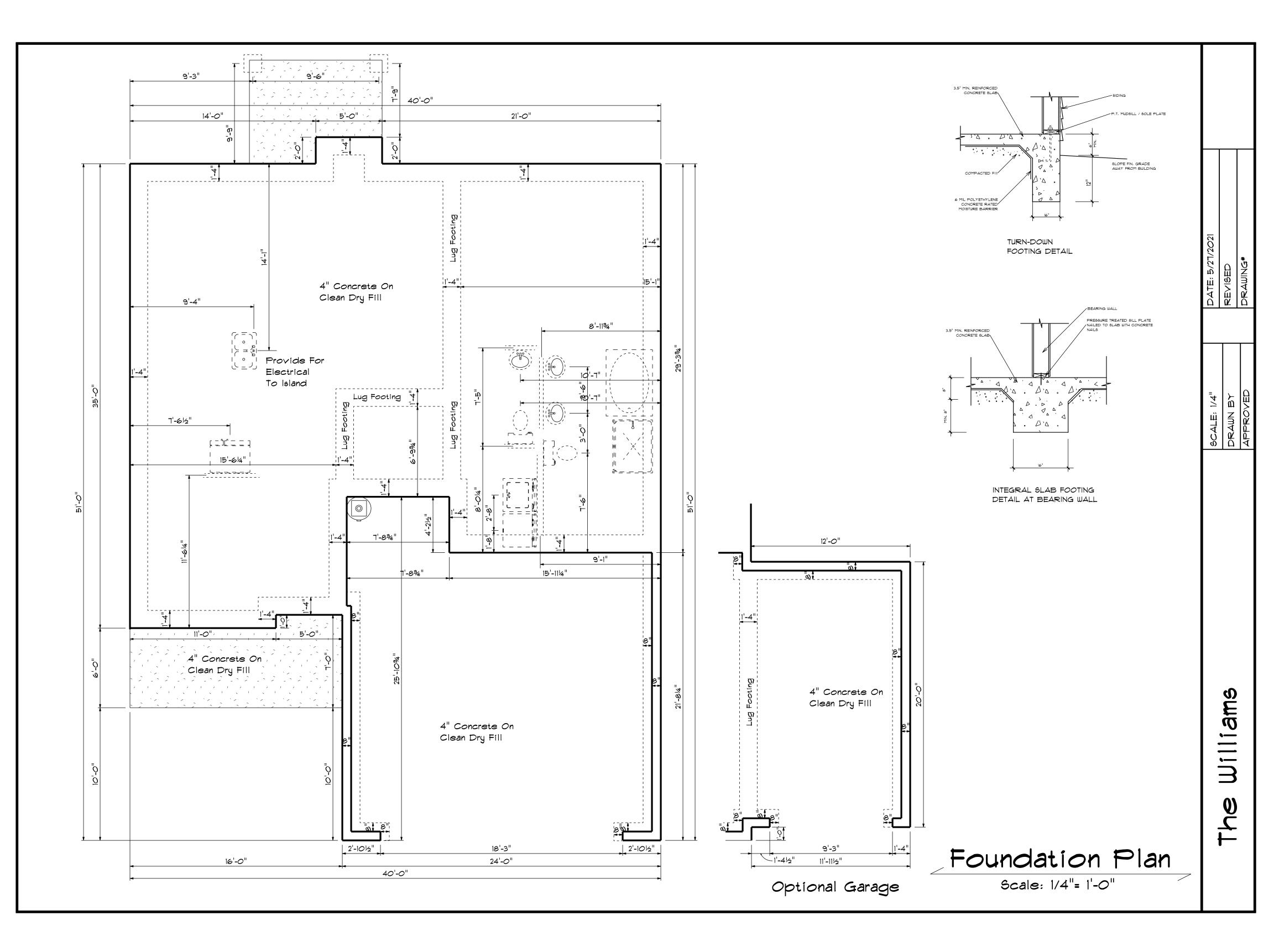


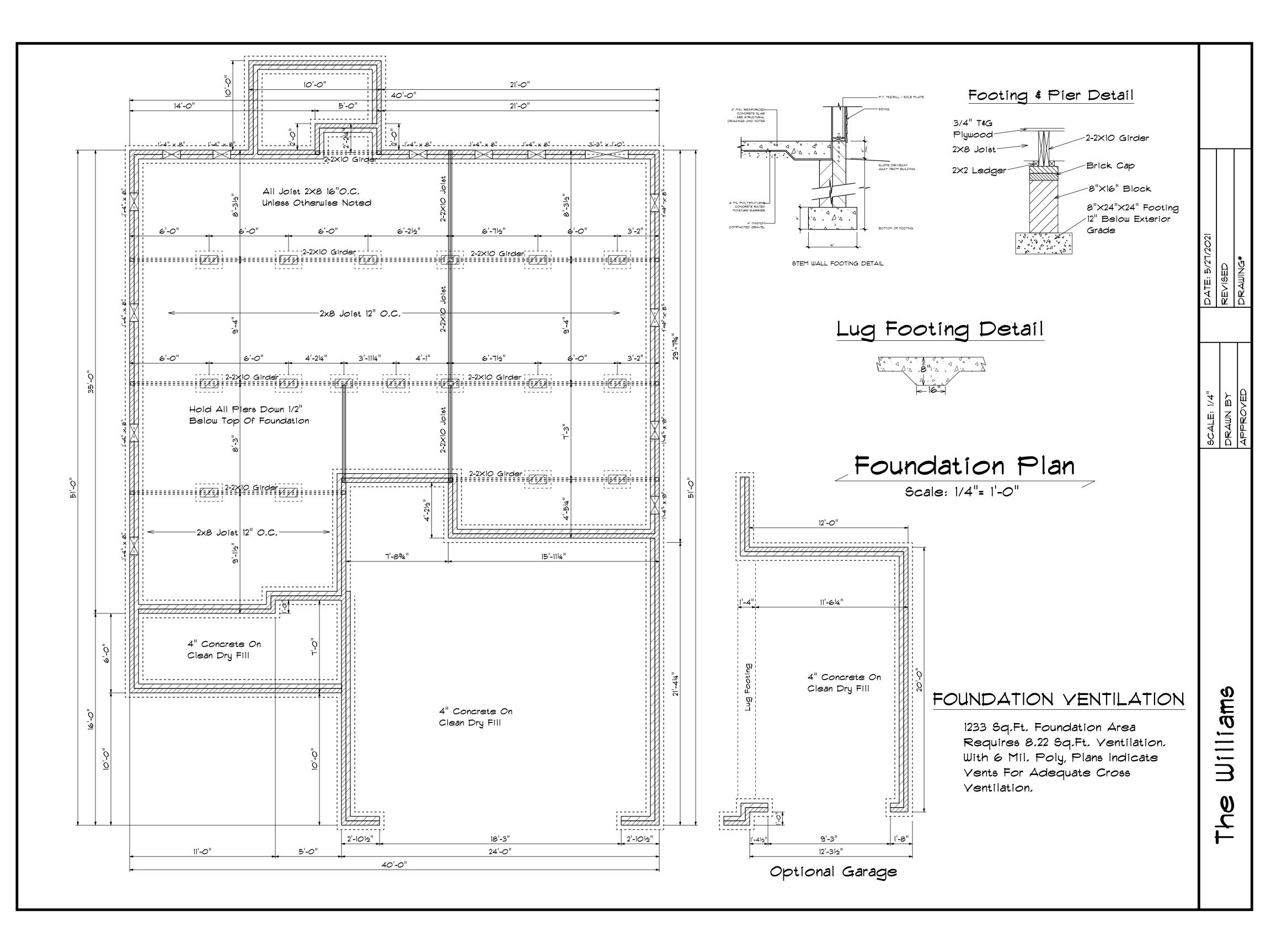
SECOND FLOOR OPENING SCHEDULE							
PRODUCT CODE	SIZE	HINGE	COUNT				
2-0 Door Unit	2'-0"	R	2				
2-4 Door Unit	2'-4"	R	2				
2-4 Door Unit	2'-4"	L	3				
2-6 Door Unit	2'-6"	R	2				
2-6 Door Unit	2'-6"	L	1				
4-0 Doublehung Door Unit	4'-0"	LR	2				
24X24 CASEMENT 1	2'-0" x 2'-0"	N	1				
28x52 single	2'-8" x 5'-2"	N	3				
28x52 twin	5'-4" x 5'-2"	NN	3				

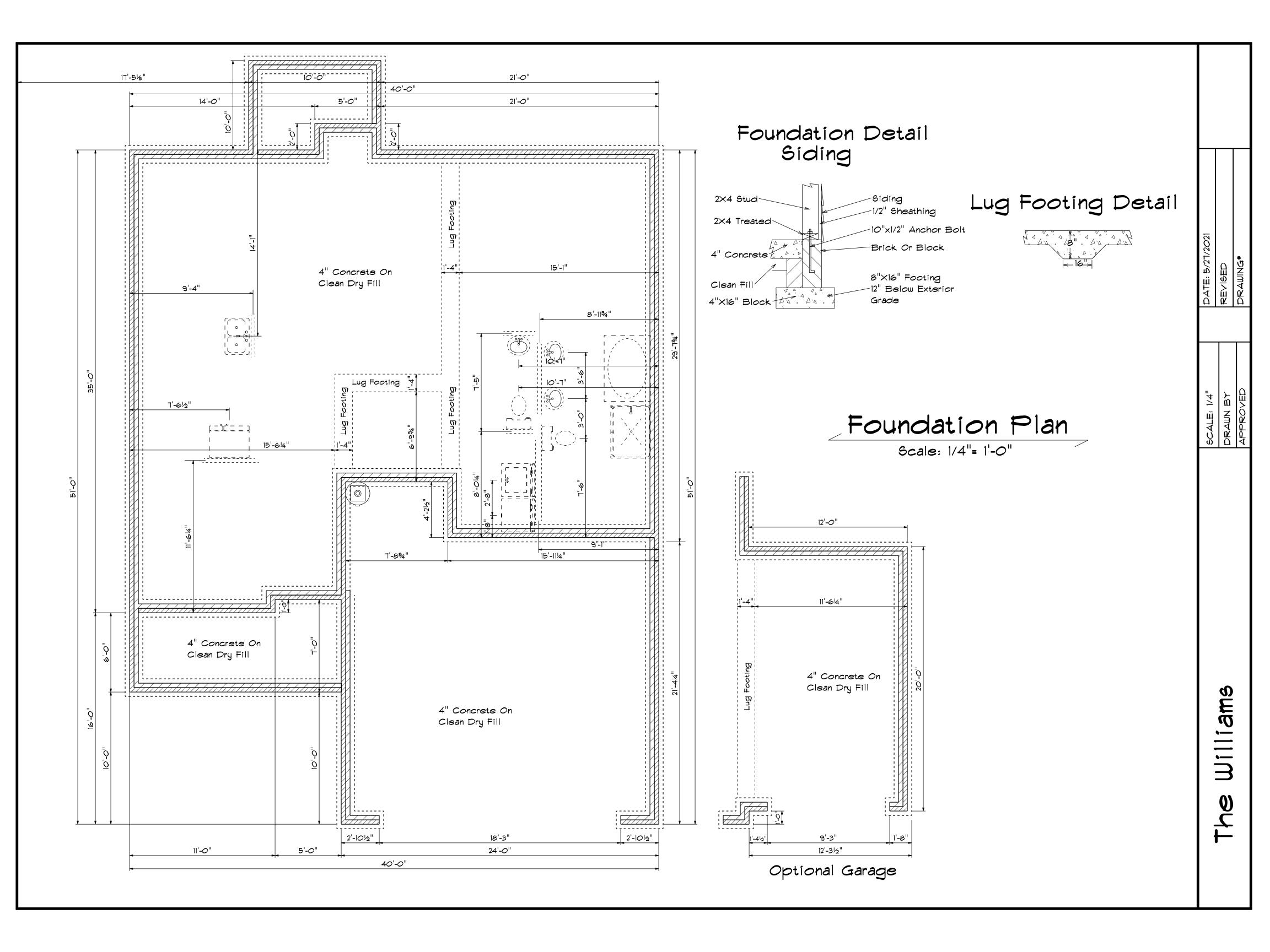
Second Floor Plan

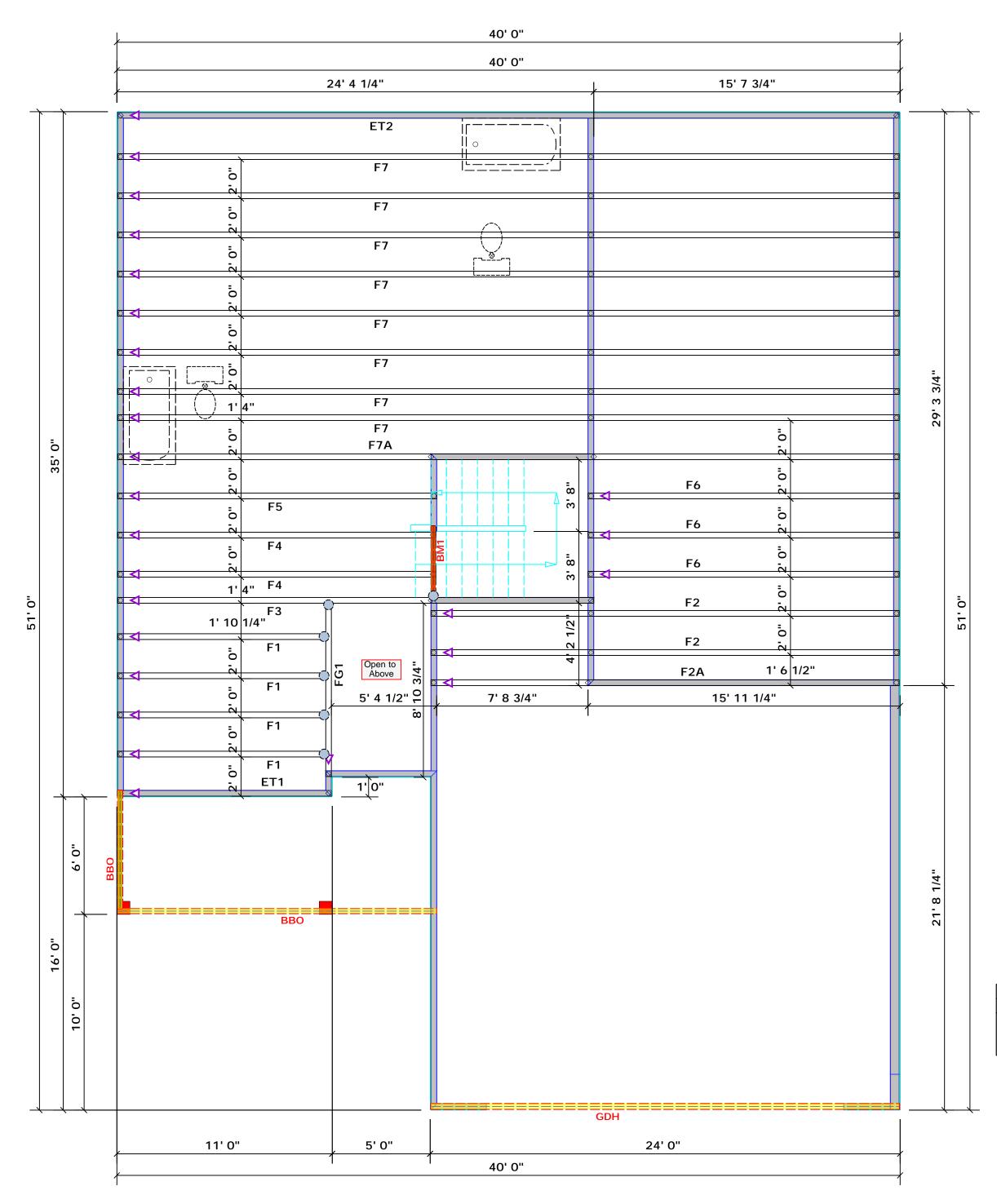
Scale: 1/4"= 1'-0"

The Williams









Dimension Notes 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
2. All interior wall dimensions are to face of frame wall unless noted otherwise All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes Plumbing drop locations shown are NOT exact.
 Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses. 3. Adjust spacing as needed not to exceed 24"oc.

	Conne	Nail Information				
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
\bigcirc	MSH422	USP	6	Varies	10d/3"	10d/3"

		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	4' 0"	2x10 SPF No.2	2	2
BM2	12' 0"	2x12 SPF No.2	2	4
GDH	24' 0"	1-3/4"x 14" LVL Kerto-S	2	2

Truss Placement Plan
Scale: 1/4"=1'

соттесн **ROOF & FLOOR TRUSSES & BEAMS**

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

David Landry

LOAD CHART FOR JACK STUDS

(8ASÉD ON TABLÉS ROCES(1) & (b)) NUMBER OF JACK STUBS REQUIRED 9 EA END OF

		 1	4EADER/	Ģī	RDER	 	-	
END REACHION (UP 10)	REQ'O STUDS FOR (2) PLY HEADER		END REACTION (UP TO)		REQ ID STUDS FOR (3) ALY HEADER	BND REACTION	60	REQUE STUDS FOR
1700	1		2550		1	340	0	1
3400	2		5100		2	680	0	2
5100	3		7650		3	1020	Ó	3
6800	4		10200	1	4	1360	0	4
8500	5		12750		5	1700	Ю	5
10200	6		15300	1	6			
11900	7							
13600	8							
15300	9							

COUNTY	Cumberland
ADDRESS	Tanna Place
MODEL	Floor
DATE REV . 01/07/21	01/07/21
DRAWN BY	DRAWN BY David Landry
SALESMAN	SALESMAN Marshall Naylor

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

PLAN

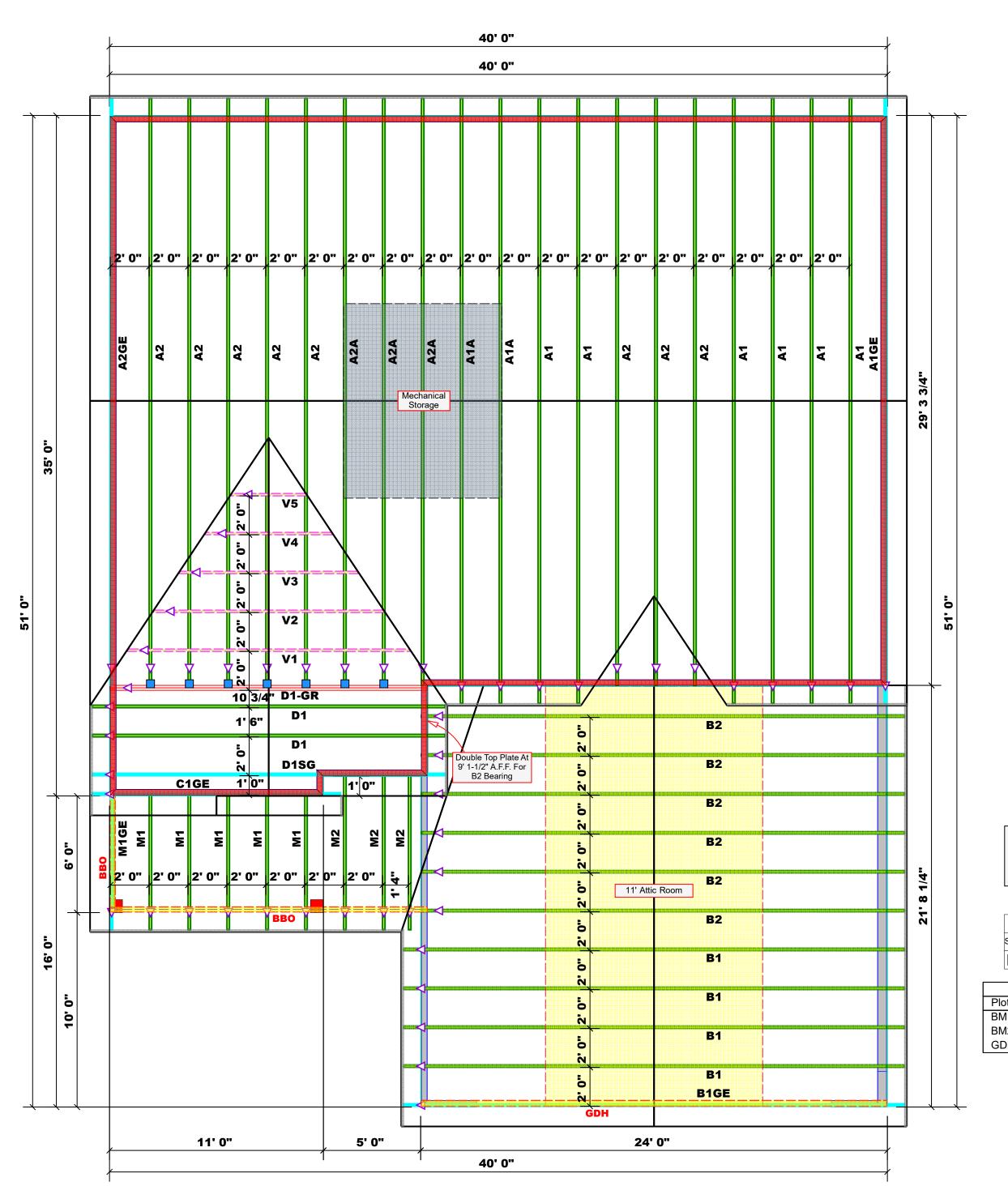
SEAL DATE

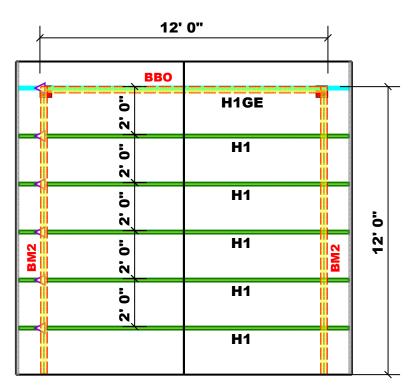
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QUOTE 7

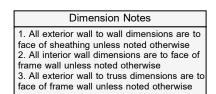
JOB NAME

BUILDER

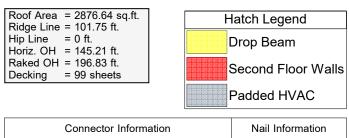




Optional Covered Porch



All Walls Shown Are Considered Load Bearing



Manuf Qty Supported Member

24' 0" 1-3/4"x 14" LVL Kerto-S 2

Product

	HUS26	USP	7	Varies	16d/3-1/2"	16d/3-1/2"	
Products							
otID	Length	Prod	uct		Plie	s Net C	Qty
/ 11	4' 0"	2x10	2x10 SPF No.2		2	2	
Л2	12' 0"	2x12	2x12 SPF No.2		2	4	

Header Truss



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature_

David Landry

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))
NUMBER OF JACK STUDS REQUIRED @ EA END OF

		HEADER/	GIRDEF	₹		
END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR
1700	1	2550	1		3400	1
3400	2	5100	2		6800	2
5100	3	7650	3		10200	3
6800	4	10200	4		13600	4
8500	5	12750	5		17000	5
10200	6	15300	6			
11900	7					
13600	8					
15300	9					
	I					

Ben Stout Real Estate	COUNTY	Cumberland
Lot 30 Forest Ridge	ADDRESS	Tanna Place
The Williams	WODEL	Roof
N/A	DATE REV. //	//
Quote #	DRAWN BY	DRAWN BY David Landry
J0621-3575	SALESMAN	SALESMAN Marshall Naylor

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

SEAL DATE

QUOTE # JOB #

JOB NAME PLAN

BUILDER



Client: Ben Stout Real Estate

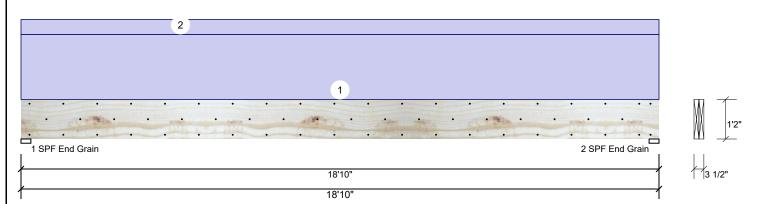
Project: Address: Date: 1/7/2021

Input by: David Landry Job Name: Lot 30 Forest Ridge Project #: J1020-4757

Page 1 of 2

Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED

Level: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Wind Type: Application: Floor Brg Live Dead Snow Const Plies: 2 Design Method: ASD 2363 0 0 0 0 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 0 2363 0 0 0 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal Temperature: Temp <= 100°F Bearings Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.500" 2363 / 0 2363 Uniform D End Grain Analysis Results 2 - SPF 3.500" 22% 2363 / 0 2363 Uniform D Comb. Analysis Actual Location Allowed Case Capacity End Moment 10589 ft-lb 9'5" 24299 ft-lb 0.436 (44%) D Uniform Grain Unbraced 10589 ft-lb 9'5" 10593 ft-lb 1.000 Uniform (100%)1'4 3/4" 9408 lb 2012 lb 0.214 (21%) D Uniform Shear

Uniform

Design Notes

1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6"

0 999.000 (L/0) 0.000 (0%)

9'5 1/16" 0.612 (L/360) 0.700 (70%) D

- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 10'1 1/2" o.c.
- 6 Bottom braced at bearings.

LL Defl inch 0.000 (L/999)

TL Defl inch 0.427 (L/516)

7 Lateral slenderness ratio based on single ply width

1 Lateral Sichae	incos ratio basca on single	pry wiatri.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	195 PLF	0 PLF	0 PLF	0 PLF	0 PLF	B1GE	
2	Uniform			Тор	45 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Above	
	Self Weight				11 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 2/26/2023

Manufacturer Info

isDesign

Client: Ben Stout Real Estate

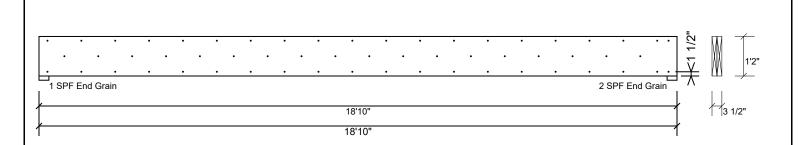
Project: Address: 1/7/2021

Input by: David Landry Job Name: Lot 30 Forest Ridge Project #: J1020-4757

Page 2 of 2

2-Ply - PASSED **Kerto-S LVL** 1.750" X 14.000" **GDH-FE**

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

	, ,	
Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

