

Trenco 818 Soundside Rd Edenton, NC 27932

Re: CG1011-R McKee-Portico20CL;Lot 1011 CarriageGlen

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource-Apex,NC.

Pages or sheets covered by this seal: I44981542 thru I44981585

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



February 26,2021

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



February 26,2021

ENGINEERING BY **TRENCO** A MI Tek Affiliate 818 Soundside Road

Edenton, NC 27932



February 26,2021





11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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.104 minin February 26,2021

> 818 Soundside Road Edenton, NC 27932



	0-	-1 <sub>1</sub> 12 10-4-12	22-5-4	22-6-4	30-7-4	41-0-0	1
	0-	·1 <sup>!</sup> 12 10-3-0	12-0-8	0-1-0	8-1-0	10-4-12	
Plate Offsets ()	X,Y)	[5:0-4-0,0-2-12], [6:0-4-0,0-2-12]					
LOADING (ps	f)	SPACING- 2-0-0	CSI.	DEFL. in	(loc) l/defl L/d	PLATES	GRIP
TCLL 20.	Ó	Plate Grip DOL 1.15	TC 0.66	Vert(LL) -0.21	13-15 >999 360	MT20	244/190
TCDL 10.	0	Lumber DOL 1.15	BC 0.58	Vert(CT) -0.32	13-15 >825 240		
BCLL 0.	0 *	Rep Stress Incr YES	WB 0.87	Horz(CT) 0.01	10 n/a n/a		
BCDL 10.	0	Code IRC2015/TPI2014	Matrix-MS	Wind(LL) 0.03	15 >999 240	Weight: 305 lb	FT = 20%
LUMBER-				BRACING-			
TOP CHORD	2x6.SP	No 2		TOP CHORD	Structural wood sheathing	n directly applied or 6-0-0 or	nurlins
BOT CHORD	2x6 SP	No.2			except end verticals and	2-0-0 oc purlins (10-0-0 ma	x ): 5-6
WEBS	2x4 SP	No.3 *Except*		BOT CHORD	Rigid ceiling directly appli	ed or 10-0-0 oc bracing F	xcept.
	2-16.9-	-10: 2x6 SP No 2 5-15 5-13 6-11: 2x4 5	P No 2	201 0110112	6-0-0 oc bracing: 11-13	ca ci i c c c c ci ciacii.g, _	
	0,0			WEBS	1 Row at midpt	5-13, 6-13	
REACTIONS.	(size	e) 16=0-3-8, 10=0-3-8, 13=0-3-8					
	Max H	orz 16=312(LC 9)					

Max Horz 16=312(LC 9) Max Uplift 16=-163(LC 12), 10=-127(LC 13), 13=-130(LC 13) Max Grav 16=789(LC 25), 10=512(LC 20), 13=2355(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-16=-686/215, 2-3=-826/212, 3-5=-672/330, 5-6=0/504, 6-8=-370/311, 8-9=-462/192, 9-10=-404/182
- BOT CHORD 15-16=-254/724, 11-13=-313/147, 10-11=-64/287
- WEBS 3-15=-443/328, 5-15=-172/940, 5-13=-1080/229, 6-13=-1074/248, 6-11=-209/846, 8-11=-485/330

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-3 to 2-3-13, Interior(1) 2-3-13 to 15-7-8, Exterior(2) 15-7-8 to 19-10-7, Interior(1) 19-10-7 to 25-4-8, Exterior(2) 25-4-8 to 29-7-7, Interior(1) 29-7-7 to 40-9-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16, 10, and 13. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

# SEAL 45844 February 26,2021





	0-1 <sub>IT</sub>	12 10-4-12	1	22-5-4	22-6-4		30-7-4		1	41-0-0	1
	0-1 <sup>[]</sup>	12 10-3-0	1	12-0-8	0-1-0		8-1-0		T	10-4-12	
Plate Offsets (X,	,Y) [5	:0-4-0,0-2-12], [6:0-4-0,	0-2-12]								
LOADING (psf)		SPACING-	2-0-0 CS		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0		Plate Grip DOL	1.15 TC	0.65	Vert(LL)	-0.21	14-16	>999	360	MT20	244/190
TCDL 10.0		Lumber DOL	1.15 BC	0.58	Vert(CT)	-0.32	14-16	>824	240		
BCLL 0.0	*	Rep Stress Incr	YES WB	0.86	Horz(CT)	0.01	11	n/a	n/a		
BCDL 10.0		Code IRC2015/TF	Pl2014 Mat	rix-MS	Wind(LL)	0.03	16	>999	240	Weight: 307 lb	FT = 20%
LUMBER-					BRACING-						
TOP CHORD	2x6 SP N	lo.2			TOP CHOP	RD	Structu	ral wood	sheathing dir	ectly applied or 6-0-0 c	oc purlins,
BOT CHORD	2x6 SP N	lo.2					except	end verti	cals, and 2-0-	-0 oc purlins (10-0-0 m	ax.): 5-6.
WEBS	2x4 SP N	lo.3 *Except*			BOT CHOF	RD	Rigid c	eiling dire	ctly applied c	or 10-0-0 oc bracing, I	Except:
:	2-17,9-11	1: 2x6 SP No.2, 5-16,5-	14,6-12: 2x4 SP No.2				6-0-0 o	c bracing	: 12-14.	0,	•
					WEBS		1 Row	at midpt	5	-14, 6-14	

REACTIONS. (size) 17=0-3-8, 11=0-3-8, 14=0-3-8 Max Horz 17=-317(LC 10) Max Uplift 17=-173(LC 12), 11=-173(LC 13), 14=-100(LC 12) Max Grav 17=791(LC 19), 11=598(LC 24), 14=2326(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-17=-684/224, 2-3=-821/226, 3-5=-668/344, 5-6=0/506, 6-8=-410/352, 8-9=-513/234, 9-11=-496/230
- BOT CHORD 16-17=-245/740, 12-14=-288/132, 11-12=-75/322
- WEBS 3-16=-439/326, 5-16=-169/938, 5-14=-1071/218, 6-14=-1049/223, 6-12=-206/840, 8-12=-476/323

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-3 to 2-3-13, Interior(1) 2-3-13 to 15-7-8, Exterior(2) 15-7-8 to 19-10-7, Interior(1) 19-10-7 to 25-4-8, Exterior(2) 25-4-8 to 29-7-7, Interior(1) 29-7-7 to 41-8-3 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 17, 11, and 14. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Edenton, NC 27932



	10-4-12	20-6-0	22-4-8	30-7-4		41-0-0	1
	10-4-12	10-1-4	'1-10-8'	8-2-12		10-4-12	l
Plate Offsets (X,Y)	_[2:0-3-0,0-1-12], [5:0-4-0,0-2-12], [6:0-4	-0,0-2-12], [9:0-3-0,0-1-12]					
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	<b>CSI.</b> TC 0.92 BC 0.82 WB 0.75 Matrix-MS	DEFL. Vert(LL) -0.: Vert(CT) -0.: Horz(CT) 0.0 Wind(LL) 0.0	in (loc) 16 19-21 28 19-21 05 11 07 19-21	l/defl L/d >999 360 >987 240 n/a n/a >999 240	PLATES MT20 Weight: 627 lb	<b>GRIP</b> 244/190 FT = 20%
2022 1010					210		2070
LUMBER- TOP CHORD 2x6 S BOT CHORD 2x6 S WEBS 2x4 S OTHERS 2x4 S	P No.2 P No.2 P No.3 P No.3		BRACING- TOP CHORD BOT CHORD WEBS	Structu except Rigid c 1 Row	ral wood sheathing of end verticals, and 2- eiling directly applied at midpt	directly applied or 5-2-11 0-0 oc purlins (5-5-11 m d or 10-0-0 oc bracing. 4-22, 5-19, 6-19, 6-12, 7	oc purlins, ax.): 5-6. '-11
REACTIONS. All b (lb) - Max Max	vearings 6-11-0 except (jt=length) 22=0-3 Horz 22=-317(LC 10) Uplift All uplift 100 lb or less at joint(s) 1 17=-419(LC 1), 14=-504(LC 1), 13= Grav All reactions 250 lb or less at joint 15=580(LC 19), 13=1056(LC 1)	-8, 11=0-3-8. 5 except 22=-202(LC 12), · 224(LC 13) (s) 17, 14 except 22=1498(	11=-173(LC 13), 18=- (LC 2), 11=1304(LC 2	112(LC 12 0), 18=838	2), 3(LC 1),		
FORCES.         (lb) - Max           TOP CHORD         2-22           7-9	Comp./Max. Ten All forces 250 (lb) o 2=-528/210, 2-4=-558/208, 4-5=-1798/39 590/205, 9-11=-542/208 22=-261/1627, 19-21=-132/1160, 18-19=- 15=-37/1019, 13-14=-37/1019, 12-13=-37 2=-1478/97, 4-21=-369/326, 5-21=-179/8 2=-400/331, 7-11=-1064/53	less except when shown. 0, 5-6=-1078/303, 6-7=-142 37/1019, 17-18=-37/1019, /1019, 11-12=-72/1184 08, 5-19=-263/186, 6-12=-1	26/343, 15-17=-37/1019, 117/333,				
<ul> <li>NOTES-</li> <li>1) Unbalanced roof lin</li> <li>2) Wind: ASCE 7-10; gable end zone an 25-4-8, Exterior(2) exposed;C-C for m</li> <li>3) Truss designed for Gable End Details</li> <li>4) Provide adequate of 5) All plates are 2x4 M</li> <li>6) Gable studs space</li> <li>7) This truss has been</li> <li>8) * This truss has be will fit between the</li> <li>9) N/A</li> </ul>	ve loads have been considered for this de Vult=130mph Vasd=103mph; TCDL=6.0 d C-C Exterior(2) -0-9-11 to 2-2-5, Interio 25-4.8 to 29-7-7, Interior(1) 29-7-7 to 41- embers and forces & MWFRS for reactic wind loads in the plane of the truss only. as applicable, or consult qualified buildin drainage to prevent water ponding. MT20 unless otherwise indicated. d at 1-4-0 oc. In designed for a 10.0 psf bottom chord live en designed for a live load of 20.0psf on bottom chord and any other members, w	esign. sef; BCDL=6.0psf; h=32ft; ( r(1) 2-2-5 to 15-7-8, Exterior 9-11 zone; cantilever left a ns shown; Lumber DOL=1. For studs exposed to wind g designer as per ANSI/TP re load nonconcurrent with the bottom chord in all area ith BCDL = 10.0psf.	Cat. II; Exp B; Encloss or(2) 15-7-8 to 19-10- nd right exposed ; en .60 plate grip DOL=1. d (normal to the face) I 1. any other live loads. as where a rectangle :	ed; MWFR 7, Interior( d vertical li 60 see Stand 3-6-0 tall b	S (envelope) 1) 19-10-7 to eft and right dard Industry y 2-0-0 wide	SE/ 458	AROLINA AL 44
10) Graphical purlin r	epresentation does not depict the size or	the orientation of the purlin	n along the top and/or	bottom ch	ord.	TEIN	OHN

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

Continued on page 2

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February 26,2021

mmm

Job	Truss	Truss Type	Qty	Ply	McKee-Portico20CL;Lot 1011 CarriageGlen	
CC1011 P	A11C		1	1		I44981550
		GABLE	1		Job Reference (optional)	
Builders FirstSource (Apex,	NC), Apex, NC - 27523,			3.430 s Fe	b 12 2021 MiTek Industries, Inc. Fri Feb 26 11:54:21 2021	Page 2
		ID:?MdgC	82XojFIR	poD?t4wJJ	vPwGb- W2BWeVgxXBNYekzWiCnBdT7YI wvPQrs4EIFt	nzhH m

## LOAD CASE(S) Standard

Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-5=-60, 5-6=-60, 6-9=-60, 9-10=-60, 18-22=-20, 18-102=-165(F=-145), 11-102=-20





L	4-5-4 10-1-2		15-9-0	20-2-0	24-7-0
	4-5-4 5-7-14		5-7-14	4-5-0	4-5-0
Plate Offsets (X,Y)	[3:0-5-12,0-2-0], [11:0-2-8,0-2-12]				
<i>. .</i>					
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.52	Vert(LL) -0.07	11 >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.62	Vert(CT) -0.14	11 >999 240	
BCLL 0.0 *	Ren Stress Incr NO	WB 0.56	Horz(CT) 0.03	9 n/a n/a	
BCDI 10.0	Code IRC2015/TPI2014	Matrix-MS	Wind(LL) 0.07	11 \0999 240	Weight: 341 lb FT - 20%
DODE 10.0	0000 11(02013/11/12014	Matrix Mo	Wind(EE) 0.07	11 2000 240	Weight: 54115 11 = 20%
			BBACING-		
			TOD CHODD	Ctructurel wood ab a athing	directly expliced or C.O.O. on purling
TOP CHORD 2X4 S			TOP CHORD	Structural wood sheathing	directly applied of 6-0-0 oc putlins,
BOT CHORD 2x6 S	P No.2 *Except*			except end verticals, and 2	2-0-0 oc purlins (6-0-0 max.): 3-8.
6-12:	2x4 SP No.3		BOT CHORD	Rigid ceiling directly applie	ed or 10-0-0 oc bracing.
WEBS 2x4 S	P No.3				
REACTIONS. (siz	ze) 9=Mechanical, 15=0-3-8				
Max I	Horz 15=152(LC 5)				
Max I	Jplift 9=-410(LC 5), 15=-315(LC 5)				
Max	Grav 9=2098(LC 1), 15=2098(LC 1)				
FORCES. (lb) - Max	. Comp./Max. Ten All forces 250 (lb) o	less except when shown			
TOP CHORD 2-3=	-2379/405 3-4=-3101/588 4-6=-3806/7	34 6-7=-3833/737 2-15=	-1976/322		
BOT CHORD 13-1	4=-412/1894 12-13=-86/454 6-11=-618	195 10-11=-511/2492 9	-10=-511/2492		
WEBS 3-13	R=_337/1528 /_13=_1181/336 11_13=_5	1/2725 1-11-165/777 7	-11-332/1619		
7 10	-0/225 7 0 - 2026/597 2 14 - 221/1913	1/2/23, 4-11=-103/777, 7	-11=-332/1019,		
7-10	=0/323, 7-9=-3020/387, 2-14=-321/1813				
NOTES					
NOTES-					
1) 2-ply truss to be co	nnected together with 10d (0.131"x3") na	alls as follows:			
Top chords connect	ted as follows: 2x4 - 1 row at 0-9-0 oc.				
Bottom chords con	nected as follows: 2x6 - 2 rows staggere	d at 0-9-0 oc, 2x4 - 1 row :	at 0-9-0 oc.		
Webs connected as	s follows: 2x4 - 1 row at 0-9-0 oc.				
2) All loads are consid	lered equally applied to all plies, except	f noted as front (F) or bac	k (B) face in the LOAD C	ASE(S) section. Ply to	
ply connections have	ve been provided to distribute only loads	noted as (F) or (B), unless	s otherwise indicated.		
3) Unbalanced roof liv	e loads have been considered for this de	sian.			IN CAD III
4) Wind: ASCE 7-10:	Vult=130mph Vasd=103mph; TCDI =6.0	nsf: BCDI =6 0nsf: h=32ft:	Cat II: Exp B: Enclosed:	MWFRS (envelope)	N'TH UAROUN
able end zone: ca	ntilever left and right exposed : end verti	cal left and right exposed.	Lumber $DOI = 1.60$ plate	arin DOI = 1.60	Not weather late
5) Provide adequate of	trainage to prevent water ponding	sai lon and ngin expected,		grip 202-1.00	C SEESPIDIN V
6) This trues has been	a designed for a 10.0 pat better abord li	a load popoonaurrant with	a any other live loads		Minu
<ul> <li>This trues has been</li> </ul>	r designed for a 10.0 psi bollom chord in	the bettern should in all are	Tariy other live loads.	O tall by 2.0.0 wide	
7) " This truss has been the	en designed for a live load of 20.0psf on	the bottom chord in all are	eas where a rectangle 3-6	-0 tall by 2-0-0 wide	S 2 0 0 1 1 2
will fit between the	bottom chord and any other members.				= : SEAL : =
8) Refer to girder(s) to	or truss to truss connections.				
<ol><li>Provide mechanica</li></ol>	I connection (by others) of truss to bearing	ng plate capable of withsta	anding 100 lb uplift at join	t(s) except (jt=lb)	: 40044 : :
9=410.					12 N 1 S
10) One RT7A USP c	onnectors recommended to connect trus	s to bearing walls due to l	JPLIFT at jt(s) 15. This co	onnection is for uplift	The second s
only and does not	consider lateral forces.				1 A. SNOWLER SS
11) Graphical purlin re	epresentation does not depict the size or	the orientation of the purl	in along the top and/or bo	ottom chord.	SUNEY CUN
12) Hanger(s) or othe	r connection device(s) shall be provided	sufficient to support conce	entrated load(s) 175 lb do	wn and 30 lb up at	1 PEW OUN
2-6-0 on bottom c	hord. The design/selection of such conr	ection device(s) is the res	ponsibility of others.		W JUT
					THILIN NY STREET
LOAD CASE(S) Stor	odard				February 26 2021
Continued or reserved	lualu				rebluary 20,2021
Continued on page 2					

All Tek Attiliate

Job	Truss	Truss Type	Qty	Ply	McKee-Portico20CL;Lot 1011 CarriageGlen	
						I44981551
CG1011-R	B01GR	MONO HIP	1	2		
				_	Job Reference (optional)	
Builders FirstSource (Apex, I	NC), Apex, NC - 27523,			8.430 s Fe	b 12 2021 MiTek Industries, Inc. Fri Feb 26 11:54:22 2021	Page 2

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## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-16=-60, 3-16=-129(F=-69), 3-8=-129(F=-69), 15-19=-20, 12-19=-44(F=-24), 9-11=-44(F=-24) Concentrated Loads (lb)

Vert: 19=-155(F)





L	6-5-4	15-9-0		1	24-7-0		
I	6-5-4	9-3-12		1	8-10-0		1
Plate Offsets (X,Y)	[2:0-3-0,0-1-12], [3:0-3-12,0-2-	0], [10:0-4-12,Edge], [11:Edge,0-1-8]					
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0. Plate Grip DOL 1.1 Lumber DOL 1.1 Rep Stress Incr YE Code IRC2015/TPI2014	-0 <b>CSI.</b> 15 TC 0.65 15 BC 0.79 15 WB 0.80 14 Matrix-MS	DEFL.         in           Vert(LL)         -0.22           Vert(CT)         -0.45           Horz(CT)         0.03           Wind(LL)         0.04	(loc) // 9-10 > 9-10 > 9 10 >	/defl L/d 999 360 654 240 n/a n/a 999 240	PLATES MT20 MT20HS Weight: 159 lb	<b>GRIP</b> 244/190 187/143 FT = 20%
LUMBER-			BRACING-				

	DIVAGING	
2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 4-11-0 oc purlins,
2x4 SP No.2 *Except*		except end verticals, and 2-0-0 oc purlins (5-1-11 max.): 3-8.
6-11: 2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 9-9-0 oc bracing.
2x4 SP No.3		
	2x4 SP No.2 2x4 SP No.2 *Except* 6-11: 2x4 SP No.3 2x4 SP No.3	2x4 SP No.2TOP CHORD2x4 SP No.2 *Except*6-11: 2x4 SP No.36-11: 2x4 SP No.3BOT CHORD2x4 SP No.3SOT CHORD

REACTIONS. (size) 9=Mechanical, 13=0-3-8 Max Horz 13=206(LC 9) Max Uplift 9=-197(LC 9), 13=-75(LC 9) Max Grav 9=970(LC 1), 13=1042(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-1152/184, 3-4=-879/195, 4-6=-1228/236, 6-7=-1247/234, 2-13=-988/214
- BOT CHORD 12-13=-351/434, 6-10=-264/119, 9-10=-221/754
- WEBS 3-12=-23/347, 4-12=-496/204, 10-12=-280/1081, 7-10=-90/693, 7-9=-1064/278, 2-12=-113/712

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 6-5-4, Exterior(2) 6-5-4 to 10-8-3, Interior(1) 10-8-3 to 24-5-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) All plates are MT20 plates unless otherwise indicated.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13 except (jt=lb) 9=197.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSIVTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

ENGINEERING BY RENCO A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932



	8-5-4	1	7-3-12	•	8-10-0		
Plate Offsets (X,Y)	[4:0-2-0,0-2-12], [6:0-3-0,0-2-3], [9:0-4-2	2,Edge], [10:0-2-4,0-2-12]	]				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.53 BC 0.71 WB 0.65 Matrix-MS	DEFL. ir Vert(LL) -0.14 Vert(CT) -0.29 Horz(CT) 0.03 Wind(LL) 0.04	n (loc) l/defl l 10-16 >999 9 10-16 >997 3 9 n/a l 10 >999	L/d 360 240 n/a 240	<b>PLATES</b> MT20 Weight: 159 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SF 4-6: 2x BOT CHORD 2x4 SF 5-11: 2 WEBS 2x4 SF SLIDER Right 2	P No.2 *Except* 6 SP No.2 P No.2 *Except* x4 SP No.3 P No.3 2x4 SP No.3 -t 2-3-14		BRACING- TOP CHORD BOT CHORD	Structural wood except end vert Rigid ceiling dir 6-0-0 oc bracing	I sheathing dired icals, and 2-0-0 ectly applied or g: 11-12.	ctly applied or 5-0-14 oc purlins (6-0-0 ma 10-0-0 oc bracing, I	oc purlins, x.): 4-6. Except:
REACTIONS. (size Max H Max U Max G	e) 9=Mechanical, 13=0-3-8 lorz 13=-180(LC 10)  plift 9=-115(LC 13), 13=-140(LC 12)  rav 9=976(LC 1), 13=1048(LC 1)						
FORCES. (lb) - Max. TOP CHORD 3-4=- 2-13=	Comp./Max. Ten All forces 250 (lb) of 1046/222, 4-5=-945/238, 5-6=-902/228, 271/136	less except when shown 6-7=-1152/235, 7-9=-126	ı. 68/247,				
BOT CHORD 12-13 WEBS 10-12	3=-174/820, 5-10=0/346, 9-10=-135/105 2=-72/825, 3-13=-1014/131	5					
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live</li> <li>2) Wind: ASCE 7-10; V gable end zone and Exterior(2) 16-5-4 to exposed;C-C for me</li> <li>3) Provide adequate di</li> <li>4) This truss has been will fit between the b</li> <li>6) Refer to girder(s) for</li> <li>7) Provide mechanical 9=115.</li> <li>8) One RT7A USP con only and does not co</li> <li>9) Graphical purlin repr</li> </ul>	e loads have been considered for this de /ult=130mph Vasd=103mph; TCDL=6.0 C-C Exterior(2) -1-0-0 to 2-0-0, Interior( 20-8-5, Interior(1) 20-8-5 to 24-7-0 zon mbers and forces & MWFRS for reactio rainage to prevent water ponding. designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on vottom chord and any other members. truss to truss connections. connection (by others) of truss to bearin unectors recommended to connect truss onsider lateral forces. resentation does not depict the size or th	sign. sif; BCDL=6.0psf; h=32ft; 1) 2-0-0 to 8-5-4, Exterior e; cantilever left and right ns shown; Lumber DOL= e load nonconcurrent with he bottom chord in all are ig plate capable of withsta to bearing walls due to U ne orientation of the purlin	; Cat. II; Exp B; Enclosed (2) 8-5-4 to 12-8-3, Inter exposed ; end vertical le 1.60 plate grip DOL=1.6( h any other live loads. eas where a rectangle 3- anding 100 lb uplift at join PLIFT at jt(s) 13. This co h along the top and/or bo	t; MWFRS (envel ior(1) 12-8-3 to 1 eft and right 0 6-0 tall by 2-0-0 v nt(s) except (jt=lb onnection is for up ttom chord.	ope) 6-5-4, vide	SE/ 458	AROLINA AL 44

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89** and **BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

# February 26,2021





Plate Offse	ts (X,Y)	[4:0-2-4,0-2-12], [5:Edge,	)-3-8], [9:0-3-	10,0-0-3], [10:0-2-12,0-2-12]							
LOADING TCLL TCDL BCLL BCDL	(psf) 20.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC 0.64 BC 0.91 WB 0.67 Matrix MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.29 12 -0.59 12 0.04 0.06	(loc) 2-13 2-13 9	l/defl >999 >497 n/a	L/d 360 240 n/a 240	PLATES MT20	<b>GRIP</b> 244/190
LUMBER-	10.0		12014	Wattix-Wi3	BRACING	0.00		>999	240	Weight. 139 lb	FT = 2078

 TOP CHORD
 2x4 SP No.2 \*Except\*
 TOP CHORD
 Structural wood sheathing directly applied or 4-9-13 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-5.

 BOT CHORD
 2x4 SP No.2 \*Except\*
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 11-12.

 WEBS
 2x4 SP No.3
 SLIDER
 Right 2x4 SP No.3 + 2-3-14

REACTIONS. (size) 9=Mechanical, 13=0-3-8 Max Horz 13=-213(LC 10) Max Uplift 9=-114(LC 13), 13=-139(LC 12) Max Grav 9=976(LC 1), 13=1048(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-4=-1004/214, 4-5=-847/246, 5-6=-843/242, 6-7=-1150/224, 7-9=-1292/247

BOT CHORD 12-13=-220/840, 6-10=-12/304, 9-10=-136/1094

WEBS 4-12=0/262, 10-12=0/715, 4-10=-91/252, 7-10=-304/175, 3-13=-1033/195

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 1-0-0 to 2-0-0, Interior(1) 2-0-0 to 10-5-4, Exterior(2) 10-5-4 to 18-8-3, Interior(1) 18-8-3 to 24-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=114.

 One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13. This connection is for uplift only and does not consider lateral forces.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







REACTIONS. (size) 9=Mechanical, 13=0-3-8 Max Horz 13=267(LC 9) Max Uplift 9=-105(LC 13), 13=-128(LC 12) Max Grav 9=970(LC 1), 13=1062(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-328/164, 3-5=-1127/253, 5-7=-1083/254, 2-13=-364/178

BOT CHORD 12-13=-158/1046, 10-12=0/728, 9-10=-89/861

WFBS 5-10=-146/481, 7-10=-239/255, 5-12=-151/574, 3-12=-266/263, 3-13=-1034/42, 7-9=-1069/87

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 12-5-4, Exterior(2) 12-5-4 to 15-5-4, Interior(1) 15-5-4 to 24-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=105

7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13. This connection is for uplift only and does not consider lateral forces.

![](_page_16_Figure_15.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_17_Figure_0.jpeg)

818 Soundside Road Edenton, NC 27932

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

ᄊ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORF USE

Job	Truss	Truss Type	Qty	Ply	McKee-Portico20CL;Lot 1011 CarriageGlen	-
CG1011-R	BOSH	SPECIAL	1	1	1449	381556
	50311		1		Job Reference (optional)	

Builders FirstSource (Apex, NC), Apex, NC - 27523,

8.430 s Feb 12 2021 MiTek Industries, Inc. Fri Feb 26 11:54:27 2021 Page 2 ID:?MdgC82XojFIRgoD?t4wJJyPwGb-pgPSnhZbXNxXGZC7tzJBQuj9\_A2oM4wkE0hcTKzhH\_g

#### LOAD CASE(S) Standard

- 3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
- Uniform Loads (plf) Vert: 1-2=-20, 2-6=-20, 6-9=-20, 9-10=-20, 11-16=-40, 17-18=-40
- 18) Dead + Uninhabitable Attic Storage: Lumber Increase=1.25, Plate Increase=1.25
- Uniform Loads (plf)
- Vert: 1-2=-20, 2-6=-20, 6-9=-20, 9-10=-20, 13-16=-20, 13-24=-60, 24-25=-20, 25-26=-60, 11-26=-20, 17-18=-40
- 19) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-56, 2-6=-61, 6-9=-42, 9-10=-37, 13-16=-20, 13-24=-50, 24-25=-20, 25-26=-50, 11-26=-20, 17-18=-30

- Horz: 2-16=21, 1-2=6, 2-6=11, 6-9=8, 9-10=13, 9-11=7
- 20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-37, 2-6=-42, 6-9=-61, 9-10=-56, 13-16=-20, 13-24=-50, 24-25=-20, 25-26=-50, 11-26=-20, 17-18=-30

- Horz: 2-16=-7, 1-2=-13, 2-6=-8, 6-9=-11, 9-10=-6, 9-11=-21
- 21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-25, 2-22=-29, 6-22=-38, 6-9=-46, 9-10=-41, 13-16=-20, 13-24=-50, 24-25=-20, 25-26=-50, 11-26=-20, 17-18=-30

- Horz: 2-16=19, 1-2=-25, 2-22=-21, 6-22=-12, 6-9=4, 9-10=9, 9-11=3
- 22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-41, 2-6=-46, 6-8=-38, 8-9=-29, 9-10=-25, 13-16=-20, 13-24=-50, 24-25=-20, 25-26=-50, 11-26=-20, 17-18=-30

Horz: 2-16=-3, 1-2=-9, 2-6=-4, 6-8=12, 8-9=21, 9-10=25, 9-11=-19

25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-50, 2-6=-50, 6-9=-20, 9-10=-20, 13-16=-20, 13-24=-50, 24-25=-20, 25-26=-50, 11-26=-20, 17-18=-30 26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-20, 2-6=-20, 6-9=-50, 9-10=-50, 13-16=-20, 13-24=-50, 24-25=-20, 25-26=-50, 11-26=-20, 17-18=-30

![](_page_18_Picture_26.jpeg)

![](_page_19_Figure_0.jpeg)

	8-4-11		14-7-0	20-7-0	22-7-0	24-7-0	
	' 8-4-11	1	6-2-5	6-0-0	2-0-0	2-0-0	
Plate Offsets (X,Y)	[10:0-3-10,0-0-3], [11:0-4-12,0-3-4], [13	:0-4-0,0-3-0]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.48 BC 0.89 WB 0.35 Matrix-MS	DEFL. Vert(LL) -0. Vert(CT) -0. Horz(CT) 0. Wind(LL) 0.	in (loc) l/defl 21 11-17 >999 43 11-17 >681 04 10 n/a 03 6 >999	L/d 360 240 n/a 240	<b>PLATES</b> MT20 Weight: 164 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-			BRACING-			_	
TOP CHORD 2x4 BOT CHORD 2x4	SP No.2 SP No.2 *Except*		TOP CHORD	Structural wood except end verti	sheathing dir cals.	ectly applied or 4-11-1	oc purlins,
6-12 WEBS 2x4	2: 2x4 SP No.3 SP No.3		BOT CHORD	Rigid ceiling dire 6-0-0 oc bracing	ectly applied o g: 12-13.	or 10-0-0 oc bracing,	Except:
SLIDER Righ	nt 2x4 SP No.3 -t 1-11-12		WEBS	1 Row at midpt	3	-14	
REACTIONS. (a Max Max Max	size) 14=0-3-8, 10=Mechanical < Horz 14=-246(LC 10) < Uplift 14=-72(LC 12), 10=-50(LC 13) < Grav 14=1040(LC 1), 10=976(LC 1)						

 
 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-14=-358/188, 2-3=-327/177, 3-5=-1062/260, 5-6=-1066/293, 6-8=-1114/198, 8-10=-1289/229

 BOT CHORD
 13-14=-105/915, 6-11=-298/184, 10-11=-120/1098

 WEBS
 3-14=-952/39, 3-13=-271/223, 5-13=-126/360, 11-13=0/697, 5-11=-149/672, 8-11=-324/180

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 12-5-4, Exterior(2) 12-5-4 to 16-8-3, Interior(1) 16-8-3 to 24-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10.

7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14. This connection is for uplift only and does not consider lateral forces.

![](_page_19_Figure_11.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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Edenton, NC 27932

![](_page_20_Figure_0.jpeg)

#### Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	McKee-Portico20CL;Lot 1011 CarriageGlen
					144981558
CG1011-R	B07H	FINK	1		
		07500		0.400	Job Reference (optional)
Builders FirstSource (	Apex, NC), Apex, NC	- 27523,		8.430 S	Feb 12 2021 MITEK Industries, Inc. Fri Feb 26 11:54:30 2021 Page 2
			ID. (WagCoz)	NOFIRGOD	?14wJJYPWGD-EF5DPJCOQIK570w1f5lu2WLgIN222RCAw_VG3l2IIII_u
LOAD CASE(S) Sta	andard				
2) Dead + 0.75 Roof	Live (balanced) + 0.75 U	ninhab. Attic Storage: Lumber Increase="	1.15. Plate Increase=1.	15	
Uniform Loads (pl	f)				
Vert: 1-2=	, =-50, 2-6=-50, 6-9=-50, 9-	10=-50, 10-11=-50, 14-17=-20, 14-25=-5	0, 12-25=-20, 18-19=-3	30	
3) Dead + Uninhabita	able Attic Without Storage	: Lumber Increase=1.25, Plate Increase=	=1.25		
Uniform Loads (pl	f)				
Vert: 1-2=	-20, 2-6=-20, 6-9=-20, 9-	10=-20, 10-11=-20, 12-17=-40, 18-19=-4	0		
18) Dead + Uninhab	itable Attic Storage: Lumb	er Increase=1.25, Plate Increase=1.25			
Uniform Loads (p	olf)				
Vert: 1-2	2=-20, 2-6=-20, 6-9=-20, 9	9-10=-20, 10-11=-20, 14-17=-20, 14-25=-	60, 12-25=-20, 18-19=	-40	
19) Dead + 0.75 Roo	of Live (bal.) + 0.75 Uninh	ab. Attic Storage + 0.75(0.6 MWFRS Win	nd (Neg. Int) Left): Lum	ber Increa	se=1.60, Plate Increase=1.60
Uniform Loads (p	olf)				
Vert: 1-2	2=-56, 2-6=-61, 6-9=-42, 9	9-10=-40, 10-11=-36, 14-17=-20, 14-25=-	50, 12-25=-20, 18-19=	-30	
Horz: 1-	2=6, 2-6=11, 6-9=8, 9-10	=10, 10-11=14, 2-17=21, 10-12=7			
20) Dead + 0.75 Roo	of Live (bal.) + 0.75 Uninh	ab. Attic Storage + 0.75(0.6 MWFRS Win	nd (Neg. Int) Right): Lur	mber Incre	ease=1.60, Plate Increase=1.60
Uniform Loads (	DIT)		50 40 05 00 40 40	00	
Vert: 1-2	2=-37, 2-6=-42, 6-9=-61, 9	-10=-29, 10-11=-25, 14-17=-20, 14-25=-	50, 12-25=-20, 18-19=	-30	
HOIZ: 1-	Z=-13, Z-b=-8, 6-9=-11, 9	-10=21, 10-11=25, 2-17=-7, 10-12=-21	d (New Just) det Develle		r Inereses 1.60 Plate Inereses 1.60
21) Dead + 0.75 Rod	DI LIVE (Dal.) + 0.75 Uninn	ab. Allic Storage + 0.75(0.6 MWFRS WIT	id (Neg. Int) 1st Paralle	ei): Lumber	r increase=1.60, Plate increase=1.60
Vort: 1-	) )25 2-620 6-042 (		50 12-25-20 18-10-	-30	
Horz: 1-	2-25, 2-0-29, 0-9-42, 3	10-8 10-11-13 2-17-19 10-12-6	50, 12-25-20, 10-13-	-50	
22) Dead + 0.75 Roc	of Live (bal.) + 0.75 Uninh	ab Attic Storage + 0.75(0.6 MWERS Win	nd (Neg. Int) 2nd Parall	el): Lumbe	er Increase=1.60. Plate Increase=1.60
Uniform Loads (r	olf)		ia (itog. iii) zila i alain		
Vert: 1-2	2=-37. 2-6=-42. 6-9=-29. 9	9-10=-29. 10-11=-25. 14-17=-20. 14-25=-	50. 12-25=-20. 18-19=	-30	
Horz: 1-	2=-13, 2-6=-8, 6-9=21, 9-	10=21, 10-11=25, 2-17=-6, 10-12=-19	,,		
25) 3rd Dead + 0.75	Roof Live (unbalanced) +	0.75 Uninhab. Attic Storage: Lumber Inc	rease=1.15, Plate Incr	ease=1.15	5
Uniform Loads (	olf)	5	,		
Vert: 1-2	2=-50, 2-6=-50, 6-9=-20, 9	9-10=-20, 10-11=-20, 14-17=-20, 14-25=-	50, 12-25=-20, 18-19=	-30	
26) 4th Dead + 0.75	Roof Live (unbalanced) +	0.75 Uninhab. Attic Storage: Lumber Inc	rease=1.15, Plate Incre	ease=1.15	5
Uniform Loads (p	olf)				

Vert: 1-2=-20, 2-6=-20, 6-9=-50, 9-10=-50, 10-11=-50, 14-17=-20, 14-25=-50, 12-25=-20, 18-19=-30

![](_page_21_Picture_3.jpeg)

![](_page_22_Figure_0.jpeg)

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS OTHERS 2x4 SP No.3 TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 13-0-0.

(lb) -

Max Horz 28=-195(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 23, 24, 25, 26, 21 except 28=-314(LC 19), 27=-235(LC 9), 20=-199(LC 24), 19=-213(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 23, 24, 25, 26, 21, 20 except 28=260(LC 9), 22=261(LC 1), 27=433(LC 19), 19=563(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

```
TOP CHORD
              2-3=-180/321
WEBS
              11-19=-266/212
```

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -1-0-0 to 2-0-0, Exterior(2) 2-0-0 to 7-10-0, Corner(3) 7-10-0 to 10-10-0, Exterior(2) 10-10-0 to 16-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

6) Gable studs spaced at 1-4-0 oc.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

![](_page_22_Figure_21.jpeg)

818 Soundside Road Edenton, NC 27932

![](_page_23_Figure_0.jpeg)

BCDL 10.0	Code IRC2015/TPI2014	Matrix-MR	Wind(LL)	0.01	7-8	>999	240	Weight: 70 lb	FT = 2
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SE	2 No.2		BRACING- TOP CHORI	D	Structu	iral wood	l sheathing d	irectly applied or 2-2-0 c	oc purlins,

except end verticals. 2x6 SP No.2 \*Except\* BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 3-8: 2x4 SP No.3

REACTIONS. (size) 9=0-3-8, 6=0-3-0, 7=0-4-15 Max Horz 9=-197(LC 10) Max Uplift 9=-79(LC 12), 6=-157(LC 12), 7=REL Max Grav 9=737(LC 19), 6=693(LC 19), 7=328(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 2-9=-632/198, 2-3=-676/134, 3-4=-700/138, 4-6=-621/201 TOP CHORD
- BOT CHORD 8-9=-23/518, 7-8=-23/518, 6-7=-23/518
- WEBS 3-8=0/363

#### NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 7-10-0, Exterior(2) 7-10-0 to 12-0-15, Interior(1) 12-0-15 to 16-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) N/A

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9 and 6. This connection is for

uplift only and does not consider lateral forces.

7) "A" indicates Released bearing: allow for upward movement at joint(s) 7.

![](_page_23_Figure_16.jpeg)

FT = 20%

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_24_Figure_0.jpeg)

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -1-0-0 to 2-0-12, Interior(1) 2-0-12 to 7-10-0, Exterior(2) 7-10-0 to 12-0-15, Interior(1) 12-0-15 to 16-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 8. This connection is for uplift only and does not consider lateral forces.

![](_page_24_Figure_5.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system. See **MSIVTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_25_Figure_0.jpeg)

Job	Truss	Truss Type	Qty	Ply	McKee-Portico20CL;Lot 1011 CarriageGlen	
CG1011-R	C06TGR	SPECIAL	5		144981	562
				<b>Z</b>	Job Reference (optional)	

8.430 s Nov 30 2020 MiTek Industries, Inc. Fri Feb 26 12:39:58 2021 Page 2 ID:?MdgC82XojFIRgoD?t4wJJyPwGb-Ri9xhSMcKm4wjXg\_QYODMIKyO287kC\_jEPkcqYzhGK?

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-5=-60, 5-7=-60, 14-15=-20, 12-13=-20, 10-12=-508(F=-488), 8-9=-508(F=-488)

Concentrated Loads (lb) Vert: 12=-2098(F)

![](_page_26_Picture_7.jpeg)

![](_page_27_Figure_0.jpeg)

	0-Q-4	7-10-0	15-7-12	15-8-0					
	0-0-4	7-9-12	7-9-12	0-0-4					
Plate Offsets (X,Y)	Plate Offsets (X,Y) [2:0-1-0,0-1-8], [9:0-0-0,0-0-0], [10:0-0-0,0-0-0], [11:0-0-0,0-0-0], [12:0-0-0,0-0-0], [13:0-0-0,0-0-0], [14:0-1-0,0-1-8], [22:0-3-0,0-3-0]								

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.66 BC 0.64 WB 0.12 Matrix-MR	DEFL.         in           Vert(LL)         -0.13           Vert(CT)         -0.20           Horz(CT)         0.01           Wind(LL)         0.17	(loc) l/defl L/d 19-20 >999 360 19-20 >917 240 16 n/a n/a 25 >999 240	PLATES         GRIP           MT20         244/190           Weight: 122 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x6 SF 8-22:2	P No.2 P No.2 P No.2 *Except* v4 SP No 3		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied	irectly applied or 5-6-4 oc purlins, or 10-0-0 oc bracing.
OTHERS 2x4 SF	° No.3				
REACTIONS. (siz Max H Max U Max G	e) 28=0-3-0, 16=0-3-0 lorz 28=195(LC 11)  plift 28=-65(LC 12), 16=-65(LC 13)  rav 28=675(LC 1), 16=675(LC 1)				
FORCES. (Ib) - Max.	Comp./Max. Ten All forces 250 (lb) or	less except when shown.			
TOP CHORD 2-28= 7-8=- 12-1:	=-572/116, 2-3=-608/55, 3-4=-545/75, 4- -509/193, 8-9=-509/193, 9-10=-490/156, 3=-545/75, 13-14=-608/55, 14-16=-572/	5=-520/104, 5-6=-492/130, 10-11=-492/130, 11-12=-52 116	6-7=-490/156, 20/104,		
BOT CHORD 27-28 21-2 16-1	3=-0/417, 26-27=-0/417, 25-26=-0/417, 2 2=-0/417, 20-21=-0/417, 19-20=-0/417, 7=-0/417	24-25=-0/417, 23-24=-0/417 18-19=-0/417, 17-18=-0/417	, 22-23=-0/417, 7,		
WEBS 8-22	=-132/366				

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 7-10-0, Exterior(2) 7-10-0 to 11-10-0, Interior(1) 11-10-0 to 16-6-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated. 5) Gable studs spaced at 1-4-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- 7)
- will fit between the bottom chord and any other members. 8) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 28 and 16. This connection is for uplift only and does not consider lateral forces.

![](_page_27_Figure_12.jpeg)

818 Soundside Road Edenton, NC 27932

![](_page_28_Figure_0.jpeg)

Plate Offsets (X,Y)-	[2:0-4-3,Edge], [4:0-4-3,0-0-0], [7:0-3-0	,0-3-0]	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.         in (loc)         l/defl         L/d         PLATES         GRIP           Vert(L)         0.42         7.9         000         200         010         010         014400
TCDL 10.0	Lumber DOL 1.15	BC 0.55	Vert(CT) -0.21 6-7 >867 240
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MR	Horz(CT) = 0.01 + 6 = n/a = n/a Wind(LL) -0.08 7-8 >999 240 Weight: 69 lb FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins,
BOT CHORD	2x4 SP No.2		except end verticals.
WEBS	2x6 SP No.2 *Except*	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
	3-7: 2x4 SP No.3		

REACTIONS. (size) 8=0-3-0, 6=0-3-0 Max Horz 8=195(LC 11) Max Uplift 8=-65(LC 12), 6=-65(LC 13) Max Grav 8=729(LC 19), 6=729(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-8=-627/192, 2-3=-708/134, 3-4=-707/134, 4-6=-627/192

BOT CHORD 7-8=0/518, 6-7=0/518 WEBS 3-7=0/378

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 7-10-0, Exterior(2) 7-10-0 to 12-0-15, Interior(1) 12-0-15 to 16-6-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8 and 6. This connection is for uplift only and does not consider lateral forces.

![](_page_28_Figure_13.jpeg)

![](_page_28_Picture_15.jpeg)

![](_page_29_Figure_0.jpeg)

			15-4-0	—
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Ippr NO	<b>CSI.</b> TC 0.19 BC 0.11 WB 0.14	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.00         15         n/r         120           Vert(CT)         -0.01         15         n/r         120           Horz(CT)         -0.00         16         n/a         n/a	PLATES         GRIP           MT20         244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-R		Weight: 117 lb FT = 20%
			BRACING	

## LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS.** All bearings 15-4-0.

(lb) - Max Horz 28=-193(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 23, 24, 25, 26, 21, 20, 19, 18 except 28=-172(LC 8), 16=-149(LC 9), 27=-171(LC 9), 17=-152(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 28, 16, 22, 23, 24, 25, 26, 27, 21, 20, 19, 18, 17

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -1-0-0 to 2-0-0, Exterior(2) 2-0-0 to 7-8-0, Corner(3) 7-8-0 to 10-8-0, Exterior(2) 10-8-0 to 16-4-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 1-4-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

![](_page_29_Figure_21.jpeg)

![](_page_29_Picture_23.jpeg)

![](_page_30_Figure_0.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_31_Figure_0.jpeg)

Plate Olisets (X, Y)	[11:0-2-0,Edge]				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.15 BC 0.13 WB 0.10 Matrix-R	DEFL.         ii           Vert(LL)         0.00           Vert(CT)         -0.00           Horz(CT)         -0.02	n (loc) l/defl L/d 0 1 n/r 120 0 1 n/r 120 2 11 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 200 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SI BOT CHORD 2x4 SI WEBS 2x6 SI	P No.2 P No.2 P No.2		BRACING- TOP CHORD	Structural wood sheathing except end verticals.	directly applied or 6-0-0 oc purlins,

WEBS

1 Row at midpt

10-31, 12-29

REACTIONS. All bearings 22-8-0.

(lb) - Max Horz 39=248(LC 11)

2x4 SP No.3

Max Uplift All uplift 100 lb or less at joint(s) 11, 31, 32, 33, 34, 35, 36, 37, 29, 28, 27, 26, 25, 24, 23 except 39=-224(LC 8), 21=-156(LC 9), 38=-240(LC 9), 22=-165(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 21, 31, 32, 33, 34, 35, 36, 37, 29, 28, 27, 26, 25, 24, 23, 22 except 39=283(LC 20), 11=259(LC 13), 38=283(LC 10)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 9-10=-238/258, 10-11=-247/273, 11-12=-247/271, 12-13=-238/252

#### NOTES-

OTHERS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 11-4-0, Exterior(2) 11-4-0 to 14-4-0, Interior(1) 14-4-0 to 22-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 1-4-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

![](_page_31_Figure_18.jpeg)

![](_page_31_Picture_20.jpeg)

![](_page_32_Figure_0.jpeg)

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.1 WEBS 2x4 SP No.3 \*Except\* 2-11,6-7: 2x6 SP No.2

REACTIONS. (size) 7=0-3-8, 11=0-3-8 Max Horz 11=248(LC 9) Max Uplift 7=-97(LC 13), 11=-120(LC 12) Max Grav 7=887(LC 1), 11=956(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 3-4=-1001/224, 4-5=-901/221, 5-6=-1024/157, 6-7=-755/134
- BOT CHORD 10-11=-158/921, 8-10=-2/643, 7-8=-86/732
- WEBS 4-10=-120/472, 4-8=-112/349, 3-11=-1008/73

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 11-4-0, Exterior(2) 11-4-0 to 14-4-0, Interior(1) 14-4-0 to 22-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are MT20 plates unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 11. This connection is for uplift only and does not consider lateral forces.

![](_page_32_Figure_14.jpeg)

![](_page_32_Picture_16.jpeg)

![](_page_33_Figure_0.jpeg)

			12-0-0	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.14	Vert(LL) -0.00 13 n/r 120	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.09	Vert(CT) -0.00 13 n/r 120	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.07	Horz(CT) -0.00 14 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-R		Weight: 86 lb FT = 20%
LUMBER-			BRACING-	

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing.

BOT CHORD

REACTIONS. All bearings 12-0-0.

Max Horz 24=-162(LC 10) (lb) -

. . . . . .

Max Uplift All uplift 100 lb or less at joint(s) 20, 21, 22, 18, 17, 16 except 24=-199(LC 8), 14=-176(LC 9), 23=-190(LC 9), 15=-171(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 24, 14, 19, 20, 21, 22, 23, 18, 17, 16, 15

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-0-0, Interior(1) 2-0-0 to 6-0-0, Exterior(2) 6-0-0 to 9-0-0, Interior(1) 9-0-0 to 12-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 1-4-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

![](_page_33_Figure_22.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_34_Figure_0.jpeg)

REACTIONS. (size) 8=0-3-8, 6=0-3-8 Max Horz 8=162(LC 11) Max Uplift 8=-71(LC 12), 6=-71(LC 13) Max Grav 8=530(LC 1), 6=530(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-440/112, 3-4=-440/112, 2-8=-451/167, 4-6=-451/167

BOT CHORD 7-8=-5/294, 6-7=-5/294

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 6-0-0, Exterior(2) 6-0-0 to 9-0-0, Interior(1) 9-0-0 to 12-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8 and 6. This connection is for uplift only and does not consider lateral forces.

![](_page_34_Figure_11.jpeg)

818 Soundside Road Edenton, NC 27932

![](_page_35_Figure_0.jpeg)

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Inco	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.85 0.38 0.08	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.05 -0.12 0.01	(loc) 6 6-7 5	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC2015	/TPI2014	Matrix	-MR	Wind(LL)	0.03	6-7	>999	240	Weight: 52 lb	FT = 20%

#### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

BRACING-TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 7=0-3-8, 5=0-3-8 Max Horz 7=157(LC 11) Max Uplift 7=-70(LC 12), 5=-50(LC 13) Max Grav 7=532(LC 1), 5=466(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-440/110, 3-4=-433/109, 2-7=-450/165, 4-5=-378/119 TOP CHORD

BOT CHORD 6-7=-32/286, 5-6=-32/286

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 6-0-0, Exterior(2) 6-0-0 to 9-0-0, Interior(1) 9-0-0 to 11-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 5. This connection is for uplift only and does not consider lateral forces.

![](_page_35_Figure_16.jpeg)

![](_page_36_Figure_0.jpeg)

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WERE 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-5-4 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 5=176/0-3-8, 3=52/Mechanical, 4=20/Mechanical

Max Horz 5=73(LC 9)

Max Uplift 3=-59(LC 12), 4=-12(LC 12) Max Grav 5=176(LC 1), 3=66(LC 19), 4=40(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 2-5-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 3 and 12 lb uplift at joint 4.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

![](_page_36_Picture_20.jpeg)

![](_page_36_Picture_22.jpeg)

![](_page_37_Figure_0.jpeg)

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

REACTIONS. (size) 7=0-3-8, 5=Mechanical

Max Horz 7=120(LC 7)

Max Uplift 7=-61(LC 8), 5=-110(LC 5) Max Grav 7=243(LC 1), 5=155(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=110.

 One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7. This connection is for uplift only and does not consider lateral forces.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3 lb down and 39 lb up at 2-0-0, and 1 lb down and 31 lb up at 4-3-8 on top chord, and 0 lb down and 18 lb up at 2-0-12, and 0 lb down and 18 lb up at 4-3-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 5-7=-20, 1-2=-60, 2-3=-60, 3-4=-60 Concentrated Loads (lb)

Vert: 5=1(B) 8=1(B)

![](_page_37_Picture_21.jpeg)

Structural wood sheathing directly applied or 4-5-4 oc purlins,

except end verticals, and 2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied or 10-0-0 oc bracing.

![](_page_37_Picture_23.jpeg)

Job	Truss	Truss Type	0e	Qty	/	Ply	McKe	e-Portico20CL;Lot	1011 CarriageGlen	144	1981574
CG1011-R	J03	JACK		1		1	Job F	Reference (optior	nal)		
				ID:?Md	nC82X	oiFIRaoD	8.430 ?t4wJJ	s Nov 30 2020 MiT	ek Industries, Inc. Fri Fel eYMXPFgHsr65xaRc	b 26 12:41:05 2021 Pag RI36wxIEaeKTfg9zh	je 1 Glv
		F	-1-0-0	4-5-4		.,					,
			1-0-0	4-5-4							
							З п			Scale -	= 1:25.2
		Ī									
			8.00	12	/						
				/							
				6							
		φ									
		4-5	2x4								
			2								
		o 1									
		1-6					8				
		1 1	$\square$								
			5				4				
			3x4 =								
			<u> </u>	4-5-4							
				4-3-4							
LOADING (psf) TCLL 20.0	SPACING- Plate Grip DOI	2-0-0	CSI. TC 0.44	DEFL. Vert(LL)	in 0.03	(loc) 4-5	l/defl >999	L/d 240	PLATES MT20	GRIP 244/190	
TCDL 10.0	Lumber DOL	1.15	BC 0.31	Vert(CT)	-0.03	4-5	>999	240			

BCLL

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

0.0

10.0

WEBS 2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Horz(CT)

-0.07

3

n/a

Structural wood sheathing directly applied or 4-5-4 oc purlins, except end verticals.

Weight: 18 lb

FT = 20%

Rigid ceiling directly applied or 10-0-0 oc bracing.

n/a

REACTIONS. (lb/size) 5=247/0-3-8, 3=113/Mechanical, 4=47/Mechanical Max Horz 5=121(LC 12) Max Uplift 3=-99(LC 12), 4=-5(LC 12)

Rep Stress Incr

Code IRC2015/TPI2014

Max Grav 5=247(LC 1), 3=130(LC 19), 4=79(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 4-5-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB 0.00

Matrix-MR

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

YES

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 99 lb uplift at joint 3 and 5 lb uplift at joint 4.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

![](_page_38_Picture_19.jpeg)

![](_page_39_Figure_0.jpeg)

WEBS 2x4 SP No.3

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 8=248/0-3-8, 4=138/Mechanical, 5=21/Mechanical (lb/size) Max Horz 8=121(LC 12) Max Uplift 4=-117(LC 12) Max Grav 8=248(LC 1), 4=164(LC 19), 5=43(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -1-0-0 to 2-1-12, Interior(1) 2-1-12 to 4-5-0 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 4.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

![](_page_39_Picture_15.jpeg)

818 Soundside Road Edenton, NC 27932

![](_page_40_Figure_0.jpeg)

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD BOT CHORD 2x4 SP No.2 WEBS

2x4 SP No.3

REACTIONS. (size) 5=0-3-8, 4=Mechanical Max Horz 5=139(LC 9) Max Uplift 5=-45(LC 8), 4=-68(LC 8) Max Grav 5=295(LC 1), 4=295(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

1-2=-319/130BOT CHORD 4-5=-246/288

WEBS 2-4=-256/217

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 7-6-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.

6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.

![](_page_40_Picture_14.jpeg)

![](_page_40_Picture_16.jpeg)

![](_page_41_Figure_0.jpeg)

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x4 SP No 2 2x4 SP No.2 BOT CHORD WEBS

- 2x4 SP No.3
- REACTIONS. 6=0-3-8, 5=Mechanical (size) Max Horz 6=147(LC 9) Max Uplift 6=-93(LC 8), 5=-66(LC 8) Max Grav 6=369(LC 1), 5=290(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-309/107, 2-6=-288/183 TOP CHORD

BOT CHORD 5-6=-243/282

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 7-6-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 3) will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.

6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6. This connection is for uplift only and does not consider lateral forces

![](_page_41_Figure_16.jpeg)

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

![](_page_41_Picture_18.jpeg)

![](_page_42_Figure_0.jpeg)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-3-2 to 3-6-8, Exterior(2) 3-6-8 to 4-10-8, Corner(3) 4-10-8 to 7-10-8, Exterior(2) 7-10-8 to 9-5-14 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8.
   N/A
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

![](_page_42_Figure_11.jpeg)

![](_page_42_Picture_13.jpeg)

![](_page_43_Figure_0.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_43_Picture_2.jpeg)

Edenton, NC 27932

February 26,2021

![](_page_44_Figure_0.jpeg)

			13-2-0	
			13-2-0	
Plate Offsets (X,Y)	[7:0-3-0,0-3-0]			
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	<b>CSI.</b> TC 0.26	DEFL.         in         (loc)         l/defl         L/d         PLATES         GR           Vert(LL)         n/a         -         n/a         999         MT20         244	<b>IP</b> I/190
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr YES	BC 0.16 WB 0.05	Vert(CT) n/a - n/a 999 Horz(CT) 0.00 5 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Weight: 53 lb	FT = 20%
LUMBER-			BRACING-	

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3 OTHERS 2x4 SP No.3

REACTIONS. All bearings 13-2-0.

(lb) -Max Horz 1=-102(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 6=-126(LC 13), 8=-126(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 6=325(LC 20), 8=325(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 6-7-0, Exterior(2) 6-7-0 to 9-7-0, Interior(1) 9-7-0 to 12-8-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1.

![](_page_44_Figure_16.jpeg)

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_45_Figure_0.jpeg)

			<u>9-2-0</u> 9-2-0					
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. TC 0.40 BC 0.29	DEFL. Vert(LL) Vert(CT)	in (l n/a n/a	loc) l/de - n/ - n/	fl L/d /a 999 /a 999	PLATES MT20	<b>GRIP</b> 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.05 Matrix-S	Horz(CT)	0.00	3 n/	'a n/a	Weight: 32 lb	FT = 20%
LUMBER-			BRACING-					

LUMBER-

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3 OTHERS 2x4 SP No.3 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 1=9-2-0, 3=9-2-0, 4=9-2-0 (size) Max Horz 1=-69(LC 8) Max Uplift 1=-31(LC 12), 3=-40(LC 13), 4=-14(LC 12) Max Grav 1=160(LC 1), 3=160(LC 1), 4=336(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-7-0, Exterior(2) 4-7-0 to 7-7-0, Interior(1) 7-7-0 to 8-8-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

![](_page_45_Picture_15.jpeg)

![](_page_45_Picture_17.jpeg)

![](_page_46_Figure_0.jpeg)

	<u> </u>		5-2-0 5-2-0						
LOADING (psf)SPACINTCLL20.0Plate GrTCDL10.0LumberBCLL0.0 *Rep StreBCDL10.0Code IR	IG- 2-0-0 ip DOL 1.15 DOL 1.15 ess Incr YES RC2015/TPI2014	CSI. TC 0.13 BC 0.08 WB 0.02 Matrix-P	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 17 lb	<b>GRIP</b> 244/190 FT = 20%

LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3OTHERS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-2-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=5-2-0, 3=5-2-0, 4=5-2-0 Max Horz 1=35(LC 11) Max Uplift 1=-21(LC 12), 3=-26(LC 13) Max Grav 1=90(LC 1), 3=90(LC 1), 4=156(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_46_Picture_17.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

A MiTek Affilia 818 Soundside Road Edenton, NC 27932

![](_page_47_Figure_0.jpeg)

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=7-0-12, 5=7-0-12, 6=7-0-12 Max Horz 1=126(LC 9) Max Uplift 5=-15(LC 13), 6=-100(LC 12) Max Grav 1=116(LC 20), 5=118(LC 1), 6=315(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-6=-253/168

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-6-6, Interior(1) 3-6-6 to 5-9-8, Exterior(2) 5-9-8 to 6-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

![](_page_47_Figure_12.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_48_Figure_0.jpeg)

	F		<u>5-0-12</u> 5-0-12			—		
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.35 BC 0.14 WB 0.03 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) 0	in (loc) n/a - n/a - .00 4	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 21 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-	2 No 3		BRACING-	Structu	ral wood	sheathing di	rectly applied or 5-0-1	2 oc purlins

BOT CHORD

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD 2x4 SP No.3 WEBS OTHERS 2x4 SP No.3

REACTIONS. (size) 1=5-0-12, 4=5-0-12, 5=5-0-12

Max Horz 1=75(LC 9) Max Uplift 1=-21(LC 12), 4=-26(LC 8), 5=-3(LC 12)

Max Grav 1=126(LC 1), 4=39(LC 20), 5=214(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 3-9-8, Exterior(2) 3-9-8 to 4-11-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1.

Structural wood sheathing directly applied or 5-0-12 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

![](_page_48_Picture_17.jpeg)

WWWWWWWWWW Summer in SEAL unun February 26,2021

![](_page_49_Figure_0.jpeg)

2x4 //

2x4

Structural wood sheathing directly applied or 3-0-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

	⊢				3-0-12						
<b>DI : 07 : 000</b>					3-0-12					1	
Plate Offsets (X,Y)	[2:0-2-0,Edge]									1	
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.05	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.12	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code IRC2015/T	PI2014	Matri	x-P						Weight: 9 lb	FT = 20%
			1								
LUMBER-					BRACING-						

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3

REACTIONS. 1=3-0-12, 3=3-0-12 (size) Max Horz 1=-22(LC 8) Max Uplift 1=-12(LC 12), 3=-10(LC 13) Max Grav 1=97(LC 1), 3=97(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1.

![](_page_49_Picture_16.jpeg)

![](_page_49_Picture_18.jpeg)

![](_page_50_Figure_0.jpeg)