

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0421-2748 1504 Gregory Circle

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15710383 thru E15710418

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



May 10,2021

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



| Job | Truss | Truss Type | Qty | Ply | 1504 Gregory Circle | - |
|--------------------|----------------------|------------|-----------|------------|--|-----------|
| | | | | | | E15710383 |
| J0421-2748 | A1-GE | GABLE | 1 | 1 | | |
| | | | | | Job Reference (optional) | |
| Comtech, Inc, Faye | teville, NC - 28314, | | 8 | .330 s Oct | 7 2020 MiTek Industries, Inc. Mon May 10 10:27:06 2021 | Page 2 |
| | | ID:DFoF | RKbNIZjVH | l?nbtIrBKZ | azEemi-VGp36oqrr?m8AemvZZrABXuY5PInfsWMGQJYou | JzHwwp |

NOTES-

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable studs spaced at 2-0-0 oc.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 52, 57, 53, 45, 39, 50, 56, 55, 54, 51, 49, 41, 40, 38 except (jt=lb) 2=139, 36=386, 58=102, 46=2257, 42=1801, 37=666.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) This truss has large uplift reaction(s) from gravity load case(s). Proper connection is required to secure truss against upward movement at the bearings. Building designer must provide for uplift reactions indicated.
- 13) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Scale = 1:99.0



| 4 | 11-8 11-7-8 18-7-8 | 25-1-4 | 4 | 2-1-0 | 50-11-0 | 54-11-0 | | |
|--|--|--|---|---|----------------------------------|--|--|--|
| Plate Offsets (X,Y) | [2:0-2-11,Edge], [17:0-5-0,0-7-4], [26:0-2 | <u>6-5-12</u> 2-8,0-3-0], [27:0-3-8,0-3-0 |)] | -11-12 | 8-10-0 | 4-0-0 | | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014 | CSI. TC 0.40 BC 0.78 WB 0.79 Matrix-S | DEFL. ir Vert(LL) -0.37 Vert(CT) -0.63 Horz(CT) 0.09 Wind(LL) 0.10 | n (loc) l/defl L/d 17-19 >953 360 17-19 >561 240 15 n/a n/a 17 >999 240 | PLATES MT20 Weight: 541 lb | GRIP 244/190 FT = 20% | | |
| LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x10 S 2-22: 2 WEBS 2x4 SF 8-19,1 REACTIONS. (siz Max H | P No.1 P 2400F 2.0E *Except* 2x6 SP No.1, 20-22: 2x10 SP No.1 P No.2 *Except* 1-17,14-15,11-25,25-27: 2x6 SP No.1 e) 2=0-3-8, 19=0-3-8, 15=0-3-8 lorz 2=199(LC 12) | | BRACING- TOP CHORDStructural wood sheathing directly applied or 3-6-5 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 7-10.BOT CHORDRigid ceiling directly applied or 9-10-14 oc bracing.WEBS1 Row at midpt 2 Rows at 1/3 ptsJOINTS1 Brace at Jt(s): 26, 27 | | | | | |
| $\begin{array}{llllllllllllllllllllllllllllllllllll$ | | | | | | | | |
| NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-16; \ MWFRS (envelope) 29-0-11 to 38-9-8, E reactions shown; Lu 3) Provide adequate d 4) This truss has been will fit between the b 6) Ceiling dead load (1 7) Bottom chord live lo 8) This truss is designer referenced standard 9) Graphical purlin rep 10) Attic room checked | e loads have been considered for this des /ult=130mph (3-second gust) Vasd=103m and C-C Exterior(2E) -0-8-11 to 4-11-8, Exterior(2R) 38-9-8 to 44-3-6, Interior(1) 4 umber DOL=1.60 plate grip DOL=1.60 rainage to prevent water ponding. designed for a 10.0 psf bottom chord live en designed for a live load of 30.0psf on th bottom chord and any other members, wit 10.0 psf) on member(s). 26-27, 11-26; Wa add (40.0 psf) and additional bottom chorc ed in accordance with the 2018 Internatio d ANSI/TPI 1. resentation does not depict the size or the d for L/360 deflection. | sign. nph; TCDL=6.0psf; BCDL Interior(1) 4-11-8 to 23-6 4-3-6 to 54-8-4 zone;C-C e load nonconcurrent with ne bottom chord in all are th BCDL = 10.0psf. all dead load (5.0psf) on d dead load (10.0 psf) app nal Residential Code sec e orientation of the purlin | =6.0psf; h=15ft; Cat. II; 12, Exterior(2R) 23-6-12 for members and force: a any other live loads. as where a rectangle 3- member(s).19-27, 11-17 blied only to room. 17-19 titons R502.11.1 and R8 along the top and/or bor | Exp C; Enclosed; 2 to 29-0-11, Interior(1) s & MWFRS for 6-0 tall by 2-0-0 wide 0 02.10.2 and ttom chord. | SEA 166 MARINE A ST | EEP. Contraction of the second | | |



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



| | 6-7-8 | 11-7-8 | 16-7-8 | 22-11-8 | 23 ₁ 1-4 | 40-1-0 | 48 | 3-11-0 | 52-11-0 | 1 |
|------------|--------------|------------------------|--------------------|-----------------|----------------------|--|----|--------|---------|---|
| | 6-7-8 | 5-0-0 | 5-0-0 | 6-4-0 | 0-1 ["] -12 | 16-11-12 | 8 | -10-0 | 4-0-0 | 1 |
| Plate Offs | sets (X,Y) [| 7:0-5-8,0-2-0], [16:0- | 5-0,0-7-8], [18:0- | 5-8,0-2-5], [20 | 0:0-4-12,0-2- | 8], [23:0-5-0,0-3-0], [24:0-4-0,0-3-0] | | | | |
| | | | | - | | | | | | |

| LOADING (ps TCLL 20. TCDL 10. BCLL 0. BCDL 10. | sf) .0 .0 .0 * .0 | SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TP | 2-0-0 1.15 1.15 YES I2014 | CSI. TC 0.86 BC 0.73 WB 0.82 Matrix-S | DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL) | in -0.60 -1.05 0.35 0.14 | (loc) 16-18 16-18 14 16-18 | l/defl >599 >339 n/a >999 | L/d 360 240 n/a 240 | PLATES MT20 M18SHS Weight: 508 lb | GRIP 244/190 244/190 FT = 20% |
|--|---------------------------------------|--|---|---|---|--------------------------------------|--|---|--|---|---|
| LUMBER- TOP CHORD BOT CHORD WEBS | 2x6 SP 2x6 SP 17-18,1 2x4 SP | No.1 2400F 2.0E *Except* 4-17: 2x10 SP 2400F 2.0 No.2 *Except* | Е | | BRACING- TOP CHOR BOT CHOR | D | Structur except Rigid ce 7-3-12 | ral wood s end vertic eiling dire oc bracing | sheathing di cals, and 2-0 ctly applied g: 16-18. | rectly applied or 3-3-4 o 0-0 oc purlins (10-0-0 ma or 10-0-0 oc bracing, E | oc purlins, ax.): 6-9. Except: |
| REACTIONS. | 7-18,10 (size Max Ho Max Gr | -16,13-14,10-22,5-24,22-) 1=0-3-8, 18=0-3-8, 14 prz 1=195(LC 12) rav 1=1139(LC 2), 18=26 | 24: 2x6 SP No.1 I=0-3-8 05(LC 28), 14=1952(| (LC 29) | JOINTS | | 1 Row a 2 Rows 1 Brace | at midpt at 1/3 pts at Jt(s): : | s 2 23, 24 | 18-24, 9-23, 3-19, 5-24 23-24, 10-23 | |

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-4701/277, 2-3=-4234/35, 3-5=-1677/3, 5-6=-123/3605, 6-7=-116/4385, 7-8=0/1429,

- 8-9=0/1429, 9-10=-344/290, 10-12=-2593/0, 12-13=-1649/42, 13-14=-1788/46

 BOT CHORD
 1-21=-284/4291, 20-21=-285/4306, 19-20=0/3953, 18-19=0/1519, 16-18=0/2211, 15-16=-10/1470

 WEBS
 18-24=-2759/295, 7-24=-1547/273, 3-20=0/2719, 5-19=-5/934, 5-18=0/1126,
- 10-16=0/1004, 12-16=0/969, 12-15=-1247/0, 13-15=-20/1769, 23-24=-6546/174, 10-23=-2207/0, 8-23=-365/218, 7-23=-303/3361, 9-23=-1304/29, 3-19=-2889/148, 2-20=-608/336, 5-24=-5169/115, 6-24=-1857/115

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-1-12 to 5-5-4, Interior(1) 5-5-4 to 21-6-2, Exterior(2R) 21-6-2 to 28-11-15, Interior(1) 28-11-15 to 36-9-8, Exterior(2R) 36-9-8 to 44-3-5, Interior(1) 44-3-5 to 52-8-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Ceiling dead load (10.0 psf) on member(s). 23-24, 10-23; Wall dead load (5.0psf) on member(s).18-24, 10-16
- 8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 16-18
 9) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.12) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See **MSIVTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601











| | 6-7-8 | 11-7-8 | 1 | 16-7-8 | 1 | 22-11-8 | 1-4 _ר 23 | | 40- | 1-0 | | | 1 | 48-11-0 | 52-11-0 | |
|--|--|---|---------------------------|--|----------|---|-----------------------------|-----------|---|--------------------------------------|---|--|--|--|---|----|
| | 6-7-8 | 5-0-0 | 1 | 5-0-0 | 1 | 6-4-0 | 0-1 ["] -12 | | 16-1 | 1-12 | | | 1 | 8-10-0 | 4-0-0 | |
| Plate Offsets (| X,Y) | [1:0-2-6,0-1-10], [6 | :0-3-0, | 0-3-12], [| 16:0-5-0 |),0 -7-8] , [′ | 18:0-5-0,0- | -4-2], [2 | 0:0-2-8,0-3-1 | 2], [24:(| 0-4-0,0-3 | 3-4] | | | | |
| LOADING (ps TCLL 20. TCDL 10. BCLL 0. BCDL 10. | sf) .0 .0 .0 * .0 | SPACING- Plate Grip D Lumber DO Rep Stress Code IRC2 | OL L Incr 018/TP | 2-0-0 1.15 1.15 YES Pl2014 | | CSI. TC BC WB Matri: | 0.68 0.70 0.77 x-S | | DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL) | in -0.55 -0.91 0.29 0.10 | (loc) 16-18 16-18 15 20-21 | l/defl >568 >340 n/a >999 | L/d 360 240 n/a 240 | PLATES MT20 Weight: 508 | GRIP 244/190 3 lb FT = 20 | 1% |
| LUMBER- TOP CHORD BOT CHORD WEBS | 2x6 SP 2x6 SP 17-18,7 2x4 SP 7-18,10 | No.1 2400F 2.0E *Exce 14-17: 2x10 SP 24(No.2 *Except*)-16,13-14,10-22,5 | pt*)0F 2.0 -24,22- |)E •24: 2x6 \$ | SP No.1 | | | | BRACING- TOP CHOF BOT CHOF WEBS | RD RD | Structu except Rigid c 1 Row 2 Rows | ral wood end verti eiling dire at midpt s at 1/3 p | sheathing d cals, and 2- ectly applied ts | directly applied or 3- 0-0 oc purlins (6-0-0 1 or 6-0-0 oc bracing 18-24, 10-16, 10-23 23-24 | 5-12 oc purlins, max.): 6-9. , 3-19, 5-24 | |
| REACTIONS. | (size Max H | e) 1=0-3-8, 18=0 orz 1=195(LC 12) | -3-8, 1 | 5=0-3-8 | | | | | JOINTS | | 1 Brac | e at Jt(s): | 23, 24 | | | |

Max Grav 1=1014(LC 2), 18=2554(LC 28), 15=2123(LC 29)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD
 1-2=-4082/252, 2-3=-3532/10, 3-5=-1317/0, 5-6=-105/2669, 6-7=-110/3235, 7-8=-182/829, 8-9=-182/829, 9-10=-546/261, 10-12=-1866/0

 BOT CHORD
 1-21=-261/3733, 20-21=-262/3745, 19-20=0/3294, 18-19=0/1143, 16-18=0/1565

 WEBS
 18-24=-2283/291, 7-24=-1365/267, 3-20=0/2324, 5-19=-6/819, 5-18=0/667, 10-16=-168/610, 12-16=0/1865, 12-15=-2186/0, 23-24=-4764/135, 10-23=-1479/0,
 - 8-23=-374/218, 7-23=-300/2824, 9-23=-868/39, 3-19=-2511/152, 2-20=-647/340,
 - 5-24=-3744/73, 6-24=-1395/112

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-1-12 to 5-5-4, Interior(1) 5-5-4 to 21-6-2, Exterior(2R) 21-6-2 to 28-11-15, Interior(1) 28-11-15 to 36-9-8, Exterior(2R) 36-9-8 to 44-3-5, Interior(1) 44-3-5 to 52-8-4 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Ceiling dead load (10.0 psf) on member(s). 23-24, 10-23; Wall dead load (5.0psf) on member(s). 18-24, 10-16
- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 16-18
 8) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify
- considering at joint(s) in considers parametric grain value using ANS/TPT if angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 Attic room checked for L/360 deflection.

SEAL 16673

May 10,2021



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| Job | Truss | Truss Type | Qty | Ply | 1504 Gregory Circle | |
|-----------------------|--------------------|----------------|-----|------------|--|-----------|
| | | | | | | E15710388 |
| J0421-2748 | A6 | PIGGYBACK BASE | 1 | 2 | | |
| | | | | _ | Job Reference (optional) | |
| Comtech, Inc, Fayette | ville, NC - 28314, | | . 8 | .330 s Oct | 7 2020 MiTek Industries, Inc. Mon May 10 10:27:28 2021 | Page 2 |

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LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-6=-60, 6-9=-60, 9-10=-60, 10-11=-81, 1-17=-20, 15-17=-20, 10-14=-20 Concentrated Loads (lb) Vert: 10=-1571(F)

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| Job | Truss | Truss Type | | Qty | Ply | 1504 Gregory Circle | e | F15710389 |
|---|---|--|--|--|---|--------------------------------------|------------------------------|---------------------------|
| J0421-2748 | A7 | PIGGYBACK BASE | | 1 | 2 | Job Poference (optiv | | |
| Comtech, Inc, Fayette | ⊥ ville, NC - 28314, | | | 8 | .330 s Oct | 7 2020 MiTek Indus | tries, Inc. Mon May 10 1 | 0:27:33 2021 Page 1 |
| -0- 01 | 11-0 2-3-8 8-8-0 11-0 2-3-8 6-4-8 | <u>14-6-0</u> <u>16-9-8</u> <u>18-8-8</u> 5-10-0 <u>2-3-8</u> <u>1-11-0</u> | + 21-6-2 + 2-9-10 | 29-1-13 7-7-11 | IDUIDRZ | 36-9-8 7-7-11 | 40-0-8 41-7-0 3-3-0 1-6-8 | Jool I / WITFYZHWWO |
| | | | | | | | | Scalo - 1:00 8 |
| | | | 8x8 = | | 2× | (4 | 8x8 = | Scale = 1.90.6 |
| Ţ | | 6.00 12 | 8 | ~ ~ | | 9 | 10 | T |
| | | 3x6 7 | | | | | 11 | 2 - |
| | | 6x8 = 5 | x8 = 10 | | 2 | | | |
| 10 | 4x4 | = 5 | 2x4 | | 4x | 15 14 $^{(8)} = 4^{(8)} = -$ | 2x4 | 0 |
| 11-5-1 | 4 | THE I | | | | 4x0 — | | |
| | | | | | | | | 8-4-0 |
| | 2 | | | | | | | |
| 4 4 1 | | 20 <mark>6 18</mark> 6 | | | | | | |
| | 4x6 = 22 2 | 21 <u>~ ⊠</u> 2x4 18 17 | | | | | | |
| | 3x4 | 5x8 = | | | | | | |
| | | 3x4 3x4 | | | | | | |
| | | 3x4 | | | | | | |
| | 2-3-8 8-8-0 | 16-9-8 14-6-0 16-7-12 18-8-8 | 21-6-2 | 29-1-13 | | 36-9-8 | 40-0-8 41-7-0 | |
| Plate Offsets (X V) [5:0 | 2-3-8 6-4-8 | 5-10-0 2-1-12 "1-11-0 0-1-12 10:0-5-4 0-4-01 [11:0-8-12 0-0-0] | ' 2-9-10 ' | /-/-11 | | 7-7-11 | ' 3-3-0 '1-6-8' | |
| | SDACING | | DEEL | in | (10.0) | | | |
| TCLL 20.0 | Plate Grip DOL 1.15 | TC 0.59 | Vert(LL) | -0.08 | (100) | >999 360 | MT20 | 244/190 |
| TCDL 10.0 BCLL 0.0 * | Lumber DOL 1.15 Rep Stress Incr NC | BC 0.34 WB 0.21 | Vert(CT) Horz(CT | -0.16) 0.08 | 13-15 17 | >999 240 n/a n/a | | |
| BCDL 10.0 | Code IRC2018/TPI2014 | Matrix-S | Wind(LL |) 0.07 | 3-21 | >999 240 | Weight: 595 lb | FT = 20% |
| LUMBER- TOP CHORD 2x10 SP N | lo.1 *Except* | | BRACIN TOP CH | G- ORD | Structur | al wood sheathing d | directly applied or 6-0-0 | oc purlins, except |
| 8-10,1-5: 2 | 2x6 SP No.1 | | | | 2-0-0 oc | c purlins (6-0-0 max. | .): 8-10. | Eveent |
| WEBS 2x4 SP No | 0.1 0.2 | | BUICH | URD | 6-0-0 oc | c bracing: 17-19,6-19 | 9. | Except: |
| REACTIONS. (size) | 12=0-3-8, 2=0-3-8, 17=0-3-8 | 3 | | | | | mun | uun. |
| Max Horz Max Uplift | 2=311(LC 8) 12=-177(LC 4), 17=-210(LC | 8) | | | | | WHATH C | ARDI |
| Max Grav | 12=2415(LC 1), 2=626(LC 2 | 1), 17=1851(LC 1) | | | | | S. AZ | STAL. |
| FORCES. (lb) - Max. Cor TOP CHORD 2-3=-396 | mp./Max. Ten All forces 250 3/0_3-4=-785/0_6-7=-395/48 | 0 (lb) or less except when shown 7-8=-2018/144 8-9=-3309/308 | 9-10=-3309/308 | 3 | | | = ;e | |
| 10-11=-2 2 21- 10 | 2787/220, 11-12=-806/71 | 0- 194/642 17 10- 1921/227 6 | : 10_ 1/9//192 | , | | | SE/ | AL 🚦 |
| 7-16=-11 | 7/1737, 15-16=-112/1750, 13 | 3-15=-175/2772, 11-13=-180/275 | 52 170 | | | | z 166 | 73 |
| 10-15=-3 | 85, 4-19=-775/218, 8-16=0/3 840/785, 10-13=0/367 | 00, 8-15=-186/1632, 9-15=-459/ | 178, | | | | E Poi | 19 |
| NOTES- | | | | | | | NGIN | EEF |
| 2-ply truss to be connect Top chords connected a | ted together with 10d (0.131 as follows: 2x6 - 2 rows stage | 'x3") nails as follows: ered at 0-9-0 oc. 2x10 - 2 rows s | taggered at 0-9 | -0 oc. | | | 11, A. ST | RZY |
| Bottom chords connected | ed as follows: $2x6 - 2$ rows sta | aggered at 0-9-0 oc. | 55 | | | | 2000 | nun. |
| All loads are considered | equally applied to all plies, e | except if noted as front (F) or bac | k (B) face in the | LOAD C | ASE(S) s | section. Ply to | | |
| 3) Unbalanced roof live loa | ads have been considered for | this design. | s otherwise ind | caled. | | | | |
| Wind: ASCE 7-16; Vult= MWFRS (envelope); Lui | 130mph (3-second gust) Vas mber DOL=1.60 plate grip DO | sd=103mph; TCDL=6.0psf; BCD DL=1.60 | L=6.0pst; h=15t | t; Cat. II; | Exp C; Er | nclosed; | | |
| 5) Provide adequate draina6) This truss has been des | age to prevent water ponding signed for a 10.0 psf bottom c | hord live load nonconcurrent witl | n anv other live | loads. | | | | |
| 7) * This truss has been de will fit between the botto | esigned for a live load of 30.0 | psf on the bottom chord in all are | eas where a rec | tangle 3-6 | 6-0 tall by | 2-0-0 wide | | |
| 8) Bearing at joint(s) 12 co | nsiders parallel to grain value | e using ANSI/TPI 1 angle to grain | n formula. Build | ing desig | ner shoul | d verify | | |
| 9) Provide mechanical con | nection (by others) of truss to | bearing plate capable of withst | anding 100 lb u | olift at joir | it(s) exce | pt (jt=lb) | | |
| 12=177, 17=210. 10) This truss is designed | in accordance with the 2018 | International Residential Code s | ections R502.1 | .1 and R | 802.10.2 | and | | |
| referenced standard A 11) See Standard Industry | NSI/TPI 1. Piggyback Truss Connectior | Detail for Connection to base tr | uss as applicat | le. or cor | sult quali | fied building | N.4- | |
| designer. | sentation does not depict the | size or the orientation of the pur | in along the top | and/or b | ottom cho | ord. | | iy 10,2021 |
| WARNING - Verify design | parameters and READ NOTES ON TH | IIS AND INCLUDED MITEK REFERENCE | PAGE MII-7473 rev | 5/19/2020 I | BEFORE US | E. | ENGINEE | RING BY |
| Design valid for use only with a truss system. Before use, the building design Bracing indi | n MITER® connectors. This design is I he building designer must verify the a icated is to prevent buckling of individ | based only upon parameters shown, and applicability of design parameters and pro- lual truss web and/or chord members only | is for an individual to perly incorporate the Additional temporate | uilding com s design int ary and per | ponent, not o the overal manent bra | ll cina | | :NCO |
| is always required for stability fabrication, storage, delivery, | y and to prevent collapse with possib erection and bracing of trusses and | le personal injury and property damage. truss systems, see ANSI/TPI1 (| For general guidance Quality Criteria, DS | e regarding B-89 and B | the CSI Buildir | g Component | 818 Soundside | A MiTek Affiliate Road |
| sarery information available | ie nom muss Plate institute, 2670 Cr | am migriway, outte ∠03 waldoff, MD 2060 | 11 | | | | Edenton NC 2 | 7932 |

| Job | Truss | Truss Type | Qty | Ply | 1504 Gregory Circle | |
|--------------------|----------------------|----------------|-----------|------------|--|---------------|
| | | | | | | E15710389 |
| J0421-2748 | A7 | PIGGYBACK BASE | 1 | 2 | | |
| | | | | | Job Reference (optional) | |
| Comtech, Inc, Faye | teville, NC - 28314, | | 8 | .330 s Oct | 7 2020 MiTek Industries, Inc. Mon May 10 10:27:3 | 3 2021 Page 2 |
| | | ID:DFo | RKbNIZiVH | ?nbtlrBKZ | azEemi-CTxHn18nUIAJvoK5ekgWnEWBiI16U86LT7 | 7MTFazHwwO |

NOTES-

13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1565 lb down and 133 lb up at 40-0-7 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-8=-60, 8-10=-60, 10-11=-60, 11-12=-81, 2-22=-20, 3-20=-20, 17-18=-20, 7-11=-20

Concentrated Loads (lb)

Vert: 11=-1565(B)

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| Job | Truss | Truss Type | Qty | Ply | 1504 Gregory Circle | F45740000 |
|--|--|--|---|-------------|---|--|
| J0421-2748 | A8 | PIGGYBACK BASE | 2 | 1 | | E15710390 |
| Comtech, Inc, Fayett | eville, NC - 28314, | | 8 | .330 s Oct | Job Reference (optional) t 7 2020 MiTek Industries, Inc. Mon I | May 10 10:27:38 2021 Page 1 |
| -0-1 | 1-0 2-3-8 8-8-0 | 14-6-0 16-9-8 18-8-8 2 | ID:DFoRKbNIZjVH' 21-6-2 29-1-13 | ?nbtIrBKZ | azEemi-ZQkAqkCwJroc?ZD2RHQhU | HD1SJiT9Oe4dP3Ew1zHwwJ |
| 0±1 | 1-0 2-3-8 ' 6-4-8 ' | 5-10-0 ' 2-3-8 '1-11-0' 2 | -9-10 ' 7-7-11 | | 7-7-11 ' 3-3-0 ' | 1-6-8 ' |
| | | | 8x8 = | 2x4 | 4 8x8 = | Scale = 1:89.7 |
| | | 6.00 12 | 8 | _25 9 | 26 10 | |
| Ī | | 3x6 7 | | | | 11 |
| | | 24 6 | | | | |
| | | 6x8 = 5x12 = | 16 | 1: | 5 14 13 5x12 | |
| ې بې | 4x4 🖘 | 5 | 2x4 | 4x8 | $3 = 4x8 = 2x4 \parallel$ | ပုံ |
| Ę | 4 | | | | | 11-8-11-11-8 |
| | 23 | | | | | à ở |
| 1 | 2 3 9 | | | | | |
| 2 4 4 4 4 4 4 4 4 | 2 | | | | | |
| 0 | 4x6 = 22 2x4 | 18 17 | | | | |
| | 3x4 | 3x4 | | | | |
| | | 3x4 | | | | |
| | | 3x6 | | | | |
| | 2-3-8 8-8-0 | 16-9-8 <u>14-6-0 16-7-12 18-8-8 2</u> 5-10-0 2-1-12 1-11-0 2 | 21-6-2 <u>29-1-13</u> 2-9-10 7-7-11 | | <u>36-9-8</u> 7-7-11 <u>40-0-8</u> 3-3-0 | 41-7-0 1-6-8 |
| Plate Offsets (X,Y) [3 | :0-4-12,0-2-1], [5:0-4-0,Edge], | <u>0-1-12</u> [7:0-7-12,0-0-0], [8:0-5-4,0-4-0], [1 | 0:0-5-4,0-4-0], [11:0-7 | -0.0-0-0] | | |
| | SPACING- 2-0-0 | | DEEL ir | | l/defl l/d PIAT | ES GRIP |
| TCLL 20.0 | Plate Grip DOL 1.15 | TC 0.65 | Vert(LL) -0.12 | 3-21 | >999 360 MT20 | 244/190 |
| BCLL 10.0 * | Rep Stress Incr YES | BC 0.45 WB 0.32 | Vert(CT) -0.25 Horz(CT) 0.17 | 3-21 17 | >783 240 n/a n/a | |
| BCDL 10.0 | Code IRC2018/TPI2014 | Matrix-S | Wind(LL) 0.14 | 3-21 | >999 240 Weigh | nt: 298 lb FT = 20% |
| LUMBER- TOP CHORD 2x10 SP | No 1 *Excent* | | BRACING- | Structur | ral wood sheathing directly applied | or 6-0-0 oc purlins, except |
| 8-10,1-5: | 2x6 SP No.1 | | | 2-0-0 00 | c purlins (4-4-4 max.): 8-10. | |
| WEBS 2x4 SP N | lo.1 lo.2 | | BOI CHORD | 6-0-0 oc | c bracing: 17-19,6-19. | bracing, Except: |
| WEDGE Left: 2x4 SP No.3 | | | WEBS | 1 Row a | at midpt 4-19 | |
| REACTIONS. (size) | 12=0-3-8 2=0-3-8 17=0-3-8 | 3 | | | | |
| Max Hor | z 2=311(LC 12) | 2) | | | | |
| Max Opi | v 12=943(LC 1), 2=633(LC 25 | i), 17=1751(LC 1) | | | | |
| FORCES. (Ib) - Max. Co | omp./Max. Ten All forces 25 | 0 (lb) or less except when shown. | | | | |
| TOP CHORD 2-3=-39 9-10=-2 | 96/0, 3-4=-807/24, 6-7=-362/10 2808/594 10-11=-2054/435 1 | 00, 7-8=-1814/336, 8-9=-2808/594, 1-12=-312/82 | | | | |
| BOT CHORD 3-21=-2 | 213/706, 20-21=-213/707, 19-2 | 0=-195/666, 17-19=-1721/480, 6-1 | 19=-1374/414, | | | |
| WEBS 4-21=0 | /384, 4-19=-795/246, 8-16=0/2 | 93, 8-15=-296/1298, 9-15=-469/22 | 24, | | | |
| 10-15= | -190/1023, 10-13=0/313 | | | | | |
| NOTES- 1) Unbalanced roof live lo | oads have been considered for | this desian. | | | | |
| 2) Wind: ASCE 7-16; Vul | t=130mph (3-second gust) Vas | sd=103mph; TCDL=6.0psf; BCDL= | 6.0psf; h=15ft; Cat. II; | Exp C; E | nclosed; | N CAP |
| 27-8-13 to 36-9-8, Ext | erior(2E) 36-9-8 to 41-5-4 zone | e;C-C for members and forces & M | WFRS for reactions sh | own; Lun | nber | ······································ |
| 3) Provide adequate drai | OL=1.60 nage to prevent water ponding | | | | | CONTRACT OF THE OWNER |
| 4) This truss has been de 5) * This truss has been de | esigned for a 10.0 psf bottom of a live load of 30 0 | hord live load nonconcurrent with a psf on the bottom chord in all area | any other live loads. s where a rectangle 3- | 6-0 tall by | / 2-0-0 wide | |
| will fit between the bot | tom chord and any other mem | bers. | ormula Building dosis | | ld vorify | SEAL |
| capacity of bearing su | face. | | ormula. Duilding desig | | 3 | 100/3 |
| Provide mechanical co 17=201. | onnection (by others) of truss to | bearing plate capable of withstan | ding 100 lb uplift at joir | it(s) 12 ex | xcept (jt=lb) | a:2: |
| This truss is designed referenced standard A | in accordance with the 2018 Ir NSI/TPI 1. | nternational Residential Code secti | ons R502.11.1 and R8 | 02.10.2 a | ind in the | VGINEE 10 |
| 9) See Standard Industry | Piggyback Truss Connection | Detail for Connection to base truss | as applicable, or cons | ult qualifi | ed building | 9. STRL |
| 10) Graphical purlin repre | esentation does not depict the | size or the orientation of the purlin | along the top and/or b | ottom cho | ord. | May 10.2021 |
| | | | | | | · , -, |

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818 Soundside Road Edenton, NC 27932



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| | -0-1170 |) | 17-0-0 | | 13-1- | | | | -11-0 | 0 | | m2-0-0 |
|-------------|----------|-----------------------------|-----------------|----------------|---------------|--------------------|-------|-------|--------|-----|----------------|----------|
| | 0-11-0 | | 16-9-8 | | 1-11- | 0' | | | 21-4-0 |) | | 1-6-8 |
| Plate Offse | ts (X,Y) | [7:0-1-15,0-0-0], [8:0-3-4, | Edge], [8:0-0-0 |),0-2-12], [14 | :0-4-0,0-3-8] | , [22:0-4-0,0-3-8] | | | | | | |
| LOADING | (psf) | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
| TCLL | 20.0 | Plate Grip DOL | 1.15 | TC | 0.03 | Vert(LL) | 0.00 | 1 | n/r | 120 | MT20 | 244/190 |
| TCDL | 10.0 | Lumber DOL | 1.15 | BC | 0.03 | Vert(CT) | 0.00 | 1 | n/r | 120 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.09 | Horz(CT) | -0.01 | 12 | n/a | n/a | | |
| BCDL | 10.0 | Code IRC2018/TF | 912014 | Matrix | (-S | | | | | | Weight: 306 lb | FT = 20% |

| LUMBER- | | BRACING- | |
|-----------|------------------------|-----------|--|
| TOP CHORD | 2x10 SP No.1 *Except* | TOP CHORD | Structural wood sheathing directly applied or 6-0-0 oc purlins, except |
| | 14-22,1-8: 2x6 SP No.1 | | 2-0-0 oc purlins (6-0-0 max.): 14-22. |
| BOT CHORD | 2x6 SP No.1 | BOT CHORD | Rigid ceiling directly applied or 10-0-0 oc bracing, Except: |
| OTHERS | 2x4 SP No.2 | | 6-0-0 oc bracing: 11-35. |

REACTIONS. All bearings 41-7-0.

(lb) - Max Horz 2=454(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 35, 25, 26, 27, 29, 30, 31, 32, 33, 34, 36, 37, 38, 40, 41, 42 except 12=-108(LC 12), 23=-103(LC 13), 43=-123(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 24, 35, 12, 2, 25, 26, 27, 29, 30, 31, 32, 33, 34, 36, 37, 38, 40, 41, 42, 43 except 23=260(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-481/174, 3-4=-396/135, 4-5=-345/117, 5-6=-293/97

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-9-2 to 3-7-11, Exterior(2N) 3-7-11 to 21-6-2 , Corner(3R) 21-6-2 to 25-10-15, Exterior(2N) 25-10-15 to 36-9-8, Corner(3E) 36-9-8 to 41-5-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Provide adequate drainage to prevent water ponding.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 35, 25, 26, 27, 29, 30, 31, 32, 33, 34, 36, 37, 38, 40, 41, 42 except (jt=lb) 12=108, 23=103, 43=123.

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



May 10,2021

TRENGINEERING BY A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932

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| Job | Truss | Truss Type | Qty | Ply | 1504 Gregory Circle | E16710202 |
|--|--|--|-----------------------------------|-------------------------|--|---|
| J0421-2748 | A11 | PIGGYBACK BASE | 2 | 1 | | E10710393 |
| Comtech, Inc, Fayette | / ville, NC - 28314, | | 8 | .330 s Oct | T 7 2020 MiTek Industries, Inc. | Mon May 10 10:26:56 2021 Page 1 |
| -0 <u>-1</u> | 1-0 2-3-8 8-8-0 | 14-6-0 16-9-8 18-8-8 2 | ID:DFoRKbNIZj\ 21-6-2 29-1 | /H?nbtIrBK | <pre>KZazEemi-oLCH0NiZCxVZz6?_; 36-9-8 36-9-9-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-9-8 36-9-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-8 36-9-9-8 36</pre> | zTgqnQUrzNNbJNCuytu0RTzHwwz |
| 0-11 | -0 2-3-8 ' 6-4-8 | ' 5-10-0 ' 2-3-8 '1-11-0' : | 2-9-10 ' 7-7-1 | 1 | ' /-/-11 ' | 3-0-0 ' |
| | | | 8x8 = | 2 | 2x4 5x8 | Scale = 1:88.4 |
| _ | | 6.00 12 | 8 | ⊠25 | 9 26 10 | 11 |
| | | 3x6 7 | | _ | | 12 m |
| | | 24 6 | | | | |
| | | 6x8 = 5x8 = | = ₁₆ | | 15 ₁₄ 13 | 4x6 |
| 5-5 | 4x4 | = 5 | 2x4 | 4 | 4x8 = 4x8 = 2x4 | 2 2 |
| 1 | 4 | | | | | 1-8 |
| | 23 | | | | | č |
| | 2 3 | | | | | |
| 1 4 1 4 1 4 1 4 | | 200 180 | | | | |
| | k=22 2 | x4 18 17 | | | | |
| | 3x4 | 5x8 = | | | | |
| | | 3x4 3x4 | | | | |
| | | 3x6 | | | | |
| | 2-3-8 8-8-0 | 16-9-8 14-6-0 16-7-12 118-8-8 | 21-6-2 <u>2</u> 9-1- ⁻ | 13 | 36-9-8 | 39-9-8 |
| | 2-3-8 6-4-8 | 5-10-0 2-1-12 1-11-0 0-1-12 | 2-9-10 7-7-1 | 1 | 7-7-11 | 3-0-0 |
| Plate Offsets (X,Y) [3:0 | I-4-12,0-2-1], [5:0-4-0,Edge], | [7:0-7-8,0-0-0], [8:0-5-4,0-4-0], [10:0 | 0-5-4,0-2-12] | | | |
| LOADING (psf) TCLL 20.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 | CSI. TC 0.66 | DEFL. ir Vert(LL) -0.13 | n (loc) 3-21 | I/defl L/d F >999 360 M | PLATES GRIP VIT20 244/190 |
| TCDL 10.0 | Lumber DOL 1.15 | BC 0.46 | Vert(CT) -0.25 | 3-21 | >778 240 | |
| BCDL 10.0 | Code IRC2018/TPI2014 | Matrix-S | Wind(LL) 0.15 | 5 3-21 | >999 240 V | Neight: 286 lb FT = 20% |
| LUMBER- | | | BRACING- | | | |
| TOP CHORD 2x6 SP No 5-8: 2x10 S | .1 *Except* SP No.1 | | TOP CHORD | Structur 2-0-0 oc | ral wood sheathing directly ap c purlins (4-7-4 max.): 8-10. | plied or 6-0-0 oc purlins, except |
| BOT CHORD 2x6 SP No | .1 | | BOT CHORD | Rigid ce | eiling directly applied or 10-0-0 | 0 oc bracing, Except: |
| WEDGE | .2 | | WEBS | 1 Row a | at midpt 4-19 | |
| Left: 2x4 SP No.3 SLIDER Right 2x4 \$ | SP No.2 -x 1-8-0 | | | | | |
| REACTIONS. (size) | 12=Mechanical, 2=0-3-8, 17 | =0-3-8 | | | | |
| Max Horz Max Holift | 2=321(LC 12) | 2) | | | | |
| Max Opint Max Grav | 12=875(LC 1), 2=638(LC 25 | i), 17=1679(LC 1) | | | | |
| FORCES. (Ib) - Max. Cor | np./Max. Ten All forces 250 |) (Ib) or less except when shown. | | | | |
| TOP CHORD 2-3=-402 10-12=-1 | /0, 3-4=-822/4, 6-7=-338/75, 484/317 | 7-8=-1672/311, 8-9=-2464/521, 9-1 | 0=-2464/521, | | | |
| BOT CHORD 3-21=-21 | 7/720, 20-21=-217/721, 19-2 | 0=-200/682, 17-19=-1649/475, 6-19 | 9=-1295/408, | | | |
| WEBS 4-21=0/3 | 2/1434, 15-16=-207/1447, 13 84, 4-19=-808/250, 8-16=0/2 | 89, 8-15=-209/1229, 12-13=-214/1218 89, 8-15=-247/1072, 9-15=-488/230 |), | | | |
| 10-15=-2 | 56/1312, 10-13=0/259 | | | | | |
| NOTES- | ids have been considered for | this design | | | | munn |
| 2) Wind: ASCE 7-16; Vult= | 130mph (3-second gust) Vas | sd=103mph; TCDL=6.0psf; BCDL=6 | .0psf; h=15ft; Cat. II; | Exp C; Er | nclosed; | TH CARO |
| 27-8-13 to 36-9-8, Exter | ior(2E) 36-9-8 to 39-9-8 zone | e;C-C for members and forces & MV | VFRS for reactions sh | 27-8-13, 1 nown; Lum | nber | N. W. W. |
| DOL=1.60 plate grip DC 3) Provide adequate draina | 0L=1.60 age to prevent water ponding | | | | 100 | S SIM |
| 4) This truss has been des | igned for a 10.0 psf bottom o | hord live load nonconcurrent with ar | ny other live loads. | | | SEAL |
| will fit between the botto | m chord and any other mem | bers. | where a rectangle 5- | 0-0 tali by | 72-0-0 wide | 16673 |
| 6) Refer to girder(s) for true7) Provide mechanical con | ss to truss connections. nection (by others) of truss to | bearing plate capable of withstand | ing 100 lb uplift at joi | nt(s) 12 ex | xcept (jt=lb) | X |
| 17=210. 8) This truss is designed in | accordance with the 2018 Ir | nternational Residential Code section | ns R502 11 1 and R8 | 02 10 2 a | and I P | NOWEER |
| referenced standard AN | SI/TPI 1. | | | | | NA OTOTILIS |
| designer. | riggyback Truss Connection | Detail for Connection to dase truss a | as applicable, or cons | suit qualifi | ea ballaing | 11 SINCE |
| 10) Graphical purlin repres | entation does not depict the | size or the orientation of the purlin a | long the top and/or b | ottom cho | ord. | May 10,2021 |
| A | | | | | | |
| Design valid for use only with | parameters and READ NOTES ON TH MiTek® connectors. This design is be building designer must verify the | his AND INCLUDED MITEK REFERENCE PAGe based only upon parameters shown, and is fo | r an individual building con | BEFORE US | | |
| building design. Bracing indi | cated is to prevent buckling of individ | lual truss web and/or chord members only. A | dditional temporary and pe | rmanent bra | | a i lilii liiU |

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Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

| Job | Truss | Truss Type | Qty | Ply | 1504 Gregory Circle | | F15710204 |
|--|---|---|--|----------------------------------|---|------------------------------------|----------------|
| J0421-2748 | A12 | PIGGYBACK BASE | 1 | 1 | Job Reference (ention | -1) | E13710394 |
| Comtech, Inc, Fayette | / /ille, NC - 28314, | | | 8.330 s Oct | 7 2020 MiTek Industrie | s, Inc. Mon May 10 10:26:59 20 |)21 Page 1 |
| -Q-11-0 | 8-8-0 | 16-9-8 18-8-8 | ID:DF0RKbNIZJ | VH?nbtIrBK2 29-1-13 7 7 11 | CazEemi-CwuQePkSUsu | 18qZkYfbDXP36SbbR_WknKfr/g | g2ozHwww |
| 0-11-0 | 0-0-0 | 0-1-0 | 2-3-10 | 7-7-11 | 7-7-11 | 3-0-0 | 0 |
| | | | 8x8 = | | 2x4 | 5x8 = | Scale = 1:79.0 |
| _ | | 6.00 12 | 7 | ⊠ ⊠ ² | 1 ⁸ 22 ⊠ ⊠ ⊠ ⊠ | 4x4 ≪ 9 10 | |
| | | 3x6 | | | | 11 | m |
| | | 5 | | | | | |
| | 6> | .8 = 20 5x12 | 2 = 15 | | ¹⁴ 13 | 12 4x6 | 5 |
| ى م | 4x4 📁 | 4 | 2x4 | | 4x8 = 4x8 = | 2x4 | 2 |
| 11 -5- | 3 | T | | | | | 8 11-5- |
| | | | | | | | 8-11- |
| | 19 | | | | | | |
| 2 | | | | | | | |
| 4-8-0 4-8-4 | g | | | | | | |
| ° 4x6 = | 18 | 17 16 | | | | | |
| | 2x4 | 6x6 = | | | | | |
| | | 18-8-8 | | | | | |
| | 8-8-0 8-8-0 | <u>16-7-12</u> <u>16-9-8</u> 7-11-12 0-1 | 21-6-2 | 29-1-13 7-7-11 | 36-9-8 7-7-11 | 39-9-8 | |
| Plate Offsets (X,Y) [4:0 | -4-0,Edge], [6:0-7-12,0-0-0], | <u>1-11-0</u> [7:0-5-4,0-4-0], [9:0-5-8,0-2-12] | | | | | |
| LOADING (psf) | SPACING- 2-0-0 | CSI. | DEFL. | in (loc) | l/defl L/d | PLATES GRIP | |
| TCLL 20.0 TCDI 10.0 | Plate Grip DOL 1.15 | TC 0.31 BC 0.28 | Vert(LL) -0. Vert(CT) -0 | 10 14 21 14-15 | >999 360 >999 240 | MT20 244/19 | 10 |
| BCLL 0.0 * | Rep Stress Incr YES | WB 0.33 Matrix-S | Horz(CT) 0. | 04 11 | n/a n/a | Weight: 280 lb FT - | 20% |
| | | | | | 2303 240 | | |
| TOP CHORD 2x6 SP No | .1 *Except* | | TOP CHORD | Structur | al wood sheathing dire | ectly applied or 6-0-0 oc purling | s, except |
| 4-7: 2x10 S BOT CHORD 2x6 SP No | SP No.1 .1 | | BOT CHORD | 2-0-0 oc Rigid ce | purlins (4-6-10 max.): iling directly applied or | 7-9. 10-0-0 oc bracing, Except: | |
| WEBS 2x4 SP No SLIDER Right 2x4 SP | .2 SP No 2 -x 1-8-0 | | WEBS | 6-0-0 oc | bracing: 5-16. | 16 | |
| | 11 Machanical 16 0 2 8 2 | 0.3.8 | 11200 | | а тара — О | | |
| Max Horz | 2=321(LC 12) | =0-3-8 | | | | | |
| Max Uplift Max Grav | 11=-61(LC 8), 16=-201(LC 1 11=886(LC 1), 16=1652(LC | 2) 1), 2=654(LC 25) | | | | | |
| FORCES (lb) - Max Cor | nn /Max Ten - All forces 25(|) (lb) or less except when shown | | | | | |
| TOP CHORD 2-3=-774 | /0, 5-6=-334/74, 6-7=-1755/3 | 46, 7-8=-2520/544, 8-9=-2520/54 | 14, | | | | |
| BOT CHORD 2-18=-17 | 04/325 8/586, 16-18=-178/586, 5-16 | =-1253/387, 6-15=-254/1534, 14 | -15=-249/1547, | | | | |
| 12-14=-2 WEBS 3-18=0/3 | 16/1246, 11-12=-221/1235 95, 7-15=0/294, 7-14=-228/1 | 051, 8-14=-487/230, 9-14=-273/1 | 352, 9-12=0/259, | | | | |
| 3-16=-68 | 4/210 | | | | | | |
| NOTES- | do have been considered for | this design | | | | | |
| 2) Wind: ASCE 7-16; Vult= | 130mph (3-second gust) Vas | d=103mph; TCDL=6.0psf; BCDL | =6.0psf; h=15ft; Cat. | II; Exp C; Er | nclosed; | | |
| MWFRS (envelope) and 27-8-13 to 36-9-8, Exter | C-C Exterior(2E) -0-9-2 to 3 ior(2E) 36-9-8 to 39-9-8 zone | -7-11, Interior(1) 3-7-11 to 21-6-2 ;;C-C for members and forces & I | , Exterior(2R) 21-6-2 MWFRS for reactions | to 27-8-13, I shown; Lum | nterior(1) ber | "TH CARO | 11, |
| DOL=1.60 plate grip DO | L=1.60 | | | | | NOR | there a |
| 4) This truss has been des 5) * This truss has been des | igned for a 10.0 psf bottom c | hord live load nonconcurrent with | any other live loads. | 0.0.0.4-11.6.4 | | | 19 1 |
| will fit between the botto | m chord and any other mem | pst on the bottom chord in all are pers. | as where a rectangle | 3-6-0 tall by | 2-0-0 wide | SEAL | N 8 |
| 6) Refer to girder(s) for trus7) Provide mechanical con | ss to truss connections. nection (by others) of truss to | bearing plate capable of withsta | nding 100 lb uplift at i | oint(s) 11 e> | cept (jt=lb) | 16673 | 1 1 |
| 16=201. 8) This truss is designed in | accordance with the 2018 Ir | ternational Residential Code sec | tions R502 11 1 and I | R802 10 2 a | nd | SP: | X |
| referenced standard AN | SI/TPI 1. | Dotail for Connection to been the | | | | PL: SNOWFER. | |
| designer. | | | | | -u bullaling | MA GINERAL | 1 III |
| 10) Graphical purlin repres | entation does not depict the | size or the orientation of the purli | n along the top and/or | r bottom cho | rd. | MA STRL | |
| | | | | | | May 10,20 |)21 |

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20-3-0 20-3-0

| Plate Offsets (X,Y) | [7:0-3-0,Edge], [17:0-4-0,0-4-8] | | | | | | | | | |
|--|---|--|--|----------------------------|--|---|--|--|------------------------------------|-----------|
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0 | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014 | CSI. TC 0.06 BC 0.03 WB 0.06 Matrix-S | DEFL. Vert(LL) Vert(CT) Horz(CT) | in 0.00 0.00 0.00 | (loc) 12 12 14 | l/defl n/r n/r n/a | L/d 120 120 n/a | PLATES MT20 Weight: 205 lb | GRIP 244/190 FT = 20% | |
| LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x4 SP 2-23,12 OTHERS 2x4 SP | P No.1 P No.1 P No.2 *Except* 2-14: 2x6 SP No.1 P No 2 | | BRACING- TOP CHORI BOT CHORI JOINTS | D D | Structu except Rigid co 1 Brace | ral wood end verti eiling dire at Jt(s): | sheathing dir cals. ectly applied o 24, 25, 26, 2 | ectly applied or 6-0-0 o or 10-0-0 oc bracing. 7, 30, 31 | oc purlins, | |
| REACTIONS. All be (Ib) - Max H Max U Max G | earings 20-3-0. lorz 23=-334(LC 10) plift All uplift 100 lb or less at joint(s) 2 17=-110(LC 13), 16=-113(LC 13), 1 irav All reactions 250 lb or less at joint 22=265(LC 19), 15=255(LC 20) | 3, 14 except 20=-120(LC 12 5=-278(LC 13) (s) 20, 21, 17, 16, 19, 18 ex | 2), 21=-108(LC 1: xcept 23=356(LC | 2), 22= 21), 14 | -283(LC | : 12), C 22), | | TH CA | ROUNT | |
| FORCES. (lb) - Max. TOP CHORD 2-23= BOT CHORD 22-23= 17-15 17-15 WEBS 2-22= NOTES- 17-16 | Comp./Max. Ten All forces 250 (lb) or =-324/55, 2-3=-302/127, 11-12=-299/109 3=-301/310, 21-22=-147/332, 20-21=-14 3=-150/336, 16-17=-145/329, 15-16=-14 =-148/328, 12-15=-145/326 | less except when shown. 9, 12-14=-322/43 7/332, 19-20=-150/336, 18- 5/329 | -19=-150/336, | | | | 1111ANS | A ST | LAS HERE | annin ann |
| Unbalanced roof live Wind: ASCE 7-16; V | e loads have been considered for this de /ult=130mph (3-second gust) Vasd=103 | sign. mph; TCDL=6.0psf; BCDL= | =6.0psf; h=15ft; C | at. II; E | xp C; E | nclosed; | | in the second | min | |

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-9-9 to 3-7-4, Exterior(2N) 3-7-4 to 10-1-8, Corner(3R) 10-1-8 to 14-6-5, Exterior(2N) 14-6-5 to 21-0-9 zone; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 23, 14 except (jt=lb) 20=120, 21=108, 22=283, 17=110, 16=113, 15=278.

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

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May 10,2021





2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-9-9 to 3-7-4, Interior(1) 3-7-4 to 10-1-8, Exterior(2R) 10-1-8 to 14-6-5, Interior(1) 14-6-5 to 19-4-12 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 8.
 6) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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| BCDL | 10.0 | Code IRC2018/TPI2014 | Matrix-S | 1012(01) 0.0 | | Weight: 128 lb FT = 20% |
|--|---|--------------------------------------|----------|--|---|--|
| LUMBEI TOP CH BOT CH WEBS OTHERS | R- ORD 2x6 SF ORD 2x6 SF 2x4 SF S 2x4 SF | 2 No.1 2 No.1 2 No.2 2 No.2 | | BRACING- TOP CHORD BOT CHORD WEBS | Structural wood sheathing di except end verticals. Rigid ceiling directly applied o 1 Row at midpt 1 | rectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing. 0-12 |

REACTIONS. All bearings 15-4-0.

(lb) - Max Horz 2=375(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 11, 12, 19, 18, 17, 16, 15, 14, 13 Max Grav All reactions 250 lb or less at joint(s) 11, 12, 2, 19, 18, 17, 16, 15, 14, 13

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-412/164, 3-4=-343/135, 4-5=-287/114

NOTES-

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-9-2 to 3-7-11, Interior(1) 3-7-11 to 15-4-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 12, 19, 18, 17, 16, 15, 14, 13.

10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



May 10,2021

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| TCLL TCDL BCLL BCDL | 20.0 10.0 0.0 * 10.0 | Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TF | 1.15 1.15 YES Pl2014 | TC BC WB Matri: | 0.20 0.20 0.22 x-S | H | Vert(LL) Vert(CT) Horz(CT) Wind(LL) | -0.02 -0.05 0.01 0.01 | 2-10 2-10 9 2-10 | >999 >999 n/a >999 | 360 240 n/a 240 | MT20 Weight: 124 lb | 244/190 FT = 20% |
|---------------------------------------|----------------------------------|--|-------------------------------|--------------------------|-----------------------------|---|--|--------------------------------|------------------------------|---------------------------------------|--|--|---------------------|
| LUMBER- TOP CHO BOT CHO WEBS | RD 2x6 SP RD 2x6 SP 2x4 SP | 2 No.1 2 No.1 2 No.2 | | | | E | BRACING- OP CHOR | D | Structu except Rigid c | ral wood end vertic eiling dire | sheathing dir cals. ctly applied o | rectly applied or 6-0-0 c or 10-0-0 oc bracing. | oc purlins, |
| SLIDER | Left 2x | 8 SP No.1 -x 4-7-14 | | | | V | VEBS | | 1 Row | at midpt | 6 | -9, 4-9 | |
| REACTIO | NS. (size | e) 9=Mechanical, 2=0-3 | -8 | | | | | | | | | | |

Max Horz 2=265(LC 12) Max Uplift 9=.143(LC 12) Max Grav 9=612(LC 1), 2=649(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-4=-719/40

BOT CHORD 2-10=-310/535, 9-10=-310/535

WEBS 4-10=0/349, 4-9=-653/379

NOTES-

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-9-2 to 3-7-11, Interior(1) 3-7-11 to 15-4-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=143.

6) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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May 10,2021

TRENGINEERING BY A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=133, 4=133.

6) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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Edenton, NC 27932



4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=129, 2=133.

6) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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A MiTek Affili 818 Soundside Road

Edenton, NC 27932



BRACING-TOP CHORD

BOT CHORD

| 11 | IM | RF | R- |
|----|----|----|----|

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 4=0-3-8

Max Horz 2=-97(LC 10)

Max Uplift 2=-28(LC 12), 4=-28(LC 13) Max Grav 2=369(LC 1), 4=369(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-308/122, 3-4=-308/122

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 3-11-8, Exterior(2R) 3-11-8 to 8-4-5, Interior(1) 8-4-5 to 8-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

6) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 6) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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TRENCO A MiTek Affiliat



Plate Offsets (X Y)-- [6:0-0-0 0-0-0] [7:0-0-0 0-0-0]

| 1 1010 0110010 (71,17 | | | | |
|---|---|---------------------------------------|---|--|
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES | CSI. TC 0.06 BC 0.04 WB 0.03 | DEFL. in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 8 n/a n/a | PLATES GRIP MT20 244/190 |
| BCDL 10.0 | Code IRC2018/TPI2014 | Matrix-S | | Weight: 60 lb FT = 20% |
| LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S | P No.1 | | BRACING- TOP CHORD Structural wood sheathing di BOT CHORD Binid ceiling directly applied | irectly applied or 6-0-0 oc purlins. or 10-0-0 oc bracing |

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 15-3-6. (lb) -Max Horz 1=74(LC 12)

2x4 SP No.2

Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 2, 8, 13, 14, 11, 10

Max Grav All reactions 250 lb or less at joint(s) 1, 9, 2, 8, 12, 13, 14, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

OTHERS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-3-15 to 4-8-11, Interior(1) 4-8-11 to 7-7-11, Exterior(2R) 7-7-11 to 12-0-8, Interior(1) 12-0-8 to 14-11-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 2, 8, 13, 14, 11, 10.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-3-15 to 4-8-11, Interior(1) 4-8-11 to 7-7-11, Exterior(2R) 7-7-11 to 12-0-8, Interior(1) 12-0-8 to 14-11-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This tasks has been designed for a too psi bottom inder the load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1. 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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¹⁾ Unbalanced roof live loads have been considered for this design.



NOTES-

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 8-1-2, Exterior(2R) 8-1-2 to 12-5-15, Interior(1) 12-5-15 to 15-9-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=147, 6=142.

7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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¹⁾ Unbalanced roof live loads have been considered for this design.



3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0ps for the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=128. 6=128.

7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=126, 6=126.

7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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Edenton, NC 27932



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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Max Horz 1=-58(LC 8) Max Uplift 1=-20(LC 13), 3=-25(LC 13)

Max Grav 1=134(LC 1), 3=134(LC 1), 4=195(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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Max Horz 1=34(LC 9) Max Uplift 1=-12(LC 13), 3=-15(LC 13)

Max Grav 1=78(LC 1), 3=78(LC 1), 4=114(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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| Plate Offsets (X,Y) | [1:0-2-2,0-1-12], [1:0-2-7,Edge], [2:0-1-7 | 7,0-1-12], [3:0-2-2,0-1-12] | | |
|--|---|---|---|--|
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014 | CSI. TC 0.01 BC 0.01 WB 0.00 Matrix-P | DEFL. in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 3 n/a n/a Weight: 5 lb FT = 20% | |
| LUMBER- | | | BRACING- | |

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD

REACTIONS. (size) 1=1-9-7, 3=1-9-7 Max Horz 1=10(LC 9) Max Uplift 1=-2(LC 12), 3=-2(LC 13) Max Grav 1=39(LC 1), 3=39(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 1-9-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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A MiTek Affilia 818 Soundside Road

Edenton, NC 27932



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

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