

RE: J1020-4756 Lot 30 Forest Ridge Trenco 818 Soundside Rd Edenton, NC 27932

Site Information: Customer: Project Name: J1020-4756 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

No.

21

22

23

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.3 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 23 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E14970005	A1	1/7/2021
2	E14970006	A1A	1/7/2021
3	E14970007	A1GE	1/7/2021
4	E14970008	A2	1/7/2021
5	E14970009	A2A	1/7/2021
6	E14970010	A2GE	1/7/2021
7	E14970011	B1	1/7/2021
8	E14970012	B1GE	1/7/2021
9	E14970013	B2	1/7/2021
10	E14970014	C1GE	1/7/2021
11	E14970015	D1	1/7/2021
12	E14970016	D1-GR	1/7/2021
13	E14970017	D1SG	1/7/2021
14	E14970018	H1	1/7/2021
15	E14970019	H1GE	1/7/2021
16	E14970020	M1	1/7/2021
17	E14970021	M1GE	1/7/2021
18	E14970022	M2	1/7/2021
19	E14970023	V1	1/7/2021
20	E14970024	V2	1/7/2021

Seal# **Truss Name** Date E14970025 V3 E14970026 V4 E14970027 V5

1/7/2021 1/7/2021 1/7/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

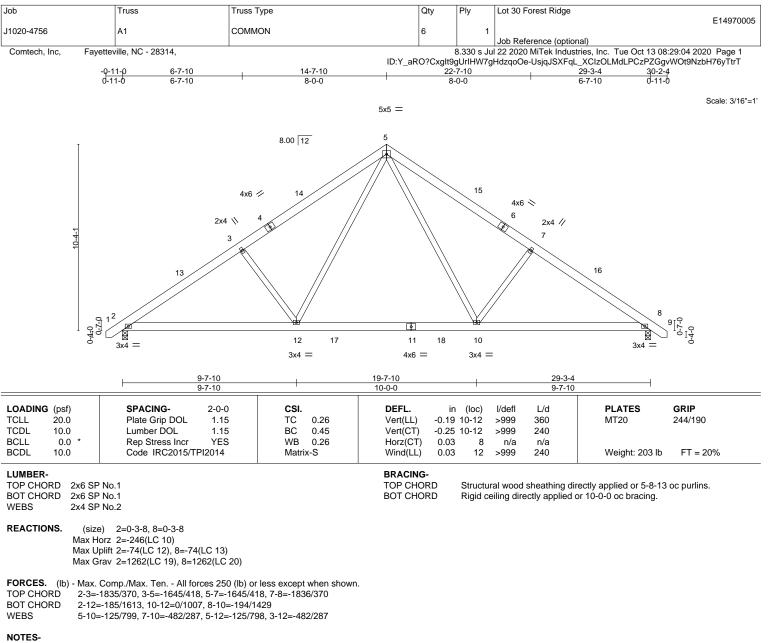
Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2021

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 37-12, Interior(1) 3-7-12 to 14-7-10, Exterior(2) 14-7-10 to 19-0-7, Interior(1) 19-0-7 to 30-0-5 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

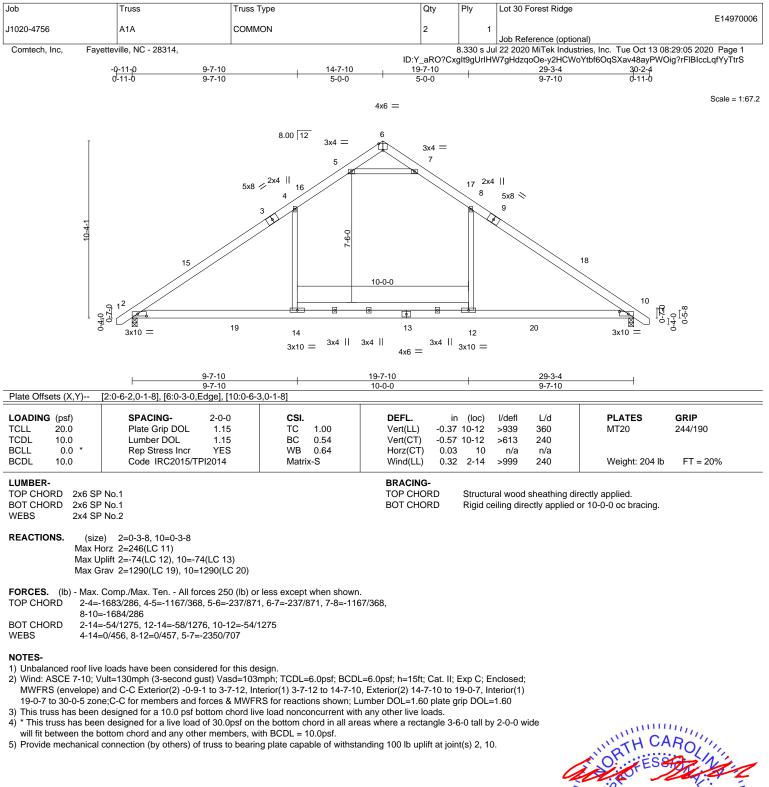
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



818 Soundside Road Edenton, NC 27932

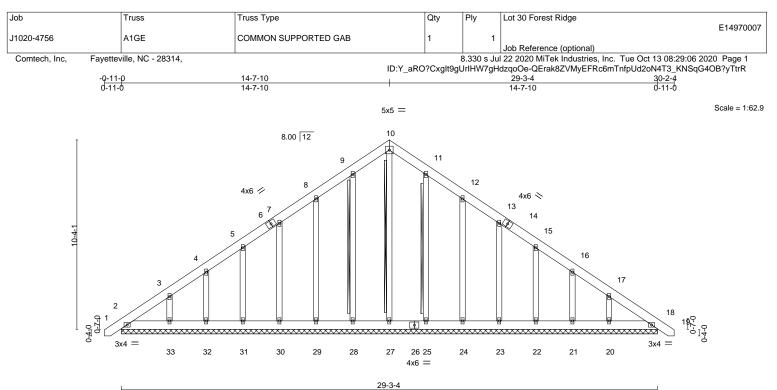




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

TRENCO A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932



29-3-4 GRIP LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) l/defl L/d PLATES TCLL 20.0 Plate Grip DOL 1.15 тс 0.04 Vert(LL) 0.00 18 n/r 120 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 BC 0.03 Vert(CT) 0.00 18 n/r 120 WB BCLL 0.0 **Rep Stress Incr** YES 0.13 Horz(CT) 0.01 18 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Weight: 255 lb FT = 20% DDACING

Brace must cover 90% of web length.

LUMBER-		BRACING-			
TOP CHORD	2x6 SP No.1	TOP CHORD	Structural wood she	eathing directly applied or 6-0-0 oc purlins.	
BOT CHORD	2x6 SP No.1	BOT CHORD	Rigid ceiling directly	/ applied or 10-0-0 oc bracing.	
OTHERS	2x4 SP No.2	WEBS	T-Brace:	2x4 SPF No.2 - 10-27, 9-28, 11-25	
			Fasten (2X) T and I	I braces to narrow edge of web with 10d	
			(0.131"x3") nails, 6ir	n o.c. with 3in minimum end distance.	

REACTIONS. All bearings 29-3-4.

(lb) - Max Horz 2=307(LC 11)

- Max Uplift 101 b or less at joint(s) 2, 28, 29, 30, 31, 32, 25, 23, 22, 21, 18 except 33=-120(LC 12), 24=-101(LC 13), 20=-118(LC 13)
- Max Grav All reactions 250 lb or less at joint(s) 2, 27, 28, 29, 30, 31, 32, 33, 25, 24, 23, 22, 21, 20, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

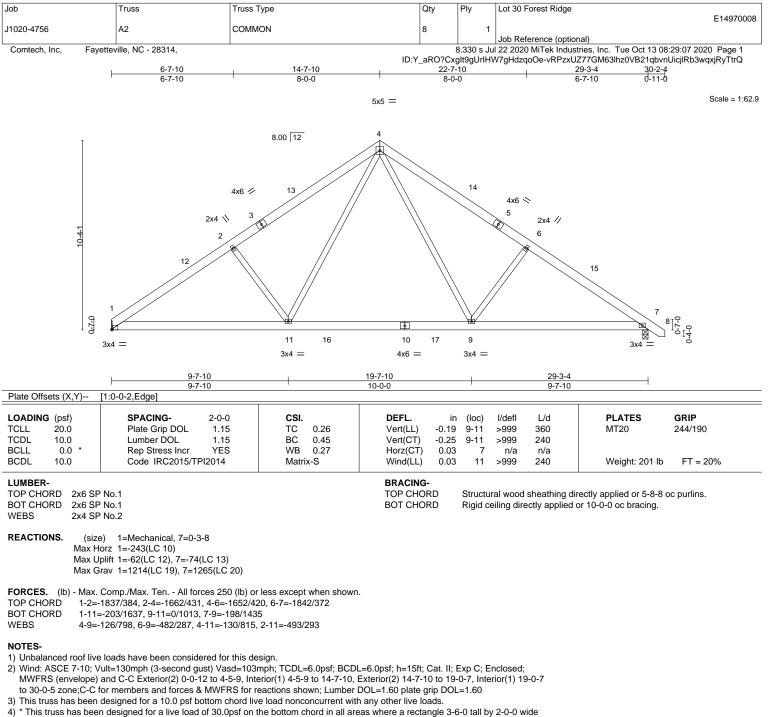
TOP CHORD 2-3=-300/229, 9-10=-233/262, 10-11=-233/262

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 28, 29, 30, 31, 32, 25, 23, 22, 21, 18 except (jt=lb) 33=120, 24=101, 20=118.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.





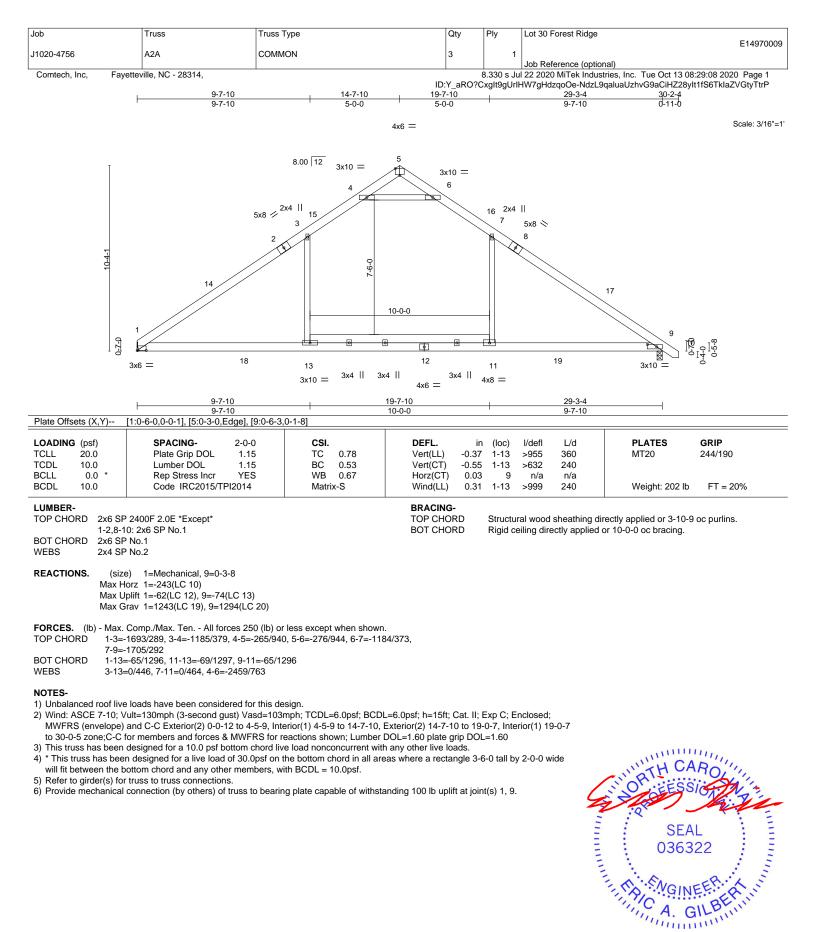


4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2 will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

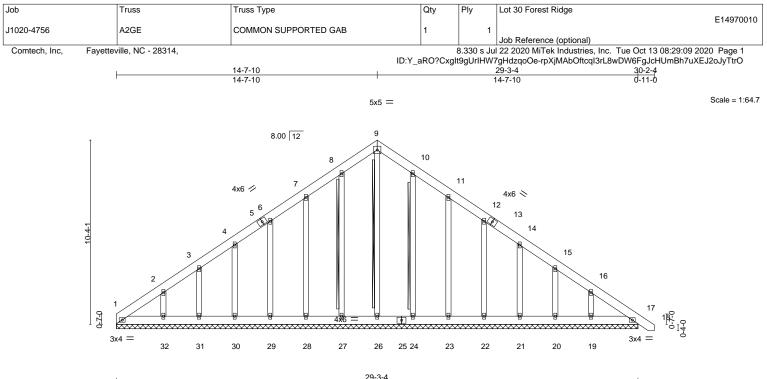
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.





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			29-3-4									
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	0.00	<u>1</u> 7	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	0.00	17	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.01	17	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-S						Weight: 253 lb	FT = 20%

LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
OTHERS	2x4 SP No.2

BRACING-TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. T-Brace: 2x4 SPF No.2 - 9-26, 8-27, 10-24 Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c.,with 3in minimum end distance. Brace must cover 90% of web length.

- REACTIONS. All bearings 29-3-4.
 - (lb) Max Horz 1=-303(LC 8)
 - Max Uplift All uplift 100 lb or less at joint(s) 1, 27, 28, 29, 30, 31, 24, 22, 21, 20, 17 except 32=-127(LC 12), 23=-101(LC 13), 19=-118(LC 13)
 - Max Grav All reactions 250 lb or less at joint(s) 1, 26, 27, 28, 29, 30, 31, 32, 24, 23, 22, 21, 20, 19, 17

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-303/231, 8-9=-233/262, 9-10=-233/262

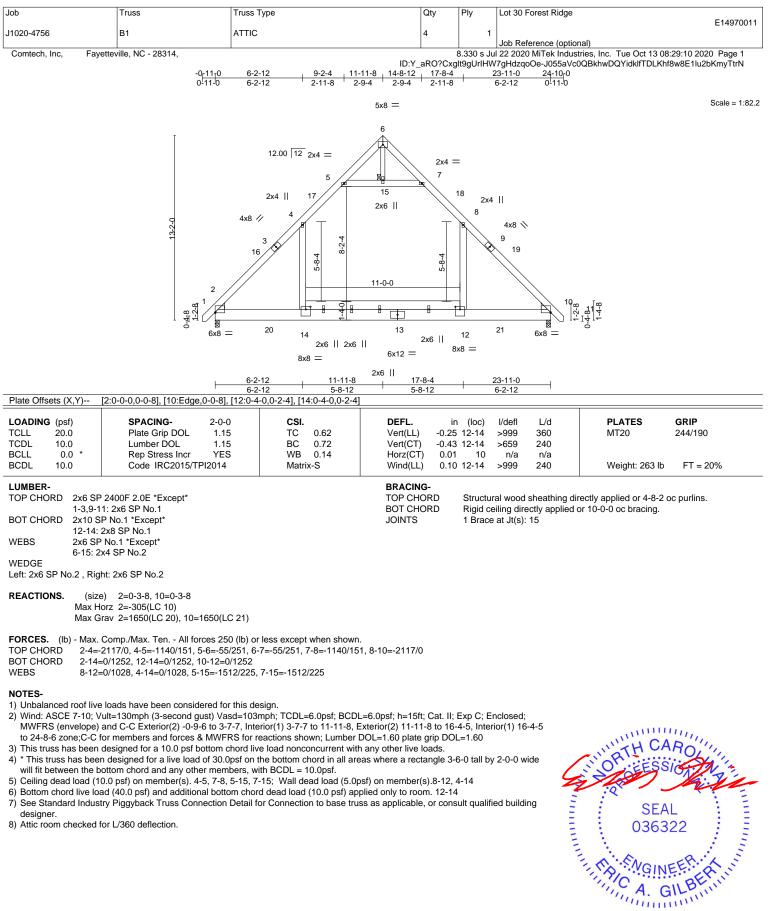
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 27, 28, 29, 30, 31, 24, 22, 21, 20, 17 except (jt=lb) 32=127, 23=101, 19=118.

10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.







October 13,2020

ENGINEERING BY ENGINEERING BY AMITEK ATFILIATE AMITEK ATFILIATE 818 Soundside Road Edenton, NC 27932

	Truss	Truss Type	Qty	Ply Lot 3) Forest Ridge		E14970012
020-4756	B1GE	GABLE	1	1 lob 5	leference (ontional)		211070012
Comtech, Inc, Fay	etteville, NC - 28314,			8.330 s Jul 22 20			3:29:11 2020 Page 1
	-0 <u>-11₁0</u>		1-11-8 14-8-12 17-8-4	4 23-11	-0 24-10 _r 0	'N?kFKF_BgmQa5	0FfaPB_Yo9tCyTtrM
	0-11-0	6-2-12 2-11-8	2-9-4 2-9-4 2-11-8	8 6-2-1	2 0-11-0		
			5x8 =				Scale = 1:77.9
			8				
	2x4	12.00 12 2x4 = $7 3x10 $ $4x6 7 6$ $2x4 $ $4 5 7 7$ $7 7$ $7 7$ $7 7$ $7 7$ $7 7$ $7 7$	2x4 9 23 11-0-0	3x10 10 - 4x	5 × x4 12 2x4 13		
	8-7-1 8-7-1 8-7-1 8-7-1					15 14 14 14 15 15 15 15 15 15 15 15 15 15 15 15 15	
	5x8 :	= 22 21 20 10x10 =	19 6x12 =	18 17 10x10 =	16 5x8 =	-	
		6-2-12 11-11-8 6-2-12 5-8-12		23-11 6-2-1			
late Offsets (X,Y)	[18:0-5-0,0-3-12], [20:0-5-0,0						
OADING (psf) CLL 20.0		0-0 CSI. .15 TC 0.97		n (loc) l/defl 2 18-20 >999	L/d 360	PLATES MT20	GRIP 244/190
CDL 10.0 CLL 0.0 *	Lumber DOL 1	.15 BC 0.67 ES WB 0.21		9 18-20 >736	240 n/a		
CDL 10.0	Code IRC2015/TPI20		Wind(LL) 0.13		240	Weight: 279 lb	FT = 20%
18-20: VEBS 2x6 SF 8-23: 2 VTHERS 2x4 SF VEDGE eft: 2x4 SP No.3 , Rig EACTIONS. (siz Max H	P No.1 *Except* 2x8 SP No.1 P No.1 *Except* x4 SP No.2 No.2 ht: 2x4 SP No.3 e) 2=0-3-8, 14=0-3-8 orz 2=-382(LC 10)		TOP CHORD BOT CHORD JOINTS		id sheathing directly irectly applied or 10 s): 23		
ORCES. (lb) - Max. OP CHORD 2-3= 10-12 OT CHORD 2-22 14-11 VEBS 10-12	1993 ['] /0, 3-4=-1620/0, 4-6=-20 2=-2061/75, 12-13=-1620/0, 1 =0/1204, 21-22=0/1206, 20-2 5=0/1203	250 (lb) or less except when show 62/75, 6-7=-1090/194, 9-10=-109 3-14=-1992/0 =0/1203, 18-20=0/1203, 17-18=0, -23=-1313/294, 9-23=-1313/294, 4	0/194, /1203, 16-17=0/1205,				
) Wind: ASCE 7-10; \ MWFRS (envelope) DOL=1.60 plate grip) Truss designed for \ Gable End Details a) All plates are 2x6 M) Gable studs spaced) This truss has been) * This truss has been will fit between the b) Ceiling dead load (1) Bottom chord live lo	gable end zone and C-C Exte DOL=1.60 vind loads in the plane of the s applicable, or consult qualifi T20 unless otherwise indicate at 2-0-0 oc. designed for a 10.0 psf botton n designed for a live load of 3 vottom chord and any other m 0.0 psf) on member(s). 6-7, 9 ad (40.0 psf) and additional bo stry Piggyback Truss Connec	Vasd=103mph; TCDL=6.0psf; BCl rrior(2) zone;C-C for members and russ only. For studs exposed to v ed building designer as per ANSI/ d. n chord live load nonconcurrent w 0.0psf on the bottom chord in all a	d forces & MWFRS for rea wind (normal to the face), s TPI 1. with any other live loads. areas where a rectangle 3- 5.0psf) on member(s).10-1 applied only to room. 18-20	actions shown; L see Standard Ind -6-0 tall by 2-0-0 18, 6-20 0	umber Justry wide	SEA 0363	EER.KI

WARNING - Verity design parameters and READ NOTES ON THIS AND INCLODED MITER REFERENCE PAGE MIT-1473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

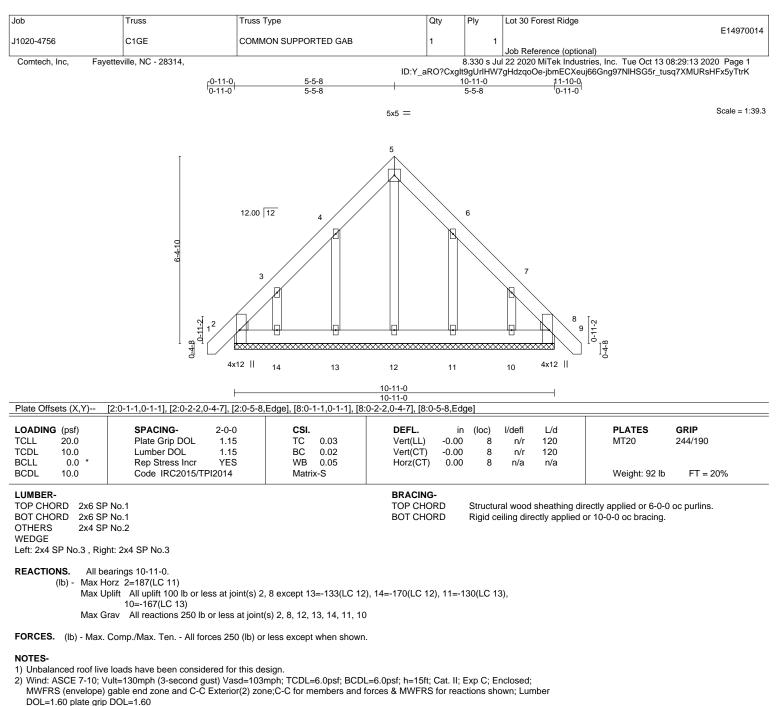
818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 30 Forest Ridge		
J1020-4756	B2	ATTIC	6	1			E14970013
Comtech, Inc, Fay	retteville, NC - 28314,			3.330 s Ji	Job Reference (optional Job 22 2020 MiTek Industri		3:29:12 2020 Page 1
		6-2-12 9-2-4 11-11 6-2-12 2-11-8 2-9-	ID:Y_aRO?CxgIt9 1-8 14-8-12 7-8-4	gUrIHW7 23	'gHdzqoOe-FOCs_BdGy <u>-11-0 24-10-</u> 0 -2-12 0-11-0		
			5x8 =				Scale = 1:82.2
	42 87 87 1 87 1 87 1 87 1 87 1 87 1 87 1	$12.00 \overline{12} _{2X4} =$ $2x4 15$	$5 \\ 2x4 = 6 \\ 14 \\ 2x6 \\ 11-0-0 \\ 12 \\ 2x6 \\ 12 \\ 2x6 \\ 8x8 \\ 2x6 \\ 8x8 \\ 2x6 \\ 12 \\ 8x8 \\ 2x6 \\ 12 \\ 8x8 \\ 2x6 \\ 12 \\ 12 \\ 12 \\ 12 \\ 12 \\ 12 \\ 12 \\$	1 1	$ \begin{array}{c} 4x8 \\ 8 \\ 17 \\ 9 \\ 6x8 \\ 9 \\ 6x8 \\ \end{array} $	1- 1-4- 6-	
		6-2-12 11-11-8 6-2-12 5-8-12	17-8-4		-2-12		
	[1:0-0-0,0-0-12], [9:Edge,0-0-8], [11:0-4-0,0-2-4], [13:0-4-0,0-2-8]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.63 BC 0.73 WB 0.14 Matrix-S	Vert(LL) -0.26 Vert(CT) -0.44 Horz(CT) 0.01	(loc) 11-13 11-13 9 11-13	l/defl L/d >999 360 >649 240 n/a n/a >999 240	PLATES MT20 Weight: 261 lb	GRIP 244/190 FT = 20%
BOT CHORD 1-2,8-1 BOT CHORD 2x10 S 11-13: 2x6 SP VEDSE 2x6 SP Left: 2x6 SP No.2 , Rig REACTIONS. (size Max H	2x8 SP No.1 P No.1 *Except* 2x4 SP No.2	1)	BRACING- TOP CHORD BOT CHORD JOINTS	Rigid co	ral wood sheathing dire eiling directly applied or e at Jt(s): 14		oc purlins.
TOP CHORD 1-3=- BOT CHORD 1-13=	Comp./Max. Ten All forces 250 -2093/0, 3-4=-1143/156, 4-5=-53/2 =0/1253, 11-13=0/1253, 9-11=0/12 =0/1035, 3-13=0/993, 4-14=-1522	253, 5-6=-48/259, 6-7=-1137/14 253					
 Wind: ASCE 7-10; V MWFRS (envelope) to 24-8-6 zone;C-C 1 This truss has been will fit between the b Ceiling dead load (1 	e loads have been considered for /ult=130mph (3-second gust) Vasa and C-C Exterior(2) 0-1-12 to 4-6 for members and forces & MWFR designed for a 10.0 psf bottom ch n designed for a live load of 30.0p bottom chord and any other memb 0.0 psf) on member(s). 3-4, 6-7, 4 ad (40.0 psf) and additional bottor for L/360 deflection.	H=103mph; TCDL=6.0psf; BCDI -9, Interior(1) 4-6-9 to 11-11-8, S for reactions shown; Lumber i ord live load nonconcurrent with sf on the bottom chord in all are ers, with BCDL = 10.0psf. -14, 6-14; Wall dead load (5.0p	Exterior(2) 11-11-8 to 16 DOL=1.60 plate grip DOL h any other live loads. eas where a rectangle 3-6 posf) on member(s).7-11, 3	4-5, Inte =1.60 6-0 tall by	rior(1) 16-4-5 / 2-0-0 wide	SEA 0363	• -

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ENGINEERING BY EREPACE A MITEK AT 818 Soundside Road Edenton, NC 27932

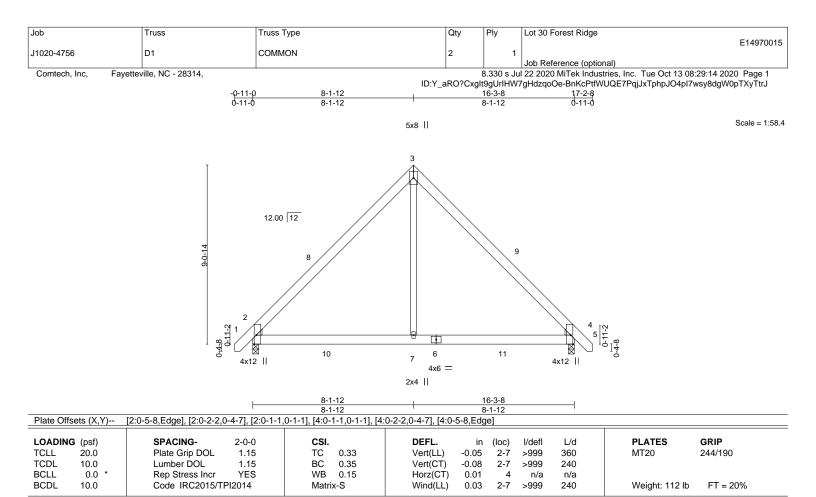
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- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8 except (jt=lb) 13=133, 14=170, 11=130, 10=167.







BRACING-

TOP CHORD

BOT CHORD

LUMBER-TOP CHORD 2x6 SP No.1

BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2 WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. (size) 4=0-3-8, 2=0-3-8 Max Horz 2=214(LC 11) Max Uplift 4=-31(LC 13), 2=-31(LC 12) Max Grav 4=824(LC 20), 2=824(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-856/187, 3-4=-856/188
- BOT CHORD 2-7=0/551, 4-7=0/551
- WEBS 3-7=0/654

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-6 to 3-7-7, Interior(1) 3-7-7 to 8-1-12, Exterior(2) 8-1-12 to 12-6-9, Interior(1) 12-6-9 to 17-0-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

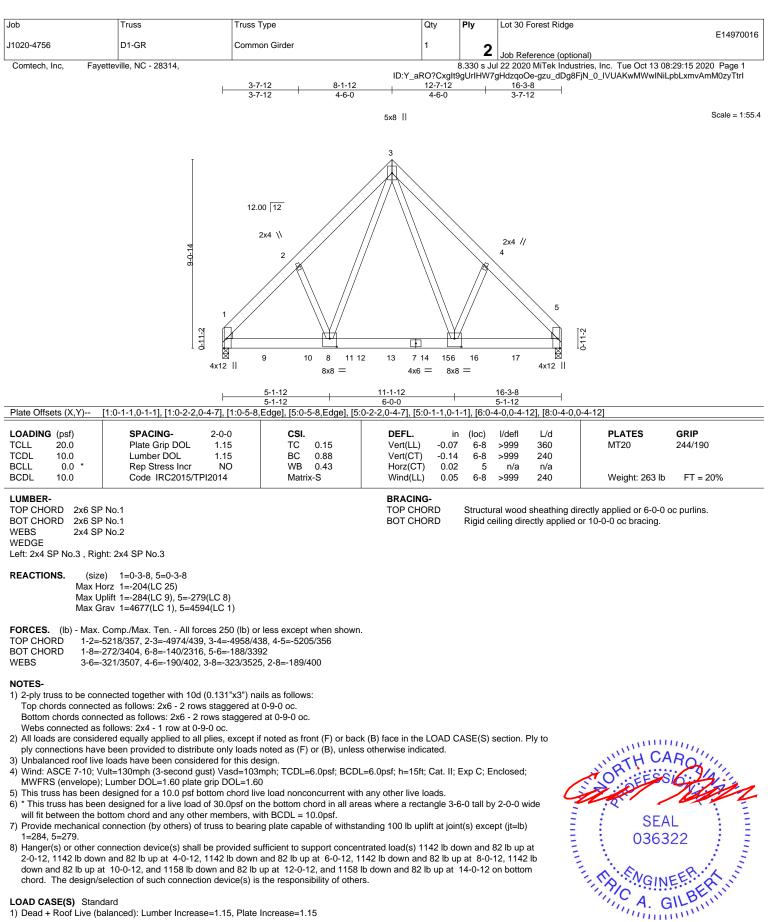
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

October 13,2020

[Job	Truss	Truss Type	Qty	Ply	Lot 30 Forest Ridge
						E14970016
	J1020-4756	D1-GR	Common Girder	1	2	
						Job Reference (optional)
	Comtech, Inc, Fayettev	rille, NC - 28314,			3.330 s Ju	22 2020 MiTek Industries, Inc. Tue Oct 13 08:29:15 2020 Page 2
			ID:Y_al	RO?Cxglt9	gUrlHW7g	gHdzqoOe-gzu_dDg8FjN_0_IVUAKwMWwINiLpbLxmvAmM0zyTtrl

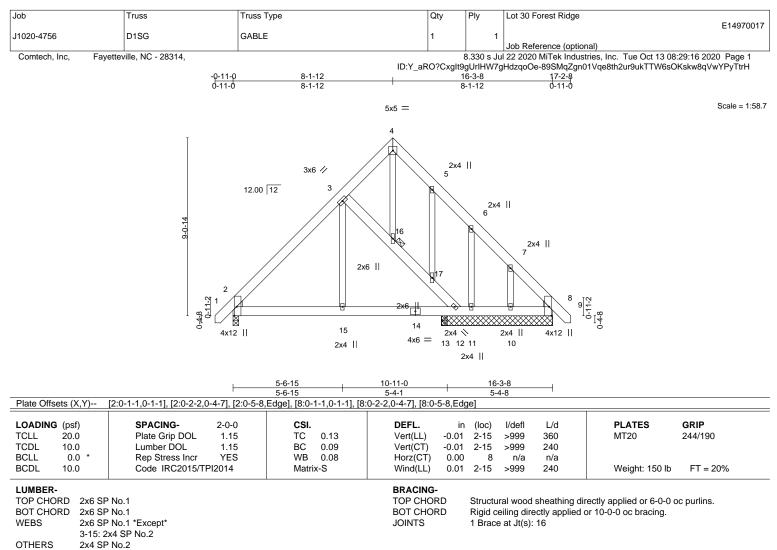
LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-3=-60, 3-5=-60, 1-5=-20 Concentrated Loads (lb)

Vert: 9=-1142(B) 10=-1142(B) 11=-1142(B) 13=-1142(B) 15=-1142(B) 16=-1142(B) 17=-1142(B)





WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. All bearings 5-8-0 except (jt=length) 2=0-3-8, 13=0-3-8.

(lb) - Max Horz 2=-267(LC 10)

- Max Uplift All uplift 100 lb or less at joint(s) 2 except 12=-281(LC 12), 11=-163(LC 13), 10=-194(LC 13)
- Max Grav All reactions 250 lb or less at joint(s) 12, 10 except 2=537(LC 1),
- 8=272(LC 22), 11=255(LC 20), 13=257(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

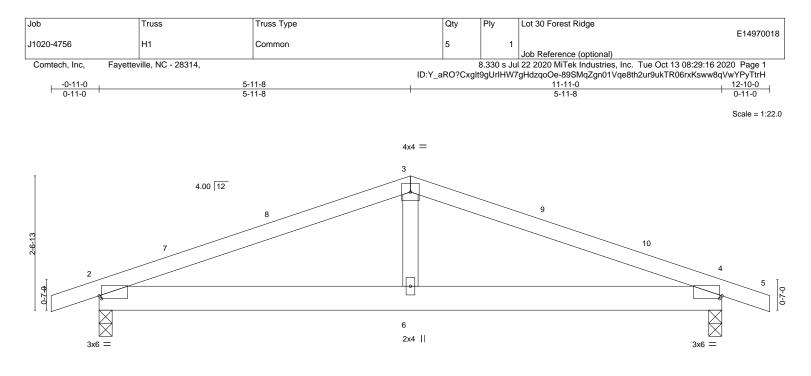
- TOP CHORD 2-3=-511/53, 7-8=-369/247
- BOT CHORD 2-15=-98/380, 13-15=-98/380, 12-13=-98/380, 11-12=-209/311, 10-11=-208/310, 8-10=-207/309
- WEBS 3-16=-438/274, 16-17=-383/246, 12-17=-467/285

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 12=281, 11=163, 10=194.



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	<u>5-11-8</u> <u>5-11-8</u>				1-11-0 5-11-8	
Plate Offsets (X,Y)	[2:0-0-8,0-0-10], [4:0-0-8,0-0-10]	1				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.29 BC 0.18 WB 0.07 Matrix-S	DEFL. ir Vert(LL) -0.01 Vert(CT) -0.03 Horz(CT) 0.01 Wind(LL) 0.04	6 >999 2-6 >999 4 n/a	L/d PLATES 360 MT20 240 n/a 240 Weight: 52	GRIP 244/190 2 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP WEBS 2x4 SP	' No.1		BRACING- TOP CHORD BOT CHORD		heathing directly applied or 6 tly applied or 8-9-15 oc braci	

REACTIONS. (size) 2=0-3-0, 4=0-3-0 Max Horz 2=-27(LC 13) Max Uplift 2=-205(LC 8), 4=-205(LC 9) Max Grav 2=529(LC 1), 4=529(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- 2-3=-806/870, 3-4=-806/870 TOP CHORD BOT CHORD
- 2-6=-734/695, 4-6=-734/695 WFBS 3-6=-379/290

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 5-11-8, Exterior(2) 5-11-8 to 10-4-5, Interior(1) 10-4-5 to 12-10-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate arip DOL=1.60

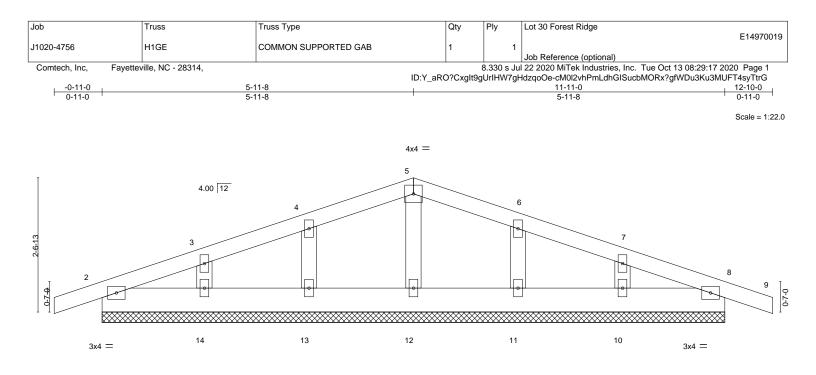
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=205, 4=205.







		<u>11-11-0</u> 11-11-0						
-OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.04	Vert(LL) -0.00	` <i>8</i>	n/r	120	MT20	244/190
CDL 10.0	Lumber DOL 1.15	BC 0.01	Vert(CT) -0.00	8	n/r	120		
CLL 0.0 *	Rep Stress Incr YES	WB 0.02	Horz(CT) 0.00	8	n/a	n/a		
3CDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 57 lb	FT = 20%

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. All bearings 11-11-0.

Max Horz 2=-46(LC 13) (lb) -

- Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 14, 11, 10
- Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 14, 11, 10.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



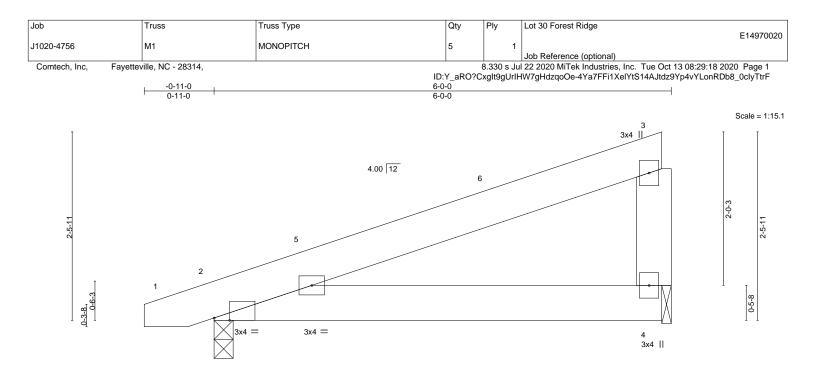


Plate Offse	ets (X,Y) [[2:0-2-7,Edge]									T	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.19	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.12	Vert(CT)	-0.03	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matrix	(-P	Wind(LL)	0.03	2-4	>999	240	Weight: 34 lb	FT = 20%
LUMBER-						BRACING-					÷	
TOP CHOP	RD 2x6 SP	No.1				TOP CHOR	RD.	Structu	ral wood	sheathing di	irectly applied or 6-0-0	oc purlins,
BOT CHOP	RD 2x6 SP	No.1						except	end verti	cals.		
WEBS	2x6 SP	No.1				BOT CHOR	RD	Rigid c	eiling dire	ectly applied	or 10-0-0 oc bracing.	

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=72(LC 8) Max Uplift 2=-105(LC 8), 4=-96(LC 8) Max Grav 2=274(LC 1), 4=223(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

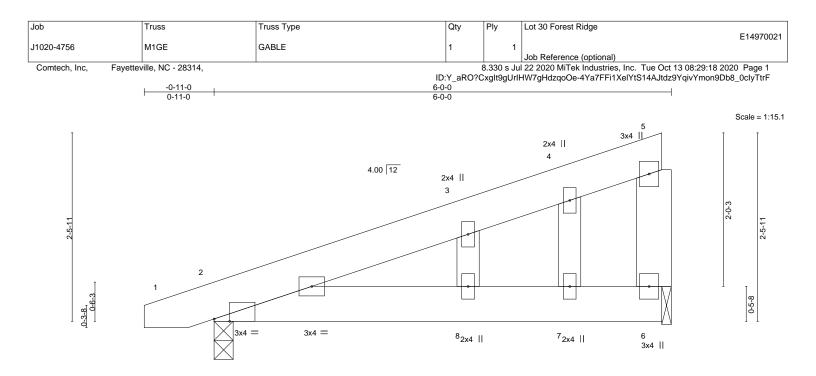
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NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-9 to 3-9-4, Interior(1) 3-9-4 to 5-9-4 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=105.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







OADING (psi	f)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.	0	Plate Grip DOL	1.15	тс	0.08	Vert(LL)	-0.01	8	>999	360	MT20	244/190
CDL 10.	0	Lumber DOL	1.15	BC	0.10	Vert(CT)	-0.02	8	>999	240		
BCLL 0.	0 *	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	6	n/a	n/a		
BCDL 10.	0	Code IRC2015/TF	PI2014	Matri	x-S	Wind(LL)	0.01	8	>999	240	Weight: 37 lb	FT = 20%
UMBER-						BRACING						
OP CHORD	2x6 SP No.1					TOP CHOP	RD	Structu	ral wood	sheathing dir	ectly applied or 6-0-0	oc purlins.
BOT CHORD	2x6 SP No.1							except	end verti	cals.	,	• •
VEBS	2x6 SP No.1					BOT CHOP					or 10-0-0 oc bracing.	
OTHERS	2x4 SP No.2							3	5		j	

Max Horz 2=102(LC 8) Max Uplift 2=-89(LC 8), 6=-79(LC 12) Max Grav 2=274(LC 1), 6=223(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

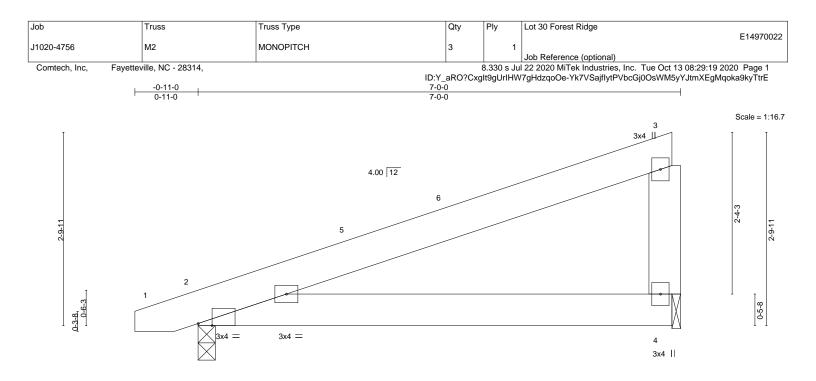
NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 1-4-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.27	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.18	Vert(CT)	-0.05	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0 Code IRC2015/TPI2014 Matrix-P			Wind(LL)	0.06	2-4	>999	240	Weight: 40 lb	FT = 20%		
LUMBER-						BRACING-						
TOP CHORD 2x6 SP No.1					TOP CHOP		Structural wood sheathing directly applied or 6-0-0 oc purlins,					
BOT CHORD 2x6 SP No.1						. 51 01101		except end verticals.				

BOT CHORD Rigid ceiling directly applied or 10-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=83(LC 8) Max Uplift 2=-119(LC 8), 4=-114(LC 8) Max Grav 2=314(LC 1), 4=263(LC 1)

2x6 SP No.1

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

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NOTES-

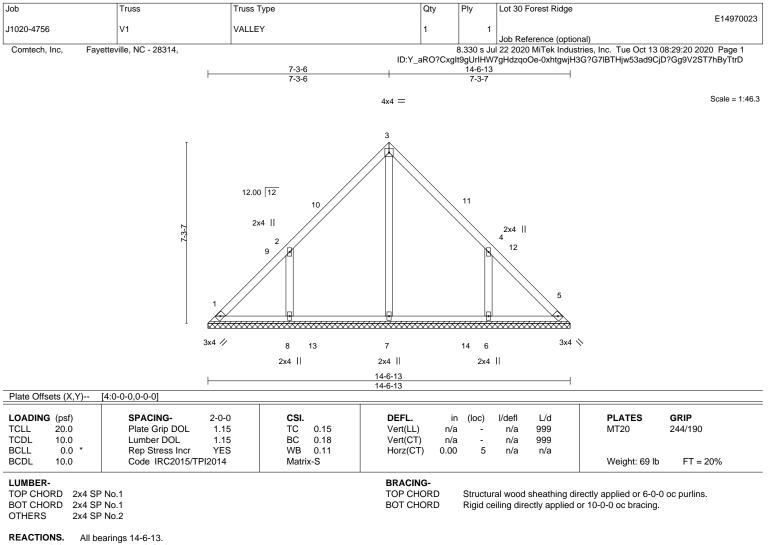
WEBS

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-9 to 3-9-4, Interior(1) 3-9-4 to 6-9-4 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=119, 4=114.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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(lb) - Max Horz 1=166(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-175(LC 12), 6=-175(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=407(LC 22), 8=427(LC 19), 6=427(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

WEBS 2-8=-380/298, 4-6=-380/298

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 7-3-6, Exterior(2) 7-3-6 to 11-8-3, Interior(1) 11-8-3 to 14-2-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

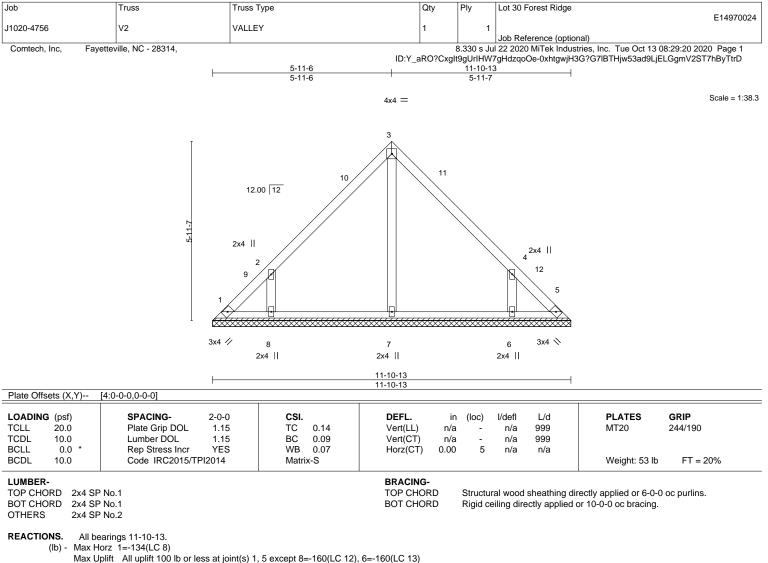
Frovide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=175, 6=175.

6) Non Standard bearing condition. Review required.



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Max Oplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-160(LC 12), 6=-160(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=338(LC 19), 6=338(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

WEBS 2-8=-357/297, 4-6=-357/297

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 5-11-6, Exterior(2) 5-11-6 to 10-4-3, Interior(1) 10-4-3 to 11-6-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

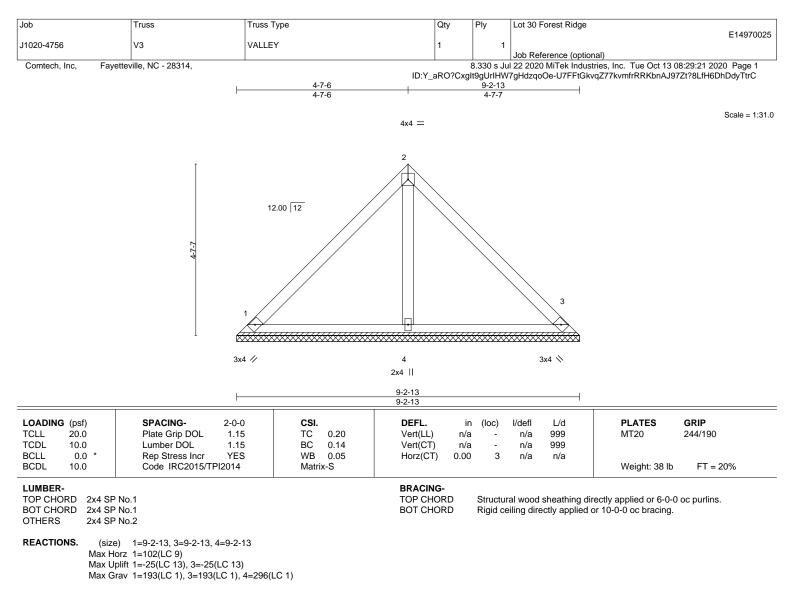
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=160, 6=160.

6) Non Standard bearing condition. Review required.







FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

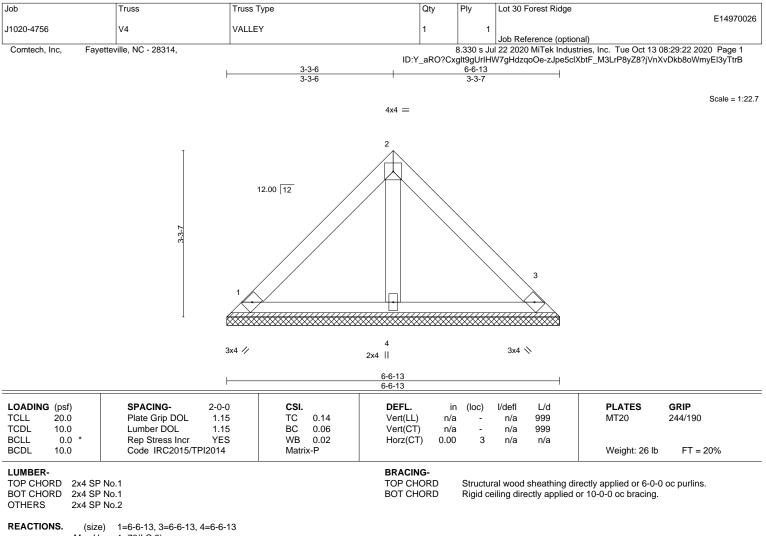
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



Max Horz 1=70(LC 9)

Max Uplift 1=-25(LC 13), 3=-25(LC 13) Max Grav 1=143(LC 1), 3=143(LC 1), 4=183(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

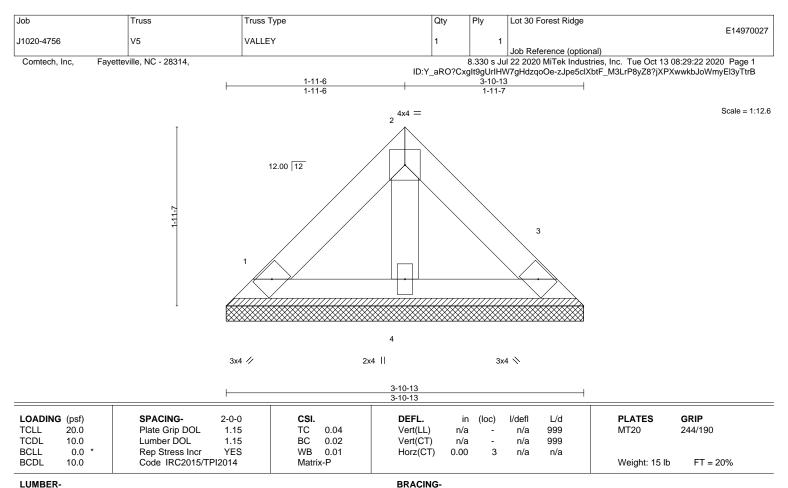
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.







TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=3-10-13, 3=3-10-13, 4=3-10-13

Max Horz 1=-38(LC 8)

Max Uplift 1=-14(LC 13), 3=-14(LC 13)

Max Grav 1=78(LC 1), 3=78(LC 1), 4=100(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

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6) Non Standard bearing condition. Review required.



Structural wood sheathing directly applied or 3-10-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



