM 2090 MAY BE USED IF OPERABLE WINDOW IS MOUNTED CLOSER THAN 24" A.F.F.

 \bigcap REV. NO. REV. DATE/INI 07/24/18 98016 04/30/98 (BS) THIS COPYRIGHTED DESIGN IS THE SOI PROPERTY OF GARRELL ASSOCIATES, AND MAY NOT BE USED OR REPRODUCE WITHOUT THE EXPRESSED WRITTEN CONSENT OF GARRELL ASSOCIATES, CAUTION: ONLY A QUALIFIED DESIGNER, ARCHITECT, CONTRACTOR, OR STRUCTURA ENGINEER SHOULD ATTEMPT TO MODIF THIS PLAN. FILE LOCATION: 18063 DRAWN BY: SHEET NO.:



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IMPACT PLOUR PLAN DESIGN.

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98016 04/30/98 (BS)

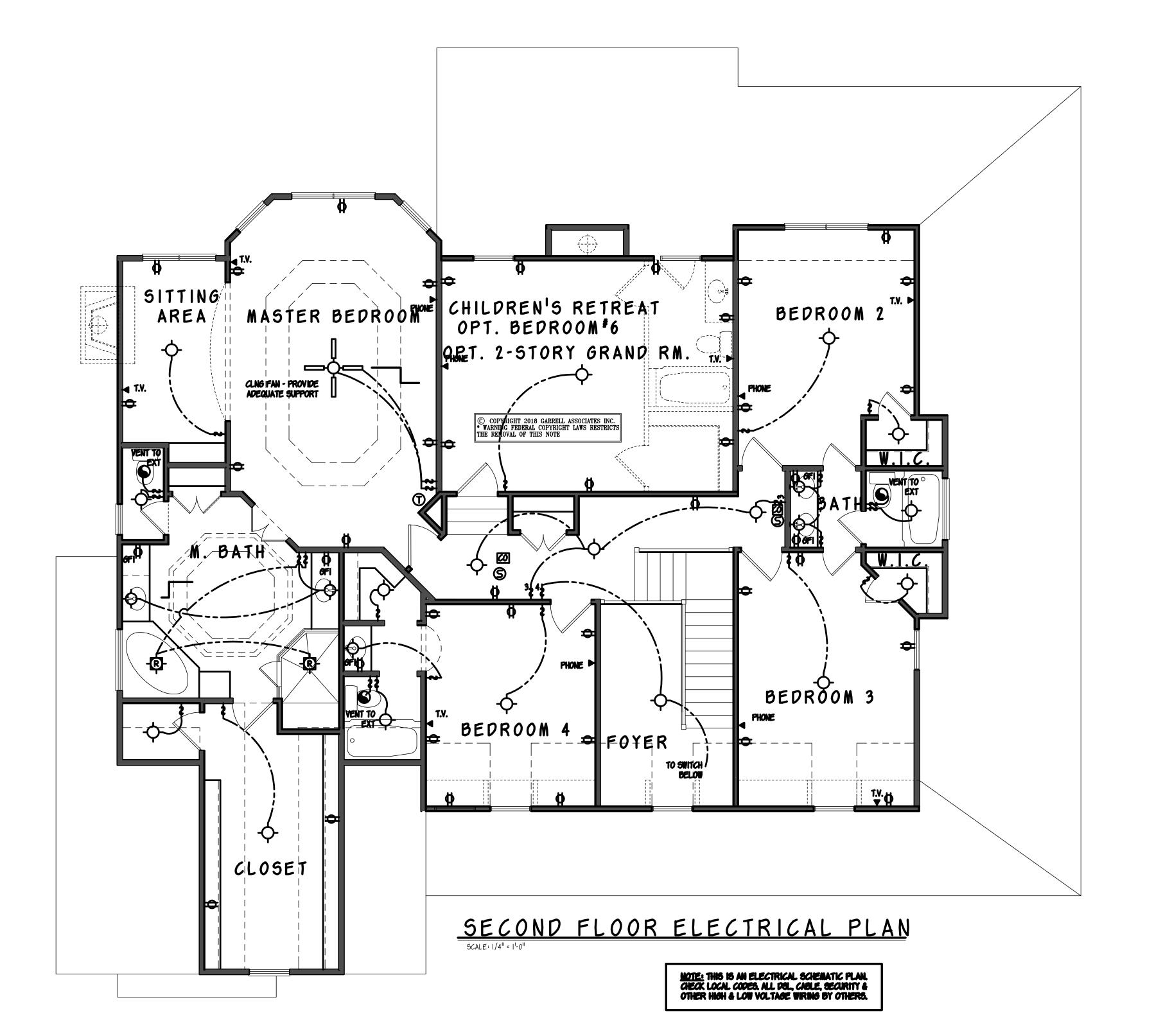
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FILE LOCATION:

18063

DRAWN BY: SHEET NO.



ELECTRICAL LEGEND

ABOVE COUNTER LOCATION

ELEPHONE OUTLET LOCATION

120 VOLT JUNCTION BOX

3-WAY WALL SWITCH

SINGLE POLE WALL SWITCH

3-WAY WALL SWITCH WITH DIMMER

CEILING MOUNTED LIGHT FIXTURE

RECESSED CEILING LIGHT FIXTURE

RECESSED LIGHT FOR WET AREA

CEILING MOUNTED FAN - PADDLE

CEILING MOUNTED FAN - EXHAUST

PORCELAIN LAMPHOLDER

DOOR BELL BUTTON

DETECTOR REQURE

SMOKE DETECTOR

JNDERCABINET LIGHT

CARBON MONOXIDE DECTECTOR

THERMOSTAT

CEILING MOUNTED FAN AND HEATER

CEILING MOUNTED GAN, LIGHT, AND HEATER

AUTOMATIC GARAGE DOOR OPENER OUTLET

SINGLE POLE WALL SWITCH WITH DIMMER

WALL BRACKET MOUNTED LIGHT FIXTURE

SUSPENDED CEILING MOUNTED LIGHT FIXTURE

RECESSED DIRECTIONAL CEILING LIGHT FIXTURE

ONE TUBE UNDER COUNTER FLUORESCENT LIGHT

EXTERIOR CAST ALUMINUM FLOOD LIGHTS

DUPLEX OUTLET (GROUNDED TYPE) WEATHER PROOF DUPLEX OUTLET (GFI) WALL SWITCH CONTROLLED DUPLEX OUTLET

LUSH FLOOR WALL SWITCH CONTROLLED PUPLEX OUTLET. OWNER VERIFY LOCATION O VOLT OUTLET OR CONNECTION

DESCRIPTION

GENERAL NOTES I. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL NATIONAL AND LOCAL CODES, REGULATIONS, AND LATEST INTERNATIONAL RESIDENTIAL BUILDING CODE WITH CURRENT ADDENDUM. 2. CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE BEFORE BEGINNING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO GARRELL ASSOCIATES, INC. FOR JUSTIFICATION OR CORRECTION BEFORE PROCEEDING WITH THE WORK. CONTRACTOR SHALL ASSUME RESPON-SIBILITY FOR ERRORS THAT ARE NOT REPORTED. 3. ALL DIMENSIONS SHALL BE READ OR CALCULATED 4. ALL PORTIONS OF FOOTINGS ARE TO BE BELOW LOCAL FROST LINES AND MUST REST ON UNDISTURBED SOIL WITH ADEQUATE BEARING CAPACITY TO SUPPORT THE STRUCTURE. A LOCAL ENGINEER MUST BE CONSULTED FOR PROPER FOOTING AND REINFORCEMENT 5. CONTRACTOR TO ENSURE COMPATABILITY OF THE BUILDING WITH ALL SITE REQUIREMENTS.

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IMPACT FLOOR PLAN DESIGN.

IM-ACT PLOUR PLAN DESIGN.

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REV. DATE/INI

07/24/18

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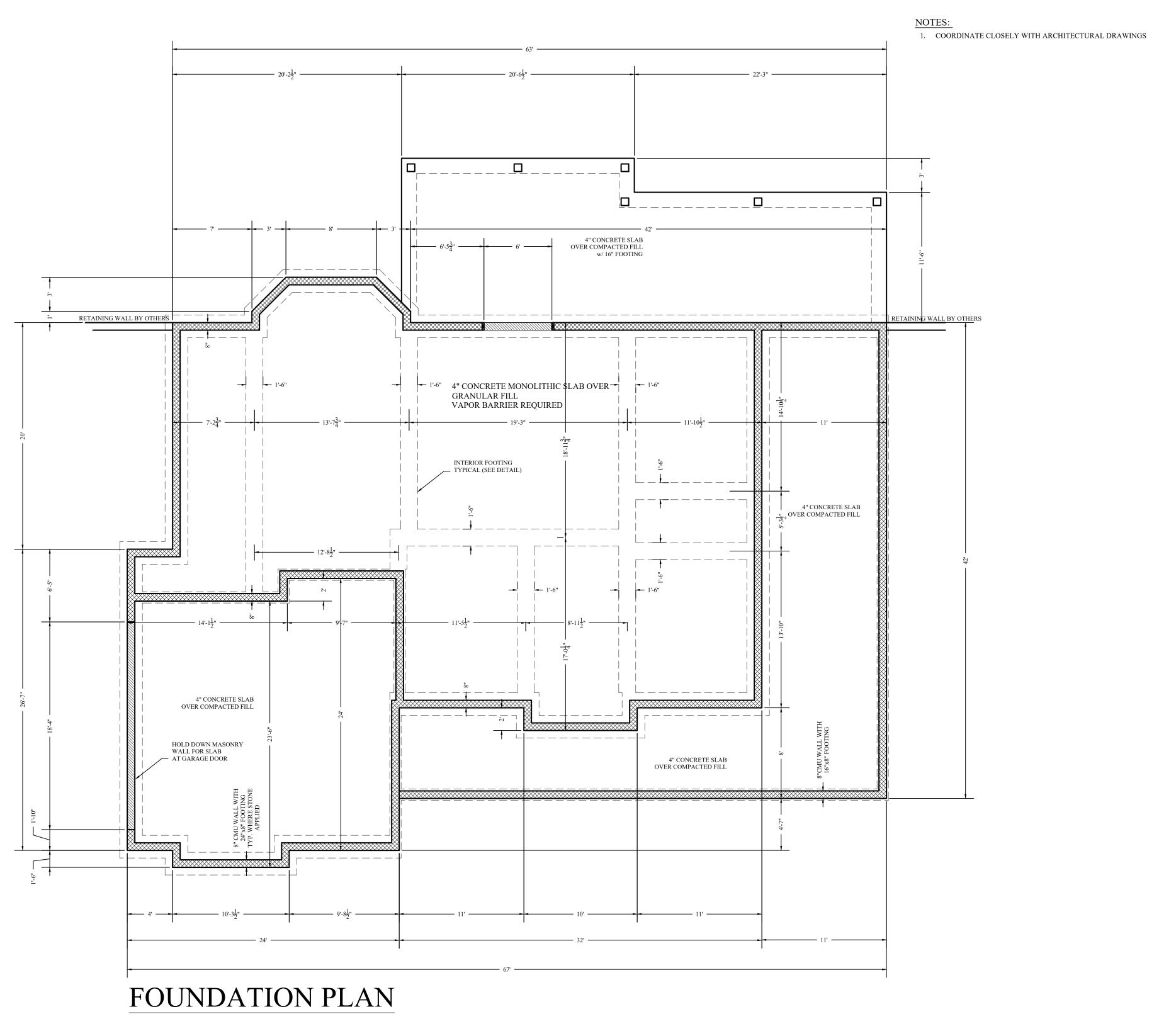
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FILE LOCATION:

18063

DRAWN BY: SHEET NO.



3/16"=1'-0"

WHITE HOUSE BUILDING ASSOCIATES LLC

WWW.WHITHOUSBBUILDING ASSOCIATES LLC

WWW.WHITHOUSBBUILDING ASSOCIATES COM
WHITHOUSBBLIAND NORTH CAROLINA
P.O. BOX 1682 LELAND, NORTH CAROLINA
910-978-5195

FOUNDATION PLAN

BER SHALL ENSURE COMPLIANCE WITH APPLICABLE BUILDING CODE
NANCES. ANY ERRORS OR OMISSIONS SHALL BE BROUGHT TO THE AT

BUCUR RESIDENCE
1185 JOE COLLINS ROAD

R1



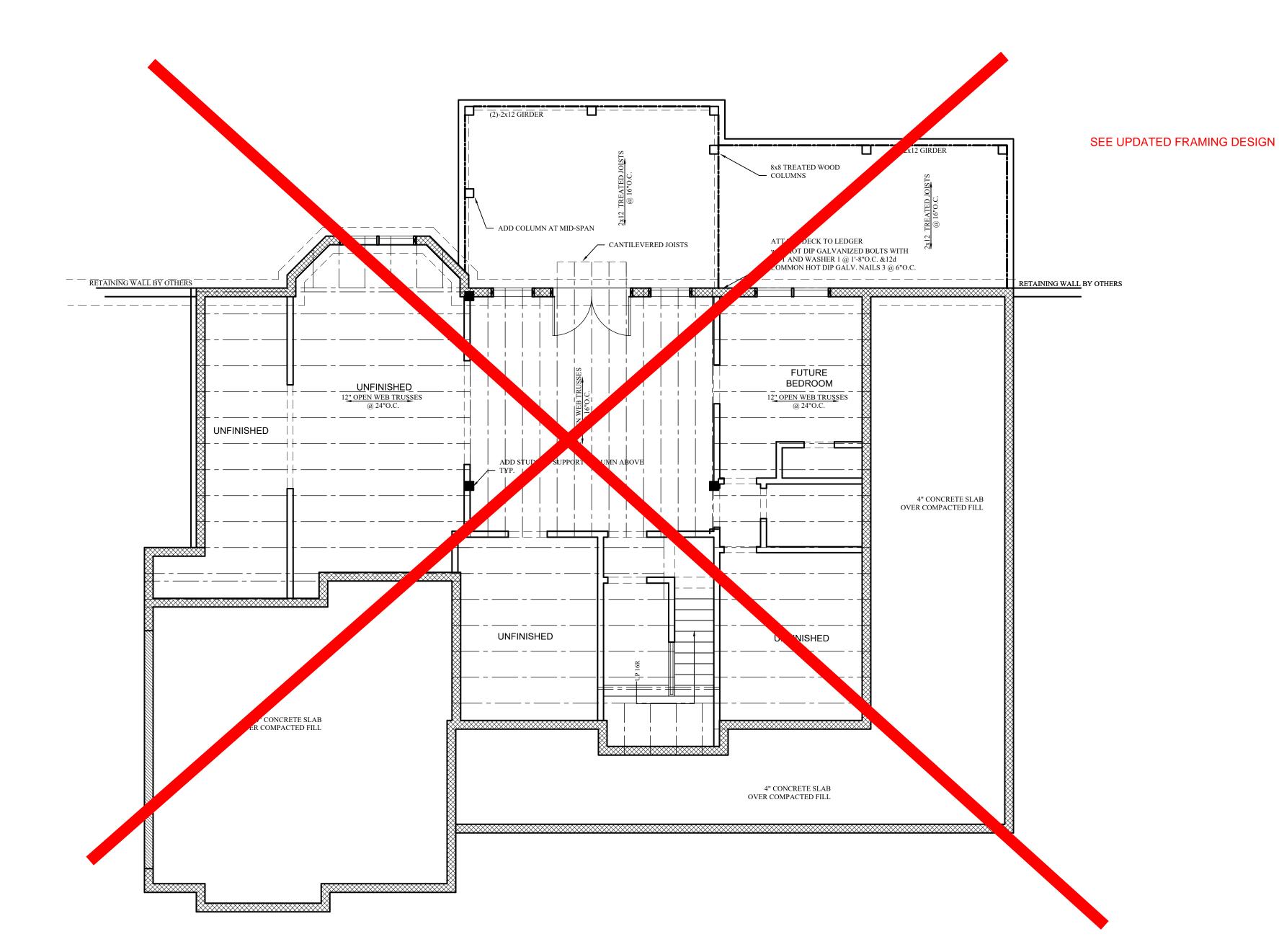
- NOTES REFER TO FRAMING DIRECTLY OVER HEAD
 COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS



BASEMENT

BUCUR RESIDENCE

2 OF 5



BASEMENT PLAN

3/16"=1'-0"



SEE UPDATED FRAMING DESIGN

COORDINATE ALL DIMENSIONS CLOSELY WITH ARCHITECTURAL DRAWINGS.
 ALL NOTES REFER TO FRAMING DIRECTLY OVER HEAD

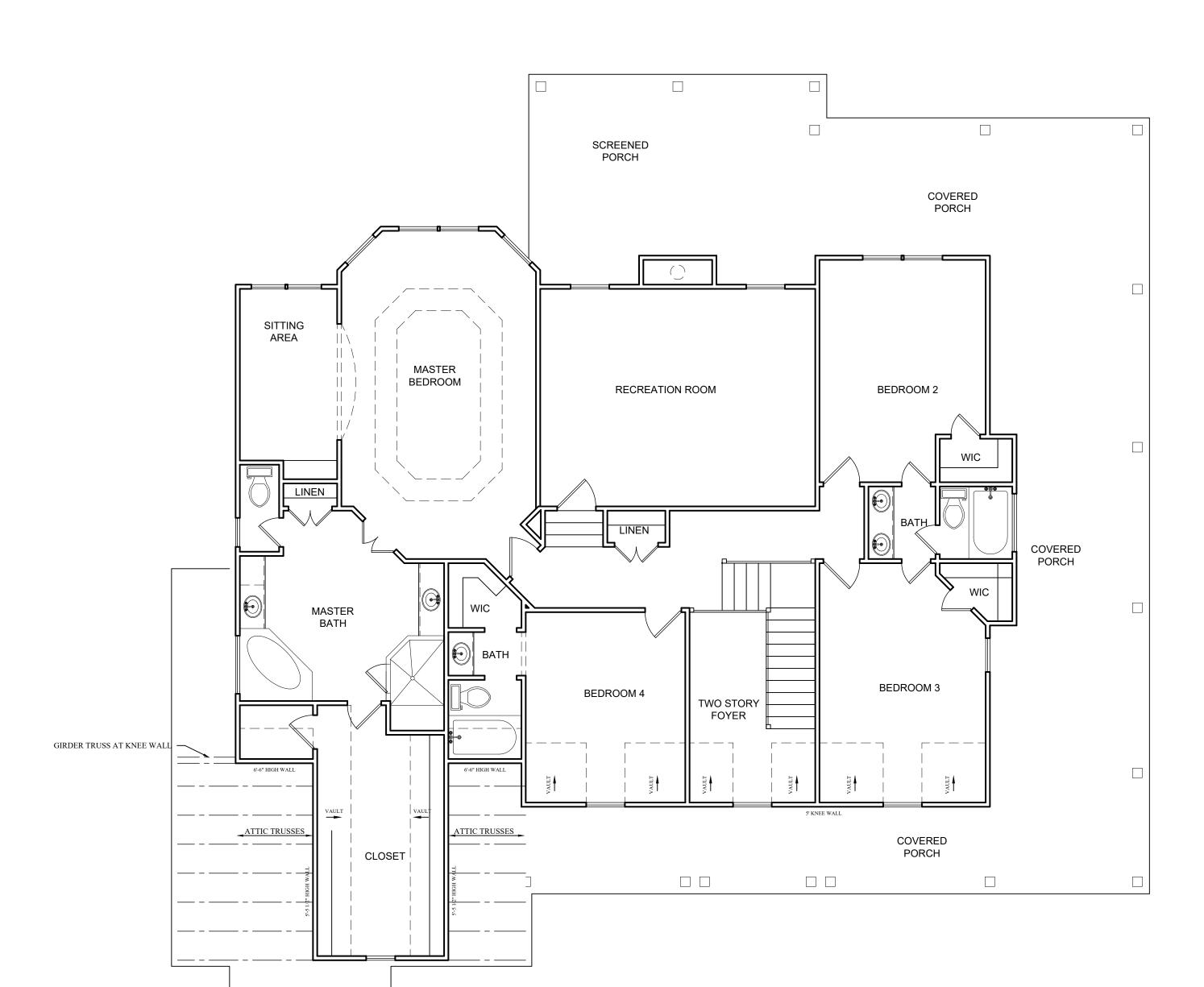
FIRST FLOOR PLAN

NA BUCUR RESIDENCE

3 OF 5

SCREENED 8x8 TREATED WOOD PORCH VAULT COVERED ADD COLUMN AT MID-SPAN CANTILEVERED JOISTS RETAINING WALL BY OTHERS RETAINING WALL BY OTHERS RETAINING WALL BELOW BREAKFAST SUN ROOM 12" OPEN WEB TRUSSES @ 24"O.C. GRAND ROOM BEDROOM 5/ STUDY 12" OPEN WEB TRUSSES @ 24"O.C. CANTILEVERED JOISTS -KITCHEN LAUNDRY LVL BEAM BY OTHERS COVERED PORCH PANTRY STORAGE GIRDER TRUSS DINING GARAGE TWD C TWO STORY GIRDER TRUSS AT KNEE WALL **FOYER** COVERED PORCH FIRST FLOOR PLAN 3/16"=1'-0" NOTES:

(2)-2x12 GIRDER



SECOND FLOOR PLAN

3/16"=1'-0"

NOTES:

1. COORDINATE DIMENSIONS WITH ARCHITECTURAL DRAWINGS



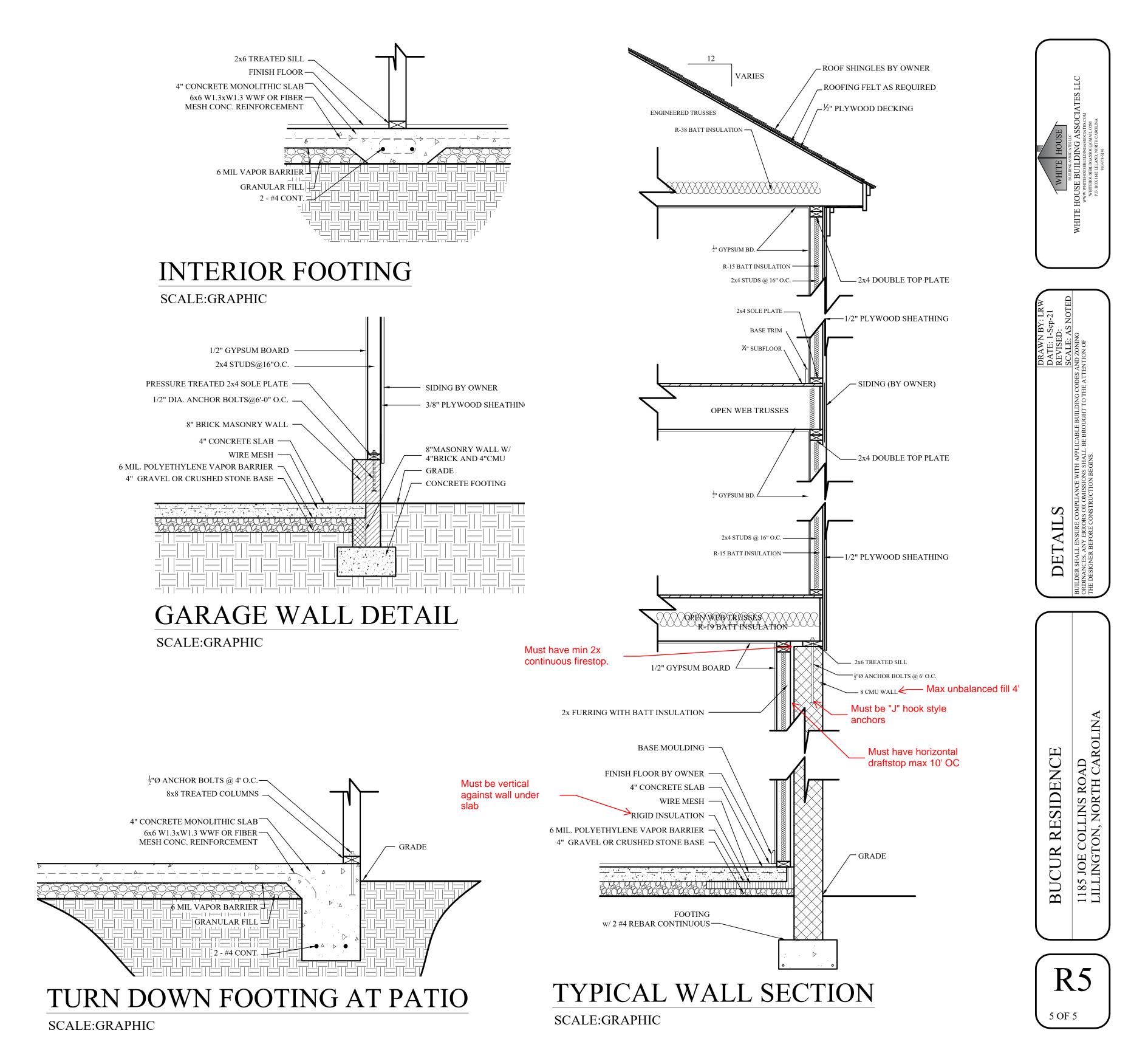
DRAWN BY: LRV
DATE: 1-Sep-21
REVISED:

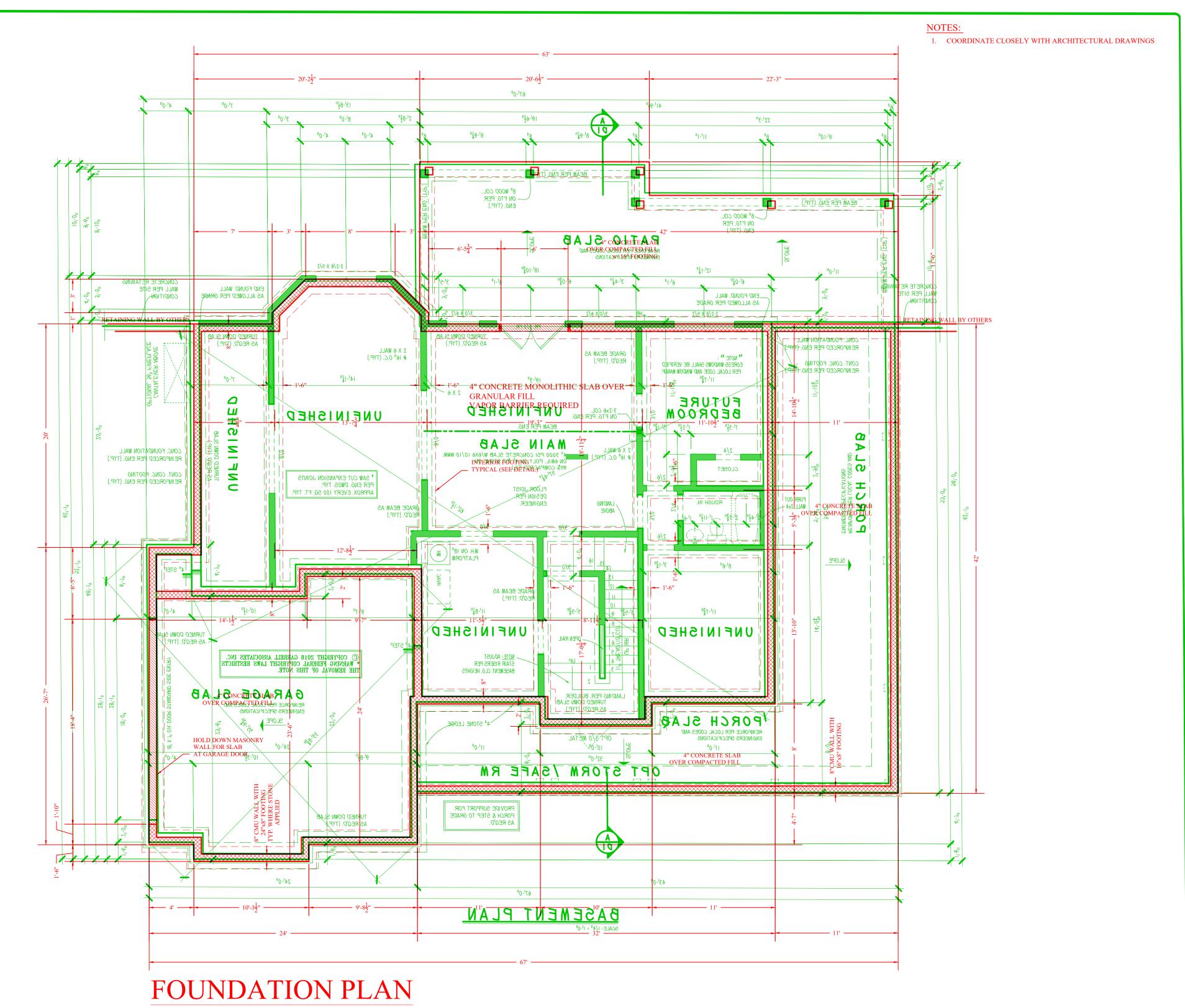
SECOND FLOOR PLAN
ILDER SHALL ENSURE COMPLIANCE WITH APPLICABLE BUILDING CC
DINANCES ANY FRRORS OR OMISSIONS SHALL BE BROLIGHT TO THE

BUCUR RESIDENCE

1418 CYPRESS CHURCH ROAD

R4
4 OF 5





GENERAL NOTES

I. ALL WORK SHALL BE FERFORMED IN ACCORDANCE MITH ALL WORK SHALL BE FERFORMED IN A AND LATEST INTERNATIONAL RESIDENTIAL BUILDING ADDITIONS AND LATEST INTERNATIONAL RESIDENTIAL BUILDING AND PURENSIONS AT THE SITE BEFORE BEGINNING CONSTRUCTION, ANY DISCREPANCIES SHALL BE AUGUSTRUCTION ANY DISCREPANCIES SHALL BE UNTIT HE AND THE CORRECTION BEFORE PROCEEDING SHALL BUTTH THE REPORTED TO BE PROPERED AND LITTLY FOR ERFORS THAT ARE NOT REPORTED.

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10. TO 24" FROM THE GRADE OF THAN OR EQUAL TO 24" THAN OR THAN OR ELUCATE.

11. THE STATE 200 THAN OR ELUCATE OR THAN OR ELUCATE.

12. MINISTRUCTURES OF THAN OR ELUCATE OR THAN OR ELUCATE.

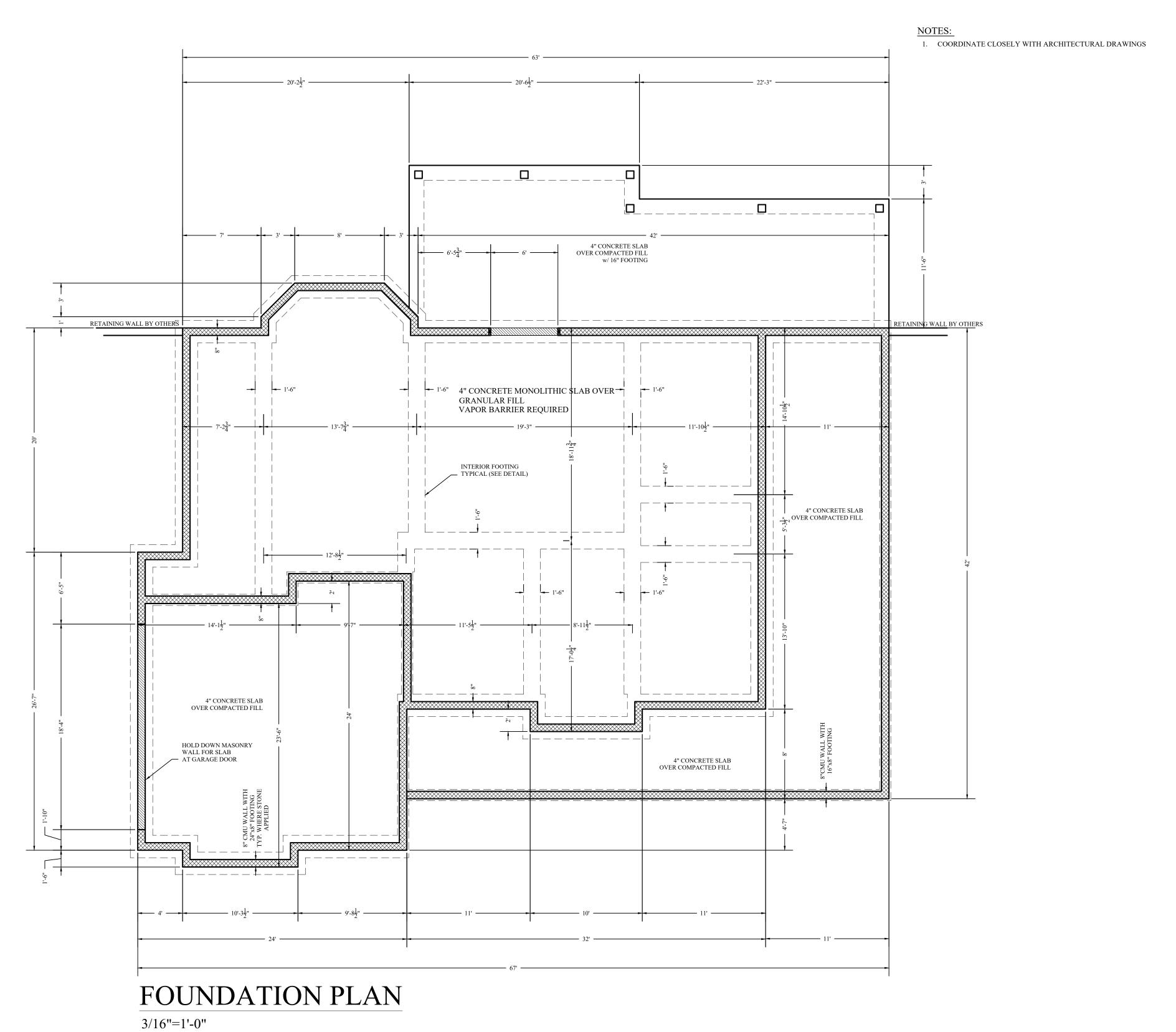
13. MINISTRUCTURES OF THAN OR ELUCATE OR THAN OR ELUCATE.

14. THE MOUNTED CLOSER THAN 24" AFF.

3/16"=1'-0"

BUCUR RESIDENCE JOE COLLINS I 1185. LILLI

1 OF 5



WHITE HOUSE BUILDING ASSOCIATES LLC
WWW.WHITEHOUSEBUILDING ASSOCIATES COM
WHITEHOUSEBUILDING ASSOCIATES COM
P.O. BOX 1082 LEGAND, NORTH CAROLINA
910-978-5195

FOUNDATION PLAN

PER SHALL ENSURE COMPLIANCE WITH APPLICABLE BUILDING CODES

BUCUR RESIDENCE
1185 JOE COLLINS ROAD

R1

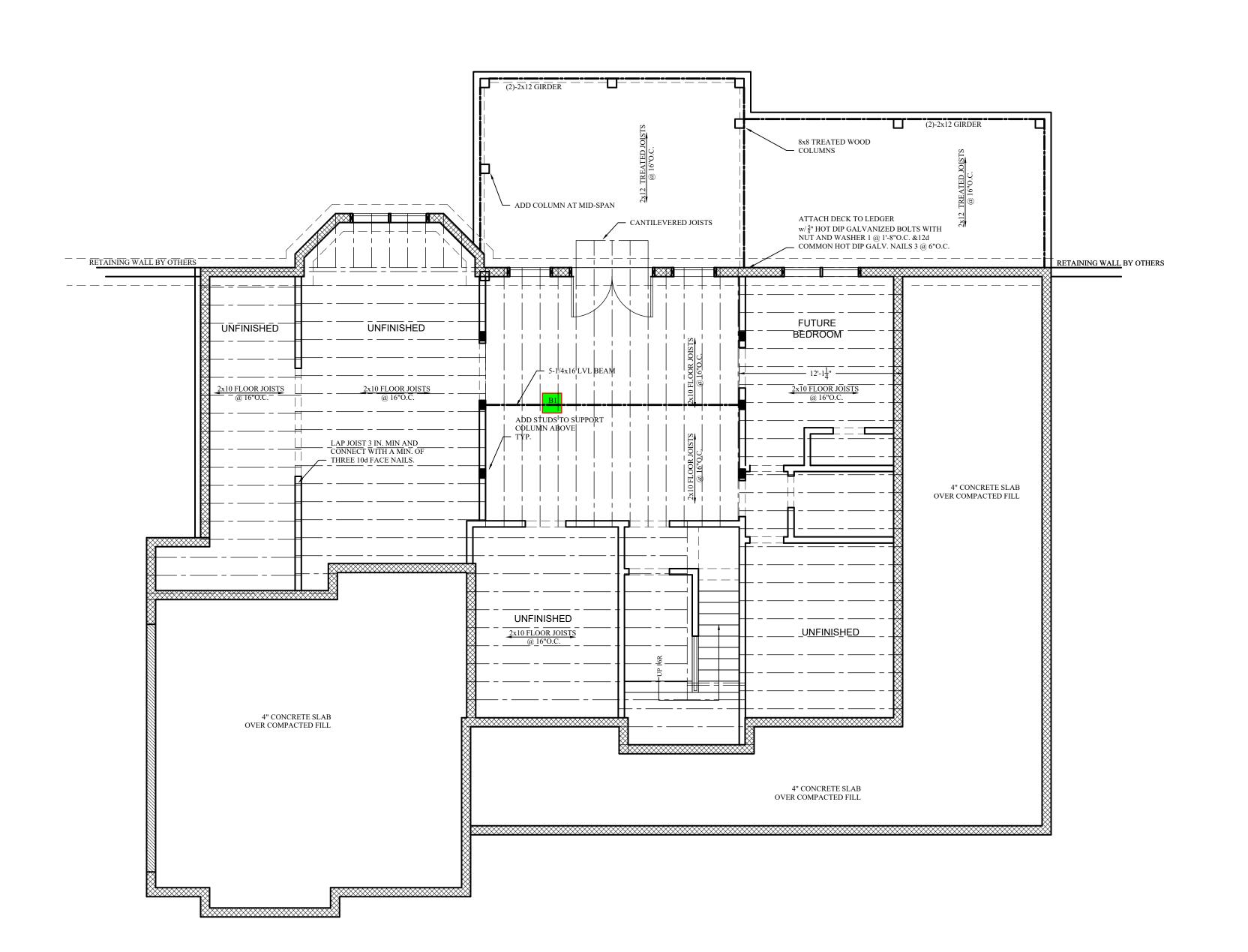
- NOTES REFER TO FRAMING DIRECTLY OVER HEAD
 COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS



BASEMENT

BUCUR RESIDENCE

2 OF 5



BASEMENT PLAN

3/16"=1'-0"



FIRST FLOOR PLAN

3/16"=1'-0"

NOTES:

- 1. COORDINATE ALL DIMENSIONS CLOSELY WITH ARCHITECTURAL DRAWINGS.
- 2. ALL NOTES REFER TO FRAMING DIRECTLY OVER HEAD



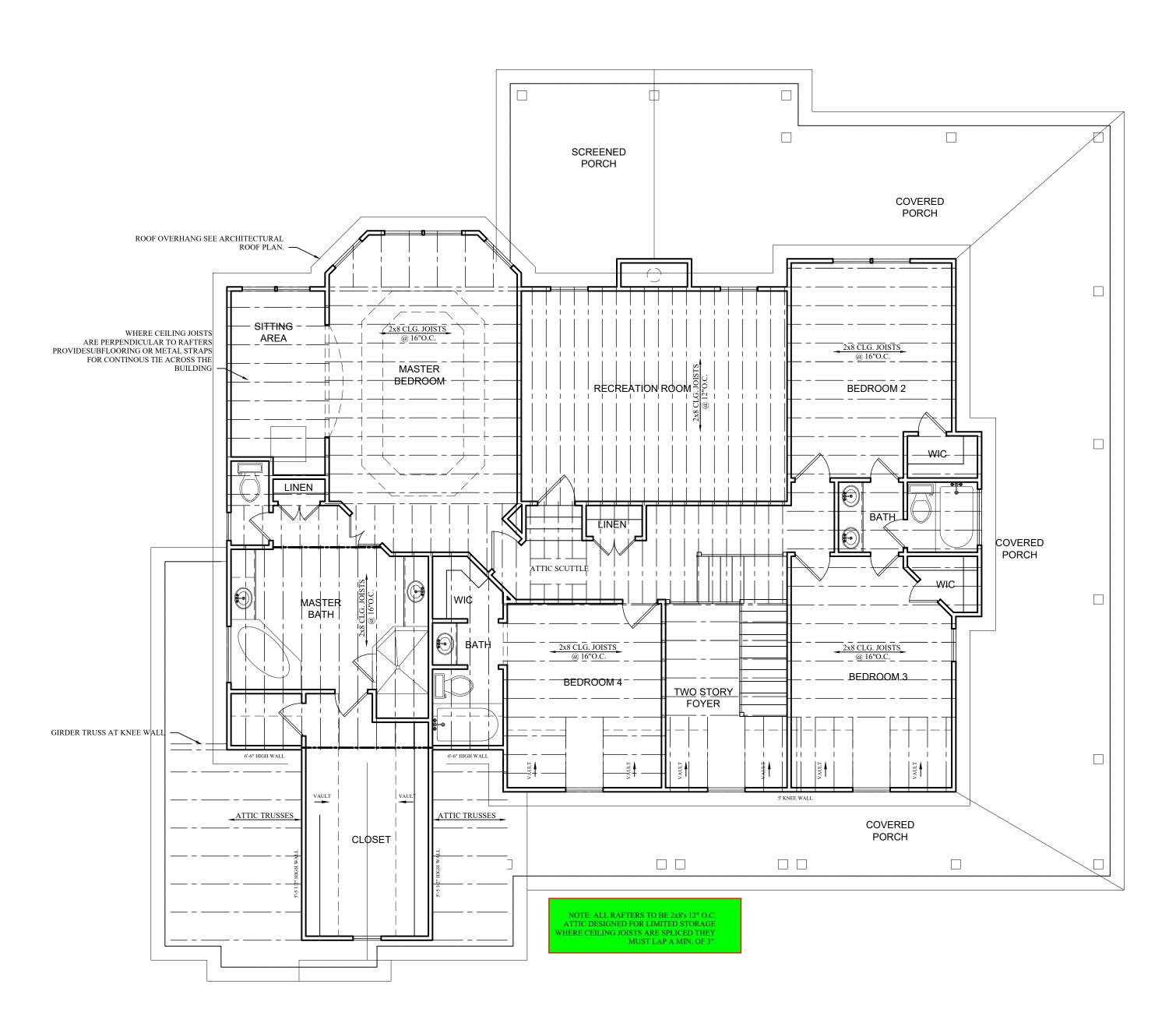
AN DATE: 23-Nov-21 REVISED:
SCALE: AS NOTE

FIRST FLOOR PLAN
BUILDER SHALL ENSURE COMPLIANCE WITH APPLICABLE BUILL
ORDINANCES. ANY ERRORS OR OMISSIONS SHALL BE BROUGHT?
THE DESIGNER BEFORE CONSTRUCTION REGINS

BUCUR RESIDENCE
1418 CYPRESS CHURCH ROAD

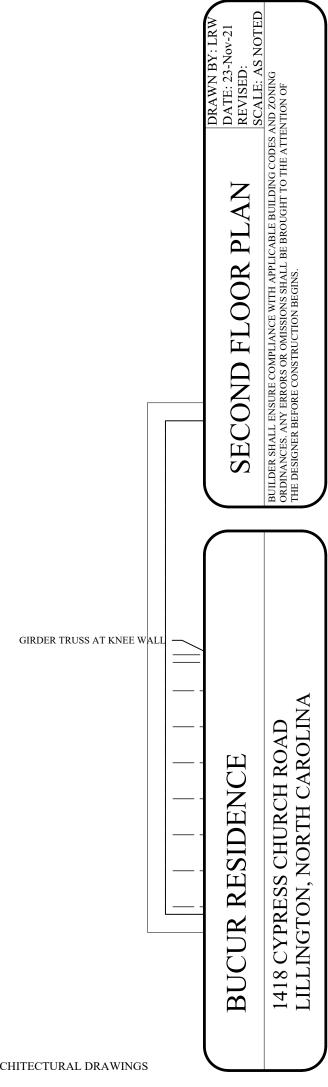
R3

3 OF 5



SECOND FLOOR PLAN

3/16"=1'-0"

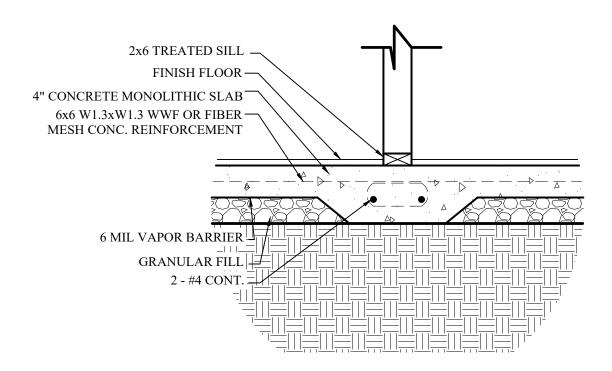


R4

4 OF 5

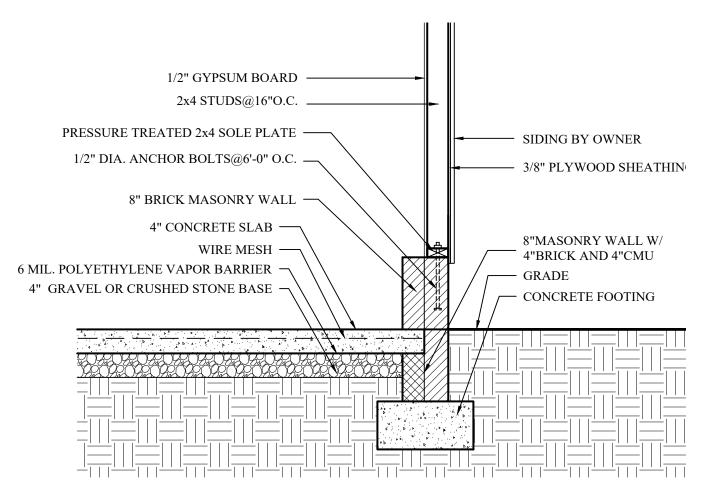
NOTES:

1. COORDINATE DIMENSIONS WITH ARCHITECTURAL DRAWINGS



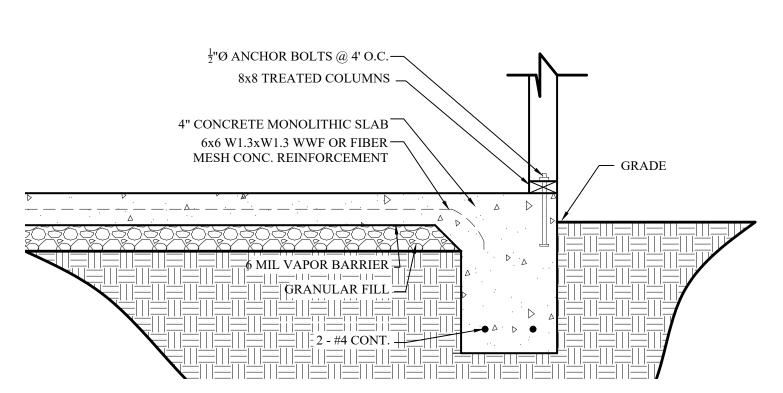
INTERIOR FOOTING

SCALE:GRAPHIC



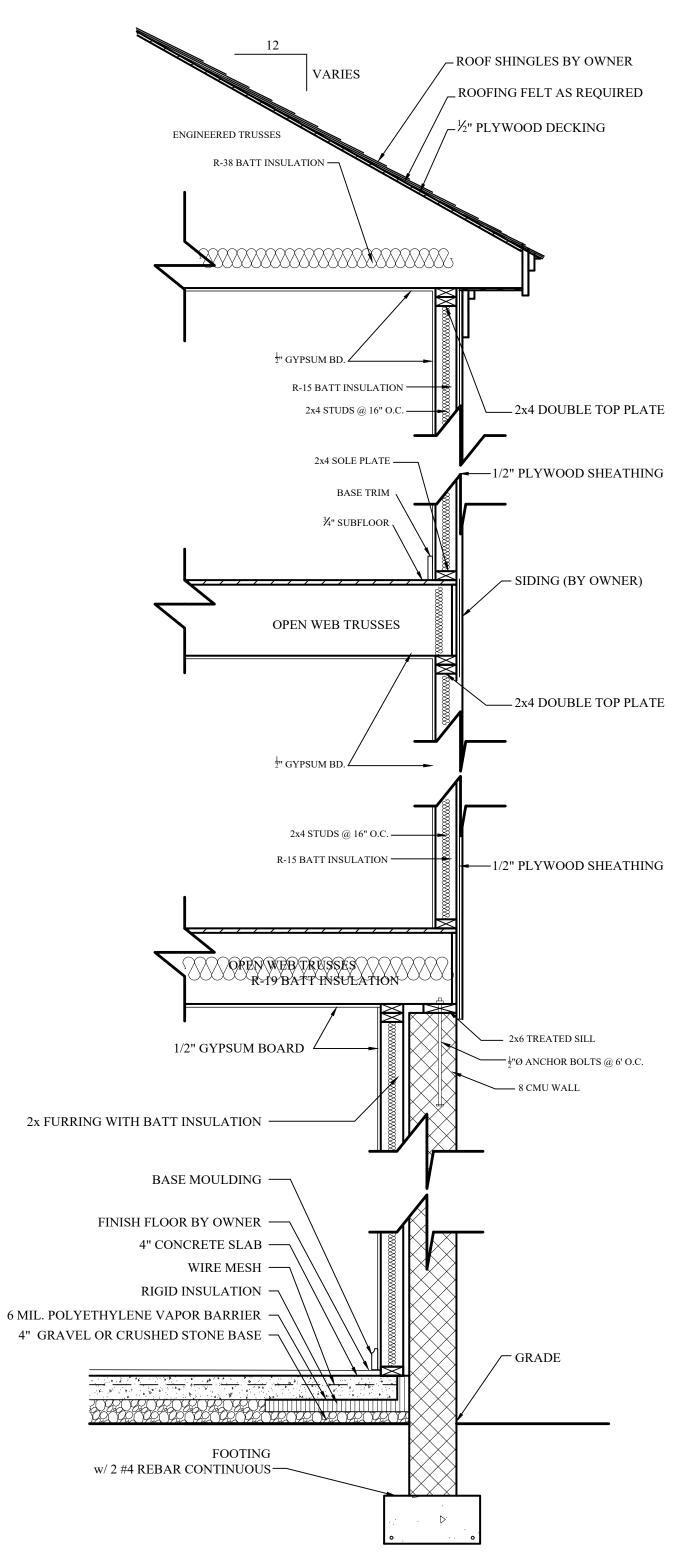
GARAGE WALL DETAIL

SCALE:GRAPHIC



TURN DOWN FOOTING AT PATIO

SCALE:GRAPHIC



TYPICAL WALL SECTION

SCALE:GRAPHIC



BUCUR RESIDENCE

DETAILS

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ser Inputs											_
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	Girder		oisture Conditi			Spar		6-8-0			
	Floor		emperature:	Temp <= 10		Spar		6-8-0			
	4		ecking:	Not Checked	i	Bearin					
	LVL LP LVL 2900F-2.0		efl. LL Span:	L / 480 L / 240		Brg 1		3.5" SPF 3.5" SPF			
	11.25		efl. LL Cant: efl. TL Span:	L / 240 L / 240		Brg 2 Brg 3		3.5" SPF 3.5" SPF			
	1.5		efl. TL Cant:	L / 120		Brg 4		3.5" SPF			
						_					
nalysis Details											
laterial Properties ame		E	Fb	Fcp	Ev	G	Density				
P LVL 2900F-2.0E		2E6	2900	750	285	125000	41.2				
esistance Factors											
oment Factor	Shear Factor			Cr-Bending	Cr-Shear	Load Sharin		Ct (E)			
24614336421787 (including Ct (E))		1		1.04	1	Yes	1	1			
are El		oosite El		Ct (E) (temp. fa	ctor for E)						
123828E+009	1.423	828E+00!	9	1							
and Combine	tions Checked	for Str	anath (East	ore include i	mnortanco	factor)					
	Description			Cd-Duration D		L L	S	W	С	٦	
	D	1		0.9	1	0	0	0	0		
	D+L	7		1	1	1	0	0	0		
oad Combinat	tions Checked	for Def	flection (To	tal Loads: De	ad + Live L	oads)					
	Description			Cd-Duration D		L	S	W	С	7	
	D	1		0.9	1	0	0	0	0		
	D+L	7		1	1	1	0	0	0		
	tions Checked									_	
	Description			Cd-Duration D		L	S	w	С		
	L	7		1	0	1	0	0	0		

BEAM DATA
SCALE:GRAPHIC

		Client: Project: Address:			Date: Input by: Job Name: Project #:	11/23/2021			Page 2 of
B1 LP-LVL 2	2900Fb-2.0	E 5.250" X	16.000"	- PASSED		Level			
Bearing Calculation									
Brg. No. Capacity 1 425	Input Length 5.5	Req'ed Length R 5.5	eaction 6015.3	MR Load Com	b. MR Load	Case MR Dead 1815.3	MR Live 4200	Uplift 0	
2 425	5.5	5.5	6015.3	D+L D+L		1815.3	4200 4200	0	
			0010.0	5.2		1010.0	4200		
Maximum Mome Mem No. Span No.	ent at Each Memi Combination		Cd CL I	Resist. Factors Mom	ent	Span-X	Mr	Mr_orig	Ratio
1 Spn 1	D+L	L		0.941 0.941	27803	9-7-6	51954	51954	0.5351
Maximum Shear	at Each Member								
Mem No. Span No.	Brg No. Max	Combination	Load C	ase Cd Res	 ac	Max She	ear Vr	Ratio	
1 Spn 1	2 Y	es D+L	L	1 1		4970	15960	0.3114	1
Maximum Deflec	tion on Span and	d Cantilever for T	ntal Load (De	ead + Live)					
Def. Span Desc.	Combination	Load Case		n Span ID Span-X	Span Ar	nalog Length L / Allov	wable L / Actual	l Ratio	1
Critical Span	D+L	L	0.5544	Spn 1 9-7-7	19-2-12	240	416.2		
Maximum Deflec	tion on Span and	d Cantilever for Li	ve Load Only	,					
Def. Span Desc.	Combination	Load Case	Max Deflection	n Span ID Span-X		nalog Length L / Allov			1
Critical Span	L	L	0.3871	Spn 1 9-7-7	19-2-12	480	596.1	0.8053	J
o SolidStart Design Version	5.0.368 (Build 2151368)) Powered by Struct Data	sset: 21062901.1457						
⁹ SolidStart Design Version	5.0.368 (Build 21.51.368)	Powered by Struct Data Client: Project:	sset: 21062901.1457	,	Date:	11/23/2021			Page 2 of

Bearing Calculation									
Brg. No. Capacity	Input Length	Reg'ed Lengt			omb. MR Load C		MR Live	Uplift	
1 425 2 425	3.5 3.5	3.5 3.5	2443.1 2443.1	D+L D+L	L L	943.1 943.1	1500 1500	0	
Maximum Momei									
Mem No. Span No. 1 Spn 1	Combination D+L	Load Ca	se Cd Cl	Resist. Factors M 0.365 0.379	oment 11662	Span-X 9-9-4	Mr 32038	Mr_orig 7702	Ratio 0.364
				0.000 0.075	11002	5.5.4	02000	7702	0.004
Maximum Shear a									_
Mem No. Span No.	Brg No. Max	Combina	tion Load	Case Cd F	Res Fac	Max Shear	r Vr	Ratio	
1 Spn 1	1 Y	es D+L	L	1	1	2161	12825	0.168	5
Maximum Deflect	ion on Span and	d Cantilever fo	r Total Load (F	(avil + bea					
Def. Span Desc.	Combination	Load Case		on Span ID Span-X	Span Ana	log Length L / Allowa	ble L / Actual	Ratio	1
Critical Span	D+L	L	0.5829	Spn 1 9-9-5	19-6-8	240	402.3		
Maximum Deflect	ion on Span and	d Cantilever fo	r Live Load On	lv					
Def. Span Desc.	Combination	Load Case		on Span ID Span-X	Span Ana	log Length L / Allowa	ble L / Actual	Ratio	1
Critical Span	L	L	0.3579	Spn 1 9-9-5	19-6-8	480	655.2		

		Cli	ent:				Date:	11/23/2021			Page 2 of
			oject:				Input by:				-
			dress:					e: Bucur Residence Be	ams		
							Project #				
3 LP-LVL 29	00Fb-2.0E	1.50	00" X 11.25	50 ! -Ply -	PASSED)		Level: Level			
earing Calculation											
g. No. Capacity	Input Lengt		Req'ed Length R			oad Co		ad Case MR Dead	MR Live	Uplift	
425	3.5		3.5	1539.8	D+L		L_L	443.3	1096.5	0	
425 425	3.5		3.5	6392.6	D+L		LL_	1149.8	5242.8	0	
425	3.5 3.5		3.5 3.5	6392.6 1539.8	D+L D+L		_LL	1149.8 443.3	5242.8 1096.5	0	
			1.0	1339.0	DVL			443.3	1090.5		
laximum Mome											
em No. Span No.	Combina	ation	Load Case		CL Resist. Fac	tors Mo		Span-X	Mr	Mr_orig	Ratio
Spn 1	D+L		LL_	1	0.237 0.246		-2421	6-5-4	32038	7702	0.0756
Spn 2	D+L D+L		LL_ _LL	1	0.237 0.246 0.237 0.246		-2423 -2423	0-0-0 0-0-0	32038 32038	7702 7702	0.0756 0.0756
Spn 3			_LLL	1	0.237 0.246		-2423	0-0-0	32038	7702	0.0756
aximum Shear			0		10	010			h	D-#	_
em No. Span No.	Brg No. I	мах	Combination	1 Load	d Case	Cd Re	es Fac	Max S	hear Vr	Ratio	'
Spn 1	2	Yes	D+L	LL_		1	1	1482	1282		
Spn 2	3	No	D+L	_LL		1	1	1343	1282		
Spn 3	3	No	D+L	_LL		1	1	1482	1282	5 0.115	6
aximum Deflec	tion on Span	and Ca	antilever for T	otal Load (Dead + Live	•)					
ef. Span Desc.	Combination		oad Case		tion Span ID S		Span	Analog Length L / All	owable L / Actua	al Ratio	7
itical Span	D+L		_L	0.0141	Spn 3 3-4-		6-5-4	240	5461		
aximum Deflec			antilever for L	ive Load O	nly						_
ef. Span Desc.	Combination	L	oad Case	Max Deflec	tion Span ID S	enan Y	Snan	Analog Length L / All		10-6-	
itical Span	L	L	<u>_L</u>	0.0105	Spn 3 3-3-		6-5-4	480	7387		
itical Span	L	L	<u>.L</u>								
itical Span	L	L	<u>u</u>								
ritical Span	L	L	<u>u</u>								
ritical Span	L	L	<u>J.</u>								J
ritical Span	L	L	<u>.</u>								
ritical Span	L	L	u.								
ritical Span	L	L	<u>J</u>								
Span Span	L	L	J.								J
Span Span	L	ι	<u>J</u>								J
Span Span	L	ι	J.								
Span Span	L	ι	<u>.</u>								
Span Span	L	ι	<u>.</u>								J
dical Span	L	ι	<u>.</u>								
ical Span	L	ı	<u>.</u>								

SEAM DATA

BUILDER SHALL ENSURE COMPLIANCE WITH APPLICABLE BUILDIN ORDINANCES. ANY ERRORS OR OMISSIONS SHALL BE BROUGHT TC THE DESIGNER BEFORE CONSTRUCTION BEGINS.

BUCUR RESIDENCE
1185 JOE COLLINS ROAD
LILLINGTON, NORTH CAROLINA

B1

1 OF 1

Date: 11/23/2021

Input by:

Job Name: Bucur Residence Beams

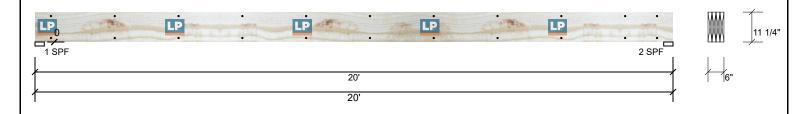
Page 1 of 2

Project #:

1.500" X 11.250" LP-LVL 2900Fb-2.0E

4-Ply - PASSED

Level: Level



User Inputs

Building Code:

Application:

Material Type:

Type:

Plies:

Design Method:

ASD

Girder

Floor

4

LVL

IBC/IRC 2015

Load Sharing: Importance:

Normal - II

Yes Moisture Condition: Dry

Temp <= 100°F

Decking: Not Checked Defl. LL Span: L/480

Material Name: LP LVL 2900F-2.0E Depth: 11.25 Width:

1.5

Defl. LL Cant: Defl. TL Span: Defl. TL Cant:

Temperature:

L/240 L / 120

L/240

Spans

Span 1: 20-0-0

Bearings

Brg 1: 3.5" SPF Brg 2: 3.5" SPF

Analysis Details

Material Properties

Е Fv Fb G Name Fcp Density 2E6 2900 285 125000 LP LVL 2900F-2.0E 750 41.2

Resistance Factors

Moment Factor 0.379342277548014 1 EI (including Ct (E))

Shear Factor Comp Perp Factor

Cr-Bendina 1.04

Cr-Shear

Load Sharing Yes

Ct 1

Ct (E)

Bare EI Composite EI 1.423828E+009

1.423828E+009

Ct (E) (temp. factor for E) 1

Load Combinations Checked for Strength (Factors include importance factor)

Comb. No.	Description	Pattern Count	Cd-Duration	D	L	S	W	С
1	D	1	0.9	1	0	0	0	0
2	D+L	1	1	1	1	0	0	0

Load Combinations Checked for Deflection (Total Loads: Dead + Live Loads)

Comb. No.	Description	Pattern Count	Cd-Duration	D	L	S	W	С	
1	D	1	0.9	1	0	0	0	0	
2	D+L	1	1	1	1	0	0	0	

Load Combinations Checked for Deflection (Live Loads)

Comb. No.	Description	Pattern Count	Cd-Duration	D	L	S	W	С	
1	L	1	1	0	1	0	0	0	

Date: 11/23/2021

Input by:

Job Name: Bucur Residence Beams

Page 2 of 2

Project #:

B2 LP-LVL 2900Fb-2.0E 1.500" X 11.250" 4-Ply - PASSED

Level: Level

Brg. No.	Capacity	Input Length	Req'ed Length	Reaction	MR Load Comb.	MR Load Case	MR Dead	MR Live	Uplift
1	425	3.5	3.5	2443.1	D+L	L	943.1	1500	0
2	425	3.5	3.5	2443.1	D+L	L	943.1	1500	0

Maximum Moment at Each Member

ſ	Mem No.	Span No.	Combination	Load Case	Cd	CL	Resist. Factors	Moment	Span-X	Mr	Mr_orig	Ratio
	1	Spn 1	D+L	L	1	0.365	0.379	11662	9-9-4	32038	7702	0.364

Maximum Shear at Each Member

Mem No.	Span No.	Brg No.	Max	Combination	Load Case	Cd	Res Fac	Max Shear	Vr	Ratio
1	Spn 1	1	Yes	D+L	L	1	1	2161	12825	0.1685

Maximum Deflection on Span and Cantilever for Total Load (Dead + Live)

Def. Span Desc.	Combination	Load Case	Max Deflection	Span ID	Span-X	Span Analog Length	L / Allowable	L / Actual	Ratio	
Critical Span	D+L	L	0.5829	Spn 1	9-9-5	19-6-8	240	402.3	0.5966	

Maximum Deflection on Span and Cantilever for Live Load Only

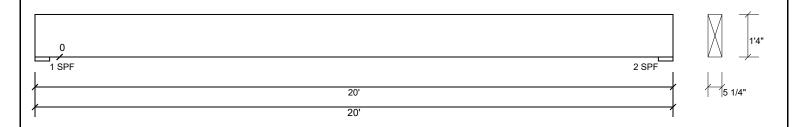
Def. Span Desc.	Combination	Load Case	Max Deflectio	n Span ID) Span-X	Span Analog Length	L / Allowable	L / Actual	Ratio
Critical Span	L	L	0.3579	Spn 1	9-9-5	19-6-8	480	655.2	0.7326

Date: 11/23/2021 Page 1 of 2

Input by: Job Name: Project #:

5.250" X 16.000" - PASSED LP-LVL 2900Fb-2.0E

Level: Level



User Inputs

Building Code:

Application:

Material Type:

Type:

Plies:

Design Method: ASD

IBC/IRC 2015

Girder

Floor

1

LVL

Load Sharing: Importance:

Temperature:

Nο

Normal - II

Moisture Condition: Dry

Temp <= 100°F Not Checked

Decking: Defl. LL Span: L/480 L/240

Material Name: LP LVL 2900F-2.0E Defl. LL Cant: Depth: 16 Defl. TL Span: L/240 Width: 5.25 Defl. TL Cant: L / 120

Analysis Details

Material Properties

Е Fν Fb G Name Fcp Density 2E6 2900 285 125000 LP LVL 2900F-2.0E 750 41.2

Resistance Factors

Moment Factor 0.94127771153381 EI (including Ct (E))

Shear Factor

Comp Perp Factor

Cr-Bendina

Cr-Shear

Load Sharing No

Spans

Span 1:

Bearings

Brg 1:

Brg 2:

20-0-0

5.5" SPF

5.5" SPF

Ct 1

Ct (E)

Bare EI

3.584000E+009

3.584000E+009

Composite EI

1

Ct (E) (temp. factor for E)

Load Combinations Checked for Strength (Factors include importance factor)

١	Comb. No.	Description	Pattern Count	Cd-Duration	D	L	S	W	С
	1	D	1	0.9	1	0	0	0	0
l	2	D+L	1	1	1	1	0	0	0

Load Combinations Checked for Deflection (Total Loads: Dead + Live Loads)

Comb. No.	Description	Pattern Count	Cd-Duration	D	L	S	W	С	
1	D	1	0.9	1	0	0	0	0	
2	D+L	1	1	1	1	0	0	0	

Load Combinations Checked for Deflection (Live Loads)

OI- N	D	D-# Ot	O-I D				14/		
Comb. No.	Description	Pattern Count	Cd-Duration	D	L	5	VV	C	
1	L	1	1	0	1	0	0	0	

ate: 11/23/2021

Input by:
Job Name:
Project #:

B1 LP-LVL 2900Fb-2.0E 5.250" X 16.000" - PASSED

Level: Level

Page 2 of 2

Bearing Calculation (N	MR: Max Reaction)
------------------------	-------------------

ſ	Brg. No.	Capacity	Input Length	Req'ed Length	Reaction	MR Load Comb.	MR Load Case	MR Dead	MR Live	Uplift
	1	425	5.5	5.5	6015.3	D+L	L	1815.3	4200	0
	2	425	5.5	5.5	6015.3	D+L	L	1815.3	4200	0

Maximum Moment at Each Member

Mem No.	Span No.	Combination	Load Case	Cd	CL	Resist. Factors	Moment	Span-X	Mr	Mr_orig	Ratio
1	Spn 1	D+L	L.	1	0.941	0.941	27803	9-7-6	51954	51954	0.5351

Maximum Shear at Each Member

Mem No.	Span No.	Brg No.	Max	Combination	Load Case	Cd	Res Fac	Max Shear	Vr	Ratio
1	Spn 1	2	Yes	D+L	L	1	1	4970	15960	0.3114

Maximum Deflection on Span and Cantilever for Total Load (Dead + Live)

Def. Span Desc.	Combination	Load Case	Max Deflec	tion Span II	O Span-X	Span Analog Length	L / Allowable	L / Actual	Ratio
Critical Span	D+L	L	0.5544	Spn 1	9-7-7	19-2-12	240	416.2	0.5767

Maximum Deflection on Span and Cantilever for Live Load Only

Def. Span Desc.	Combination	Load Case	Max Deflection	n Span ID	Span-X	Span Analog Length	L / Allowable	L / Actual	Ratio
Critical Span	L	L	0.3871	Spn 1	9-7-7	19-2-12	480	596.1	0.8053

Date: 11/23/2021

Input by:

Job Name: Bucur Residence Beams

6-8-0

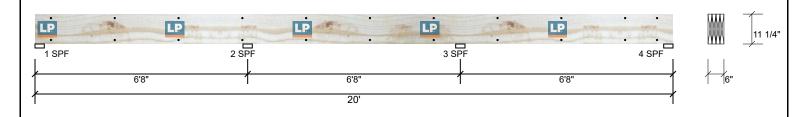
6-8-0

6-8-0

Page 1 of 2

Project #:

LP-LVL 2900Fb-2.0E 1.500" X 11.250" 4-Ply - PASSED Level: Level



User Inputs

ASD Design Method: Load Sharing: Yes Spans IBC/IRC 2015 **Building Code:** Importance: Normal - II Span 1: Span 2: Type: Girder Moisture Condition: Dry Application: Floor Temperature: Temp <= 100°F Span 3: Plies: 4 Decking: Not Checked Bearings Material Type: LVL Defl. LL Span: L/480

Brg 1: 3.5" SPF Material Name: LP LVL 2900F-2.0E Defl. LL Cant: L/240 Brg 2: 3.5" SPF Depth: 11.25 Defl. TL Span: L/240 Brg 3: 3.5" SPF Width: 1.5 Defl. TL Cant: L / 120 Brg 4: 3.5" SPF

Analysis Details

Material Properties

Е Fcp Fb Fv G Name Density 2E6 2900 285 125000 LP LVL 2900F-2.0E 750 41.2

Resistance Factors

Moment Factor Shear Factor Comp Perp Factor Cr-Bending Cr-Shear Load Sharing Ct Ct (E) 0.246143364217874 1 EI (including Ct (E)) 1.04 1 Yes

Bare El Composite EI Ct (E) (temp. factor for E)

1.423828E+009 1.423828E+009 1

Load Combinations Checked for Strength (Factors include importance factor)

Comb. No.	Description	Pattern Count	Cd-Duration	D	L	S	W	С
1	D	1	0.9	1	0	0	0	0
2	D+L	7	1	1	1	0	0	0

Load Combinations Checked for Deflection (Total Loads: Dead + Live Loads)

Comb. No.	Description	Pattern Count	Cd-Duration	D	L	S	W	С	
1	D	1	0.9	1	0	0	0	0	
2	D+L	7	1	1	1	0	0	0	

Load Combinations Checked for Deflection (Live Loads)

Comb. No.	Description	Pattern Count	Cd-Duration	D	L	S	W	С
1	L	7	1	0	1	0	0	0

Date: 11/23/2021

Input by:

Job Name: Bucur Residence Beams

Page 2 of 2

Project #:

B3 LP-LVL 2900Fb-2.0E 1.500" X 11.250" 4-Ply - PASSED

Level: Level

Bearing Calculation (MR: Max Reaction)

Brg. N	lo. Capacity	Input Length	Req'ed Length	Reaction	MR Load Comb.	MR Load	Case MR Dead	MR Live	Uplift	
1	425	3.5	3.5	1539.8	D+L	L_L	443.3	1096.5	0	
2	425	3.5	3.5	6392.6	D+L	LL_	1149.8	5242.8	0	
3	425	3.5	3.5	6392.6	D+L	_LL	1149.8	5242.8	0	
4	425	3.5	3.5	1539.8	D+L	L_L	443.3	1096.5	0	

Maximum Moment at Each Member

Mem No.	Span No.	Combination	Load Case	Cd	CL	Resist. Factors	Moment	Span-X	Mr	Mr_orig	Ratio
1	Spn 1	D+L	LL_	1	0.237	0.246	-2421	6-5-4	32038	7702	0.0756
2	Spn 2	D+L	LL_	1	0.237	0.246	-2423	0-0-0	32038	7702	0.0756
3	Spn 3	D+L	_LL	1	0.237	0.246	-2423	0-0-0	32038	7702	0.0756

Maximum Shear at Each Member

Mem No.	Span No.	Brg No.	Max	Combination	Load Case	Cd	Res Fac	Max Shear	Vr	Ratio
1	Spn 1	2	Yes	D+L	LL_	1	1	1482	12825	0.1156
2	Spn 2	3	No	D+L	_LL	1	1	1343	12825	0.1047
3	Spn 3	3	No	D+L	_LL	1	1	1482	12825	0.1156

Maximum Deflection on Span and Cantilever for Total Load (Dead + Live)

Def. Span Desc.	Combination	Load Case	Max Deflection	Span ID	Span-X	Span Analog Length	L / Allowable	L / Actual	Ratio
Critical Span	D+L	L_L	0.0141	Spn 3	3-4-4	6-5-4	240	5461.1	0.0439

Maximum Deflection on Span and Cantilever for Live Load Only

Def. Span Desc.	Combination	Load Case	Max Deflec	tion Span II) Span-X	Span Analog Length	L / Allowable	L / Actual	Ratio
Critical Span	L	L_L	0.0105	Spn 3	3-3-12	6-5-4	480	7387.5	0.065