

GENERAL NOTES:

GENERAL: ALL NOTES ARE FOR SUPPLEMENTING THE PLANS AND SPECIFICATIONS AND ARE IN NO WAY TO BE CONSIDERED AS EXCLUDING ANY ITEM IN THEM.

CONTRACTOR TO OBTAIN ALL MISC. UTILITIES AND UTILITY CLEARANCES AND EXCAVATION PERMITS.

CODE: DESIGN AND CONSTRUCTION TO BE IN ACCORD WITH THE 2018 NC RESIDENTIAL CODE (NCRS) AND THE PARTICULAR CODES AS REFERENCED IN NCRS.

DESIGN CRITERIA:

FLOOR LIVE:	40 PSF
ROOF LIVE:	20 PSF
ATTIC LOAD:	20 PSF
WIND SPEED:	110 MPH
WALL COMPONENT:	24 PSF
NET UPLIFT:	20 PSF

FOUNDATION: EXCAVATION FOR AND BEARING MATERIAL FOR FOUNDATIONS SHOULD BE SUPERVISED AND APPROVED BY PWD PRIOR TO FOOTING INSTALLATION.

MATERIAL SATISFACTORY FOR CONTROLLED FILL AND BACKFILL MATERIAL AROUND AND ABOVE FOOTINGS SHALL INCLUDE CLEAN SOIL OR BANKRUN SAND AND GRAVEL (GW, GC, SG, SM, ML & CL), BUT EXCLUDE HIGHLY PLASTIC CLAYS (MH & CH) OR HIGH SHRINK SWELL SOILS. THE FILL MATERIALS SHALL BE FREE FROM TOPSOIL, ORGANIC CONTAMINATED SOIL AND ROCK FRAGMENTS HAVING A MAJOR DIMENSION GREATER THAN FOUR (4) INCHES, AND SHALL CONTAIN NO ICE OR SNOW.

FOOTINGS ARE DESIGNED FOR AN ASSUMED SOIL BEARING PRESSURE OF 2000 PSF.

CARE SHOULD BE TAKEN TO ASSURE THAT DURING PLACING OF CONCRETE FOOTINGS ON GRADE NO ORGANIC MATTER, SALTS, OR CLAYS ARE MIXED WITH THE CONCRETE.

CONCRETE: REINFORCED CONCRETE TO HAVE THE FOLLOWING COMPRESSIVE STRENGTH (F_c)

SLAB ON GRADE:	3000 PSI
FOOTINGS:	3000 PSI

EXPOSED CONCRETE SHALL BE AIR-ENTRAINED.

GROUT FOR BASE PLATES SHALL BE NON-SHRINKABLE GROUT AND SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH AT 28 DAYS OF 5,000 P.S.I.

REINFORCING STEEL: ASTM A615 GRADE 60.

WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185.

REINFORCING STEEL MARKED CONTINUOUS (CONT.) SHALL BE LAPPED 48 X BAR DIAMETER AT SPLICES. ALL REINFORCING STEEL SHALL BE HELD SECURELY IN PLACE TO PREVENT DISLOCATION DURING THE POURING OPERATION.

SLAB REINFORCING BARS SHALL BE SUPPORTED ON HIGH CHAIRS AND BAR SPACERS OF SUITABLE DESIGN. "HOOKING" OF WELDED WIRE FABRIC SHALL NOT BE PERMITTED.

DETAILING OF ALL CONCRETE STEEL REINFORCEMENT SHALL BE IN ACCORDANCE WITH THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (A.C.I. 315-89).

NO CONCRETE SHALL BE PLACED UNTIL ALL EMBEDDED WORK HAS BEEN INSTALLED, TESTED AND INSPECTED.

EXCEPT AS OTHERWISE SHOWN, MINIMUM PROTECTION (CONCRETE COVER) FOR REINFORCING STEEL SHALL BE AS FOLLOWS:

CONCRETE SURFACES EXPOSED TO SOIL:
1 1/2" FOR SLABS
3" FOR FOOTINGS

INTERIOR CONCRETE SURFACES:
3/4" FOR SLABS

CONCRETE SURFACES EXPOSED TO WEATHER:
1 1/2" FOR SLABS

WOOD: ALL WOOD TO BE SOUTHERN YELLOW PINE (SYP) NO. 2 OR HIGHER. ALL FASTENERS AND HANGERS TO BE HOT DIPPED GALVANIZED (AT A MINIMUM), PRESERVATIVE PRESSURE TREATMENT TO BE IN ACCORDANCE WITH AWPA STANDARD M4-D6 & U1-D7. MINIMUM PRESERVATION TREATMENTS:

POSTS: UC4A
ALL OTHER WOOD MEMBERS: UC3B

ROOF SHEATHING C-D GRADE "APA" EXTERIOR STRUCTURAL PANELS OR APPROVED EQUAL. PLACE WITH LONG DIMENSION PERPENDICULAR TO FRAMING. STAGGER END JOINTS. FASTEN WITH 8d HOT-DIPPED GALVANIZED BOX NAILS AT 6" O.C. AT ALL SUPPORTED EDGES, EXCEPT WITHIN THE FIRST 4' FROM ROOF EDGE. FASTENERS WITHIN THE FIRST 4' SHALL BE AT 4" O.C.

ALL MULTI-PLY LAMINATED VENEER LUMBER (LVL) HEADERS LINTELS & STUD COLUMNS SHALL BE CONNECTED SUCH THAT THEY ACT AS A SINGLE MEMBER.

LVL SPECS: F_b=2900 PSI F_v=285 PSI E=1,900,000 PSI

TIMBER TRUSS

- TRUSS FABRICATOR TO VERIFY FIELD DIMENSIONS WITH GENERAL CONTRACTOR.
- ALL TIMBER TRUSSES SHALL BE DESIGNED FOR:
110 MPH WIND SPEED
TOP CHORD LL = 30 PSF
TOP CHORD DL = 10 PSF
BOT CHORD LL = 20 PSF (GENERAL ATTIC)
BOT CHORD DL = 15 PSF
NET UPLIFT = 25 PSF
IN ACCORDANCE WITH GOVERNING LOAD COMBINATIONS PER IBC 1605.
- TRUSS SUPPLIER SHALL SUBMIT SHOP DRAWINGS WITH NORTH CAROLINA REGISTERED ENGINEER SEAL BEFORE FABRICATION.
- ALL MEMBERS TO BE SYP NO. 2 OR HIGHER

MASONRY: LOAD BEARING CONCRETE MASONRY CONSTRUCTION TO BE IN ACCORDANCE WITH ASCE 5/ACI 530/TMS 402-08, "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES", AND ASCE 6/ACI 530.1/TMS/602-08, "SPECIFICATIONS FOR MASONRY STRUCTURES." BEARING WALLS AND PIERS TO CONSIST ENTIRELY OF LOAD BEARING UNITS.

ALL MASONRY CONSTRUCTION TO BE IN ACCORDANCE WITH ACCEPTABLE INDUSTRY STANDARDS AND METHODS OF CONSTRUCTION.

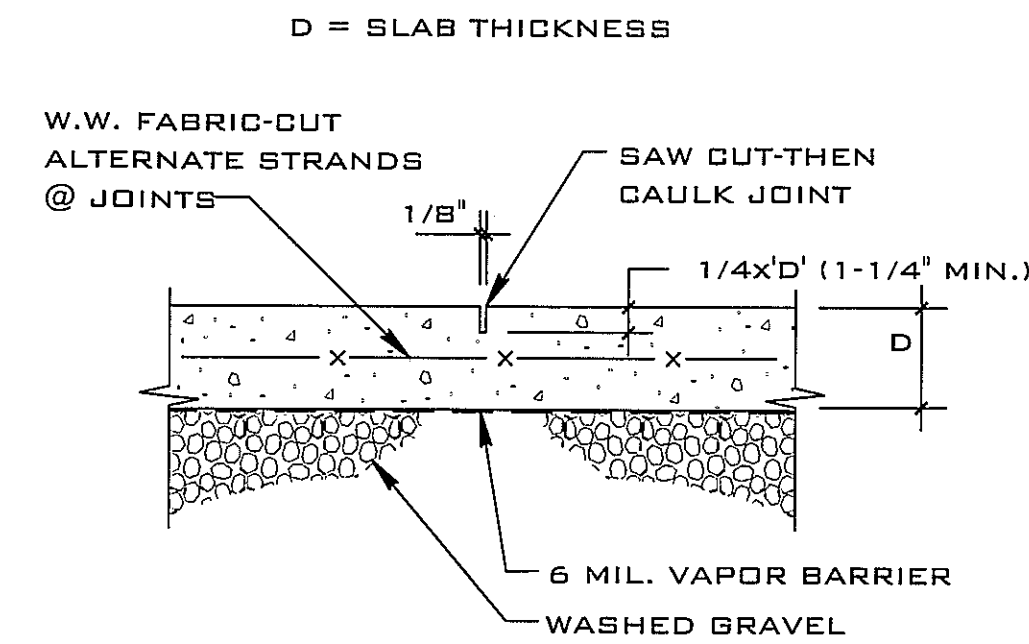
PROVIDE DUR D WAL OR EQUAL EVERY BLOCK COURSE BELOW FINISHED FLOOR AND EVERY OTHER COURSE ABOVE FINISHED FLOOR UNLESS OTHERWISE SHOWN ON THE ARCHITECTURAL SECTION. ALL HORIZONTAL WALL REINFORCING TO BE TRUSSED AND GALVANIZED. AT CORNERS AND INTERSECTIONS HORIZONTAL WALL REINFORCING TO BE FULLY LAPPED WITH TRUSSED GALVANIZED CORNERS AND TEES.

HOLLOW LOAD BEARING MASONRY UNITS SHALL CONFORM TO ASTM C90 REGULAR WEIGHT (UNLESS NOTED OTHERWISE). SOLID LOAD BEARING CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C145. THE UNIT MASONRY SHALL HAVE A NET UNIT COMPRESSIVE STRENGTH OF 2,000 PSI. THE COMPRESSIVE STRENGTH OF THE UNITS SHALL BE DETERMINED IN ACCORDANCE WITH ASTM C140 '70, STANDARD METHODS OF SAMPLING AND TESTING CONCRETE MASONRY UNITS. A 28 DAY PRISM STRENGTH VALUE OF 1500 PSI HAS BEEN USED IN THE DESIGN.

MORTAR FOR CONCRETE MASONRY SHALL CONFORM TO THE REQUIREMENTS OF THE ASTM SPECIFICATION FOR MORTAR UNIT MASONRY ASTM C270, TYPE M OR S. GROUT SHALL CONFORM TO ASTM C476. CONCRETE GROUT USED TO FILL CORES IN MASONRY UNITS SHALL HAVE A 28 DAY STRENGTH OF 3,000 PSI MIN.

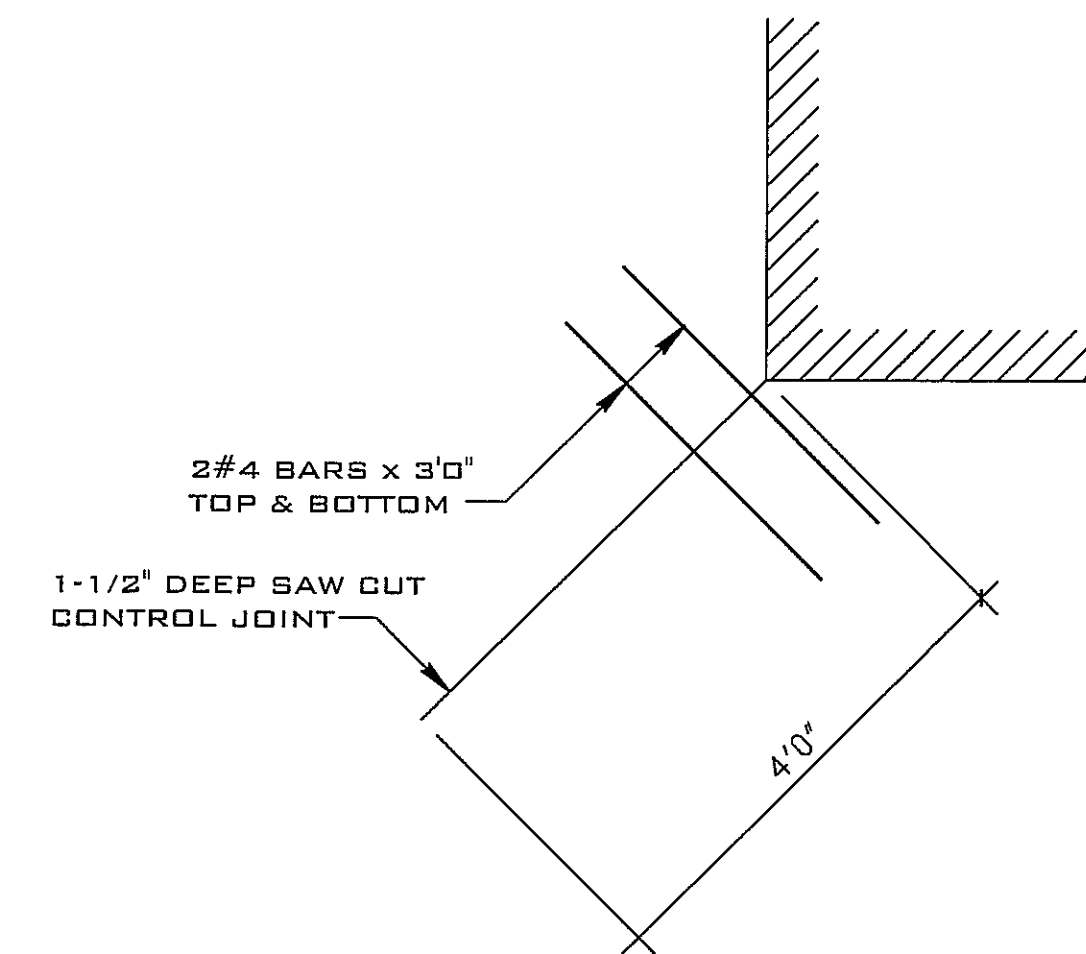
ALL MASONRY TO BE LAID IN TYPE M OR S MORTAR WITH FULL HEAD AND BED JOINT.

MARK	DATE	REVISION

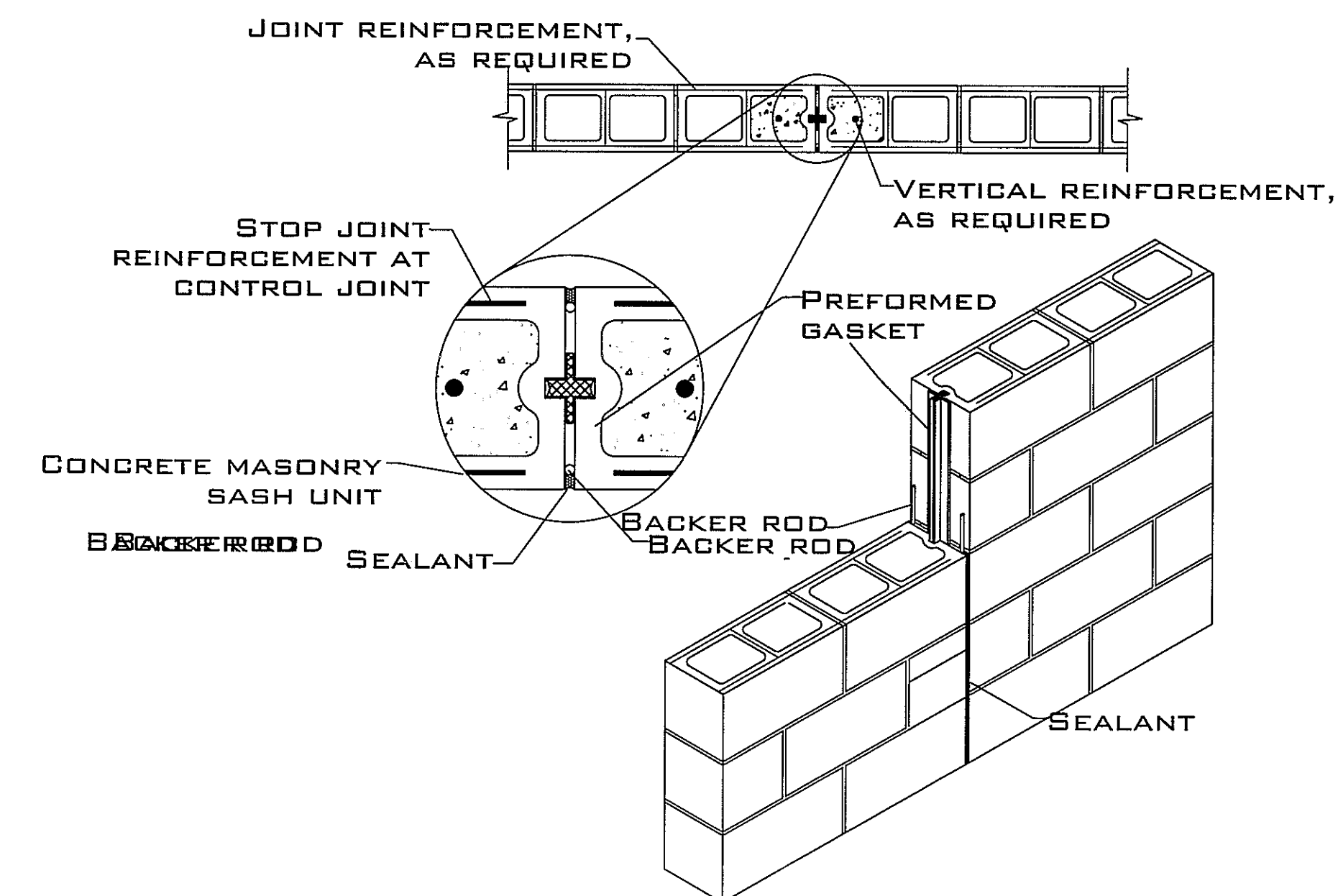


PROVIDE CONTROL JOINTS BETWEEN CONSTRUCTION JOINTS WITH SPACING NOT TO EXCEED IN FEET 3 TIMES THE SLAB THICKNESS IN INCHES IN EACH DIRECTION. CONTROL JOINTS TO BE FORMED WHILE CONCRETE IS STILL PLASTIC OR SAW CUT WITHIN 8 HOURS OF PLACING CONCRETE.

D1A SAWED CONTROL JOINT (S.J.)
DET



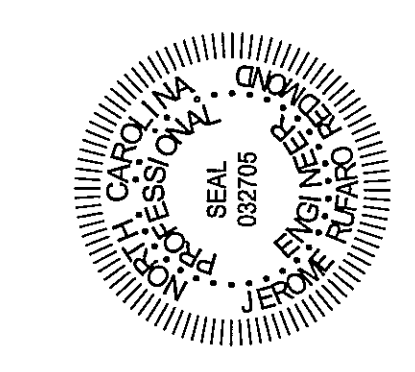
D1B TYPICAL DIAGONAL SAW CUT SLAB CONTROL JOINT DETAIL
DET



D40 MASONRY CONTROL JOINT
DET PROVIDE AT 16' INTERVAL MAX

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JEROME RUFARD REDMOND, PE
BUILDING ENGINEERING & DESIGN
8209A MARKET STREET STE. 222
WILMINGTON, NC 28411
910.915.6529
JREDNCS@YAHOO.COM



BB 1675-2 PLAN
HARNETT COUNTY, NC

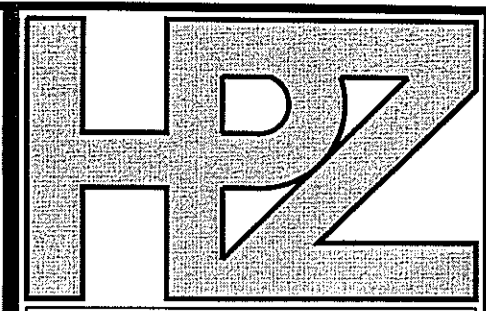
DRAWING TITLE
GENERAL NOTES

DRAWN BY: JRR
CHECKED BY: JRR

SCALE: AS SHOWN

DATE: 06.02.19

PROJECT SHEET
02019173 **S1**



House Plan Zone, LLC

House Plan Zone, LLC. Fax: 1-800-574-1387
Email: HPZplans@comcast.net



Plan ID:

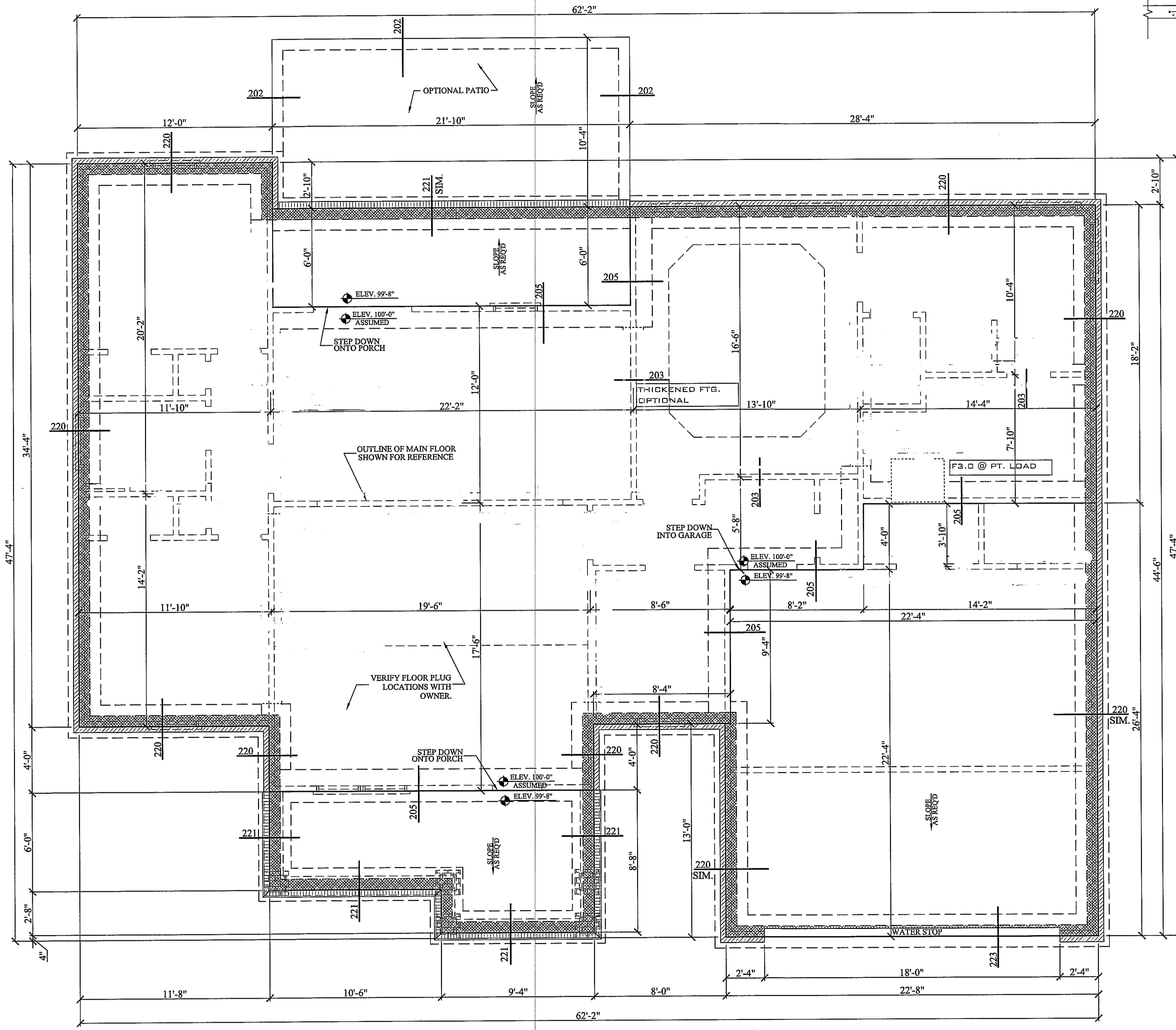
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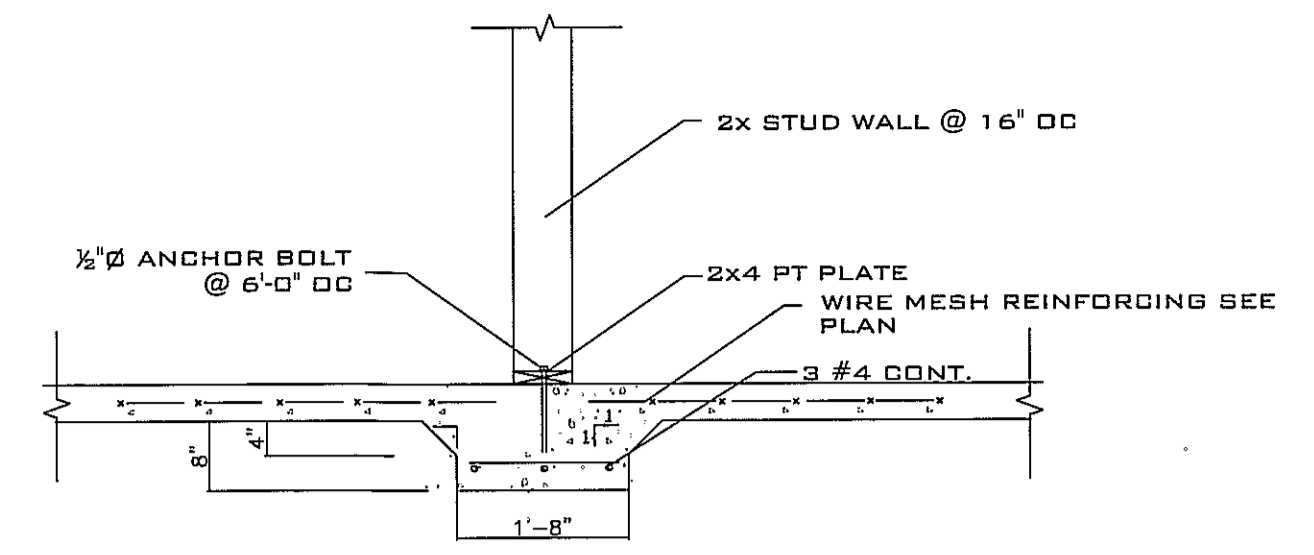
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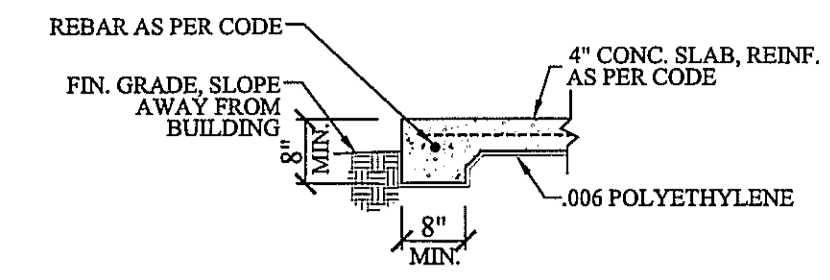
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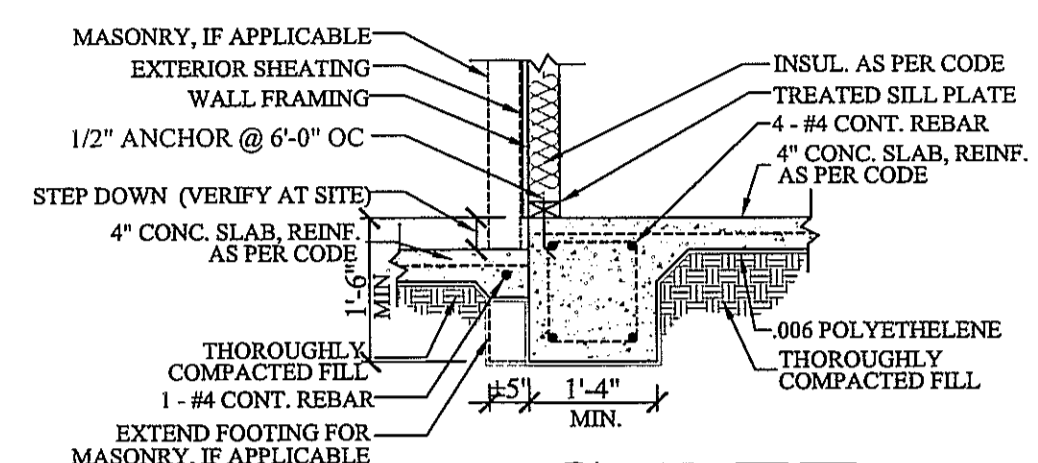
201 FOUNDATION PLAN
SCALE: 1/4" = 1'-0"



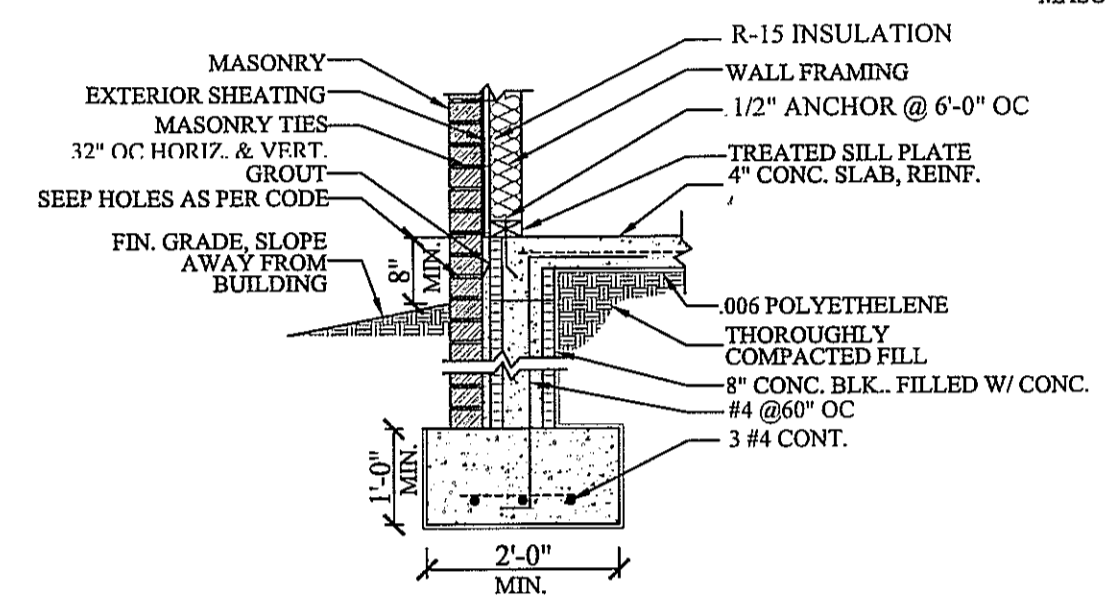
203 THICKENED SLAB
SCALE: NTS



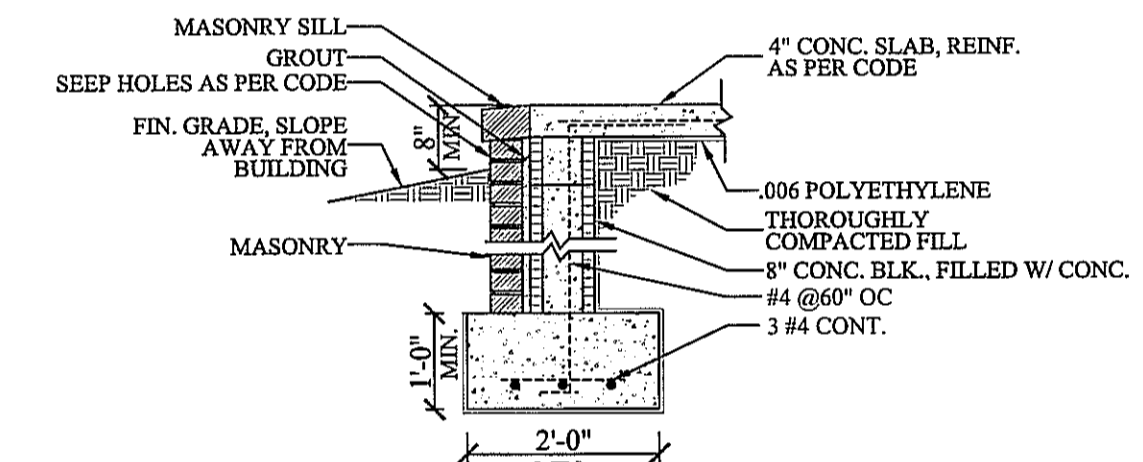
202 FTG. DET.



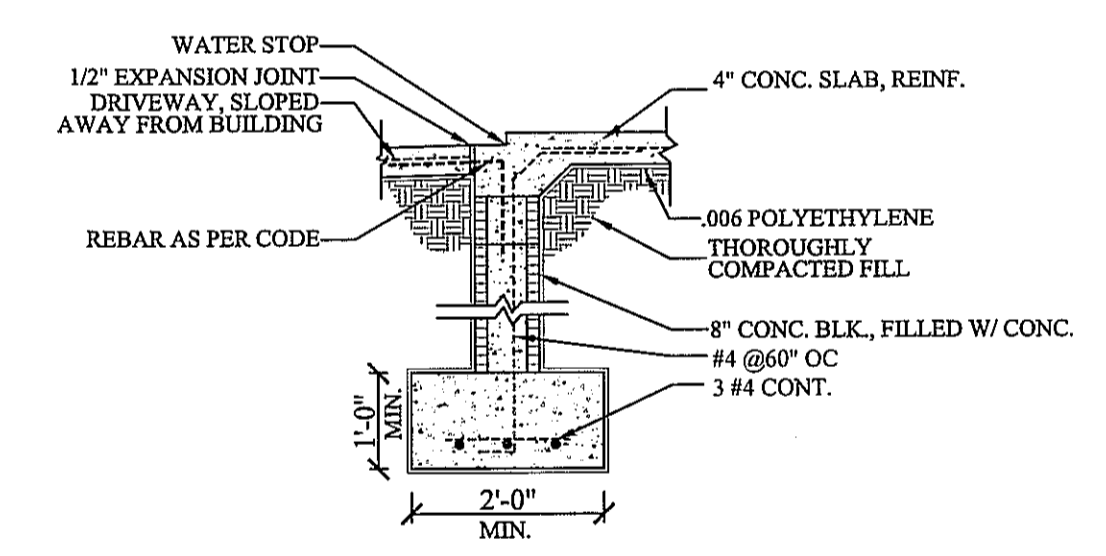
205 FTG. DET.



220 FTG. DET.



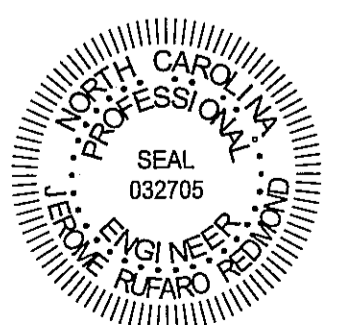
221 FTG. DET.



223 FTG. DET.

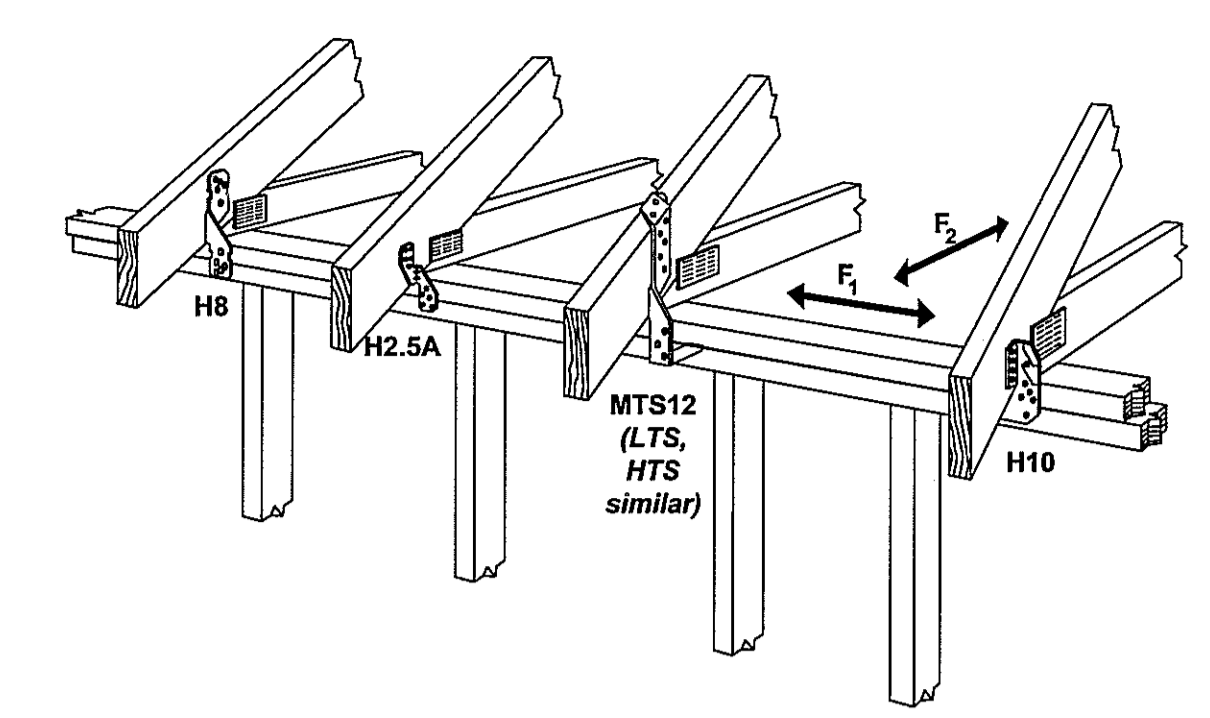
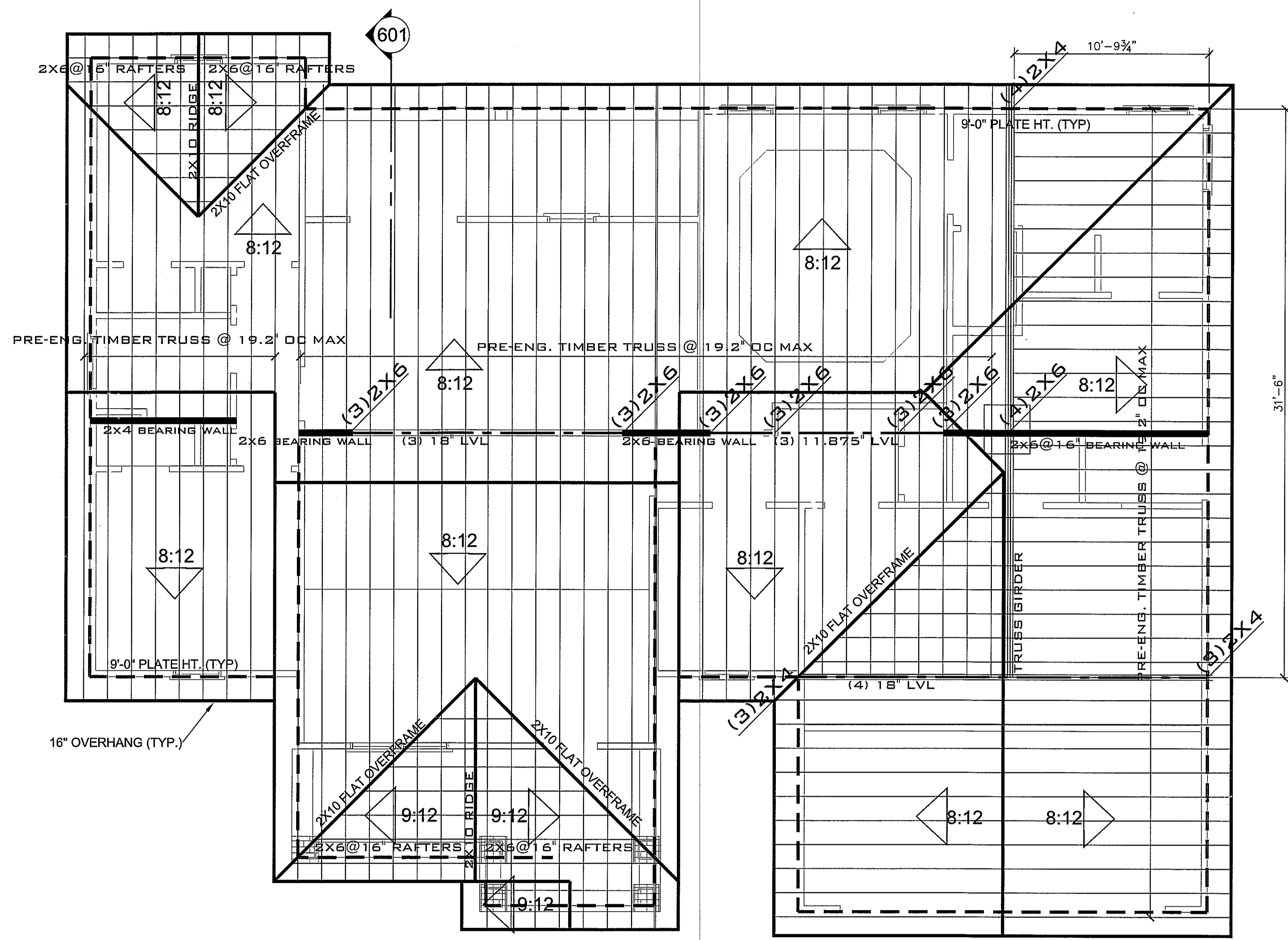
FOUNDATION NOTES:

1. FLOOR LIVE LOAD 40 PSF
2. ROOF LIVE 20 PSF
3. WF2.0 - 2'-0"WX16"D CONT. WALL FOOTING W/3 #4 OR 2 #5 CONT.
- WF1.8 - 1'-8"WX16"D CONT. WALL FOOTING W/3 #4 OR 2 #5 CONT.
- F3.0 - 36"X36"X12"D W/ 4 #4 EW
4. WALL: 2X4@16" OC
5. WOOD: SPF NO. 2 OR HIGHER
6. CONCRETE: $f'_c = 3000$ PSI
7. MASONRY: $f'_m = 1500$ PSI
8. ASSUMED SOIL BEARING: 2000 PSF
9. 4" CONCRETE SLAB ON GRADE CONCRETE W/6X6-W1.4XW1.4 REINFORCING OVER 6 MIL VAPOR BARRIER ON COMPACTED FILL
10. SLAB PERIMETER INSULATION: R-15 FOR 24"

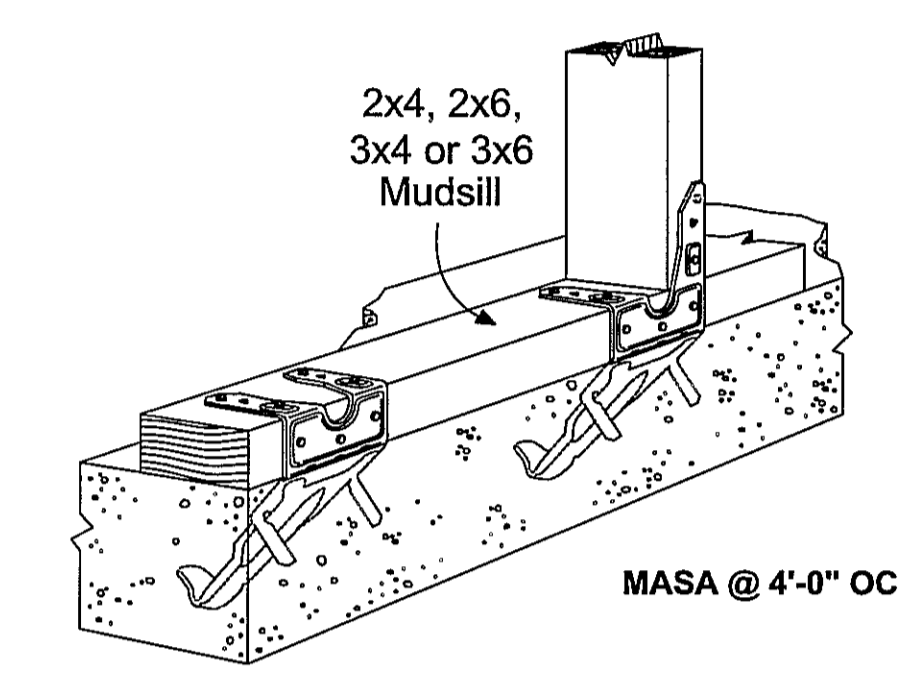


JEROME RUFARD REDMOND, PE
BUILDING ENGINEERING & DESIGN
8209A MARKET ST. STE 222
WILMINGTON, NC 28411
P: 910.915.6529
JRUFARD@JRUFARDAE.COM

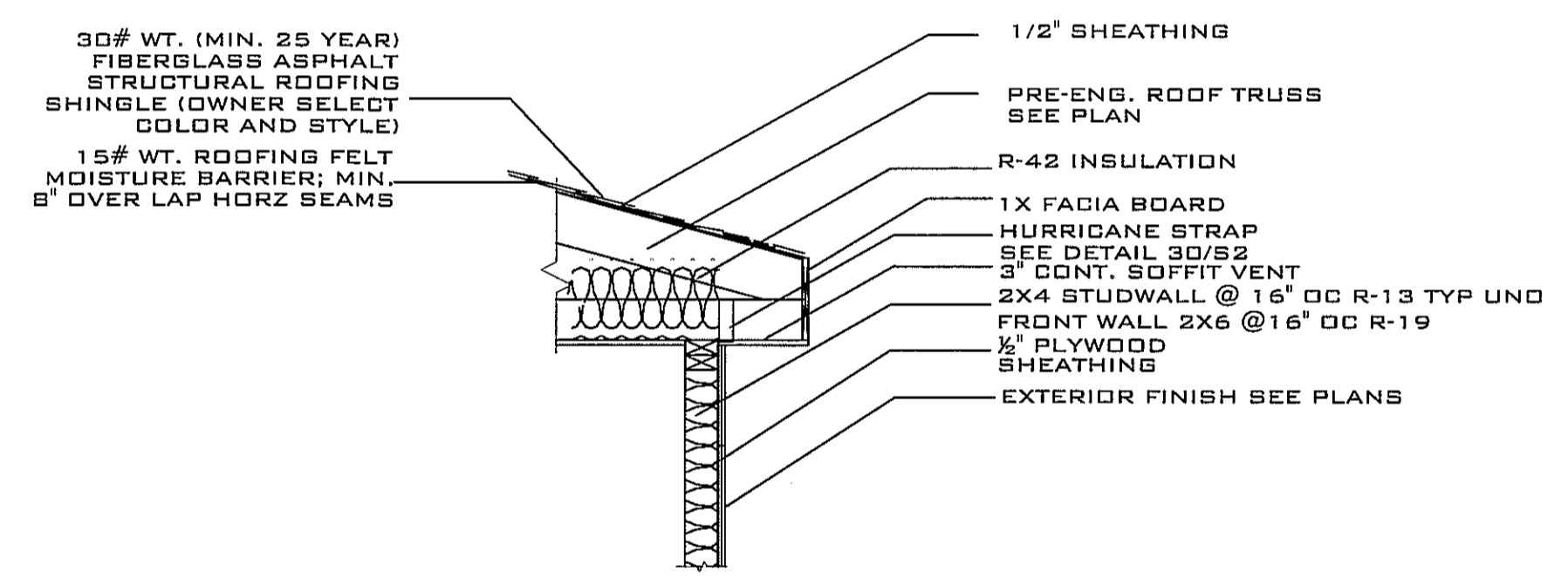
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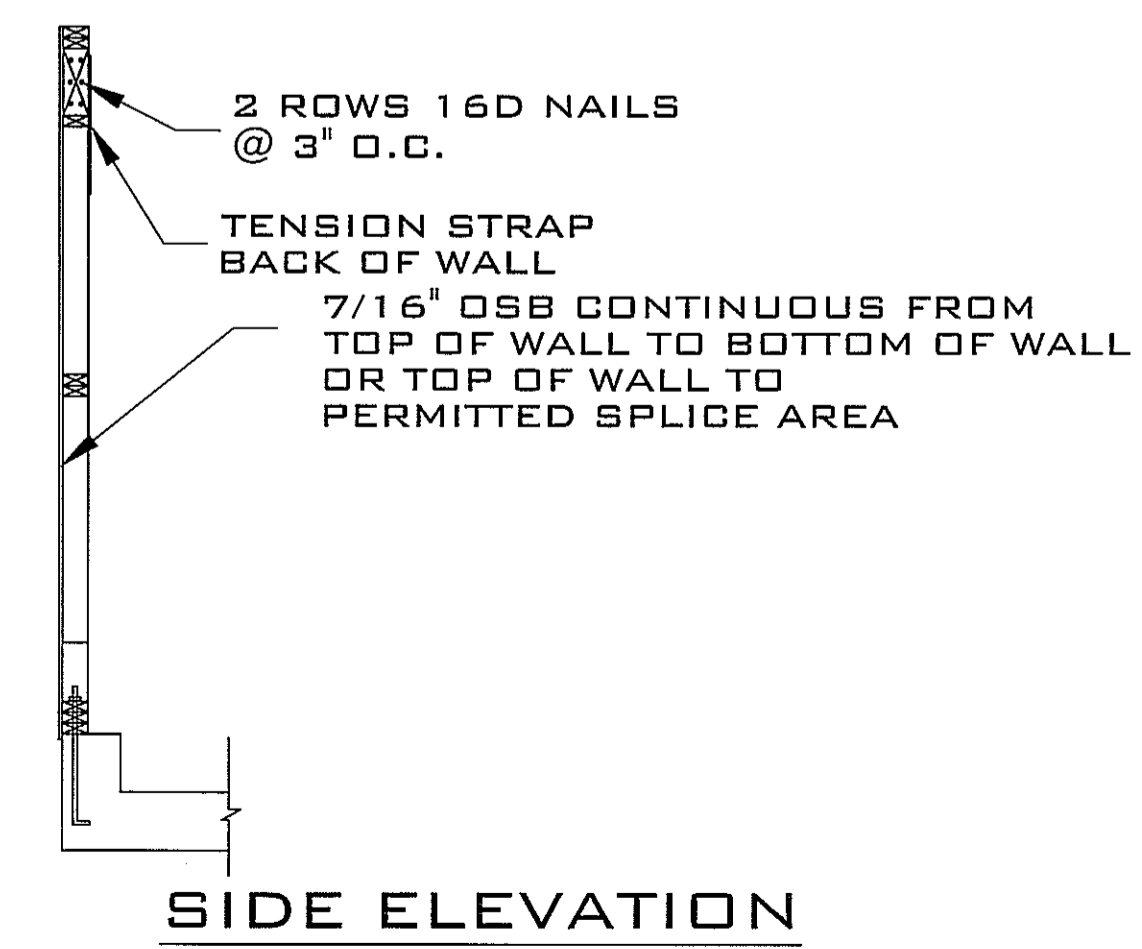
D30 DET HURRICANE TIE OPTIONS
NOTE: FASTENING SCHEDULE PER MANUFACTURER'S RECOMMENDATIONS



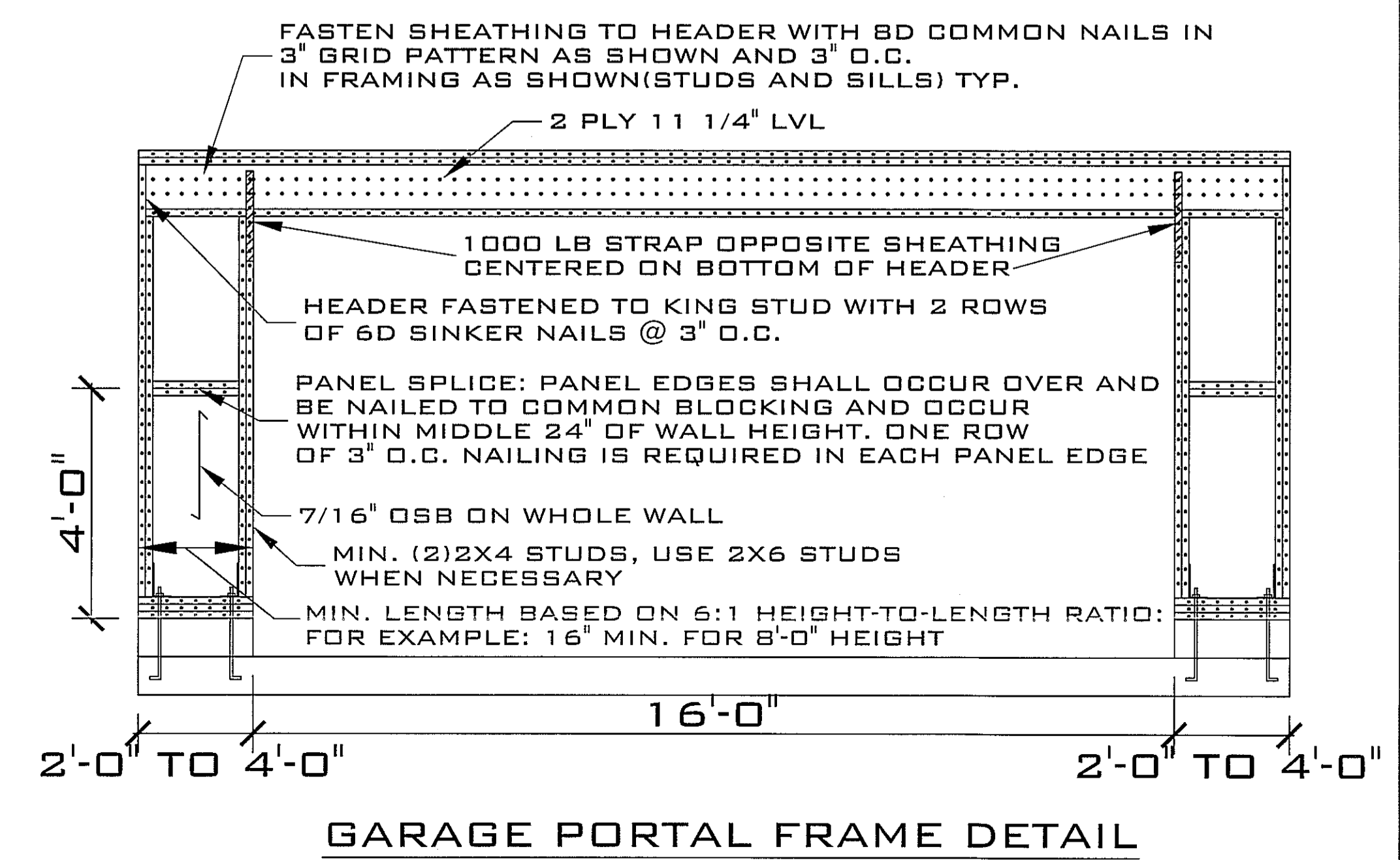
D32 DET ANCHORAGE - ALTERNATIVE
NOTE: FASTENING SCHEDULE PER MANUFACTURER'S RECOMMENDATIONS



3 52 ROOF @ BEARING WALL DETAIL
SCALE: NTS



SIDE ELEVATION



GARAGE PORTAL FRAME DETAIL

ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"
NOTE:

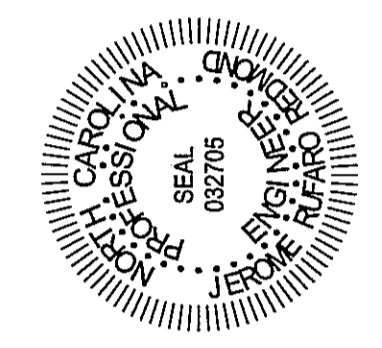
1. ROOF LIVE LOAD 20 PSF
2. CEILING LIVE: 10 PSF ATTIC STORAGE: 20 PSF
3. WINDOW HEADER: (2)2X8 W/ 1 JACK AND 1 KING STUD SUPPORT
4. HEADERS TO BE (2) 2X8 HEADER UND
5. ATTIC INSULATION: R-42

TIMBER TRUSS

1. TRUSS FABRICATOR TO VERIFY FIELD DIMENSIONS WITH GENERAL CONTRACTOR.
2. ALL TIMBER TRUSSES SHALL BE DESIGNED FOR:
 - 110 MPH WIND SPEED
 - TOP CHORD LL = 30 PSF
 - TOP CHORD DL = 10 PSF
 - BOT CHORD LL = 20 PSF (GENERAL ATTIC)
 - BOT CHORD DL = 15 PSF
 - NET UPLIFT = 25 PSF
 IN ACCORDANCE WITH GOVERNING LOAD COMBINATIONS PER IBC 1605.
3. TRUSS SUPPLIER SHALL SUBMIT SHOP DRAWINGS WITH NORTH CAROLINA REGISTERED ENGINEER SEAL BEFORE FABRICATION.
4. ALL MEMBERS TO BE SYP NO. 2 OR HIGHER
5. PROVIDE ANCHORAGE AT ALL BEARING LOCATIONS
6. SEE SHEET 5 FOR CATHEDRAL CEILING PROFILE IN GREAT ROOM

DESIGNER HAS BEEN REGISTERED BY THE BOARD OF ARCHITECTS OF THE STATE OF NORTH CAROLINA TO THE EXCLUSIVE PRACTICE OF ARCHITECTURE SINCE 1987. THIS PROJECT IS BEING DESIGNED BY THE ARCHITECT UNDER THE CLOSE PERSONAL SUPERVISION AND CONTROL OF THE ARCHITECT. THE ARCHITECT HAS REVIEWED THE ENGINEER'S DESIGN AND APPROVES THE SAME FOR CONSTRUCTION. THE ARCHITECT'S REVIEW IS LIMITED TO THE INFORMATION PROVIDED TO HIM BY THE ENGINEER. THE ARCHITECT IS NOT RESPONSIBLE FOR THE DESIGN OR CONSTRUCTION OF THE STRUCTURE.

JEROME RUFARO REDMOND, PE
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8209A MARKET STREET, STE. 222
WILMINGTON, NC 28411
910.915.6529
JREDNDS@YAHOO.COM



BB 1675-2 PLAN
HARNETT COUNTY, NC

DRAWING TITLE	
ROOF FRAMING PLAN AND STRUCTURAL DETAILS	
DRAWN BY:	JRR
CHECKED BY:	JRR
SCALE:	AS SHOWN
DATE:	06.02.19
PROJECT	SHEET
02019173	S2