

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 29043-29043A WEST-SHERMAN 106-21-180

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I48675510 thru I48675593

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



November 5,2021

Sevier, Scott

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



|                                    | 6-4-2                     | 12-4-12     | 18-6-0    | 24-6-0   | 37-10-4  | 44-0-0                                  |
|------------------------------------|---------------------------|-------------|-----------|----------|--|---|
|                                    | 6-4-2                     | 6-0-10      | 6-1-4     | 6-0-0    | 13-4-4   | 6-1-12                                  |
| Plate Offsets (X,Y)                | [7:0-3-9,0-2-0], [16:0-2- | 8,0-3-8]    |           |          |  |   |
| LOADING (psf)                      | SPACING-                  | 2-0-0       | CSI.      | DEFL.    | in (loc) l/defl L/d  | PLATES GRIP                             |
| TCLL 20.0                          | Plate Grip DOL            | 1.15        | TC 0.46   | Vert(LL) | -0.28 12-14 >999 240   | MT20 244/190                            |
| TCDL 10.0                          | Lumber DOL                | 1.15        | BC 0.80   | Vert(CT) | -0.51 12-14 >882 180   |   |
| BCLL 0.0 *                         | Rep Stress Incr           | YES         | WB 0.79   | Horz(CT) | 0.10 12 n/a n/a  |   |
| BCDL 10.0                          | Code IRC2015/             | TPI2014     | Matrix-MS |          |  | Weight: 355 lb FT = 20%                 |
| LUMBER-                            |                           |             |           | BRACING  | 3-   | L                                       |
| TOP CHORD 2x6 S<br>BOT CHORD 2x6 S | SP No.2<br>SP No.2        |             |           | TOP CHO  | ORD Structural wood sheathing except end verticals.            | directly applied or 4-11-13 oc purlins, |
| WEBS 2x4 S                         | SP No.3                   |             |           | BOT CHO  | DRD Rigid ceiling directly applied<br>6-0-0 oc bracing: 10-12. | d or 10-0-0 oc bracing, Except:         |
|                                    |                           |             |           | WEBS     | 1 Row at midpt   | 3-15, 6-15, 6-14, 8-12                  |
| REACTIONS. (si                     | ze) 18=0-3-8, 12=0-3-8    | 8, 10=0-3-8 |           |          | ·  |   |

(size) 18=0-3-8, 12=0-3-8, 10=0-3-8
 Max Horz 18=-308(LC 12), 12=-197(LC 13), 10=-74(LC 13)
 Max Grav 18=1486(LC 1), 12=1910(LC 1), 10=207(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD
   1-2=-1733/392, 2-3=-2101/524, 3-5=-1452/522, 5-6=-1456/523, 6-8=-1527/505, 1-18=-1434/344

   BOT CHORD
   17-18=-117/327, 16-17=-220/1541, 15-16=-150/1869, 14-15=-86/1337, 12-14=-164/1145

   WEBS
   2-17=-759/253, 2-16=0/376, 3-16=-60/724, 3-15=-985/262, 5-15=-280/874,
- 6-15=-231/257, 6-14=-281/79, 8-14=0/279, 8-12=-1712/382, 9-12=-417/289, 1-17=-288/1559

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 18, 12, and 10. This connection is for uplift only and does not consider lateral forces.







|                     | 6-4-2 12-4-12                     | 18-6-0    | 24-6-0   | 37-10-4                           | 44-0-0                                |
|---------------------|-----------------------------------|-----------|----------|-----------------------------------|---------------------------------------|
|                     | 6-4-2 6-0-10                      | 6-1-4     | 6-0-0    | 13-4-4                            | 6-1-12                                |
| Plate Offsets (X,Y) | [7:0-3-9,0-2-0], [16:0-2-8,0-3-8] | -         |          |                                   |                                       |
| DADING (psf)        | SPACING- 2-0-0                    | CSI.      | DEFL.    | in (loc) l/defl L/d               | PLATES GRIP                           |
| CLL 20.0            | Plate Grip DOL 1.15               | TC 0.46   | Vert(LL) | -0.28 12-14 >999 240              | MT20 244/190                          |
| CDL 10.0            | Lumber DOL 1.15                   | BC 0.80   | Vert(CT) | -0.51 12-14 >882 180              |                                       |
| CLL 0.0 *           | Rep Stress Incr YES               | WB 0.79   | Horz(CT) | 0.10 12 n/a n/a                   |                                       |
| BCDL 10.0           | Code IRC2015/TPI2014              | Matrix-MS |          |                                   | Weight: 355 lb FT = 20%               |
| UMBER-              | •                                 | ·         | BRACING- |                                   |                                       |
| OP CHORD 2x6 SF     | P No.2                            |           | TOP CHOP | RD Structural wood sheathing di   | rectly applied or 4-11-13 oc purlins, |
| OT CHORD 2x6 SF     | ° No.2                            |           |          | except end verticals.             |                                       |
| VEBS 2x4 SF         | ° No.3                            |           | BOT CHOP | RD Rigid ceiling directly applied | or 10-0-0 oc bracing, Except:         |
|                     |                                   |           |          | 6-0-0 oc bracing: 10-12.          |                                       |
|                     |                                   |           | WEBS     | 1 Row at midpt                    | 3-15, 6-15, 6-14, 8-12                |
| FACTIONS (size      | e) 18=Mechanical 12=0-3-8 10=0-1  | 3-8       |          |                                   |                                       |

TIONS. (size) 18=Mechanical, 12=0-3-8, 10=0-3-8 Max Horz 18=-308(LC 13) Max Uplift 18=-164(LC 12), 12=-197(LC 13), 10=-74(LC 13) Max Grav 18=1486(LC 1), 12=1910(LC 1), 10=207(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD
   1-2=-1733/392, 2-3=-2101/524, 3-5=-1452/522, 5-6=-1456/523, 6-8=-1527/505, 1-18=-1434/344

   BOT CHORD
   17-18=-117/327, 16-17=-220/1541, 15-16=-150/1869, 14-15=-86/1337, 12-14=-164/1145

   WEBS
   2-17=-759/253, 2-16=0/376, 3-16=-60/724, 3-15=-985/262, 5-15=-280/874.
- WEBS 2-17=-759/253, 2-16=0/376, 3-16=-60/724, 3-15=-985/262, 5-15=-280/874, 6-15=-231/257, 6-14=-281/79, 8-14=0/279, 8-12=-1712/382, 9-12=-417/289, 1-17=-288/1559

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 18=164.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12 and 10. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932



| LUMBER-   |        |      | BRACING-  |                                |                                       |
|-----------|--------|------|-----------|--------------------------------|---------------------------------------|
| TOP CHORD | 2x6 SP | No.2 | TOP CHORD | Structural wood sheathing d    | irectly applied or 4-11-3 oc purlins, |
| BOT CHORD | 2x6 SP | No.2 |           | except end verticals.          |                                       |
| WEBS      | 2x4 SP | No.3 | BOT CHORD | Rigid ceiling directly applied | or 10-0-0 oc bracing, Except:         |
|           |        |      |           | 2-2-0 oc bracing: 10-11.       |                                       |
|           |        |      | WEBS      | 1 Row at midpt                 | 4-12, 6-12, 6-11, 7-10                |
|           |        |      |           | 2 Rows at 1/3 pts              | 9-10                                  |

REACTIONS. (size) 15=0-3-8, 10=0-3-8 Max Horz 15=-173(LC 13) Max Uplift 15=-160(LC 12), 10=-166(LC 13) Max Grav 15=1508(LC 1), 10=1508(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 1-3=-1762/436, 3-4=-2146/591, 4-5=-1492/539, 5-6=-1495/540, 6-7=-1598/494, 1-15=-1456/378
- BOT CHORD
   13-14=-315/1568, 12-13=-312/1911, 11-12=-230/1396, 10-11=-310/1239

   WEBS
   3-14=-776/277, 3-13=0/393, 4-13=-101/733, 4-12=-993/302, 5-12=-294/907, 6-12=-283/211, 1-14=-330/1587, 7-10=-1660/453

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Bearing at joint(s) 15 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 15 and 10. This connection is for uplift only and does not consider lateral forces.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-1473 TeV. OF 192/2020 DELIGNE OGE. Design valid for use only with MITeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

| Job                   | Truss             | Truss Type          | Qty     | Ply       | WEST-SHERMAN 106-21-180                                |           |
|-----------------------|-------------------|---------------------|---------|-----------|--|-----------|
|                       |                   |                     |         |           |  | l48675513 |
| 29043-29043A          | AH                | Roof Special Girder | 1       | 2         |  |           |
|                       |                   |                     |         | <b>_</b>  | Job Reference (optional)                               |           |
| 84 Components (Dunn), | Dunn, NC - 28334, |                     | . 8     | 520 s Aug | 27 2021 MiTek Industries, Inc. Thu Nov 4 12:58:11 2021 | Page 2    |
|                       |                   | ID:2bG0dF           | ?FQwaXc | Mo_jiugPa | yMcRK-tJ5RXDTm7C2b1EHUHVua1Jcv?FwExNS_fZYuw            | SyMZrA    |

#### NOTES-

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 139 lb down and 120 lb up at 23-11-4, 139 lb down and 120 lb up at 25-11-4, 139 lb down and 120 lb up at 25-11-4, 139 lb down and 120 lb up at 25-11-4, 139 lb down and 120 lb up at 35-11-4, 139 lb down and 120 lb up at 35-11-4, 139 lb down and 120 lb up at 35-11-4, 139 lb down and 120 lb up at 35-11-4, 139 lb down and 120 lb up at 35-11-4, 139 lb down and 120 lb up at 37-11-4, 139 lb down and 120 lb up at 39-11-4, 139 lb down and 120 lb up at 35-11-4, and 139 lb down and 120 lb up at 35-11-4, 139 lb down and 120 lb up at 35-11-4, 139 lb down and 120 lb up at 35-11-4, 139 lb down and 120 lb up at 35-11-4, and 139 lb down and 120 lb up at 43-11-4, and 139 lb down and 120 lb up at 43-11-4, and 139 lb down and 120 lb up at 43-11-4, and 139 lb down at 25-11-4, 73 lb down at 21-11-4, 73 lb down at 23-11-4, 73 lb down at 27-11-4, 73 lb down at 33-11-4, 73 lb down at 35-11-4, 73 lb down at 37-11-4, 73 lb down at 33-11-4, 73 lb down at 35-11-4, 73 lb down at 33-11-4, 73 lb down at 41-11-4, and 73 lb down at 43-11-4, and 74 lb down at 45-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 4-5=-60, 5-11=-60, 12-23=-20

Concentrated Loads (lb)

Vert: 18=-1135(F) 26=-99(F) 27=-99(F) 28=-99(F) 29=-99(F) 30=-99(F) 31=-99(F) 32=-99(F) 33=-99(F) 33=-99(F) 35=-99(F) 35=-99(F) 35=-99(F) 37=-101(F) 38=-59(F) 39=-59(F) 40=-59(F) 40=-59(F) 41=-59(F) 42=-59(F) 43=-59(F) 44=-59(F) 45=-59(F) 46=-59(F) 44=-59(F) 48=-59(F) 48=-59(







| 5-(   | 0-0 13-10-8   | 22-0-2  | 30-3-8   | 38-6-14  | 47-0-0   |  |  |  |
|---|---|---|--|--|--|--|--|--|
| Plate Offsets (X,Y)   | [2:0-0-0,0-0-4], [3:0-3-0,0-1-8], [10:0-3-{   | 3,0-2-0], [13:0-3-8,0-2-8],   | [18:0-2-0,0-3-8]   | 000  | 002  |  |  |  |
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 *<br>BCDL 10.0  | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014   | CSI.<br>TC 0.96<br>BC 0.39<br>WB 0.87<br>Matrix-MS  | DEFL.         in           Vert(LL)         -0.44           Vert(CT)         -0.88           Horz(CT)         0.12   | (loc) l/defl L/d<br>15-16 >999 240<br>15-16 >636 180<br>12 n/a n/a   | PLATES         GRIP           MT20         197/144           Weight: 329 lb         FT = 20% |  |  |  |
| LUMBER-<br>TOP CHORD 2x6 SF<br>BOT CHORD 2x6 SF<br>WEBS 2x4 SF<br>4-18,5<br>WEDGE<br>Left: 2x4 SP No.3  | P No.2<br>P DSS<br>P No.3 *Except*<br>-16,6-15,10-15,11-13: 2x4 SP No.2 or 2)   | 4 SPF No.2  | BRACING-<br>TOP CHORD<br>BOT CHORD<br>WEBS   | BRACING-<br>TOP CHORDStructural wood sheathing directly applied or 2-6-11 oc purlins<br>except end verticals, and 2-0-0 oc purlins (2-4-3 max.): 5-11.BOT CHORDRigid ceiling directly applied or 8-0-15 oc bracing.<br>WEBS1 Row at midpt5-18, 6-15, 11-13 |  |  |  |  |
| REACTIONS. (siz<br>Max H<br>Max U<br>Max G  | e) 12=Mechanical, 2=0-3-8<br>lorz 2=187(LC 12)<br>Jplift 12=-320(LC 13), 2=-171(LC 13)<br>Grav 12=1874(LC 1), 2=1935(LC 1)  |   |  |  |  |  |  |  |
| FORCES.         (lb) - Max.           TOP CHORD         2-3=           8-10:         2-19:           BOT CHORD         2-19:           13-11         3-19:           WEBS         3-19:           6-16:         11-13:  | Comp./Max. Ten All forces 250 (lb) or<br>-5022/1033, 3-4=-5249/1184, 4-5=-4859<br>=-4920/1012, 10-11=-3196/650, 11-12=-<br>=-1109/4737, 18-19=-739/3274, 16-18=-<br>5=-650/3196<br>=-1011/367, 4-19=-424/1727, 4-18=-617<br>=-407/230, 6-15=-505/135, 8-15=-469/2<br>3=-718/3532  | less except when shown<br>/1069, 5-6=-5374/1134, 6<br>1792/416<br>958/4422, 15-16=-1133/5<br>/3311, 5-18=-2899/675, 5<br>16, 10-15=-405/1927, 10-   | 5-8=-4920/1012,<br>5372,<br>5-16=-280/1201,<br>13=-1407/434,   |  |  |  |  |  |
| <ol> <li>NOTES-         <ol> <li>Unbalanced roof live</li> <li>Wind: ASCE 7-10; \<br/>gable end zone and<br/>DOL=1.60</li> <li>Provide adequate d</li> <li>This truss has been</li> <li>This truss has been</li> <li>This truss has been</li> <li>Refer to girder(s) for</li> <li>Provide mechanical<br/>12=320.</li> <li>One H2.5A Simpson<br/>connection is for up</li> <li>Graphical purlin rep</li> </ol> </li> </ol> | e loads have been considered for this de<br>/ult=130mph Vasd=103mph; TCDL=6.0p<br>I C-C Exterior(2) zone;C-C for members<br>rainage to prevent water ponding.<br>designed for a 10.0 psf bottom chord liv<br>en designed for a live load of 20.0psf on t<br>pottom chord and any other members.<br>r truss to truss connections.<br>connection (by others) of truss to bearin<br>n Strong-Tie connectors recommended t<br>lift only and does not consider lateral for<br>resentation does not depict the size or th | sign.<br>psf; BCDL=6.0psf; h=30ft;<br>and forces & MWFRS for<br>e load nonconcurrent with<br>the bottom chord in all are<br>ng plate capable of withsta<br>o connect truss to bearing<br>ces.<br>ne orientation of the purlin | ; Cat. II; Exp B; Enclosed;<br>reactions shown; Lumbe<br>h any other live loads.<br>eas where a rectangle 3-6<br>anding 100 lb uplift at join<br>g walls due to UPLIFT at j<br>h along the top and/or bott | MWFRS (envelope)<br>r DOL=1.60 plate grip<br>6-0 tall by 2-0-0 wide<br>t(s) except (jt=lb)<br>jt(s) 2. This<br>tom chord.  | SEAL<br>044925<br>M. SEVILLA   |  |  |  |







| 5-(   | 0-0 12-1-8   | 20-8-6  | 29-5-0  | 38-1-10  | 47-0-0   |
|---|--|---|---|--|--|
| 5-0   |  | 8-6-14  | 8-8-10  | 8-8-10   | 8-10-6   |
| Plate Offsets (X, Y)  | [2:0-0-0,0-0-4], [3:0-5-0,0-3-4], [12:0-3-4]   | 3,0-2-8]  |   |  |  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0  | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014  | <b>CSI.</b><br>TC 0.61<br>BC 0.40<br>WB 0.76<br>Matrix-MS   | DEFL.         in           Vert(LL)         -0.30           Vert(CT)         -0.61           Horz(CT)         0.11                            | (loc) l/defl L/d<br>14-15 >999 240<br>14-15 >922 180<br>11 n/a n/a   | PLATES         GRIP           MT20         197/144           Weight: 331 lb         FT = 20%                                   |
| LUMBER-<br>TOP CHORD 2x6 SF<br>BOT CHORD 2x6 SF<br>WEBS 2x4 SF<br>4-15,5<br>WEDGE<br>Left: 2x4 SP No.3  | P No.2<br>P DSS<br>P No.3 *Except*<br>-14,9-14,10-12: 2x4 SP No.2 or 2x4 SPF   | - No.2  | BRACING-<br>TOP CHORD<br>BOT CHORD<br>WEBS  | Structural wood sheathing dii<br>except end verticals, and 2-0<br>Rigid ceiling directly applied<br>1 Row at midpt 1 | rectly applied or 3-1-2 oc purlins,<br>-0 oc purlins (3-0-11 max.): 4-10.<br>or 7-11-13 oc bracing.<br>0-11, 3-17, 5-14, 10-12 |
| REACTIONS. (siz<br>Max H<br>Max U<br>Max G  | e) 11=Mechanical, 2=0-3-8<br>lorz 2=209(LC 12)<br>Jplift 11=-327(LC 9), 2=-221(LC 8)<br>Grav 11=1874(LC 1), 2=1935(LC 1)   |   |   |  |  |
| FORCES.         (lb) - Max.           TOP CHORD         2-3=           9-10.         9-10.           BOT CHORD         2-18.           12-1.         12-1.           WEBS         3-17.           7-14.         7-14.   | Comp./Max. Ten All forces 250 (lb) or<br>-5052/1034, 3-4=-3777/779, 4-5=-4301/9<br>=-2693/552, 10-11=-1793/422<br>=-1153/4773, 17-18=-1145/4772, 15-17=<br>4=-552/2693<br>=-1477/399, 4-17=-14/639, 4-15=-290/12<br>=-502/231, 9-14=-343/1594, 9-12=-1386  | less except when shown<br>923, 5-7=-4067/848, 7-9=<br>758/3321, 14-15=-922/4<br>282, 5-15=-500/259, 5-14<br>/441, 10-12=-634/3096                     | -4067/848,<br>1299,<br>=-270/86,  |  |  |
| <ul> <li>NOTES-</li> <li>1) Unbalanced roof live</li> <li>2) Wind: ASCE 7-10; \<br/>gable end zone and<br/>DOL=1.60</li> <li>3) Provide adequate d</li> <li>4) All plates are 4x6 M</li> <li>5) This truss has been</li> <li>6) * This truss has been</li> <li>6) * This truss has been</li> <li>will fit between the b</li> <li>7) Refer to girder(s) fo</li> <li>8) Provide mechanical<br/>11=327.</li> </ul> | e loads have been considered for this de<br>/ult=130mph Vasd=103mph; TCDL=6.0<br>I C-C Exterior(2) zone;C-C for members<br>rainage to prevent water ponding.<br>T20 unless otherwise indicated.<br>designed for a 10.0 psf bottom chord live<br>in designed for a live load of 20.0psf on to<br>pottom chord and any other members.<br>r truss to truss connections.<br>connection (by others) of truss to bearing | sign.<br>bsf; BCDL=6.0psf; h=30ft;<br>and forces & MWFRS for<br>e load nonconcurrent with<br>he bottom chord in all are<br>g plate capable of withsta | Cat. II; Exp B; Enclosed;<br>reactions shown; Lumber<br>n any other live loads.<br>eas where a rectangle 3-6<br>anding 100 lb uplift at joint | MWFRS (envelope)<br>DOL=1.60 plate grip<br>-0 tall by 2-0-0 wide<br>(s) except (jt=lb)                               | SEAL<br>044925   |

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



M. 55,2021





| 5-  | 0-0 14-1-8<br>0-0 9-1-8  | 24-11-13<br>10-10-5   |   | 35-10-3<br>10-10-5   | 47-0-0   |
|---|--|---|---|--|--|
| Plate Offsets (X,Y)   | [2:0-0-0,0-0-4], [3:0-4-12,0-3-12]   |   |   |  |  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0  | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014  | <b>CSI.</b><br>TC 0.95<br>BC 0.42<br>WB 0.98<br>Matrix-MS   | DEFL. ir<br>Vert(LL) -0.24<br>Vert(CT) -0.50<br>Horz(CT) 0.13   | 1 (loc) l/defl L/d<br>14-16 >999 240<br>14-16 >999 180<br>14-16 >999 180<br>11 n/a n/a   | PLATES         GRIP           MT20         197/144           Weight: 331 lb         FT = 20%                     |
| LUMBER-<br>TOP CHORD 2x6 SI<br>BOT CHORD 2x6 SI<br>WEBS 2x4 SI<br>5-16,9<br>WEDGE<br>Left: 2x4 SP No.3  | P No.2<br>P DSS<br>P No.3 *Except*<br>-11: 2x4 SP No.2 or 2x4 SPF No.2   |   | BRACING-<br>TOP CHORD<br>BOT CHORD<br>WEBS  | Structural wood sheathing c<br>2-0-0 oc purlins (3-7-11 ma:<br>Rigid ceiling directly applied<br>1 Row at midpt<br>2 Rows at 1/3 pts | directly applied, except end verticals, and<br>x.): 4-10.<br>I or 7-8-13 oc bracing.<br>3-16, 5-16, 7-12<br>9-11 |
| REACTIONS. (siz<br>Max H<br>Max L<br>Max C  | e) 11=Mechanical, 2=0-3-8<br>forz 2=248(LC 12)<br>Jplift 11=-321(LC 9), 2=-180(LC 8)<br>Grav 11=1874(LC 1), 2=1935(LC 1)   |   |   |  |  |
| FORCES.         (lb) - Max.           TOP CHORD         2-3=           BOT CHORD         2-17           11-1         11-1           WEBS         3-16           9-12         9-12   | Comp./Max. Ten All forces 250 (lb) or<br>-5106/1045, 3-4=-3514/717, 4-5=-3073/<br>=-1212/4831, 16-17=-1202/4832, 14-16=<br>2=-471/2139<br>=-1787/494, 4-16=-54/1031, 5-16=-859/2<br>=-103/1217, 9-11=-2638/583   | less except when shown<br>*18, 5-7=-3560/747, 7-9=<br>815/3609, 12-14=-726/3<br>258, 7-14=-32/440, 7-12=                                      | -2626/512<br>1285,<br>-1046/322,  |  |  |
| NOTES-<br>1) Unbalanced roof liv.<br>2) Wind: ASCE 7-10; V<br>gable end zone and<br>DOL=1.60<br>3) Provide adequate d<br>4) This truss has been<br>will fit between the I<br>6) Refer to girder(s) for<br>7) Provide mechanical | e loads have been considered for this de<br>Vult=130mph Vasd=103mph; TCDL=6.0p<br>I C-C Exterior(2) zone;C-C for members<br>rainage to prevent water ponding.<br>designed for a 10.0 psf bottom chord liv<br>en designed for a live load of 20.0psf on t<br>pottom chord and any other members, w<br>r truss to truss connections.<br>I connection (by others) of truss to bearing | sign.<br>bsf; BCDL=6.0psf; h=30ft;<br>and forces & MWFRS for<br>e load nonconcurrent with<br>he bottom chord in all are<br>th BCDL = 10.0psf. | Cat. II; Exp B; Enclosed<br>reactions shown; Lumbe<br>n any other live loads.<br>eas where a rectangle 3- | I; MWFRS (envelope)<br>or DOL=1.60 plate grip<br>6-0 tall by 2-0-0 wide  | NUMERIA CAROLINA<br>OFFESSION<br>SEAL  |

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=321.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
  connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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| 5-0-0  | 16-1-8  | 26-3-13   |   | 36-6-3  |   | 47-0-0  |                                    |
|--|---|---|---|---|---|---|------------------------------------|
| 5-0-0<br>Plate Offsets (X Y) [2:0-0-0 0-0-4] [3:0-3-0  | 11-1-8<br>0-3-4] [16:0-4-12:0-4-8]  | 10-2-5  |   | 10-2-5  |   | 10-5-13   |                                    |
| LOADING (psf)         SPACING-           TCLL 20.0         Plate Grip DOL           TCDL 10.0         Lumber DOL           BCLL 0.0 *         Rep Stress Incr           Code IRC2015/T         Code IRC2015/T  | 2-0-0 <b>CSI.</b><br>1.15 TC<br>1.15 BC<br>YES WB<br>Pl2014 Matrix-   | 0.47 Vert(LL<br>0.90 Vert(C<br>0.93 Horz(C<br>MS  | in<br>.) -0.26<br>T) -0.56<br>T) 0.14                                     | (loc) l/defl<br>15-16 >999<br>16-17 >998<br>12 n/a  | L/d<br>240<br>180<br>n/a  | PLATES<br>MT20<br>Weight: 346 lb  | <b>GRIP</b><br>197/144<br>FT = 20% |
| LUMBER-<br>TOP CHORD 2x6 SP No.2<br>BOT CHORD 2x6 SP No.2<br>WEBS 2x4 SP No.3 *Except*<br>6-16,9-12: 2x4 SP No.2 or 2x4<br>WEDGE<br>Left: 2x4 SP No.3  | SPF No.2  | BRACII<br>TOP CH<br>BOT CH<br>WEBS  | NG-<br>Hord<br>Hord   | Structural wood s<br>except end vertic<br>Rigid ceiling dire<br>1 Row at midpt<br>2 Rows at 1/3 pts | sheathing direct<br>als, and 2-0-0<br>ctly applied or 6<br>6-16<br>s 9-12 | tly applied or 2-8-14<br>oc purlins (4-0-3 max<br>3-8-8 oc bracing.<br>5, 8-13<br>2 | oc purlins,<br>): 5-11.            |
| REACTIONS.         (size)         12=Mechanical, 2=0           Max Horz         2=286(LC 12)           Max Uplift         12=-315(LC 9), 2=-7           Max Grav         12=1909(LC 2), 2=1  | )-3-8<br>175(LC 12)<br>1935(LC 1)   |   |   |   |   |   |                                    |
| FORCES. (lb) - Max. Comp./Max. Ten All fc<br>TOP CHORD 2-3=-5091/961, 3-4=-5480/11<br>8-9=-2218/435  | orces 250 (lb) or less except w<br>45, 4-5=-3225/699, 5-6=-2800   | vhen shown.<br>6/666, 6-8=-3098/660,  |   |   |   |   |                                    |
| BOT CHORD 2-17=-1167/4793, 16-17=-928<br>12-13=-392/1790   | 8/3448, 15-16=-723/3129, 13-  | -15=-619/2788,  |   |   |   |   |                                    |
| WEBS 3-17=-1011/333, 4-17=-341/1<br>8-15=-71/533, 8-13=-1040/31  | 824, 4-16=-794/332, 5-16=-13<br>7, 9-13=-127/1272, 9-12=-24   | 31/1070, 6-16=-624/216,<br>08/531   |   |   |   |   |                                    |
| <ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads have been considered with the second s</li></ul> | dered for this design.<br>hph; TCDL=6.0psf; BCDL=6.0<br>C for members and forces & I<br>ponding.<br>bottom chord live load noncor<br>d of 20.0psf on the bottom cho<br>her members, with BCDL = 10<br>ns.<br>f truss to bearing plate capab | Dpsf; h=30ft; Cat. II; Exp B<br>MWFRS for reactions sho<br>ncurrent with any other live<br>ord in all areas where a re<br>0.0psf. | ; Enclosed;<br>wn; Lumber<br>e loads.<br>ctangle 3-6-<br>uplift at joint( | MWFRS (envelo)<br>DOL=1.60 plate<br>-0 tall by 2-0-0 wi<br>(s) except (jt=lb)                       | pe)<br>grip<br>de   | SEAL<br>OCOTESSI<br>SEAL<br>04492   | ROLAN<br>Source                    |

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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| L  | 5-0-0 9-3-8 18-   | 1-8 <u>1</u> 21-4-12  | 25-9-12  | 33-6-0   | 36-10-8  | 47-0-0  |                                    |
|--|---|---|--|--|--|---|------------------------------------|
| I  | 5-0-0 4-3-8 8-1   | 0-0 3-3-4   | 4-5-0  | 7-8-4  | 3-4-8  | 10-1-8  |                                    |
| Plate Offsets (X,Y)  | [2:0-0-0,0-0-4], [3:0-5-0,0-3-4], [14:0-3-0   | 0,0-3-8], [16:0-3-0,0-4-4],   | [18:0-5-4,0-4-4]   |  |  |   |                                    |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0   | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014   | CSI.<br>TC 0.78<br>BC 0.97<br>WB 0.89<br>Matrix-MS  | DEFL.<br>Vert(LL) -<br>Vert(CT) -<br>Horz(CT)  | in (loc)<br>0.29 16<br>0.57 16<br>0.24 12          | l/defl L/d<br>>999 240<br>>985 180<br>n/a n/a  | PLATES<br>MT20<br>Weight: 369 lb  | <b>GRIP</b><br>244/190<br>FT = 20% |
| LUMBER-<br>TOP CHORD 2x6 SF<br>BOT CHORD 2x6 SF<br>WEBS 2x4 SF<br>WEDGE<br>Left: 2x4 SP No.3   | 9 No.2<br>9 No.2<br>9 No.3  |   | BRACING-<br>TOP CHORE<br>BOT CHORE<br>WEBS   | 9 Structu<br>except<br>9 Rigid c<br>1 Row          | ural wood sheathing dir<br>end verticals, and 2-0-<br>æiling directly applied o<br>at midpt 4- | ectly applied or 2-6-0 c<br>0 oc purlins (3-3-4 ma<br>r 2-2-0 oc bracing.<br>-17, 8-13, 10-12 | oc purlins,<br>x.): 5-9.           |
| REACTIONS. (size<br>Max H<br>Max U<br>Max G  | e) 2=0-3-8, 12=Mechanical<br>lorz 2=218(LC 12)<br>lplift 2=-203(LC 12), 12=-116(LC 8)<br>irav 2=1935(LC 1), 12=1874(LC 1)   |   |  |  |  |   |                                    |
| FORCES.         (lb) - Max.           TOP CHORD         2-3=-<br>8-9=-<br>8-9=-<br>15-16           BOT CHORD         2-19-<br>15-16           WEBS         3-18-<br>6-15=<br>10-13                     | Comp./Max. Ten All forces 250 (lb) or<br>-5016/1085, 3-4=-4190/959, 4-5=-4078/3<br>-1808/518, 9-10=-2092/540<br>=-1114/4728, 18-19=-1109/4727, 17-18-<br>6=-707/3667, 14-15=-432/2294, 13-14=-<br>-1048/227, 4-18=-371/209, 5-17=-4/39<br>=-1247/371, 8-15=-341/1724, 8-14=-476<br>3=-48/657, 10-12=-2148/495 | less except when shown<br>224, 5-6=-4196/1011, 6-8<br>e-930/3877, 16-17=-717/3<br>412/2214, 12-13=-321/14<br>5, 5-16=-216/1222, 6-16=<br>/137, 8-13=-1117/271, 9- | =-3557/885,<br>643,<br>35<br>-191/973,<br>13=-147/710,                                     |  |  |   |                                    |
| NOTES-<br>1) Unbalanced roof live<br>2) Wind: ASCE 7-10; V<br>gable end zone and<br>DOL=1.60<br>3) Provide adequate di<br>4) This truss has been<br>5) * This truss has been<br>will fit between the b | e loads have been considered for this de<br>/ult=130mph Vasd=103mph; TCDL=6.0<br>C-C Exterior(2) zone;C-C for members<br>rainage to prevent water ponding.<br>designed for a 10.0 psf bottom chord liv<br>n designed for a live load of 20.0psf on i<br>bottom chord and any other members, w                 | sign.<br>ssf; BCDL=6.0psf; h=30ft;<br>and forces & MWFRS for<br>e load nonconcurrent with<br>the bottom chord in all are<br>ith BCDL = 10.0psf.                   | Cat. II; Exp B; Encl<br>reactions shown; Lu<br>n any other live load<br>as where a rectang | osed; MWFR<br>umber DOL=′<br>s.<br>le 3-6-0 tall b | S (envelope)<br>1.60 plate grip<br>y 2-0-0 wide  | OPTESS  | ROLIN                              |

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=116.

One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
connection is for uplift only and does not consider lateral forces.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



![](_page_11_Figure_0.jpeg)

| <b> </b>   | 5-0-0 9-3-8 14-8-8  | 21-4-12  | 27-6-0  | 33-6-0                                       | 34-10-8  | 40-9-8   | 47-0-0  |                                    |
|--|---|--|---|--|--|--|---|------------------------------------|
| Plate Offsets (X,Y)  | [2:0-0-0,0-0-4], [3:0-5-0,0-3-4], [7:0-0-0,   | 0-0-0], [9:0-6-0,0-3-2], [1:   | 5:0-3-0,0-3-8], [19:0-5   | -4,0-4-4]                                    | 1-4-0  | 5-11-0   | 0-2-0   |                                    |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0   | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014   | CSI.<br>TC 0.51<br>BC 0.97<br>WB 0.75<br>Matrix-MS   | DEFL.<br>Vert(LL) -0<br>Vert(CT) -0<br>Horz(CT) 0                           | in (loc)<br>.26 17-18<br>.53 17-18<br>.21 12 | l/defl L<br>>999 24<br>>999 18<br>n/a n/                       | /d<br>40<br>30<br>/a                                 | PLATES<br>MT20<br>Weight: 381 lb  | <b>GRIP</b><br>244/190<br>FT = 20% |
| LUMBER-<br>TOP CHORD 2x6 SI<br>BOT CHORD 2x6 SI<br>WEBS 2x4 SI<br>WEDGE<br>Left: 2x4 SP No.3   | P No.2<br>P No.2<br>P No.3  |  | BRACING-<br>TOP CHORD<br>BOT CHORD<br>WEBS                                  | Structu<br>except<br>Rigid co<br>1 Row       | ral wood shea<br>end verticals,<br>eiling directly<br>at midpt | athing direct<br>and 2-0-0 c<br>applied or 2<br>8-16 | ly applied or 3-0-7 c<br>c purlins (3-7-6 ma<br>-2-0 oc bracing.<br>i, 9-15, 9-14 | oc purlins,<br>x.): 7-9.           |
| REACTIONS. (siz<br>Max H<br>Max L<br>Max C   | ze) 2=0-3-8, 12=Mechanical<br>Horz 2=234(LC 12)<br>Jplift 2=-220(LC 12), 12=-116(LC 13)<br>Grav 2=1935(LC 1), 12=1874(LC 1)   |  |   |  |  |  |   |                                    |
| FORCES.         (lb) - Max           TOP CHORD         2-3=           8-9=         8-9=           BOT CHORD         2-20           16-1         3-19           8-17         10-1 | . Comp./Max. Ten All forces 250 (lb) or<br>-5033/1106, 3-5=-4140/950, 5-6=-4186/<br>-2836/773, 9-10=-2121/588, 10-11=-187<br>=-1137/4747, 19-20=-1131/4746, 18-19=<br>7=-547/2932, 15-16=-318/1854, 14-15=-<br>=-1138/298, 5-19=-353/150, 6-18=0/262<br>(=-183/999, 8-16=-1238/365, 9-16=-314/<br>4=-84/434, 10-13=-723/257, 11-13=-366 | less except when shown<br>988, 6-7=-3714/907, 7-8=<br>3/449, 11-12=-1815/442<br>899/3802, 17-18=-8260<br>318/1835, 13-14=-331/16<br>,6-17=-475/212, 7-17=-2<br>1528, 9-15=-341/98, 9-14<br>/1800 | 1.<br>-3537/903,<br>3801,<br>512<br>35/1283,<br>=-280/107,                  |  |  |  |   |                                    |
| NOTES-<br>1) Unbalanced roof liv<br>2) Wind: ASCE 7-10; '<br>gable end zone and<br>DOL=1.60<br>3) Provide adequate d<br>4) This truss has been                                   | e loads have been considered for this de<br>Vult=130mph Vasd=103mph; TCDL=6.0j<br>d C-C Exterior(2) zone;C-C for members<br>drainage to prevent water ponding.<br>designed for a 10.0 psf bottom chord liv  | sign.<br>osf; BCDL=6.0psf; h=30ft<br>and forces & MWFRS for<br>e load nonconcurrent witi   | ; Cat. II; Exp B; Enclos<br>reactions shown; Lur<br>h any other live loads. | sed; MWFR<br>nber DOL=1                      | S (envelope)<br>.60 plate grip                                 | X  | UNTH CA   | ROLIN                              |

- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=116.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_11_Picture_7.jpeg)

![](_page_11_Picture_9.jpeg)

![](_page_12_Figure_0.jpeg)

| L  | 6-8-8  | 12-4-12   | 18-6-0                        | 24-6-0   | 30-9-8   | 38-0-0   |                                    |
|--|--|---|-------------------------------|--|--|--|------------------------------------|
|  | 6-8-8  | 5-8-4   | 6-1-4                         | 6-0-0  | 6-3-8  | 7-2-8  | 1                                  |
| Plate Offsets (X,Y)  | [11:0-3-0,0-3-8], [13:0-2-8  | ,0-3-8]   |                               |  |  |  |                                    |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0 | SPACING-<br>Plate Grip DOL<br>Lumber DOL<br>Rep Stress Incr<br>Code IRC2015/TP   | 2-0-0 <b>CSI.</b><br>1.15 TC<br>1.15 BC<br>YES WB<br>I2014 Matr | 0.49<br>0.36<br>0.67<br>ix-MS | DEFL.         in           Vert(LL)         -0.07           Vert(CT)         -0.15           Horz(CT)         0.09 | (loc) I/defl L/d<br>13 >999 240<br>12-13 >999 180<br>9 n/a n/a   | PLATES<br>MT20<br>Weight: 324 lb   | <b>GRIP</b><br>244/190<br>FT = 20% |
| LUMBER-<br>TOP CHORD 2x6 SF<br>BOT CHORD 2x6 SF<br>WEBS 2x4 SF<br>REACTIONS. (siz<br>Max H<br>Max L<br>Max C                         | P No.2<br>P No.2<br>P No.3<br>e) 15=0-3-8, 9=Mechani<br>forz 15=-126(LC 13)<br>Jplift 15=-122(LC 12), 9=-1<br>Grav 15=1508(LC 1), 9=15 | cal<br>30(LC 13)<br>08(LC 1)                                    |                               | BRACING-<br>TOP CHORD<br>BOT CHORD<br>WEBS   | Structural wood sheathing<br>except end verticals, and 2<br>Rigid ceiling directly applied<br>1 Row at midpt | directly applied or 4-10-3<br>-0-0 oc purlins (5-7-1 ma<br>d or 10-0-0 oc bracing.<br>3-12, 4-12, 5-11, 6-11 | oc purlins,<br>ĸ.): 3-5.           |

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 1-2=-1821/429, 2-3=-2140/559, 3-4=-1672/528, 4-5=-1672/528, 5-6=-1576/484,
- 6-8=-1567/384, 1-15=-1452/363, 8-9=-1442/366
- BOT CHORD 13-14=-306/1619, 12-13=-258/1825, 11-12=-201/1393, 10-11=-262/1330
- WEBS 2-14=-754/264, 2-13=-49/415, 3-13=-100/741, 4-12=-374/166, 5-12=-108/696,
  - 5-11=-267/113, 6-10=-456/215, 1-14=-317/1621, 8-10=-277/1430

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Bearing at joint(s) 15 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=130.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 15. This
  connection is for uplift only and does not consider lateral forces.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_12_Picture_19.jpeg)

![](_page_12_Picture_20.jpeg)

![](_page_13_Figure_0.jpeg)

|  | 7-8-8   | 12-4-12 15-1-8                                     | 24-6-0  | 31-1-4  | 38-0-0   |
|--|---|--|---|---|--|
| Plate Offsets (X,Y)  | [4:0-6-0,0-3-2]   | 4-8-4 2-8-12                                       | 9-4-8   | 0-7-4   | 6-10-12  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014 | CSI.<br>TC 0.56<br>BC 0.47<br>WB 0.70<br>Matrix-MS | DEFL.<br>Vert(LL) -0.0<br>Vert(CT) -0.2<br>Horz(CT) 0.0 | in (loc) l/defl L/d<br>18 12-13 >999 240<br>00 12-13 >999 180<br>18 10 n/a n/a                                    | PLATES         GRIP           MT20         244/190           Weight: 330 lb         FT = 20%                                 |
| LUMBER-<br>TOP CHORD 2x6 S<br>BOT CHORD 2x6 S<br>WEBS 2x4 S  | P No.2<br>P No.2<br>P No.3  |  | BRACING-<br>TOP CHORD<br>BOT CHORD<br>WEBS              | Structural wood sheathing di<br>except end verticals, and 2-0<br>Rigid ceiling directly applied<br>1 Row at midpt | rectly applied or 4-9-9 oc purlins,<br>I-0 oc purlins (5-4-13 max.): 4-5.<br>or 10-0-0 oc bracing.<br>4-13, 5-12, 6-12, 7-12 |

REACTIONS. (size) 16=0-3-8, 10=0-3-8 Max Horz 16=-143(LC 13) Max Uplift 16=-138(LC 12), 10=-145(LC 13) Max Grav 16=1508(LC 1), 10=1508(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 1-3=-1962/471, 3-4=-2114/595, 4-5=-1583/524, 5-6=-1502/570, 6-7=-1583/486, 7-9=-1541/384, 1-16=-1444/376, 9-10=-1445/371
- BOT CHORD 14-15=-332/1732, 13-14=-201/1614, 12-13=-191/1389, 11-12=-265/1310
- WEBS 3-15=-672/259, 3-14=-28/279, 4-14=-199/692, 4-13=-257/186, 5-13=-83/506, 7-11=-480/216, 1-15=-335/1701, 9-11=-284/1422

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Bearing at joint(s) 16 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16 and 10. This connection is for uplift only and does not consider lateral forces.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_13_Picture_16.jpeg)

![](_page_13_Picture_18.jpeg)

![](_page_14_Figure_0.jpeg)

WEBS

1 Row at midpt

5-12: 2x4 SP No.2 or 2x4 SPF No.2 **REACTIONS.** (size) 16=0-3-8, 10=0-3-8 Max Horz 16=-160(LC 13) Max Uplift 16=-152(LC 12), 10=-158(LC 13)

Max Uplift 16=-152(LC 12), 10=-158(LC 13) Max Grav 16=1508(LC 1), 10=1508(LC 1)

 
 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 1-2=-1764/430, 2-3=-2141/582, 3-4=-1612/546, 4-5=-1384/523, 5-7=-1547/617, 7-8=-1582/496, 8-9=-1541/390, 1-16=-1457/373, 9-10=-1445/376

 BOT CHORD
 14-15=-310/1569, 13-14=-302/1901, 12-13=-158/1347, 11-12=-270/1309

 WEBS
 2-15=-772/277, 2-14=0/381, 3-14=-112/701, 3-13=-898/282, 4-13=-129/451, 5-13=-83/461, 5-12=-251/234, 7-12=-321/222, 8-11=-476/221, 1-15=-325/1590, 9-11=-289/1421

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Bearing at joint(s) 16 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

capacity of bearing surface.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16 and 10. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

ORT arriter and the second second SEAL 44925 minin November 5,2021

3-13, 5-12, 7-12, 8-12

ENGINEERING BY **TREENCO** A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932

![](_page_15_Figure_0.jpeg)

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 14=128.

One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9. This
connection is for uplift only and does not consider lateral forces.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_15_Picture_6.jpeg)

TREENCO AMITEK Affiliate 818 Soundside Road

Edenton, NC 27932

![](_page_16_Figure_0.jpeg)

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=119.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7. This connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_16_Picture_8.jpeg)

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![](_page_17_Figure_0.jpeg)

|  | 2-9-8                        | 8-3-1                                    | 10-11-14              | 13-8-10          | 19-6-8         | 25-4  | -8                                 | 3                             | 1-8-8             | 38-0-8         |                            |
|--|------------------------------|--|-----------------------|------------------|----------------|---|------------------------------------|-------------------------------|-------------------|----------------|----------------------------|
|  | 2-9-8                        | 5-5-9                                    | 2-8-13                | 2-8-12           | 5-9-14         | 5-10  | )-0                                | ' 6                           | 6-4-0             | 6-4-0          | 1                          |
| Plate Offsets (X,Y)  | ) [2:1-0-7                   | 7,0-2-8], [2:1-2-8,0                     | 0-2-2], [15:0-3-4     | l,0-2-0], [18:0  | 0-5-12,0-4-4]  |   |                                    |                               |                   |                |                            |
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0  |                              | SPACING-<br>Plate Grip DOL<br>Lumber DOL | 2-0-0<br>1.15<br>1.15 | CSI.<br>TC<br>BC | 0.69<br>0.90   | DEFL.<br>Vert(LL) -<br>Vert(CT) -   | in (lo<br>-0.22 12-1<br>-0.44 12-1 | c) l/defl<br>6 >999<br>6 >999 | L/d<br>240<br>180 | PLATES<br>MT20 | <b>GRIP</b><br>244/190     |
| BCDL 10.0  |                              | Code IRC2015/T                           | PI2014                | WB<br>Matrix     | 0.97<br>(-MS   | Horz(CT)  | 0.12                               | 2 n/a                         | n/a               | Weight: 327 lb | FT = 20%                   |
| LUMBER-  |                              |  |                       |                  |                | BRACING-  |                                    |                               |                   |                |                            |
| TOP CHORD 2x<br>8-'  | 6 SP No.2 * <br>11: 2x6 SP [ | Except*<br>DSS                           |                       |                  |                | TOP CHORD Structural wood sheathing directly applied or 3-5-13 oc purlins,<br>except end verticals, and 2-0-0 oc purlins (4-7-10 max.); 8-11. |                                    |                               |                   |                | oc purlins,<br>ax.): 8-11. |
| BOT CHORD 2x6 SP No.2 *Except*<br>2-22: 2x8 SP No.2 *2:0: 2x6 SP DSS 7-17: 2x4 SP No.3 |                              |  |                       | BOT CHORD        | ) Rigi<br>10-( | Rigid ceiling directly applied or 8-2-11 oc bracing. Except: 10-0-0 oc bracing: 17-18   |                                    |                               |                   |                |                            |
| WEBS 2x  | 4 SP No.3                    | - ,                                      | ,                     |                  |                | WEBS  | 1 R                                | ow at midpt                   | 1                 | 0-12           |                            |
| REACTIONS.   | (size) 12=                   | =0-3-8, 2=0-3-8                          |                       |                  |                |   |                                    |                               |                   |                |                            |

Max Horz 2=320(LC 12) Max Uplift 12=-198(LC 13), 2=-190(LC 12) Max Grav 12=1520(LC 2), 2=1580(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD
   2-3=-3072/714, 3-5=-2384/595, 5-6=-2342/713, 6-7=-1421/493, 7-8=-1899/528, 8-9=-1883/444, 9-10=-1115/299

   BOT CHORD
   2-21=-807/2721, 19-21=-807/2721, 18-19=-399/1613, 7-18=-91/822, 12-16=-299/1115
- WEBS 16-18=-390/1743, 8-18=-536/114, 8-16=-1163/406, 5-19=-269/198, 3-21=0/375, 3-19=-813/303, 6-19=-310/878, 9-16=-251/1363, 10-12=-1896/513

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12 and 2. This connection is for uplift only and does not consider lateral forces.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 8) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

![](_page_17_Figure_15.jpeg)

![](_page_17_Picture_17.jpeg)

![](_page_18_Figure_0.jpeg)

|  |                           | 2-9-8                              | 8-3-1  | 13-  | 8-10   | 19-6-8                                      |                      | 23-4-8                                | в ,                          | 26-0-0  | 1                             | 36-0                     | 0-0 13                        | 8-0-8                                 |
|--|---------------------------|------------------------------------|--|--|--|---|----------------------|---------------------------------------|------------------------------|---|-------------------------------|--------------------------|-------------------------------|---------------------------------------|
|  |                           | 2-9-8 '                            | 5-5-9  | 5-   | 5-9 '  | 5-9-14                                      |                      | 3-10-0                                | <u> </u>                     | 2-7-8   | <u> </u>                      | 10-0                     | 0-0                           | 2-0-8                                 |
| Plate Offsets (X,  | Y)                        | [2:1-2-8,0                         | -2-2], [2:1-0-7,0  | -2-8], [8:0-5-4,   | ,0-2-12], [11  | :0-3-0,0-3-4],                              | [16:0-5-4            | ,0-4-8], [ <sup>•</sup>               | 19:0-5-                      | 12,0-4-0                                      | ]                             |                          |                               |                                       |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0   | *                         | SP<br>Pla<br>Lur<br>Rej<br>Cod     | ACING-<br>tte Grip DOL<br>mber DOL<br>p Stress Incr<br>de IRC2015/TF | 2-0-0<br>1.15<br>1.15<br>YES<br>Pl2014                         | CSI.<br>TC<br>BC<br>WB<br>Matri  | 0.49<br>0.90<br>0.68<br>x-MS                | Di<br>Va<br>Ha       | EFL.<br>ert(LL)<br>ert(CT)<br>orz(CT) | in<br>-0.13<br>-0.27<br>0.11 | (loc)<br>22-27<br>22-27<br>13                 | l/defl<br>>999<br>>999<br>n/a | L/d<br>240<br>180<br>n/a | PLATES<br>MT20<br>Weight: 371 | <b>GRIP</b><br>244/190<br>lb FT = 20% |
| LUMBER-           TOP CHORD         2x6 SP No.2           BOT CHORD         2x6 SP No.2 *Except*           2-23: 2x8 SP No.2, 2-21: 2x6 SP DSS, 7-18: 2x4 SP No.3           WEBS         2x4 SP No.3 |                           |                                    |  | BI<br>TC<br>BC   | BRACING-         TOP CHORD       Structural wood sheathing directly applied or 3-5-14 oc purlins, except end verticals, and 2-0-0 oc purlins (5-8-6 max.): 8-11.         BOT CHORD       Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 8-11-14 oc bracing: 2-22         8-30 oc bracing: 20-22.       10-00 oc bracing: 18-19 |   |                      |                                       |                              | -14 oc purlins,<br>max.): 8-11.<br>ı, Except: |                               |                          |                               |                                       |
| REACTIONS.       (size)       2=0-3-8, 13=0-3-8         Max Horz       2=320(LC 12)         Max Uplift       2=-190(LC 12), 13=-198(LC 13)         Max Grav       2=1580(LC 1), 13=1509(LC 1)        |                           |                                    |  |  |  |   |                      |                                       |                              |   |                               |                          |                               |                                       |
| FORCES. (lb) -<br>TOP CHORD  | - Max.<br>2-3=-3<br>8-9=- | Comp./Ma<br>3073/709,<br>1624/422. | ax. Ten All for<br>, 3-5=-2382/590<br>, 9-10=-1624/42                | ces 250 (lb) or<br>, 5-6=-2345/7 <sup>-</sup><br>2. 10-11=-331 | <sup>-</sup> less except<br>10, 6-7=-141<br>/104, 11-12:   | when shown<br>7/487, 7-8=-<br>=-391/93, 12- | 1871/537<br>13=-1466 | ,<br>5/291                            |                              |   |                               |                          |                               |                                       |
| BOT CHORD  | 2-22=<br>14-16            | -803/272<br>=-274/10               | 1, 20-22=-803/2<br>91  | 721, 19-20=-3  | 94/1614, 7-  | 19=-125/870,                                | 16-17=-4             | 127/1748,                             |                              |   |                               |                          |                               |                                       |

WEBS 17-19=-27/4/1091 5-20=-272/205, 3-22=-2/378, 3-20=-815/303, 6-20=-313/872, 10-16=-192/960, 10-14=-1379/381

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 13. This connection is for uplift only and does not consider lateral forces.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 8) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

SEAL 044925 MGINEEPHER

![](_page_18_Picture_14.jpeg)

![](_page_19_Figure_0.jpeg)

|                      | 2-9-8 8-3-1                                 | 13-8-10 19-6-8                                | 21-4-8 26-             | -6-0 34-0-0                    | 38-0-8                               |  |  |  |
|----------------------|---|---|------------------------|--------------------------------|--------------------------------------|--|--|--|
| Plate Offects (X V)  |   | 5-5-9 ' $5-9-14$                              | 1-10-0' 5-             |                                | 4-0-8                                |  |  |  |
|                      | [2.1-0-7,0-2-0], [2.1-2-0,0-2-2], [7.0-4-1] | <u>, , , , , , , , , , , , , , , , , , , </u> | .0-4-0,0-4-0], [11.0-4 | -0,0-7-0], [20.0-0-0,0-4-0]    |                                      |  |  |  |
| LOADING (psf)        | <b>SPACING-</b> 2-0-0                       | CSI.  | DEFL. ir               | n (loc) l/defl L/d             | PLATES GRIP                          |  |  |  |
| TCLL 20.0            | Plate Grip DOL 1.15                         | TC 0.64                                       | Vert(LL) -0.17         | 20-21 >999 240                 | MT20 197/144                         |  |  |  |
| TCDL 10.0            | Lumber DOL 1.15                             | BC 0.90                                       | Vert(CT) -0.34         | 20-21 >999 180                 | MT18HS 244/190                       |  |  |  |
| BCLL 0.0 *           | Rep Stress Incr YES                         | WB 0.95                                       | Horz(CT) 0.11          | 13 n/a n/a                     |                                      |  |  |  |
| BCDL 10.0            | Code IRC2015/TPI2014                        | Matrix-MS                                     |                        |                                | Weight: 363 lb FT = 20%              |  |  |  |
| LUMBER-              |   | · ·   | BRACING-               |                                |                                      |  |  |  |
| TOP CHORD 2x6        | SP No.2 *Except*                            |   | TOP CHORD              | Structural wood sheathing di   | rectly applied or 3-5-14 oc purlins, |  |  |  |
| 8-11                 | : 2x6 SP DSS                                |   |                        | except end verticals, and 2-0  | )-0 oc purlins (5-8-7 max.): 8-11.   |  |  |  |
| BOT CHORD 2x6        | SP No.2 *Except*                            |   | BOT CHORD              | Rigid ceiling directly applied | or 8-3-4 oc bracing. Except:         |  |  |  |
| 2-24                 | I: 2x8 SP No.2, 2-22: 2x6 SP DSS, 7-19: 2   | x4 SP No.3                                    |                        | 10-0-0 oc bracing: 19-20       |                                      |  |  |  |
| WEBS 2x4             | SP No.3 *Except*                            |   | WEBS                   | 1 Row at midpt 8               | 8-18, 8-16, 9-16                     |  |  |  |
| 10-1                 | 6: 2x4 SP No.2 or 2x4 SPF No.2, 11-13: 2    | x4 SP No.2                                    |                        | 2 Rows at 1/3 pts              | 11-13                                |  |  |  |
|                      |   |   | JOINTS                 | 1 Brace at Jt(s): 25, 27       |                                      |  |  |  |
| REACTIONS. (         | size) 2=0-3-8, 13=0-3-8                     |   |                        |                                |                                      |  |  |  |
| Ma                   | (Horz 2=320(LC 12)                          |   |                        |                                |                                      |  |  |  |
| Max                  | C = 190(LC + 12), 13 = -198(LC + 13)        |   |                        |                                |                                      |  |  |  |
| Ma                   | (Grav 2=1580(LC 1), 13=1591(LC 2)           |   |                        |                                |                                      |  |  |  |
| FORCES. (lb) - Ma    | ax Comp /Max Ten - All forces 250 (lb) or   | less except when shown                        |                        |                                |                                      |  |  |  |
| TOP CHORD 2-         | 3=-3073/704. 3-5=-2382/585. 5-6=-2349/7     | 07. 6-7=-1415/483. 7-8=-191                   | 1/554.                 |                                |                                      |  |  |  |
| 8-                   | 9=-1478/405, 9-10=-1478/405                 |   | ,                      |                                |                                      |  |  |  |
| BOT CHORD 2-         | 23=-798/2721, 21-23=-798/2721, 20-21=-3     | 91/1653, 7-20=-172/1098, 16                   | 5-18=-389/1648,        |                                |                                      |  |  |  |
| 13                   | -16=-174/686                                |   |                        |                                |                                      |  |  |  |
| WEBS 18              | -20=-391/1770, 8-20=-272/144, 8-18=-652     | /185, 8-16=-390/134, 9-16=-6                  | 659/236,               |                                |                                      |  |  |  |
| 16                   | -25=-264/1433, 10-25=-210/1178, 5-21=-2     | 276/209, 3-23=-2/379, 3-21=-8                 | 314/302,               |                                |                                      |  |  |  |
| 6-                   | 21=-313/866, 11-26=-1100/294, 13-26=-15     | 80/409, 25-27=-87/414, 26-2                   | 7=-591/140,            |                                |                                      |  |  |  |
| 10                   | -27=-950/217                                |   |                        |                                |                                      |  |  |  |
| NOTES                |   |   |                        |                                | MILLIN                               |  |  |  |
| 1) Unbalanced roof   | live leads have been considered for this de | sian  |                        |                                | WHY CARO                             |  |  |  |
| I) Ulibalaliceu luul |   | siyii.  |                        |                                |                                      |  |  |  |

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) All plates are MT20 plates unless otherwise indicated.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 13. This connection is for uplift only and does not consider lateral forces.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

![](_page_19_Picture_10.jpeg)

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Edenton, NC 27932

![](_page_20_Figure_0.jpeg)

|  | 9-9-10<br>9-9-10  | 19-0-0<br>9-2-6                                    | 25-6-0<br>6-6-0  | 32-0-0<br>6-6-0  | 38-0-8<br>6-0-8  |
|--|---|--|--|--|--|
| Plate Offsets (X,Y)  | [5:0-5-4,0-2-12]  |  |  | 1  |  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0 | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014   | CSI.<br>TC 0.48<br>BC 0.54<br>WB 0.93<br>Matrix-MS | DEFL.         in         (loc           Vert(LL)         -0.10         9-1           Vert(CT)         -0.20         14-1           Horz(CT)         0.06 | c) l/defl L/d<br>l2 >999 240<br>6 >999 180<br>9 n/a n/a  | PLATES         GRIP           MT20         244/190           Weight: 315 lb         FT = 20%                     |
| LUMBER-<br>TOP CHORD 2x6 SI<br>BOT CHORD 2x6 SI<br>WEBS 2x4 SI<br>REACTIONS. (siz<br>Max H<br>Max L<br>Max C                         | P No.2<br>P No.2<br>P No.3<br>te) 2=0-3-8, 9=0-3-8<br>Horz 2=319(LC 12)<br>Jplift 2=-186(LC 12), 9=-80(LC 9)<br>Grav 2=1577(LC 1), 9=1568(LC 2) |  | BRACING-<br>TOP CHORD Stru<br>exce<br>BOT CHORD Rigi<br>WEBS 1 Ro  | uctural wood sheathing direct<br>ept end verticals, and 2-0-0<br>id ceiling directly applied or<br>ow at midpt 3-1 | ctly applied or 4-0-14 oc purlins,<br>oc purlins (6-0-0 max.): 5-7.<br>8-10-14 oc bracing.<br>4, 5-12, 6-12, 7-9 |

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2669/567, 3-5=-1814/463, 5-6=-1368/415, 6-7=-1368/415

BOT CHORD 2-16=-661/2287, 14-16=-661/2287, 12-14=-354/1509, 9-12=-188/758

WEBS 3-16=0/403, 3-14=-901/352, 5-14=-81/604, 5-12=-324/144, 6-12=-429/198, 7-12=-182/1128, 7-9=-1442/359

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.8) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

![](_page_20_Picture_14.jpeg)

![](_page_20_Picture_15.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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![](_page_21_Figure_0.jpeg)

| Job                   | Truss             | Truss Type | Qty     | Ply        | WEST-SHERMAN 106-21-180                                |           |
|-----------------------|-------------------|------------|---------|------------|--|-----------|
|                       |                   |            |         |            |  | 148675529 |
| 29043-29043A          | B4GR              | Hip Girder | 1       | 2          |  |           |
|                       |                   |            |         | <b></b>    | Job Reference (optional)                               |           |
| 84 Components (Dunn), | Dunn, NC - 28334, |            | 8       | .520 s Aug | 27 2021 MiTek Industries, Inc. Thu Nov 4 12:58:34 2021 | Page 2    |
|                       |                   | ID:2bG     | 0dF?FQw | aXcMo iiu  | aPavMcRK-il 8M5lBiGxKInYv8apzTA3ubWm anxMvedcD         | dvMZap    |

### NOTES-

9) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 378 lb down and 86 lb up at 1-8-12, 317 lb down and 65 lb up at 3-8-12, and 431 lb down and 109 lb up at 5-8-12, and 4634 lb down and 544 lb up at 6-10-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

### LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
  - Vert: 1-5=-60, 5-7=-60, 7-8=-60, 12-17=-20, 10-11=-20, 9-10=-20 Concentrated Loads (lb)

Vert: 15=-4634(F) 20=-378(F) 21=-317(F) 22=-431(F)

![](_page_22_Picture_11.jpeg)

<sup>10)</sup> One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9. This connection is for uplift only and does not consider lateral forces.

![](_page_23_Figure_0.jpeg)

WEDGE

Right: 2x4 SP No.3

- REACTIONS. (size) 9=0-3-8, 2=0-3-8 Max Horz 2=71(LC 12) Max Uplift 9=-576(LC 8), 2=-449(LC 9) Max Grav 9=2677(LC 1), 2=2803(LC 1)
- FORCES.
   (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

   TOP CHORD
   2-3=-5351/1072, 3-4=-7976/1756, 4-5=-7976/1756, 5-7=-7820/1843, 7-8=-7820/1843, 8-9=-4940/1248
- BOT CHORD
   2-14=-878/4692, 13-14=-885/4680, 12-13=-1930/9065, 11-12=-1930/9065, 10-11=-1032/4300, 9-10=-1027/4297

   WEBS
   3-14=0/443, 3-13=-871/3732, 4-13=-761/401, 5-13=-1239/340, 5-12=0/537, 5-11=-1401/232, 7-11=-773/405, 8-11=-786/3967

#### NOTES-

 2-ply truss to be connected together with 10d (0.120"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9. This connection is for uplift only and does not consider lateral forces.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

#### Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 November 5,2021

Million,

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SEAL

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44925

ORTH

![](_page_23_Picture_21.jpeg)

| Job                   | Truss             | Truss Type | Qty     | Ply        | WEST-SHERMAN 106-21-180                                |           |
|-----------------------|-------------------|------------|---------|------------|--|-----------|
|                       |                   |            |         |            |  | 148675530 |
| 29043-29043A          | BH                | Hip Girder | 1       | 2          |  |           |
|                       |                   |            |         | <b>_</b>   | Job Reference (optional)                               |           |
| 84 Components (Dunn), | Dunn, NC - 28334, |            | 8       | 520 s Aug  | 27 2021 MiTek Industries, Inc. Thu Nov 4 12:58:40 2021 | Page 2    |
|                       |                   | ID:2bG0    | dF?FQwa | aXcMo_jiug | PayMcRK-XvLPc8qyI6hU0i?3U5wNiRJwbxjPEVTFKZ4xF          | RGyMZqj   |

#### NOTES-

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 114 lb down and 119 lb up at 6-0-0, 95 lb down and 118 lb up at 8-0-12, 95 lb down and 118 lb up at 12-0-12, 95 lb down and 118 lb up at 14-0-12, 95 lb down and 118 lb up at 18-0-12, 95 lb down and 118 lb up at 12-0-12, 95 lb down and 118 lb up at 18-0-12, 95 lb down and 118 lb up at 22-3-12, 95 lb down and 118 lb up at 24-3-12, 95 lb down and 118 lb up at 26-3-12, 95 lb down and 118 lb up at 24-3-12, 95 lb down and 118 lb up at 26-3-12, 95 lb down and 118 lb up at 22-3-12, 95 lb down and 118 lb up at 22-3-12, 95 lb down and 118 lb up at 22-3-12, 95 lb down and 118 lb up at 22-3-12, 95 lb down and 118 lb up at 22-3-12, 95 lb down and 118 lb up at 22-3-12, 95 lb down and 118 lb up at 28-3-12, and 95 lb down and 118 lb up at 30-3-12, and 28 lb down and 276 lb up at 32-3-12, 70 lb down at 18-0-12, 70 lb down at 12-0-12, 70 lb down at 10-0-12, 70 lb down at 12-0-12, 70 lb down at 22-3-12, 70 lb down at 23-3-12, and 70 lb down at 30-3-12, and 19 lb down and 88 lb up at 32-3-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-8=-60, 8-9=-60, 15-18=-20

Concentrated Loads (lb)

Vert: 14=-163(B) 3=-95(B) 12=-57(B) 5=-95(B) 8=-191(B) 10=54(B) 21=-95(B) 22=-95(B) 23=-95(B) 24=-95(B) 25=-95(B) 25=-95(B) 26=-95(B) 27=-95(B) 28=-95(B) 29=-95(B) 30=-95(B) 31=-95(B) 32=-95(B) 32=-95(B) 33=-57(B) 34=-57(B) 35=-57(B) 35

![](_page_24_Picture_9.jpeg)

![](_page_25_Figure_0.jpeg)

![](_page_25_Figure_1.jpeg)

| <b> </b>   | 8-0-0   | 15-4-15   | 22-11-9  | 30-4-8  | <u>36-0-8</u> <u>38-0-8</u><br><u>5-8-0</u> <u>2-0-0</u>  |
|--|---|---|--|---|---|
| Plate Offsets (X,Y)  | [11:0-3-8,0-2-0]  | 7 7 17  | 1010   | 1 4 14  | 200   |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0 | SPACING- 2-<br>Plate Grip DOL 1.<br>Lumber DOL 1.<br>Rep Stress Incr YI<br>Code IRC2015/TPI201                              | 0-0 <b>CSI.</b><br>15 TC 0.4<br>15 BC 0.6<br>ES WB 0.9<br>4 Matrix-MS | DEFL.           10         Vert(LL)         -0           167         Vert(CT)         -0           167         Horz(CT)         0           163         Horz(CT)         0 | in (loc) I/defl L/d<br>.19 14-15 >999 240<br>.39 14-15 >999 180<br>.09 10 n/a n/a                               | PLATES         GRIP           MT20         244/190           Weight: 257 lb         FT = 20%                      |
| LUMBER-<br>TOP CHORD 2x6 S<br>BOT CHORD 2x6 S<br>WEBS 2x4 S  | P No.2<br>P No.2<br>P No.3  |   | BRACING-<br>TOP CHORD<br>BOT CHORD<br>WEBS   | Structural wood sheathing c<br>except end verticals, and 2-<br>Rigid ceiling directly applied<br>1 Row at midot | lirectly applied or 4-1-6 oc purlins,<br>0-0 oc purlins (3-7-7 max.): 3-7, 8-9.<br>0 r 9-0-11 oc bracing.<br>4-14 |
| REACTIONS. (si<br>Max<br>Max<br>Max  | ze) 2=0-3-8, 10=0-3-8<br>Horz 2=108(LC 12)<br>Uplift 2=-154(LC 9), 10=-148(L0<br>Grav 2=1577(LC 1), 10=1515(L               | C 8)<br>.C 1)   |  |   |   |
| FORCES.         (lb) - Max           TOP CHORD         2-3:           8-9:         8-9:  | Comp./Max. Ten All forces 2<br>=-2753/574, 3-4=-3475/780, 4-6<br>=-2008/430, 9-10=-1411/295<br>7. 455/2378, 15 47, 458/2372 | 250 (lb) or less except whe<br>=-3436/770, 6-7=-3438/77               | en shown.<br>2, 7-8=-2579/546,   |   |   |

 
 BOT CHORD
 2-17=-455/2378, 15-17=-458/2372, 14-15=-664/3473, 12-14=-426/2277, 11-12=-392/1863

 WEBS
 3-17=0/304, 3-15=-286/1362, 4-15=-506/226, 6-14=-490/218, 7-14=-275/1401, 8-12=-103/526, 8-11=-1382/363, 9-11=-501/2333

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_25_Figure_13.jpeg)

![](_page_25_Picture_15.jpeg)

![](_page_26_Figure_0.jpeg)

Scale = 1:69.0

![](_page_26_Figure_2.jpeg)

|  | 10-0-0  | <u>19-2-4</u>                                      | 28-4-8   | 34-8-8 38-0-8                 |
|--|---|--|--|-------------------------------|
|  | 10-0-0  | 9-2-4  | 9-2-4  | 6-4-0 3-4-0                   |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           TCDL         10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IBC2015/TPI2014 | CSI.<br>TC 0.67<br>BC 0.59<br>WB 0.63<br>Matrix-MS | DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.14         13         >999         240           Vert(CT)         -0.29         11-13         >999         180           Horz(CT)         0.08         9         n/a         n/a | H PLATES GRIP<br>MT20 197/144 |

TOP CHORD

BOT CHORD

| TOP CHORD | 2x6 SP No.2                       |
|-----------|-----------------------------------|
| BOT CHORD | 2x6 SP No.2                       |
| WEBS      | 2x4 SP No.3 *Except*              |
|           | 8-10: 2x4 SP No.2 or 2x4 SPF No.2 |

Structural wood sheathing directly applied or 3-3-1 oc purlins, except end verticals, and 2-0-0 oc purlins (3-8-2 max.): 3-6, 7-8. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 9=0-3-8 Max Horz 2=140(LC 12) Max Uplift 2=-121(LC 12), 9=-119(LC 9) Max Grav 2=1577(LC 1), 9=1515(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-2655/561, 3-4=-2941/698, 4-6=-2939/697, 6-7=-2477/541, 7-8=-2224/480, 8-9=-1439/320

2-15=-445/2267, 13-15=-448/2260, 11-13=-412/2175, 10-11=-454/2129 BOT CHORD 3-15=0/398, 3-13=-224/931, 4-13=-652/290, 6-13=-208/984, 6-11=0/336, WEBS 7-10=-1327/362, 8-10=-552/2554

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_26_Picture_19.jpeg)

![](_page_26_Picture_21.jpeg)

![](_page_27_Figure_0.jpeg)

Scale = 1:69.0

![](_page_27_Figure_2.jpeg)

| L  | 12-0-0   | 19-2-4   | 26-4-8   | 33-4-8  | 38-0-8   |  |  |  |  |
|--|--|--|--|---|--|--|--|--|--|
|  | 12-0-0   | 7-2-4  | 7-2-4  | 7-0-0   | 4-8-0  |  |  |  |  |
| Plate Offsets (X,Y)  | [10:0-1-12,0-2-0]  |  |  |   |  |  |  |  |  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0   | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014  | <b>CSI.</b><br>TC 0.37<br>BC 0.61<br>WB 0.64<br>Matrix-MS  | DEFL.         in         (loc)           Vert(LL)         -0.12         15-18         :           Vert(CT)         -0.27         15-18         :           Horz(CT)         0.08         9 | l/defl L/d<br>>999 240<br>>999 180<br>n/a n/a       | PLATES         GRIP           MT20         197/144           Weight: 272 lb         FT = 20% |  |  |  |  |
| LUMBER-<br>TOP CHORD 2x6 SF<br>BOT CHORD 2x6 SF<br>WEBS 2x4 SF<br>8-10: 2  | BRACING-     TOP CHORD     2x6 SP No.2     TOP CHORD     Structural wood sheathing directly applied or 4-3-4 oc purlins, except end verticals, and 2-0-0 oc purlins (4-5-12 max.): 4-6, 7-8.       WEBS     2x4 SP No.3 *Except*<br>8-10: 2x4 SP No.2 or 2x4 SPF No.2     BOT CHORD     Rigid ceiling directly applied or 9-4-4 oc bracing.  |  |  |   |  |  |  |  |  |
| REACTIONS.         (size)         2=0-3-8, 9=0-3-8           Max Horz         2=171(LC 12)           Max Uplift         2=-141(LC 12), 9=-120(LC 13)           Max Grav         2=1577(LC 1), 9=1515(LC 1)   |  |  |  |   |  |  |  |  |  |
| FORCES.         (lb) - Max.           TOP CHORD         2-3=           7-8=           BOT CHORD         2-15           WEBS         3-15           7-10  | FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       2-3=-2737/656, 3-4=-2394/553, 4-5=-2415/622, 6-7=-2348/533, 7-8=-2303/500, 8-9=-1449/335         BOT CHORD       2-15=-605/2383, 13-15=-399/2073, 11-13=-393/2028, 10-11=-480/2236         WEBS       3-15=-354/235, 4-15=0/480, 4-13=-158/571, 5-13=-508/224, 6-13=-143/630, 6-11=0/375, 7-10=-1249/372, 8-10=-571/2625 |  |  |   |  |  |  |  |  |
| NOTES-<br>1) Unbalanced roof liv.<br>2) Wind: ASCE 7-10; V<br>gable end zone and<br>DOL=1.60<br>3) Provide adequate d<br>4) This truss has been<br>5) * This truss has been<br>will fit between the I<br>6) Bearing at joint(s) 9<br>capacity of bearing | e loads have been considered for this de<br>/ult=130mph Vasd=103mph; TCDL=6.0<br>I C-C Exterior(2) zone;C-C for members<br>rainage to prevent water ponding.<br>designed for a 10.0 psf bottom chord live<br>n designed for a live load of 20.0psf on t<br>sottom chord and any other members, w<br>considers parallel to grain value using A<br>surface.  | sign.<br>bsf; BCDL=6.0psf; h=30ft;<br>and forces & MWFRS for<br>e load nonconcurrent with<br>he bottom chord in all are<br>th BCDL = 10.0psf.<br>.NSI/TPI 1 angle to grain f | Cat. II; Exp B; Enclosed; MWFRS<br>eactions shown; Lumber DOL=1.6<br>any other live loads.<br>as where a rectangle 3-6-0 tall by 2<br>prmula. Building designer should v                   | (envelope)<br>50 plate grip<br>2-0-0 wide<br>verify | OR FESSION N   |  |  |  |  |

One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_27_Picture_6.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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![](_page_28_Figure_0.jpeg)

|  |  | 7-3-10  | 14-0-0   | 1   | 24-4-8  |  | 31-   | -4-8  | 38-0-8   |                                    |
|--|--|---|--|---|---|--|---|---|--|------------------------------------|
|  | 1  | 7-3-10  | 6-8-7  |   | 10-4-7  |  | 7-  | 0-0   | 6-8-0  | 1                                  |
| Plate Offsets (  | X,Y)   | [11:0-1-12,0-2-0], [14:0-   | 3-12,0-4-8]  |   |   |  |   |   |  |                                    |
| LOADING (ps<br>TCLL 20.<br>TCDL 10.<br>BCLL 0.<br>BCDL 10.   | sf)<br>.0<br>.0<br>.0 *<br>.0  | SPACING-<br>Plate Grip DOL<br>Lumber DOL<br>Rep Stress Incr<br>Code IRC2015/T | 2-0-0<br>1.15<br>1.15<br>YES<br>PI2014                   | <b>CSI.</b><br>TC 0.55<br>BC 0.67<br>WB 0.63<br>Matrix-MS | DEFL.<br>Vert(LL) -0.<br>Vert(CT) -0.<br>Horz(CT) 0 | in (loc)<br>.17 12-14<br>.34 12-14<br>.07 10 | l/defl<br>>999<br>>999<br>n/a                                 | L/d<br>240<br>180<br>n/a                              | PLATES<br>MT20<br>Weight: 279 lb   | <b>GRIP</b><br>197/144<br>FT = 20% |
| LUMBER-<br>TOP CHORD         2x6 SP No.2           BOT CHORD         2x6 SP No.2           WEBS         2x4 SP No.3 *Except*           9-11: 2x4 SP No.2 or 2x4 SPF No.2 |  |   |  |   | BRACING-<br>TOP CHORD<br>BOT CHORD<br>WEBS          | Struct<br>excep<br>Rigid<br>1 Rov            | tural wood s<br>ot end vertica<br>ceiling direc<br>v at midpt | heathing dire<br>als, and 2-0-<br>tly applied o<br>6- | ectly applied or 4-3-7 c<br>0 oc purlins (4-5-7 ma:<br>or 9-5-1 oc bracing.<br>-14, 6-12 | oc purlins,<br>x.): 5-7, 8-9.      |
| REACTIONS.   | REACTIONS. (size) 2=0-3-8, 10=0-3-8<br>Max Horz 2=210(LC 12)<br>Max Uplift 2=-158(LC 12), 10=-142(LC 13)<br>Max Grav 2=1577(LC 1), 10=1515(LC 1) |   |  |   |   |  |   |   |  |                                    |
| FORCES. (Ib<br>TOP CHORD   | o) - Max.<br>2-3=-:<br>8-9=-:  | Comp./Max. Ten All fc<br>2739/614, 3-5=-2232/54<br>2278/504, 9-101439/3       | rces 250 (lb) or l<br>4, 5-6=-1914/534                   | ess except when shown.<br>4, 6-7=-1888/518, 7-8=-2        | 201/515,  |  |   |   |  |                                    |
| BOT CHORD<br>WEBS  | 2-15=<br>3-14=<br>8-12=  |   | 2368, 12-14=-43<br>, 6-14=-327/143,<br>63, 9-11=-570/2   | 4/2024, 11-12=-490/223<br>6-12=-368/134, 7-12=-3<br>571   | 3<br>6/580,   |  |   |   |  |                                    |
| NOTES-<br>1) Unbalanced<br>2) Wind: ASCE<br>gable end z<br>DOL=1.60  | I roof live<br>∃ 7-10; V<br>one and  | loads have been consid<br>ult=130mph Vasd=103n<br>C-C Exterior(2) zone;C-     | lered for this des<br>nph; TCDL=6.0ps<br>C for members a | ign.<br>sf; BCDL=6.0psf; h=30ft;<br>nd forces & MWFRS for | Cat. II; Exp B; Enclos<br>reactions shown; Lun      | sed; MWFI<br>nber DOL=                       | RS (envelop<br>=1.60 plate ç                                  | e)<br>grip  |  |                                    |

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

- 6) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_28_Picture_8.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see MSISTPT (Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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![](_page_29_Figure_0.jpeg)

|  | 8-3-10  |   | 16-0-0  |  |   | 22-4-8                                    |                              | 29-4-8                                |  | 38-0-8  |   |                                    |
|--|---|---|---|--|---|---|------------------------------|---------------------------------------|--|---|---|------------------------------------|
|  | 1   | 8-3-10  |   | 7-8-7  |   | 6-4-7                                     | 1                            |                                       | 7-0-0  | I   | 8-8-0   |                                    |
| Plate Offsets (2   | X,Y)  | [10:0-3-8,0-2-0]  |   |  |   |   |                              |                                       |  |   |   |                                    |
| LOADING (ps<br>TCLL 20.<br>TCDL 10.<br>BCLL 0.<br>BCDL 10.           | if)<br>.0<br>.0 *<br>.0 *                               | SPACING-<br>Plate Grip DOL<br>Lumber DOL<br>Rep Stress Incr<br>Code IRC2015/TPI:  | 2-0-0<br>1.15<br>1.15<br>YES<br>2014                                | CSI.<br>TC 0.<br>BC 0.<br>WB 0.<br>Matrix-M                            | .65<br>.49<br>.92<br>IS                                   | DEFL.<br>Vert(LL)<br>Vert(CT)<br>Horz(CT) | in<br>-0.11<br>-0.21<br>0.07 | (loc)<br>10-12<br>13-15<br>9          | l/defl<br>>999<br>>999<br>n/a                    | L/d<br>240<br>180<br>n/a                            | PLATES<br>MT20<br>Weight: 282 lb  | <b>GRIP</b><br>197/144<br>FT = 20% |
| LUMBER-<br>TOP CHORD<br>BOT CHORD<br>WEBS                            | 2x6 SP<br>2x6 SP<br>2x4 SP<br>8-10: 2:                  | No.2<br>No.2<br>No.3 *Except*<br>x4 SP No.2 or 2x4 SPF No   | .2  |  |   | BRACING-<br>TOP CHOR<br>BOT CHOR<br>WEBS  | RD<br>RD                     | Structu<br>except<br>Rigid c<br>1 Row | ral wood<br>end verti<br>eiling dire<br>at midpt | sheathing di<br>cals, and 2-0<br>ectly applied<br>8 | rectly applied or 4-2-10<br>0-0 oc purlins (3-8-15 m<br>or 9-3-0 oc bracing.<br>3-9, 3-13, 5-12 | oc purlins,<br>ax.): 5-6, 7-8.     |
| REACTIONS.   | (size)<br>Max Ho<br>Max Up<br>Max G                     | e) 2=0-3-8, 9=0-3-8<br>orz 2=248(LC 12)<br>plift 2=-172(LC 12), 9=-164<br>rav 2=1577(LC 1), 9=1515  | 4(LC 13)<br>5(LC 1)   |  |   |   |                              |                                       |  |   |   |                                    |
| FORCES. (Ib<br>TOP CHORD<br>BOT CHORD<br>WEBS                        | o) - Max.<br>2-3=-2<br>8-9=-<br>2-15=<br>3-15=<br>7-10= | Comp./Max. Ten All forco<br>2717/606, 3-5=-2073/530,<br>1429/373<br>-627/2341, 13-15=-627/23<br>-0/309, 3-13=-684/279, 5-1<br>1030/379, 8-10=-555/247 | es 250 (lb) or<br>5-6=-1734/51<br>41, 12-13=-3<br>3=-51/537, 6<br>7 | less except wh<br>17, 6-7=-2032/5<br>84/1752, 10-12<br>-12=-25/501, 7- | nen shown.<br>i15, 7-8=-221<br>=-483/2178<br>i12=-548/139 | 10/495,<br>9,                             |                              |                                       |  |   |   |                                    |
| NOTES-<br>1) Unbalanced<br>2) Wind: ASCE<br>gable end zo<br>DOL=1.60 | I roof live<br>E 7-10; V<br>one and                     | loads have been consider<br>ult=130mph Vasd=103mph<br>C-C Exterior(2) zone;C-C f  | ed for this de<br>n; TCDL=6.0p<br>for members                       | esign.<br>osf; BCDL=6.0p:<br>and forces & M\                           | osf; h=30ft; Ca<br>WFRS for re                            | at. II; Exp B; En<br>actions shown;       | closed<br>Lumbe              | ; MWFR<br>er DOL=′                    | S (envelo<br>1.60 plate                          | ope)<br>grip  |   |                                    |

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

- 6) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

![](_page_29_Picture_8.jpeg)

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![](_page_30_Figure_0.jpeg)

| · · ·   | 2-9-8 3-9-10 3-9-10   | 7-7-4   | 1-6-8   | 7-10-0   |   | 10-8-0   |   |
|---|---|---|---|--|---|--|---|
| Plate Offsets (X,Y)   | [2:1-2-8,0-2-2], [2:1-0-7,0-2-8], [9:0-2-1  | 2,0-5-0], [19:0-6-4,0-4-12]   | ]   |  |   |  |   |
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 *<br>BCDL 10.0                                  | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014  | CSI.<br>TC 0.43<br>BC 0.86<br>WB 0.85<br>Matrix-MS  | DEFL.<br>Vert(LL) -0<br>Vert(CT) -0<br>Horz(CT) 0                 | in (loc)<br>12 20-22<br>25 20-22<br>.10 13     | l/defl L/d<br>>999 240<br>>999 180<br>n/a n/a   | PLATES<br>MT20<br>Weight: 355 lb   | <b>GRIP</b><br>244/190<br>FT = 20%            |
| LUMBER-<br>TOP CHORD 2x6<br>BOT CHORD 2x6<br>2-24<br>WEBS 2x4<br>REACTIONS. (s<br>Max<br>Max<br>Max | SP No.2<br>SP No.2 *Except*<br>: 2x8 SP No.2, 2-21,13-17: 2x6 SP DSS, 7<br>SP No.3<br>size) 13=0-3-8, 2=0-3-8<br>< Horz 2=287(LC 12)<br>< Uplif 13=-184(LC 13), 2=-186(LC 12)<br>< Gray 13=1509(I C 1)  | 7-18: 2x4 SP No.3   | BRACING-<br>TOP CHORD<br>BOT CHORD<br>WEBS                        | Structu<br>except<br>Rigid c<br>1 Row<br>1 Row | ural wood sheathing d<br>end verticals, and 2-<br>æiling directly applied<br>at midpt<br>at midpt | directly applied or 3-7-10<br>-0-0 oc purlins (4-8-10 n<br>d or 8-11-11 oc bracing.<br>7-19<br>4-20, 7-20, 8-19, 11-13 | 0 oc purlins,<br>nax.): 6-8, 9-12.<br>Except: |
| FORCES. (lb) - Ma<br>TOP CHORD 2<br>8-6<br>BOT CHORD 2<br>13<br>WEBS 4-2<br>8-<br>11                | ax. Comp./Max. Ten All forces 250 (lb) o<br>3=-3239/762, 3-4=-2718/665, 4-6=-2008/5<br>9=-2316/603, 9-10=-2068/475, 10-11=-127<br>23=-825/2880, 22-23=-825/2880, 20-22=-6<br>-16=-327/1278<br>22=-27/484, 4-20=-819/306, 6-20=-45/474<br>16=-141/659, 9-16=-1380/442, 3-22=-605/<br>-13=-1826/469 | less except when shown<br>36, 6-7=-1699/536, 7-8=-<br>8/327<br>52/2397, 19-20=-379/164<br>7-20=-134/419, 16-19=-2<br>217, 3-23=-48/313, 10-16 | n.<br>1633/518,<br>49, 16-18=-90/520,<br>295/1131,<br>=-236/1255, |  |   |  |   |

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 4x4 MT20 unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13 and 2. This connection is for uplift only and does not consider lateral forces.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

![](_page_30_Figure_11.jpeg)

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![](_page_31_Figure_0.jpeg)

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.

8) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

SEAL 044925 MGINEED, HELL November 5,2021

![](_page_31_Picture_5.jpeg)

![](_page_32_Figure_0.jpeg)

![](_page_32_Figure_1.jpeg)

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![](_page_33_Figure_0.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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![](_page_34_Figure_0.jpeg)

|              | <u>8-4-5</u><br>8-4-5 |           |               |         |        |     | i             |          |  |
|--------------|-----------------------|-----------|---------------|---------|--------|-----|---------------|----------|--|
| OADING (psf) | SPACING- 2-0-0        | CSI.      | DEFL.         | n (loc) | l/defl | L/d | PLATES        | GRIP     |  |
| CLL 20.0     | Plate Grip DOL 1.15   | TC 0.59   | Vert(LL) 0.0  | 7 4-7   | >999   | 240 | MT20          | 244/190  |  |
| CDL 10.0     | Lumber DOL 1.15       | BC 0.43   | Vert(CT) -0.1 | 4 4-7   | >687   | 180 |               |          |  |
| SCLL 0.0 *   | Rep Stress Incr NO    | WB 0.00   | Horz(CT) 0.0  | 1 2     | n/a    | n/a |               |          |  |
| 3CDL 10.0    | Code IRC2015/TPI2014  | Matrix-MP | · · · ·       |         |        |     | Weight: 44 lb | FT = 20% |  |

TOP CHORD

BOT CHORD

### LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No 2

REACTIONS. (size) 3=Mechanical, 2=0-4-9, 4=Mechanical

Max Horz 2=139(LC 8)

Max Uplift 3=-121(LC 12), 2=-106(LC 8)

Max Grav 3=237(LC 1), 2=450(LC 1), 4=173(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 3=121.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 13 lb down and 59 lb up at 2-9-7, 13 lb down and 59 lb up at 2-9-8, and 39 lb down and 102 lb up at 5-7-6, and 39 lb down and 102 lb up at 5-7-7 on top chord , and 4 lb down at 2-9-7, 4 lb down at 2-9-8, and 23 lb down at 5-7-6, and 23 lb down at 5-7-7 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-60, 4-5=-20

Concentrated Loads (lb)

Vert: 9=-25(F=-13, B=-13) 10=-6(F=-3, B=-3) 11=-41(F=-21, B=-21)

![](_page_34_Picture_23.jpeg)

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

![](_page_34_Picture_24.jpeg)

![](_page_35_Figure_0.jpeg)

| LOADING (psf) | SPACING- 2-0-0       | CSI.      | DEFL. in (loc) I/defl L/d   | PLATES GRIP            |
|---------------|----------------------|-----------|-----------------------------|------------------------|
| TCLL 20.0     | Plate Grip DOL 1.15  | TC 0.43   | Vert(LL) -0.01 5-9 >999 240 | MT20 244/190           |
| TCDL 10.0     | Lumber DOL 1.15      | BC 0.25   | Vert(CT) -0.02 5-9 >999 180 |                        |
| BCLL 0.0 *    | Rep Stress Incr NO   | WB 0.00   | Horz(CT) 0.01 3 n/a n/a     |                        |
| BCDL 10.0     | Code IRC2015/TPI2014 | Matrix-MP |                             | Weight: 44 lb FT = 20% |
|               | 1                    |           |                             | -                      |

TOP CHORD2x6 SP No.2BOT CHORD2x6 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings Mechanical except (jt=length) 2=0-4-15, 5=0-4-15. (lb) - Max Horz 2=139(LC 8)

Max Horz 2=139(LC 8) Max Uplift All uplift 100 lb or less at joint(s) 4 except 3=-112(LC 12), 2=-116(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 3, 4, 5 except 2=407(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 3=112.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 13 lb down and 59 lb up at 2-9-8, and 13 lb down and 59 lb up at 2-9-9, and 34 lb down and 99 lb up at 5-7-7 on top chord, and 4 lb down at 2-9-8, and 4 lb down at 2-9-9, and 19 lb down at 5-7-7 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf) Vert: 1-3=-60, 4-6=-20 Concentrated Loads (lb)
  - Vert: 5=-11(F) 11=-4(F) 12=-6(F=-3, B=-3)

![](_page_35_Picture_22.jpeg)

![](_page_35_Picture_23.jpeg)


TOP CHORD

BOT CHORD

| LUMBER-   |             |
|-----------|-------------|
| TOP CHORD | 2x6 SP No.2 |
|           | 2VE ED No 2 |

BOT CHORD2x6 SP No.2WEBS2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=113(LC 12) Max Uplift 2=-45(LC 12), 8=-45(LC 13) Max Grav 2=1187(LC 1), 8=1187(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-1869/166, 3-4=-1504/251, 4-5=-105/797, 5-6=-105/797, 6-7=-1504/251, 7-8=-1869/166
- 7-8=-1869/166
- BOT CHORD
   2-12=-13/1557, 10-12=-13/1557, 8-10=-13/1557

   WEBS
   7-10=0/430, 3-12=0/430, 4-6=-2448/403

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (5.0 psf) on member(s). 3-4, 6-7, 4-6; Wall dead load (5.0 psf) on member(s).7-10, 3-12

6) Bottom chord live load (20.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 10-12

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.

8) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





| 1                   |                                |           |      | 25-0-0   |       |         |           |               |                           | 1         |
|---------------------|--------------------------------|-----------|------|----------|-------|---------|-----------|---------------|---------------------------|-----------|
|                     |                                |           |      | 25-0-0   |       |         |           |               |                           |           |
| Plate Offsets (X,Y) | [9:0-3-0,Edge], [25:0-1-12,0-2 | 2-0]      |      |          |       |         |           |               |                           |           |
|                     |                                |           |      |          |       |         |           |               |                           |           |
| LOADING (psf)       | SPACING- 2-                    | -0-0 CSI. |      | DEFL.    | in    | (loc)   | l/defl    | L/d           | PLATES                    | GRIP      |
| TCLL 20.0           | Plate Grip DOL 1               | 1.15 TC   | 0.05 | Vert(LL) | -0.00 | Ì16     | n/r       | 120           | MT20                      | 244/190   |
| TCDL 10.0           | Lumber DOL 1                   | 1.15 BC   | 0.02 | Vert(CT) | 0.00  | 16      | n/r       | 90            |                           |           |
| BCLL 0.0 *          | Rep Stress Incr Y              | YES WB    | 0.06 | Horz(CT) | 0.00  | 16      | n/a       | n/a           |                           |           |
| BCDL 10.0           | Code IRC2015/TPI20             | 14 Matri  | x-S  |          |       |         |           |               | Weight: 191 lb            | FT = 20%  |
|                     |                                |           |      |          |       |         |           |               | -                         |           |
| LUMBER-             |                                |           |      | BRACING- |       |         |           |               |                           |           |
| TOP CHORD 2x6 S     | PNo 2                          |           |      | TOP CHOR | סי    | Structu | al wood s | sheathing dir | rectly applied or 6-0-0 o | c nurlins |

 TOP CHORD
 Structural wood sheathing directly applied or 6-0-0 oc purlins.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 25-0-0.

2x6 SP No 2

2x4 SP No.3

(lb) - Max Horz 2=113(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 26, 27, 28, 29, 30, 22, 21, 20, 19, 18 Max Grav All reactions 250 lb or less at joint(s) 2, 24, 23, 26, 27, 28, 29, 30, 22, 21, 20, 19, 18, 16

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

BOT CHORD

OTHERS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.







|  | 7-8-0   |   | 21-10-4  | 22 <sub>1</sub> Q-0  | 29-8-0   |                                    |
|--|---|---|--|--|--|------------------------------------|
|  | 7-8-0   |   | 14-2-4   | 0-1-12   | 7-8-0  |                                    |
| Plate Offsets (X,Y)  | [8:0-0-0,0-0-11]  |   |  |  |  |                                    |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0 | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014   | <b>CSI.</b><br>TC 0.45<br>BC 0.77<br>WB 0.59<br>Matrix-MS           | <b>DEFL.</b> in<br>Vert(LL) -0.37<br>Vert(CT) -0.58<br>Horz(CT) 0.01 | l (loc) l/defl L/d<br>9-11 >716 240<br>9-11 >453 180<br>9 n/a n/a                  | PLATES<br>MT20<br>Weight: 191 lb                       | <b>GRIP</b><br>244/190<br>FT = 20% |
| LUMBER-<br>TOP CHORD 2x6 SF<br>BOT CHORD 2x6 SF<br>WEBS 2x4 SF   | P No.2<br>P No.2<br>P No.3  |   | BRACING-<br>TOP CHORD<br>BOT CHORD<br>WEBS                           | Structural wood sheathing di<br>Rigid ceiling directly applied<br>1 Row at midpt 5 | rectly applied or 6-0-0<br>or 6-0-0 oc bracing.<br>5-9 | oc purlins.                        |
| REACTIONS. (siz<br>Max H<br>Max U<br>Max G   | e) 2=0-3-8, 9=0-3-8<br>lorz 2=140(LC 16)<br> plift 2=-144(LC 12), 9=-189(LC 13)<br> rav 2=835(LC 23), 9=1610(LC 1)  |   |  |  |  |                                    |
| FORCES.         (lb) - Max.           TOP CHORD         2-3=-           BOT CHORD         2-11=           WEBS         3-11=         | Comp./Max. Ten All forces 250 (lb) o<br>1272/258, 3-5=-1279/446, 5-7=0/542, 7<br>=-186/1096, 9-11=-35/398, 8-9=-415/19<br>=-471/335, 5-11=-265/1042, 5-9=-1081/ | r less except when shown.<br>'-8=-144/582<br>4<br>216, 7-9=-520/345 |  |  |  |                                    |

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.







|              |                       |          | 8-4-0<br>8-4-0 |         |        |     |               |          |
|--------------|-----------------------|----------|----------------|---------|--------|-----|---------------|----------|
| OADING (psf) | <b>SPACING-</b> 2-0-0 | CSI.     | DEFL. ii       | ) (loc) | l/defl | L/d | PLATES        | GRIP     |
| TCLL 20.0    | Plate Grip DOL 1.15   | TC 0.11  | Vert(LL) -0.00 | 4-6     | >999   | 240 | MT20          | 244/190  |
| CDL 10.0     | Lumber DOL 1.15       | BC 0.09  | Vert(CT) -0.01 | 4-6     | >999   | 180 |               |          |
| BCLL 0.0 *   | Rep Stress Incr YES   | WB 0.07  | Horz(CT) 0.00  | 4       | n/a    | n/a |               |          |
| 3CDL 10.0    | Code IRC2015/TPI2014  | Matrix-P | ( <i>'</i>     |         |        |     | Weight: 50 lb | FT = 20% |

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. (size) 2=0-3-8, 4=0-3-8 Max Horz 2=43(LC 16)

Max Uplift 2=-62(LC 12), 4=-62(LC 13) Max Grav 2=390(LC 1), 4=390(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-383/101, 3-4=-383/101 TOP CHORD

BOT CHORD 2-6=-1/278, 4-6=-1/278

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



| L  | 7-8-0  |  | 21-10-4   | 22 <sub>1</sub> Q-0  | 29-8-0   |  |
|--|--|--|---|--|--|--|
|  | 7-8-0  |  | 14-2-4  | 0-1-12   | 7-8-0  |  |
| Plate Offsets (X,Y)  | [4:0-4-0,0-4-8], [8:0-0-0,0-0-11], [10:0-4   | -0,0-4-8]  |   |  |  |  |
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 *  | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES  | <b>CSI.</b><br>TC 0.45<br>BC 0.77<br>WB 0.59   | <b>DEFL.</b> ir<br>Vert(LL) -0.37<br>Vert(CT) -0.58<br>Horz(CT) 0.01                    | n (loc) l/defl L/d<br>/ 9-11 >716 240<br>8 9-11 >453 180<br>9 n/a n/a  | PLATES         GRIP           MT20         244/190 |  |
| BCDL 10.0  | Code IRC2015/1FI2014   | Matrix-MS  |   |  | Weight. 287 lb FT = 20%                            |  |
| LUMBER-<br>TOP CHORD2x6 SP No.2BRACING-<br>TOP CHORDBOT CHORD2x6 SP No.2DO CHORDStructural wood sheathing directly applied or 6-0-0 oc bracing.<br>BOT CHORDWEBS2x4 SP No.3WEBS1 Row at midptStructural wood sheathing directly applied or 6-0-0 oc bracing.<br>WEBSREACTIONS(size) 2=0-3-8, 9=0-3-8<br>Max Horz 2=140(LC 16)<br>Max Uplift 2=-144(LC 12), 9=-189(LC 13)<br>Max Grav 2=835(LC 23), 9=1610(LC 1)BRACING-<br>TOP CHORD<br>WEBSTOP CHORD<br>Rigid ceiling directly applied or 6-0-0 oc bracing.<br>WEBS |  |  |   |  |  |  |
| FORCES.         (lb) - Max.           TOP CHORD         2-3=-           BOT CHORD         2-11=           WEBS         3-11=   | Comp./Max. Ten All forces 250 (lb) o<br>1272/258, 3-5=-1279/446, 5-7=0/542, 7<br>=-186/1096, 9-11=-35/398, 8-9=-415/19<br>=-471/335, 5-11=-265/1042, 5-9=-1081/  | r less except when shown.<br>'-8=-144/582<br>4<br>216, 7-9=-520/345  |   |  |  |  |
| NOTES-<br>1) Unbalanced roof live<br>2) Wind: ASCE 7-10; V<br>gable end zone and<br>DOL=1.60<br>3) Truss designed for v<br>Gable End Details a<br>4) All plates are 2x4 M<br>5) Gable studs spaced   | e loads have been considered for this de<br>/ult=130mph Vasd=103mph; TCDL=6.0<br>C-C Exterior(2) zone;C-C for members<br>vind loads in the plane of the truss only.<br>s applicable, or consult qualified buildin<br>T20 unless otherwise indicated.<br>at 2-0-0 oc. | esign.<br>psf; BCDL=6.0psf; h=30ft;<br>and forces & MWFRS for n<br>For studs exposed to win<br>g designer as per ANSI/TF | Cat. II; Exp B; Enclosed<br>reactions shown; Lumbe<br>d (normal to the face), s<br>I 1. | t; MWFRS (envelope)<br>er DOL=1.60 plate grip<br>see Standard Industry | TH CAROUND   |  |

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
   7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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| Job                   | Truss             | Truss Type    | Qty | Ply        | WEST-SHERMAN 106-21-180                                |           |
|-----------------------|-------------------|---------------|-----|------------|--|-----------|
|                       |                   |               |     |            |  | 148675547 |
| 29043-29043A          | EGR               | COMMON GIRDER | 1   | 3          |  |           |
|                       |                   |               |     |            | Job Reference (optional)                               |           |
| 84 Components (Dunn), | Dunn, NC - 28334, |               | 8   | .520 s Aug | 27 2021 MiTek Industries, Inc. Thu Nov 4 12:59:01 2021 | Page 2    |

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## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-6=-60, 6-8=-60, 9-15=-20

Concentrated Loads (lb) Vert: 14=-3804(B) 13=-1854(B) 10=-1854(B) 18=-1854(B) 19=-1854(B) 20=-1854(B) 21=-1854(B) 22=-1488(B)





Design valid for Use only with with exerconnectors. This design is based only upon parameters shown, and is to an individual outing component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



| Job                   | Truss             | Truss Type          | Qty  | Ply        | WEST-SHERMAN 106-21-180                                |           |
|-----------------------|-------------------|---------------------|--|------------|--|-----------|
|                       |                   |                     |  |            |  | 148675548 |
| 29043-29043A          | FGR               | Roof Special Girder | 1  | 2          |  |           |
|                       |                   |                     |  | <b>_</b>   | Job Reference (optional)                               |           |
| 84 Components (Dunn), | Dunn, NC - 28334, |                     | 8  | .520 s Aug | 27 2021 MiTek Industries, Inc. Thu Nov 4 12:59:02 2021 | Page 2    |
|                       |                   | ID:2bG0dF           | ID:2bG0dF?FQwaXcMo_jiugPayMcRK-u7gjEg5l6tTMf4hHnjJXb4DgWpFNOQOUO_P5D?yMZqN |            |  |           |

## NOTES-

12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1210 lb down and 139 lb up at 1-9-12, 1210 lb down and 148 lb up at 3-9-12, 1466 lb down and 184 lb up at 7-9-12, and 1466 lb down and 184 lb up at 1-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 6-10=-20

Concentrated Loads (lb)

Vert: 11=-1210(B) 12=-1210(B) 13=-1466(B) 14=-1466(B) 15=-1466(B) 16=-1466(B)





Scale = 1:50.0



| 0 <sub>1</sub> 4 <sub>1</sub> 8 2-8-0  | 8-8-14   | 14-9-12  | 20-4-14   | 26-0-0  | 28-8-0                                  |
|--|--|--|---|---|---|
| 0-4-8 2-3-8  | 6-0-14   | 6-0-14   | 5-7-2   | 5-7-2   | 2-8-0                                   |
| Plate Offsets (X,Y)  | [2:0-1-10,0-2-0]   |  |   |   |   |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr NO<br>Code IRC2015/TPI2014 | CSI.<br>TC 0.43<br>BC 0.43<br>WB 0.84<br>Matrix-MS | DEFL.         in         (loc)         I/defl           Vert(LL)         -0.05         15-16         >999           Vert(CT)         -0.10         15-16         >999           Horz(CT)         0.02         9         n/a | L/d PLATES<br>240 MT20<br>180<br>n/a Weight: 184  | <b>GRIP</b><br>244/190<br>4 lb FT = 20% |
| LUMBER-<br>TOP CHORD 2x6 SI<br>BOT CHORD 2x6 SI<br>WEBS 2x4 SI   | P No.2<br>P No.2<br>P No.3   |  | BRACING-<br>TOP CHORD Structural woo<br>2-0-0 oc purlin<br>BOT CHORD Rigid ceiling di<br>WEBS 1 Row at mido   | d sheathing directly applied or 6-6<br>s (6-0-0 max.): 3-8.<br>rectly applied or 10-0-0 oc bracin<br>t 4-13 | 0-0 oc purlins, except<br>g.            |

REACTIONS. (size) 2=0-3-8, 13=0-3-8, 9=0-3-8 Max Horz 2=31(LC 39) Max Uplift 2=-99(LC 12), 13=-275(LC 9), 9=-90(LC 13)

Max Opint 2=-99(LC 12), 13=-275(LC 9), 9=-90(LC 13)Max Grav 2=634(LC 23), 13=1411(LC 1), 9=555(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-777/144, 3-4=-1039/240, 4-5=-59/403, 5-7=-105/507, 7-8=-750/178, 8-9=-836/147
- BOT CHORD 2-16=-97/648, 15-16=-104/647, 13-15=-185/1039, 12-13=-118/750, 11-12=-90/711, 9-11=-84/722
- WEBS 3-15=-95/408, 4-13=-1482/289, 5-13=-456/203, 7-13=-1309/283

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 13, and 9. This connection is for uplift only and does not consider lateral forces.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 22 lb down and 69 lb up at 2-8-0, 25 lb down and 60 lb up at 4-0-12, 25 lb down and 60 lb up at 6-0-12, 25 lb down and 60 lb up at 8-0-12, 25 lb down and 60 lb up at 10-0-12, 25 lb down and 60 lb up at 10-0-12, 25 lb down and 60 lb up at 14-7-4, 25 lb down and 60 lb up at 16-7-4, 25 lb down and 60 lb up at 12-0-12, 25 lb down and 60 lb up at 12-0-12, 25 lb down and 60 lb up at 20-7-4, 25 lb down and 60 lb up at 22-7-4, and 25 lb down and 60 lb up at 24-7-4, and 33 lb down and 78 lb up at 26-0-0 on top chord, and 26 lb down and 11 lb up at 28-8-0, 15 lb down at 40-12, 15 lb down at 60-12, 15 lb down at 16-7-4, 15 lb down at 16-7-4, 15 lb down at 12-7-4, and 15 lb down at 24-7-4, and 26 lb down at 26-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard





| Job                   | Truss             | Truss Type | Qty | Ply       | WEST-SHERMAN 106-21-180                                       |
|-----------------------|-------------------|------------|-----|-----------|---|
|                       |                   |            |     |           | 148675549   |
| 29043-29043A          | FH                | Hip Girder | 1   | 1         |   |
|                       |                   |            |     |           | Job Reference (optional)                                      |
| 84 Components (Dunn), | Dunn, NC - 28334, |            | 8   | 520 s Aug | 27 2021 MiTek Industries, Inc. Thu Nov 4 12:59:04 2021 Page 2 |

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## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-8=-60, 8-10=-60, 17-21=-20

Concentrated Loads (lb)

Vert: 6=-4(B) 16=11(B) 5=-4(B) 7=-4(B) 12=-11(B) 8=-5(B) 11=-17(B) 24=-4(B) 25=-4(B) 26=-4(B) 27=-4(B) 28=-4(B) 29=-4(B) 30=-4(B) 31=-4(B) 32=-4(B) 32=-4(B) 33=-11(B) 33=-11(B) 33=-11(B) 33=-11(B) 33=-11(B) 40=-11(B) 41=-11(B) 42=-11(B) 43=-11(B) 43=-11(B)





Scale = 1:52.5



| 0 <sub>1</sub> 4 <sub>1</sub> 8   | 4-8-0   | 14-9-12   | 17-5-8   | 24-0-0  | 25-10-8 28-8-0   |
|---|---|---|--|---|--|
| 0-4-8   | 4-3-8   | 10-1-12   | 2-7-12   | 6-6-8   | 1-10-8 2-9-8   |
| Plate Offsets (X,Y)   | [15:0-3-4,0-2-0]  |   |  |   | 1  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0          | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014   | <b>CSI.</b><br>TC 0.23<br>BC 0.94<br>WB 0.63<br>Matrix-MS   | DEFL.         in           Vert(LL)         -0.06           Vert(CT)         -0.13           Horz(CT)         0.03 | (loc) I/defl L/d<br>17-19 >999 240<br>17-19 >999 180<br>10 n/a n/a                              | PLATES         GRIP           MT20         244/190           Weight: 191 lb         FT = 20% |
| LUMBER-<br>TOP CHORD 2x6<br>BOT CHORD 2x6<br>6-16<br>WEBS 2x4<br>REACTIONS. (5  | SP No.2<br>SP No.2 *Except*<br>.9-12: 2x4 SP No.3<br>SP No.3<br>ize) 2=0-3-8. 10=0-3-8. 17=0-3-8  |   | BRACING-<br>TOP CHORD<br>BOT CHORD   | Structural wood sheathing di<br>2-0-0 oc purlins (6-0-0 max.)<br>Rigid ceiling directly applied | rectly applied or 6-0-0 oc purlins, except<br>: 3-8.<br>or 2-2-0 oc bracing.                 |
| Max Horz 2=46(LC 12)<br>Max Uplift 2=-77(LC 12), 10=-76(LC 13), 17=-204(LC 8)<br>Max Grav 2=568(LC 23), 10=496(LC 24), 17=1360(LC 1)          |   |   |  |   |  |
| FORCES.         (lb) - Ma           TOP CHORD         2-3           BOT CHORD         2-4           10         4-1           WEBS         4-1 | x. Comp./Max. Ten All forces 250 (lb) ol<br>=-657/109, 3-4=-524/135, 4-5=-77/563, 8-<br>9=-24/525, 17-19=-112/350, 6-15=-321/1-<br>12=-77/514<br>9=0/317, 4-17=-976/282, 5-17=-577/188,<br>5=-682/100, 8-14=0/284 | less except when shown.<br>9=-764/155, 9-10=-654/159<br>46, 14-15=-57/692, 13-14=-109<br>15-17=-460/226, 5-15=-170/65 | /716,<br>6,  |   |  |

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 10, and 17. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







Scale = 1:49.9



|  | 6-3-8   | 14-5-4<br>8-1-12                                   | 2-7-12  | 21-7-8  | <u>25-6-0</u> <u>28-3-8</u><br><u>3-10-8</u> <u>2-9-8</u>                                       |
|--|---|--|---|---|---|
| Plate Offsets (X,  | Y) [2:0-0-0,0-0-0], [5:0-0-0,0-0-0], [12:0-   | 5-8,0-4-0]   | 2112  | 400   | 200   |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0   | SPACING-         2-0-0           Plate Grip DOL         1.15           Lumber DOL         1.15           *         Rep Stress Incr         YES           Code         IRC2015/TPI2014 | CSI.<br>TC 0.21<br>BC 0.88<br>WB 0.48<br>Matrix-MS | DEFL. ir<br>Vert(LL) -0.03<br>Vert(CT) -0.05<br>Horz(CT) 0.02 | n (loc) I/defl L/d<br>8 10-11 >999 240<br>5 10-11 >999 180<br>2 7 n/a n/a   | PLATES         GRIP           MT20         244/190           Weight: 193 lb         FT = 20%    |
| LUMBER-           TOP CHORD         2x6 SP No.2           BOT CHORD         2x6 SP No.2 *Except*           4.13,6-9: 2x4 SP No.3           WEBS         2x4 SP No.3  |   |  | BRACING-<br>TOP CHORD<br>BOT CHORD                            | Structural wood sheathing<br>2-0-0 oc purlins (6-0-0 ma<br>Rigid ceiling directly applie<br>6-0-0 oc bracing: 12-13 | g directly applied or 6-0-0 oc purlins, except<br>ix.): 2-5.<br>ed or 6-0-0 oc bracing. Except: |
| REACTIONS.         (size)         1=0-3-8, 7=0-3-8, 14=0-3-8           Max Horz         1=-75(LC 13)           Max Uplift         1=-74(LC 12), 7=-94(LC 13), 14=-159(LC 8)           Max Grav         1=463(LC 23), 7=465(LC 1), 14=1426(LC 1)  |   |  |   |   |   |
| FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       1-2=-564/149, 2-3=-429/176, 4-5=-335/154, 5-6=-413/143, 6-7=-612/184         BOT CHORD       1-16=-40/432, 4-12=-632/192, 10-11=-154/746, 7-9=-104/487         WEBS       3-16=-43/501, 3-14=-1230/329, 12-14=-883/268, 3-12=-173/724, 4-11=-87/604, 6-11=-410/157 |   |  |   |   |   |

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1, 7, and 14. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







Scale = 1:49.8



|   | 8-3-8  | <u>14-5-4</u><br>6-1-12  | 17-1-0                                    | 19-7                                    | -8  | <u>25-6-0</u><br>5-10-8                                | 28  | - <u>3-8</u>                       |
|---|--|--|---|---|---|--|---|------------------------------------|
| Plate Offsets (X,Y)   | [7:0-0-6,0-1-8], [11:0-3-4,0-2-0]  | 0-1-12   | 2-1-12                                    | 2-0-                                    | 0   | 5-10-0   | Σ-  | 5-0                                |
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 *<br>BCDL 10.0                                  | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014  | CSI.<br>TC 0.37<br>BC 0.98<br>WB 0.92<br>Matrix-MS                                       | DEFL.<br>Vert(LL)<br>Vert(CT)<br>Horz(CT) | in (lo<br>-0.03 9-<br>-0.07 15-<br>0.03 | oc) l/defl<br>10 >999<br>18 >999<br>7 n/a       | L/d<br>240<br>180<br>n/a                               | <b>PLATES</b><br>MT20<br>Weight: 197 lb                   | <b>GRIP</b><br>244/190<br>FT = 20% |
| LUMBER-<br>TOP CHORD 2x6<br>BOT CHORD 2x6<br>4-12<br>WEBS 2x4<br>REACTIONS. ((<br>Maz<br>Ma:<br>Ma: | SP No.2<br>SP No.2 *Except*<br>,6-8: 2x4 SP No.3<br>SP No.3<br>size) 1=0-3-8, 7=Mechanical, 13=0-3-8<br>( Horz 1=-75(LC 13)<br>( Uplift 1=-95(LC 12), 7=-66(LC 13), 13=-7<br>( Grav 1=493(LC 23), 7=398(LC 1), 13=17 | 118(LC 8)<br>138(LC 1)   | BRACING-<br>TOP CHOP<br>BOT CHOP          | RD Stru<br>2-0<br>RD Rig                | uctural wooc<br>-0 oc purlins<br>id ceiling dir | I sheathing dire<br>(10-0-0 max.):<br>ectly applied of | ectly applied or 6-0-0 o<br>: 2-5.<br>r 2-2-0 oc bracing. | oc purlins, except                 |
| FORCES.(lb) - MaTOP CHORD1-BOT CHORD1-WEBS2-5-  | ax. Comp./Max. Ten All forces 250 (lb) o<br>2=-528/124, 2-3=0/488, 5-6=-254/108, 6-7<br>15=-70/377, 13-15=-73/371, 9-10=-240/86<br>15=0/326, 2-13=-848/177, 3-13=-745/208<br>11=-608/120, 5-10=0/310, 6-10=-698/281  | r less except when shown.<br>=-652/213<br>6, 7-8=-156/535<br>11-13=-456/145, 3-11=-75/46 | 67,                                       |   |   |  |   |                                    |
| NOTES-<br>1) Unbalanced roof  | ive loads have been considered for this d  | esign.   |   | alaaadi MAAA                            | (EDC (anyol                                     | )  |   |                                    |

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 13. This connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







| ł   |  | 10-3-8  | 14   | 1-5-4            | 17-1-0                                    |                              |   |  | 25-6-0                                       |  | 28-3-8                               |
|---|--|---|--|------------------|---|------------------------------|---|--|--|--|--------------------------------------|
|   | ()())                                    | 10-3-8  | 4-   | 1-12             | <u>2-7-12</u>                             |                              |   |  | 8-5-0  |  | 2-9-8                                |
| Plate Offsets                                   | s (X,Y)                                  | [3:0-4-12,0-2-0], [5:0-0-0,0-0-0], [6:0-2-0   | ,0-2-0], [7:0-0-6,0-1                              | -8], [10:0-6-0   | ,0-2-8]                                   |                              |   |  |  |  |                                      |
| LOADING (<br>TCLL 2<br>TCDL 1<br>BCLL<br>BCDL 1 | (psf)<br>20.0<br>10.0<br>0.0 *<br>10.0   | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014 | CSI.<br>TC 0.38<br>BC 0.86<br>WB 0.45<br>Matrix-MS |                  | DEFL.<br>/ert(LL)<br>/ert(CT)<br>Horz(CT) | in<br>-0.07<br>-0.16<br>0.04 | (loc)<br>9-10<br>9-10<br>7                | l/defl<br>>999<br>>999<br>n/a                    | L/d<br>240<br>180<br>n/a                     | PLATES<br>MT20<br>Weight: 207 I  | <b>GRIP</b><br>244/190<br>b FT = 20% |
| LUMBER-<br>TOP CHORI<br>BOT CHORI<br>WEBS       | D 2x6 SF<br>D 2x6 SF<br>4-11,6<br>2x4 SF | 2 No.2<br>2 No.2 *Except*<br>-8: 2x4 SP No.3<br>2 No.3                              |  | B<br>T<br>B<br>V | BRACING-<br>OP CHORE<br>OT CHORE<br>VEBS  | )<br>)                       | Structu<br>2-0-0 o<br>Rigid ce<br>1 Row a | ral wood<br>c purlins<br>eiling dire<br>at midpt | sheathing di<br>(10-0-0 max<br>ectly applied | irectly applied or 6-0-<br>): 3-5.<br>or 6-0-0 oc bracing.<br>3-12, 6-10 | 0 oc purlins, except                 |
| REACTION  | <b>S.</b> (siz<br>Max H                  | e) 1=0-3-8, 7=Mechanical, 12=0-3-8<br>lorz 1=-92(LC 13)                             |  |                  |   |                              |   |  |  |  |                                      |

Max Horz 1=-92(LC 13) Max Uplift 1=-68(LC 12), 7=-45(LC 13), 12=-121(LC 13) Max Grav 1=447(LC 23), 7=-337(LC 24), 12=1601(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-524/138, 3-4=0/274, 4-5=-3/265, 5-6=-83/346, 6-7=-563/151

BOT CHORD 1-14=-140/436, 4-10=-372/201, 9-10=-178/864, 6-9=0/254, 7-8=-121/475

WEBS 2-14=-39/2/229, 3-14=-0/4/20, 3-12=-1517/330, 10-12=-857/399, 3-10=-184/906, 5-10=-377/198, 6-10=-1032/379

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 12. This connection is for uplift only and does not consider lateral forces.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







| L  | 6-3-2   | 12-3-8  | 14-5-4 15-7-8  | 21-7-14   | 28-3-8   |
|--|---|---|--|---|--|
| I  | 6-3-2   | 6-0-6   | 2-1-12 1-2-4   | 6-0-6   | 6-7-10   |
| Plate Offsets (X,Y)  | [6:0-1-6,0-1-8]   |   |  |   |  |
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 *<br>BCDL 10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014 | <b>CSI.</b><br>TC 0.23<br>BC 0.19<br>WB 0.55<br>Matrix-MS | DEFL.         i           Vert(LL)         0.02           Vert(CT)         -0.02           Horz(CT)         0.02 | n (loc) l/defi L/d<br>2 7-18 >999 240<br>3 7-18 >999 180<br>1 6 n/a n/a                           | PLATES         GRIP           MT20         244/190           Weight: 205 lb         FT = 20% |
| LUMBER-<br>TOP CHORD 2x6 S<br>BOT CHORD 2x6 S<br>WEBS 2x4 S        | P No.2<br>P No.2<br>P No.3  |   | BRACING-<br>TOP CHORD<br>BOT CHORD   | Structural wood sheathing di<br>2-0-0 oc purlins (10-0-0 max.<br>Rigid ceiling directly applied o | rectly applied or 6-0-0 oc purlins, except<br>): 3-4.<br>or 6-0-0 oc bracing.                |

## REACTIONS. (size) 1=0-3-8, 6=Mechanical, 9=0-3-8 Max Horz 1=-109(LC 13)

Max Uplift 1=-75(LC 12), 6=-89(LC 13), 9=-90(LC 12)

Max Grav 1=478(LC 23), 6=451(LC 24), 9=1405(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 1-2=-616/159, 3-4=0/272, 4-5=0/251, 5-6=-549/148
- BOT CHORD 1-12=-125/484, 11-12=-125/484, 7-8=-48/446, 6-7=-48/446
- WEBS 2-12=0/265, 2-11=-581/228, 3-11=-32/323, 3-9=-612/128, 4-9=-566/114, 4-8=-19/277,

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

5-8=-633/241, 5-7=0/286

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 9. This connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







REACTIONS. (size) 5=Mechanical, 2=0-3-8 Max Horz 2=100(LC 12) Max Uplift 5=-96(LC 13), 2=-135(LC 12) Max Grav 5=879(LC 1), 2=941(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-2092/485, 3-4=-1227/327, 4-5=-1271/314
- BOT CHORD 2-8=-422/1968, 6-8=-429/1964, 5-6=-143/1026
- WEBS 3-6=-981/294, 4-6=-7/583

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
- connection is for uplift only and does not consider lateral forces.







Scale = 1:39.1



| L  |  | 5-2-4  | 12  | -0-6  | 1  | 18-10-8   | 22-0-1  | J                                  |
|--|--|--|---|---|--|---|---|------------------------------------|
| F  |  | 5-2-4  | 6-1   | 10-2  |  | 6-10-2  | 3-1-8   | 1                                  |
| Plate Offsets  | (X,Y)                                      | [2:0-3-6,Edge]   |   |   |  |   |   |                                    |
| LOADING (p<br>TCLL 20<br>TCDL 10<br>BCLL 0<br>BCDL 10  | osf)<br>0.0<br>0.0<br>0.0 *<br>0.0         | SPACING-<br>Plate Grip DOL<br>Lumber DOL<br>Rep Stress Incr<br>Code IRC2015/TPI2 | 2-0-0 <b>CSI.</b><br>1.15 TC<br>1.15 BC<br>NO WB<br>2014 Matrix | 0.52 Vert<br>0.66 Vert<br>0.72 Horz<br>x-MS | FL. in<br>t(LL) -0.15<br>t(CT) -0.31<br>z(CT) 0.04 | (loc) l/defl L/d<br>9-11 >999 240<br>9-11 >863 180<br>6 n/a n/a                         | PLATES<br>MT20<br>Weight: 136 lb  | <b>GRIP</b><br>244/190<br>FT = 20% |
| LUMBER-<br>TOP CHORD<br>BOT CHORD<br>WEBS<br>REACTIONS | 0 2x6 SP<br>0 2x6 SP<br>2x4 SP<br>0. (size | No.2<br>No.2<br>No.3<br>2) 2=0-3-8, 6=0-3-8                                      |   | BRA<br>TOP<br>BOT                           | ACING-<br>? CHORD                                  | Structural wood sheath<br>except<br>2-0-0 oc purlins (3-5-1<br>Rigid ceiling directly a | ning directly applied or 4-2-14<br>0 max.): 3-5.<br>oplied or 9-10-15 oc bracing. | oc purlins,                        |
|  | Max He                                     | orz 2=38(LC 8)   |   |   |  |   |   |                                    |

```
Max Uplift 2=-292(LC 8), 6=-211(LC 8)
Max Grav 2=1213(LC 1), 6=1188(LC 1)
```

## FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2821/638 3-4=-3449/786 4-5=-3449/786 5-6=-2080/453

BOT CHORD 2-11=-540/2661. 9-11=-542/2637. 8-9=-351/1808. 6-8=-347/1825

WFBS 3-11=0/345. 3-9=-175/951. 4-9=-484/261. 5-9=-368/1729. 5-8=0/284

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 31 lb down and 71 lb up at 6-0-12, 31 lb down and 71 lb up at 8-0-12, 31 lb down and 71 lb up at 10-0-12, 31 lb down and 71 lb up at 12-0-12, 31 lb down and 71 lb up at 13-11-4, and 31 lb down and 71 lb up at 15-11-4, and 31 lb down and 71 lb up at 17-11-4 on top chord, and 95 lb down and 46 lb up at 2-0-12, 89 lb down and 40 lb up at 4-0-12, 20 lb down at 6-0-12, 20 lb down at 8-0-12, 20 lb down at 10-0-12, 20 Ib down at 12-0-12, 20 lb down at 13-11-4, 20 lb down at 15-11-4, and 20 lb down at 17-11-4, and 108 lb down and 53 lb up at 19-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-60, 3-5=-60, 5-7=-60, 12-15=-20





| Job                   | Truss             | Truss Type          | Qty     | Ply        | WEST-SHERMAN 106-21-180                                |           |
|-----------------------|-------------------|---------------------|---------|------------|--|-----------|
| 222.42.222.424        |                   |                     |         |            |  | 148675556 |
| 29043-29043A          | GH                | Roof Special Girder | 1       | 1          |  |           |
|                       |                   |                     |         |            | Job Reference (optional)                               |           |
| 84 Components (Dunn), | Dunn, NC - 28334, |                     | 8       | .520 s Aug | 27 2021 MiTek Industries, Inc. Thu Nov 4 12:59:13 2021 | Page 2    |
|                       |                   | ID:2bG0dF?F         | QwaXcMo | o_jiugPayN | IcRK-3FqtXQDfWFsoTm1PwW?6YOBXjF2TTSH6wCaA6sy           | yMZqC     |

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 10=-18(F) 9=-18(F) 4=-15(F) 18=-15(F) 19=-15(F) 20=-15(F) 21=-15(F) 22=-15(F) 23=-15(F) 24=-95(F) 25=-89(F) 26=-18(F) 27=-18(F) 28=-18(F) 29=-18(F) 30=-18(F) 31=-108(F)





Scale = 1:40.4



|   | 5-0-0 7-1-  | 8  | 16-10-8  |   | 22-0-0   |
|---|---|--|--|---|--|
|   | 5-0-0 2-1   | 8  | 9-9-0  |   | 5-1-8  |
| Plate Offsets (X,Y)   | [2:0-2-2,Edge], [3:0-4-12,0-3-0], [6:0-0-   | 0,0-0-11]  |  |   |  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           *         BCDL | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014       | CSI.<br>TC 0.70<br>BC 0.48<br>WB 0.24<br>Matrix-MS | <b>DEFL.</b> ir<br>Vert(LL) -0.08<br>Vert(CT) -0.19<br>Horz(CT) 0.04 | n (loc) I/defi L/d<br>8 8-10 >999 240<br>9 8-10 >999 180<br>4 6 n/a n/a   | PLATES         GRIP           MT20         244/190           Weight: 139 lb         FT = 20% |
| LUMBER-<br>TOP CHORD 2x6 SF<br>BOT CHORD 2x6 SF<br>WEBS 2x4 SF  | P No.2<br>P No.2<br>P No.3  |  | BRACING-<br>TOP CHORD  | Structural wood sheathing dir<br>except<br>2-0-0 oc purlins (4-4-9 max.): | ectly applied or 4-11-6 oc purlins,<br>4-5.  |
|   |   |  | BOT CHORD<br>WEBS  | Rigid ceiling directly applied of 1 Row at midpt 4                        | or 10-0-0 oc bracing.<br>-8  |
| REACTIONS. (siz<br>Max H<br>Max U<br>Max C  | e) 2=0-3-8, 6=0-3-8<br>łorz 2=-51(LC 17)<br>Jplift 2=-151(LC 8), 6=-82(LC 8)<br>Grav 2=940(LC 1), 6=940(LC 1) |  |  |   |  |

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2064/455, 3-4=-1880/391, 4-5=-1387/334, 5-6=-1604/317

BOT CHORD 2-11=-361/1933, 10-11=-363/1930, 8-10=-268/1714, 6-8=-205/1400

WEBS 3-10=-257/135, 4-10=0/518, 4-8=-434/75, 5-8=0/390

NOTES1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







Scale = 1:40.4



| L  | 5-0-0  | 9-1-8  | 14-10-8  |   | 22-0-0                            |                                    |
|--|--|--|--|---|-----------------------------------|------------------------------------|
|  | 5-0-0  | 4-1-8  | 5-9-0  |   | 7-1-8                             |                                    |
| Plate Offsets (X,Y)  | [2:0-1-14,Edge], [3:0-4-12,0-3-0]  |  |  |   |                                   |                                    |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014          | CSI.<br>TC 0.24<br>BC 0.43<br>WB 0.25<br>Matrix-MS | DEFL.         ir           Vert(LL)         -0.05           Vert(CT)         -0.10           Horz(CT)         0.03 | (loc) l/defl L/d<br>10-11 >999 240<br>10-11 >999 180<br>6 n/a n/a         | PLATES<br>MT20<br>Weight: 140 lb  | <b>GRIP</b><br>244/190<br>FT = 20% |
| LUMBER-<br>TOP CHORD 2x6 S<br>BOT CHORD 2x6 S<br>WEBS 2x4 S  | P No.2<br>P No.2<br>P No.3   |  | BRACING-<br>TOP CHORD  | Structural wood sheathing dir<br>except<br>2-0-0 oc purlins (6-0-0 max.): | ectly applied or 4-11-7 o<br>4-5. | c purlins,                         |
| REACTIONS. (siz<br>Max  <br>Max  <br>Max   | ze) 2=0-3-8, 6=0-3-8<br>Horz 2=-68(LC 17)<br>Uplift 2=-115(LC 12), 6=-94(LC 13)<br>Grav 2=940(LC 1), 6=940(LC 1) |  | Derenend   |   | , 10 0 0 0 0 Diability.           |                                    |

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-2079/473, 3-4=-1494/357, 4-5=-1194/339, 5-6=-1426/318
- BOT CHORD 2-11=-379/1951, 10-11=-375/1956, 8-10=-187/1301, 6-8=-169/1202
- WEBS 3-10=-696/211, 4-10=-21/421, 5-8=0/320

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







| L  | 5-0-0  | 11-1-8  | 12-10-8  | 1 2   | 2-0-0  |
|--|--|---|--|---|--|
|  | 5-0-0  | 6-1-8   | 1-9-0  | 1   | 9-1-8  |
| Plate Offsets (X,Y)  | [2:0-1-14,Edge], [3:0-4-12,0-3-0]  |   |  |   |  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0 | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014                            | <b>CSI.</b><br>TC 0.46<br>BC 0.44<br>WB 0.67<br>Matrix-MS | DEFL. ir<br>Vert(LL) 0.06<br>Vert(CT) -0.12<br>Horz(CT) 0.02 | n (loc) l/defl L/d<br>5 7-13 >999 240<br>2 7-13 >999 180<br>2 6 n/a n/a   | PLATES         GRIP           MT20         244/190           Weight: 141 lb         FT = 20% |
| LUMBER-<br>TOP CHORD 2x6 SF<br>BOT CHORD 2x6 SF<br>WEBS 2x4 SF   | P No.2<br>P No.2<br>P No.3   |   | BRACING-<br>TOP CHORD  | Structural wood sheathing dir<br>except<br>2-0-0 oc purlins (6-0-0 max.): | ectly applied or 4-11-6 oc purlins,<br>4-5.  |
| REACTIONS. (siz<br>Max H<br>Max U<br>Max C   | e) 6=0-3-8, 2=0-3-8<br>lorz 2=92(LC 12)<br>Jplift 6=-90(LC 13), 2=-130(LC 12)<br>Grav 6=879(LC 1), 2=941(LC 1) |   | BUTCHORD   | Kigid celling directly applied t  | n 10-0-0 oc bracing.   |

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-2111/494, 3-4=-1273/329, 4-5=-1091/359, 5-6=-1333/323
- BOT CHORD 2-10=-428/1985, 8-10=-422/1992, 7-8=-154/1067, 6-7=-165/1096

WEBS 3-8=-953/279, 4-8=-23/307, 5-7=-7/293

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 2. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





|        |         |                      |           | 6-1-8          |                  |                        |
|--------|---------|----------------------|-----------|----------------|------------------|------------------------|
| LOADIN | G (psf) | SPACING- 2-0-0       | CSI.      | DEFL. in (     | (loc) l/defl L/d | PLATES GRIP            |
| TCLL   | 20.0    | Plate Grip DOL 1.15  | TC 0.23   | Vert(LL) 0.02  | 4-7 >999 240     | MT20 244/190           |
| TCDL   | 10.0    | Lumber DOL 1.15      | BC 0.18   | Vert(CT) -0.04 | 4-7 >999 180     |                        |
| BCLL   | 0.0 *   | Rep Stress Incr YES  | WB 0.00   | Horz(CT) 0.00  | 2 n/a n/a        |                        |
| BCDL   | 10.0    | Code IRC2015/TPI2014 | Matrix-MP |                |                  | Weight: 33 lb FT = 20% |

# LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

**REACTIONS.** (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=134(LC 12)

Max Uplift 3=-88(LC 12), 2=-35(LC 12)

Max Grav 3=159(LC 1), 2=307(LC 1), 4=113(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.





|  | <u>−−−−−−−−−−−−−−−−−−−−−−−−−−−−−−−−−−−−</u>   |  |  |   |   |                                    |  |  |  |  |
|--|---|--|--|---|---|------------------------------------|--|--|--|--|
| Plate Offsets (X,Y)  | [1:Edge,0-3-0]  |  | 0-1-0  |   |   |                                    |  |  |  |  |
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0   | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr NO<br>Code IRC2015/TPI2014  | CSI.<br>TC 0.54<br>BC 0.91<br>WB 0.03<br>Matrix-MP   | DEFL.         in           Vert(LL)         -0.07           Vert(CT)         -0.15           Horz(CT)         0.01   | (loc) l/defl L<br>3-6 >968 24<br>3-6 >484 18<br>1 n/a n   | /d PLATES<br>40 MT20<br>80<br>/a Weight: 70 lb                    | <b>GRIP</b><br>244/190<br>FT = 20% |  |  |  |  |
| LUMBER-<br>TOP CHORD 2x6 SF<br>BOT CHORD 2x6 SF<br>WEBS 2x4 SF   | P No.2<br>P No.2<br>P No.3  |  | BRACING-<br>TOP CHORD<br>BOT CHORD   | Structural wood shear Rigid ceiling directly  | athing directly applied or 6-0-0<br>applied or 10-0-0 oc bracing. | oc purlins.                        |  |  |  |  |
| REACTIONS. (siz<br>Max H<br>Max U<br>Max G   | e) 1=0-3-8, 3=Mechanical<br>lorz 1=116(LC 12)<br> plift 1=-112(LC 12), 3=-192(LC 12)<br> rav 1=1041(LC 1), 3=1155(LC 1)   |  |  |   |   |                                    |  |  |  |  |
| FORCES.         (lb) - Max.           WEBS         2-3=-   | Comp./Max. Ten All forces 250 (lb) or<br>318/165  | less except when shown   | ι.   |   |   |                                    |  |  |  |  |
| <ul> <li>NOTES-</li> <li>1) 2-ply truss to be corr<br/>Top chords connect<br/>Bottom chords connected as</li> <li>2) All loads are considi-<br/>ply connections hav</li> <li>3) Wind: ASCE 7-10; \vices y gable end zone and<br/>DOL=1.60</li> <li>4) This truss has been<br/>will fit between the b</li> <li>6) Refer to girder(s) for</li> <li>7) Provide mechanical<br/>3=192.</li> <li>8) One H2.5A Simpsor<br/>connection is for upl</li> <li>9) Hanger(s) or other or<br/>2-2-4, and 859 lb dor<br/>responsibility of other</li> </ul> | anected together with 10d (0.120"x3") na<br>ed as follows: 2x6 - 2 rows staggered at<br>ected as follows: 2x6 - 2 rows staggered<br>follows: 2x4 - 1 row at 0-9-0 oc.<br>ered equally applied to all plies, except i<br>e been provided to distribute only loads<br>/ult=130mph Vasd=103mph; TCDL=6.0;<br>C-C Exterior(2) zone;C-C for members<br>designed for a 10.0 psf bottom chord liv<br>n designed for a 10.0 psf bottom chord liv<br>n designed for a live load of 20.0psf on<br>sottom chord and any other members.<br>connection (by others) of truss to bearir<br>n Strong-Tie connectors recommended to<br>lift only and does not consider lateral for<br>somnection device(s) shall be provided si<br>wwn and 116 lb up at 4-2-4 on bottom chars. | ills as follows:<br>0-9-0 oc.<br>d at 0-9-0 oc.<br>f noted as front (F) or bac<br>noted as (F) or (B), unles<br>sf; BCDL=6.0psf; h=30ft;<br>and forces & MWFRS for<br>e load nonconcurrent with<br>the bottom chord in all are<br>ng plate capable of withsta<br>o connect truss to bearing<br>ces.<br>ufficient to support concer<br>hord. The design/selection | k (B) face in the LOAD CA<br>s otherwise indicated.<br>; Cat. II; Exp B; Enclosed; I<br>reactions shown; Lumber<br>h any other live loads.<br>eas where a rectangle 3-6-<br>anding 100 lb uplift at joint(<br>g walls due to UPLIFT at jt<br>ntrated load(s) 859 lb dowr<br>n of such connection devic | SE(S) section. Ply to<br>MWFRS (envelope)<br>DOL=1.60 plate grip<br>0 tall by 2-0-0 wide<br>s) except (jt=lb)<br>(s) 1. This<br>n and 116 lb up at<br>e(s) is the | SEA<br>0449   | AROLINE AL                         |  |  |  |  |
| LOAD CASE(S) Stan<br>1) Dead + Roof Live (b  | dard<br>alanced): Lumber Increase=1.15, Plate   | Increase=1.15  |  |   | CONGIN  | EEVIEN                             |  |  |  |  |

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 3-4=-20

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



(IIIIIIII) November 5,2021

| Job                   | Truss             | Truss Type       | Qty      | Ply       | WEST-SHERMAN 106-21-180                                |           |
|-----------------------|-------------------|------------------|----------|-----------|--|-----------|
|                       |                   |                  |          |           |  | I48675561 |
| 29043-29043A          | J2GR              | Jack-Open Girder | 1        | 2         |  |           |
|                       |                   |                  |          | <b>_</b>  | Job Reference (optional)                               |           |
| 84 Components (Dunn), | Dunn, NC - 28334, |                  | . 8      | 520 s Aug | 27 2021 MiTek Industries, Inc. Thu Nov 4 12:59:23 2021 | Page 2    |
|                       |                   | ID:2bG00         | dF?FQwa⊁ | CMo_jiugF | ayMcRK-nARferLw9K6NgJnKVdBSyVbF0HPup8RaDI?iS           | HyMZq2    |

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 7=-859(B) 8=-859(B)





|                         | 010   |  |       |  |  |  |  |  |  |
|-------------------------|---|--|-------|--|--|--|--|--|--|
| LOADING (psf)           | SPACING- 2-0-0                              | CSI. DEFL. in (loc) //defl L/d PLATES GRIP   |       |  |  |  |  |  |  |
| TCDL 10.0               | Lumber DOL 1.15                             | BC 0.04 Vert(CT) -0.00 7 >999 240 M120 244/190<br>BC 0.04 Vert(CT) -0.00 7 >999 180  | )     |  |  |  |  |  |  |
| BCLL 0.0 *<br>BCDL 10.0 | Rep Stress Incr YES<br>Code IRC2015/TPI2014 | WB         0.00         Horz(CT)         0.00         3         n/a         n/a           Matrix-MP         Weight: 19 lb         FT = | - 20% |  |  |  |  |  |  |

## LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=76(LC 12)

Max Uplift 3=-42(LC 12), 2=-29(LC 12)

Max Grav 3=75(LC 1), 2=192(LC 1), 4=55(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-1-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



| LOADING (psf)SPACING-<br>2-0-0TCLL20.0Plate Grip DOL1.15TCDL10.0Lumber DOL1.15BCLL0.0 *Rep Stress IncrYESDOD1.901.901.90 | CSI.<br>TC 0.08<br>BC 0.03<br>WB 0.00 | <b>DEFL.</b> ir<br>Vert(LL) -0.00<br>Vert(CT) -0.00<br>Horz(CT) 0.00 | (loc) l/defl L/d<br>8 >999 240<br>8 >999 180<br>2 n/a n/a | PLATES         GRIP           MT20         244/190 |
|--|---------------------------------------|--|---|--|
| BCDL 10.0 Code IRC2015/1P12014   | Matrix-MR                             |  |   | Weight: 20 lb $FI = 20\%$                          |
| LUMBER-<br>TOP CHORD 2x6 SP No.2<br>BOT CHORD 2x6 SP No.2  |                                       | BRACING-<br>TOP CHORD  | Structural wood sheathing dir                             | rectly applied or 3-1-8 oc purlins,                |
| WEBS 2x4 SP No.3   |                                       | BOT CHORD  | Rigid ceiling directly applied                            | 10-0-0 oc bracing                                  |

REACTIONS. (size) 5=Mechanical, 2=0-3-8 Max Horz 2=63(LC 12)

Max Uplift 5=-20(LC 12), 2=-34(LC 12) Max Grav 5=109(LC 1), 2=189(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5 and 2. This connection is for uplift only and does not consider lateral forces.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







|                        |                 | 1      | 1-0   | )-0  |          |       |         | 2-1-8     |               |                        |             |
|------------------------|-----------------|--------|-------|------|----------|-------|---------|-----------|---------------|------------------------|-------------|
| Plate Offsets (X,Y) [3 | 3:0-4-4,0-2-12] |        |       |      |          |       |         |           |               |                        |             |
| LOADING (psf)          | SPACING-        | 2-0-0  | CSI.  |      | DEFL.    | in    | (loc)   | l/defl    | L/d           | PLATES                 | GRIP        |
| TCLL 20.0              | Plate Grip DOL  | 1.15   | TC    | 0.04 | Vert(LL) | -0.00 | ` 6     | >999      | 240           | MT20                   | 244/190     |
| TCDL 10.0              | Lumber DOL      | 1.15   | BC    | 0.07 | Vert(CT) | -0.00 | 6       | >999      | 180           |                        |             |
| BCLL 0.0 *             | Rep Stress Incr | NO     | WB    | 0.02 | Horz(CT) | 0.00  | 2       | n/a       | n/a           |                        |             |
| BCDL 10.0              | Code IRC2015/T  | PI2014 | Matri | x-MP |          |       |         |           |               | Weight: 19 lb          | FT = 20%    |
| LUMBER-                |                 |        |       |      | BRACING  |       |         |           |               |                        |             |
| TOP CHORD 2x6 SP       | No.2            |        |       |      | TOP CHO  | RD    | Structu | ral wood  | sheathing dir | ectly applied or 3-1-8 | oc purlins, |
| BOT CHORD 2x6 SP       | No.2            |        |       |      |          |       | except  | end verti | cals, and 2-0 | -0 oc purlins: 3-4.    |             |

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD2x6 SP No.2WEBS2x4 SP No.3REACTIONS.(size)5=Mechanical, 2=0-3-8

ONS. (size) 5=Mechanical, 2=0-3-8 Max Horz 2=37(LC 12) Max Uplift 5=-26(LC 9), 2=-34(LC 12)

Max Grav 5=115(LC 1), 2=192(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5 and 2. This connection is for uplift only and does not consider lateral forces.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 77 lb down and 31 lb up at 1-0-0, and 16 lb down and 37 lb up at 2-0-12 on top chord, and 24 lb down and 14 lb up at 1-0-0, and 9 lb down and 11 lb up at 2-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
  - Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-7=-20 Concentrated Loads (lb)
    - Vert: 11=-9(B)







TOP CHORD

BOT CHORD

|    | 18/ |   |      |
|----|-----|---|------|
| LU | אוע | ю | - R- |

TOP CHORD2x6 SP No.2BOT CHORD2x6 SP No.2WEBS2x4 SP No.3

REACTIONS. (size) 5=Mechanical, 2=0-3-8

Max Horz 2=56(LC 12) Max Uplift 5=-33(LC 12), 2=-37(LC 12)

Max Grav 5=128(LC 1), 2=191(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5 and 2. This
  connection is for uplift only and does not consider lateral forces.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 25 lb down and 52 lb up at 2-11-12, and 16 lb down and 52 lb up at 2-0-0 on top chord, and 19 lb down at 2-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-7=-20 Concentrated Loads (lb) Vert: 4=-14(F) 6=-7(F)



Structural wood sheathing directly applied or 3-1-8 oc purlins,

except end verticals, and 2-0-0 oc purlins; 3-4.

Rigid ceiling directly applied or 10-0-0 oc bracing.





|         |         |                      |           | 200                       |                        |
|---------|---------|----------------------|-----------|---------------------------|------------------------|
| LOADING | i (psf) | SPACING- 2-0-0       | CSI.      | DEFL. in (loc) l/defl L/d | PLATES GRIP            |
| TCLL    | 20.0    | Plate Grip DOL 1.15  | TC 0.03   | Vert(LL) -0.00 7 >999 240 | MT20 244/190           |
| TCDL    | 10.0    | Lumber DOL 1.15      | BC 0.01   | Vert(CT) -0.00 7 >999 180 |                        |
| BCLL    | 0.0 *   | Rep Stress Incr YES  | WB 0.00   | Horz(CT) 0.00 3 n/a n/a   |                        |
| BCDL    | 10.0    | Code IRC2015/TPI2014 | Matrix-MP |                           | Weight: 13 lb FT = 20% |

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=55(LC 12)

Max Uplift 3=-25(LC 12), 2=-28(LC 12)

Max Grav 3=43(LC 1), 2=155(LC 1), 4=34(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 2-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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Edenton, NC 27932



|           |            |                 |        |       |      |          | 2-0-0 |       |        |     | 1             |          |
|-----------|------------|-----------------|--------|-------|------|----------|-------|-------|--------|-----|---------------|----------|
| Plate Off | sets (X,Y) | [2:0-2-14,Edge] |        | 1     |      |          |       |       |        |     |               |          |
| LOADIN    | G (psf)    | SPACING-        | 2-0-0  | CSI.  |      | DEFL.    | in    | (loc) | l/defl | L/d | PLATES        | GRIP     |
| TCLL      | 20.0       | Plate Grip DOL  | 1.15   | TC    | 0.03 | Vert(LL) | -0.00 | 5     | >999   | 240 | MT20          | 244/190  |
| TCDL      | 10.0       | Lumber DOL      | 1.15   | BC    | 0.01 | Vert(CT) | -0.00 | 5     | >999   | 180 |               |          |
| BCLL      | 0.0 *      | Rep Stress Incr | YES    | WB    | 0.00 | Horz(CT) | 0.00  | 4     | n/a    | n/a |               |          |
| BCDL      | 10.0       | Code IRC2015/TF | PI2014 | Matri | x-MP |          |       |       |        |     | Weight: 12 lb | FT = 20% |

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

#### REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=43(LC 8) Max Uplift 3=-12(LC 12), 2=-60(LC 8), 4=-1(LC 12) Max Grav 3=39(LC 1), 2=155(LC 1), 4=29(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 2-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





|  |   |  |   |  | 1-7-7                         |                          |                                 |                                    |
|--|---|--|---|--|-------------------------------|--------------------------|---------------------------------|------------------------------------|
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 *<br>BCDL 10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014 | CSI.<br>TC 0.10<br>BC 0.01<br>WB 0.00<br>Matrix-MP | DEFL.<br>Vert(LL) (<br>Vert(CT) (<br>Horz(CT) ( | in (loc)<br>0.00 7<br>0.00 7<br>0.00 2 | l/defl<br>>999<br>>999<br>n/a | L/d<br>240<br>180<br>n/a | PLATES<br>MT20<br>Weight: 13 lb | <b>GRIP</b><br>244/190<br>FT = 20% |

BRACING-TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

**REACTIONS.** (size) 3=Mechanical, 2=0-5-3, 4=Mechanical

Max Horz 2=44(LC 8)

Max Uplift 3=-5(LC 12), 2=-123(LC 8), 4=-10(LC 1)

Max Grav 3=14(LC 1), 2=240(LC 1), 4=23(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2. This
- connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 1-8-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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|                |                 |                               |               |            |              |                      | 2-8-7         |            |                |            |                |                        |
|----------------|-----------------|-------------------------------|---------------|------------|--------------|----------------------|---------------|------------|----------------|------------|----------------|------------------------|
| LOADIN<br>TCLL | G (psf)<br>20.0 | SPACING-<br>Plate Grip DOL    | 2-0-0<br>1.15 | CSI.<br>TC | 0.06         | DEFL.<br>Vert(LL)    | in<br>-0.00   | (loc)<br>7 | l/defl<br>>999 | L/d<br>240 | PLATES<br>MT20 | <b>GRIP</b><br>244/190 |
| TCDL<br>BCLL   | 10.0<br>0.0 *   | Lumber DOL<br>Rep Stress Incr | 1.15<br>YES   | BC<br>WB   | 0.02<br>0.00 | Vert(CT)<br>Horz(CT) | -0.00<br>0.00 | 7<br>3     | >999<br>n/a    | 180<br>n/a |                |                        |
| BCDL           | 10.0            | Code IRC2015/TP               | 12014         | Matri      | x-MP         |                      |               |            |                |            | Weight: 17 lb  | FT = 20%               |

BRACING-TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-4-9, 4=Mechanical

Max Horz 2=62(LC 8)

Max Uplift 3=-27(LC 12), 2=-82(LC 8)

Max Grav 3=57(LC 1), 2=213(LC 1), 4=44(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 2-8-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





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TOP CHORD 2x6 SP No.2

BOT CHORD 2x6 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-7-12 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (Ib/size) 3=62/Mechanical, 2=301/0-4-15, 4=5/Mechanical Max Horz 2=75(LC 8) Max Uplift 3=-37(LC 12), 2=-101(LC 8) Max Grav 3=62(LC 1), 2=301(LC 1), 4=45(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 70 lb down and 48 lb up at 2-9-8, and 14 lb down and 56 lb up at 2-9-8 on top chord, and 8 lb down and 9 lb up at 2-9-8, and 7 lb down at 2-9-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-60, 4-5=-20 Concentrated Loads (lb)

Vert: 10=3(F=6, B=-3)



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|  |   |   | 3-7-12  |   |
|--|---|---|---|---|
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 *<br>BCDL 10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014 | <b>CSI.</b><br>TC 0.06<br>BC 0.05<br>WB 0.00<br>Matrix-MP | DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.00         7         >999         240           Vert(CT)         -0.00         4-7         >999         180           Horz(CT)         0.00         2         n/a         n/a | PLATES         GRIP           MT20         244/190           Weight: 21 lb         FT = 20% |

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

**REACTIONS.** (size) 3=Mechanical, 2=0-4-9, 4=Mechanical

Max Horz 2=75(LC 8)

Max Uplift 3=-40(LC 12), 2=-82(LC 8)

Max Grav 3=86(LC 1), 2=246(LC 1), 4=63(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2. This
- connection is for uplift only and does not consider lateral forces.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 14 lb down and 56 lb up at 2-9-8, and 14 lb down and 56 lb up at 2-9-8 on top chord, and 7 lb down at 2-9-8, and 7 lb down at 2-9-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf) Vert: 1-3=-60, 4-5=-20 Concentrated Loads (lb) Vert: 9=-6(F=-3, B=-3)



Structural wood sheathing directly applied or 3-7-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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Edenton, NC 27932



TOP CHORD

BOT CHORD

| <br>8.4 |   | - | •  |
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TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

REACTIONS. (lb/size) 3=41/Mechanical, 4=20/Mechanical, 2=152/0-3-8 Max Horz 2=53(LC 12) Max Uplift 3=-23(LC 12), 2=-28(LC 12)

Max Grav 3=41(LC 1), 4=32(LC 3), 2=152(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 1-10-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.


TOP CHORD

BOT CHORD

| LUMBER       | २-            |                                      |            |              |              | BRACING  |       |     |      |
|--------------|---------------|--------------------------------------|------------|--------------|--------------|----------|-------|-----|------|
| BCLL<br>BCDL | 0.0 *<br>10.0 | Rep Stress Incr<br>Code IRC2015/TPI2 | YES<br>014 | WB<br>Matrix | 0.00<br>«-MP | Horz(CT) | 0.00  | 3   | n/a  |
| TCDI         | 10.0          | Lumber DOI                           | 1 15       | BC           | 0.05         | Vert(CT) | -0.00 | 4-9 | >999 |
|              |               |                                      |            |              |              |          |       |     |      |

## LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No 2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=91(LC 12)

Max Uplift 3=-53(LC 12), 2=-33(LC 12)

Max Grav 3=89(LC 1), 2=240(LC 1), 4=64(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2. This connection is for uplift only and does not consider lateral forces.



n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 22 lb

Structural wood sheathing directly applied or 3-10-15 oc purlins.

FT = 20%





Max Uplift All uplift 100 lb or less at joint(s) 3, 2, 4, 5

Max Grav All reactions 250 lb or less at joint(s) 3, 4, 5 except 2=271(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.







|               | 1                                     |             | 6-0-0                            |                        |
|---------------|---------------------------------------|-------------|----------------------------------|------------------------|
| LOADING (psf) | SPACING- 2-0-0<br>Plate Grip DOI 1 15 | <b>CSI.</b> | <b>DEFL.</b> in (loc) I/defl L/d | PLATES GRIP            |
| TCDL 10.0     | Lumber DOL 1.15                       | BC 0.17     | Vert(CT) -0.03 4-7 >999 180      | WI 20 244/130          |
| BCDL 10.0     | Code IRC2015/TPI2014                  | Matrix-MP   |                                  | Weight: 33 lb FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

**REACTIONS.** (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=131(LC 12)

Max Uplift 3=-86(LC 12), 2=-34(LC 12)

Max Grav 3=155(LC 1), 2=303(LC 1), 4=110(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





| LOADING (psf)   TCLL 20.0   TCDL 10.0   BCLL 0.0   BCDL 10.0 | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014 | <b>CSI.</b><br>TC 0.09<br>BC 0.06<br>WB 0.00<br>Matrix-P | DEFL. i<br>Vert(LL) n/:<br>Vert(CT) n/:<br>Horz(CT) 0.00 | n (loc) l/defl L/d<br>a - n/a 999<br>a - n/a 999<br>) n/a n/a | PLATES GRIP   MT20 244/190   Weight: 21 lb FT = 20% |
|--|--|--|--|---|---|
| LUMBER-<br>TOP CHORD 2x6 SP                                  | No.2   |  | BRACING-<br>TOP CHORD                                    | Structural wood sheathing dir                                 | ectly applied or 3-7-0 oc purlins,                  |
| BOT CHORD 2x6 SP   | No.2   |  |  | except end verticals.   |   |

REACTIONS. (size) 1=3-7-0, 3=3-7-0

Max Horz 1=80(LC 12) Max Uplift 1=-1(LC 12), 3=-49(LC 12)

Max Grav 1=137(LC 1), 3=137(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.







|        |         |                       |           | 1-11-0                    |                        |
|--------|---------|-----------------------|-----------|---------------------------|------------------------|
| LOADIN | G (psf) | <b>SPACING-</b> 2-0-0 | CSI.      | DEFL. in (loc) I/defl L/d | PLATES GRIP            |
| TCLL   | 20.0    | Plate Grip DOL 1.15   | TC 0.03   | Vert(LL) -0.00 7 >999 240 | MT20 244/190           |
| TCDL   | 10.0    | Lumber DOL 1.15       | BC 0.01   | Vert(CT) -0.00 7 >999 180 |                        |
| BCLL   | 0.0 *   | Rep Stress Incr YES   | WB 0.00   | Horz(CT) 0.00 3 n/a n/a   |                        |
| BCDL   | 10.0    | Code IRC2015/TPI2014  | Matrix-MP |                           | Weight: 13 lb FT = 20% |

BRACING-TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=53(LC 12)

Max Uplift 3=-23(LC 12), 2=-28(LC 12)

Max Grav 3=41(LC 1), 2=152(LC 1), 4=32(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 1-11-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





|                 |                 |                                       | 1                      | 3-11-0   |  |
|-----------------|-----------------|---------------------------------------|------------------------|--|--|
| LOADING<br>TCLL | i (psf)<br>20.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15 | <b>CSI.</b><br>TC 0.08 | DEFL. in (loc) I/defl L/d PLATES GRIP   Vert(LL) -0.00 7 >999 240 MT20 244/190 |  |
| TCDL            | 10.0            | Lumber DOL 1.15                       | BC 0.06                | Vert(CT) -0.01 4-7 >999 180  |  |
| BCLL            | 0.0 *           | Rep Stress Incr YES                   | WB 0.00                | Horz(CT) 0.00 2 n/a n/a  |  |
| BCDL            | 10.0            | Code IRC2015/TPI2014                  | Matrix-MP              | Weight: 23 lb FT = 20%   |  |

BRACING-TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=91(LC 12)

Max Uplift 3=-54(LC 12), 2=-30(LC 12)

Max Grav 3=97(LC 1), 2=222(LC 1), 4=70(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2. This connection is for uplift only and does not consider lateral forces.



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Edenton, NC 27932

Structural wood sheathing directly applied or 3-11-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



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TOP CHORD 2x6 SP No.2

BOT CHORD 2x6 SP No.2

REACTIONS. (Ib/size) 3=23/Mechanical, 4=-0/Mechanical, 2=189/0-3-8 Max Horz 2=53(LC 12) Max Uplift 3=-20(LC 12), 2=-35(LC 12) Max Grav 3=23(LC 1), 4=21(LC 3), 2=189(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



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BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 1-10-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



|   |  | I I                        | 2-8-0   |                            |
|---|--|----------------------------|---|----------------------------|
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15 | CSI.<br>TC 0.03<br>BC 0.02 | DEFL. in (loc) l/defl L/d   Vert(LL) -0.00 7 >999 240   Vert(CT) -0.00 7 >999 180 | PLATES GRIP   MT20 244/190 |
| BCLL 0.0 *<br>BCDL 10.0                 | Rep Stress Incr YES<br>Code IRC2015/TPI2014              | WB 0.00<br>Matrix-MP       | Horz(CT) 0.00 3 n/a n/a   | Weight: 16 lb FT = 20%     |

BRACING-

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=67(LC 12)

Max Uplift 3=-35(LC 12), 2=-28(LC 12)

Max Grav 3=62(LC 1), 2=176(LC 1), 4=46(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 2-8-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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|         |       |                      | 0-3-0     | 2 4 0                     |                        |
|---------|-------|----------------------|-----------|---------------------------|------------------------|
| LOADING | (psf) | SPACING- 2-0-0       | CSI.      | DEFL. in (loc) I/defl L/d | PLATES GRIP            |
| TCLL    | 20.0  | Plate Grip DOL 1.15  | TC 0.03   | Vert(LL) -0.00 7 >999 240 | MT20 244/190           |
| TCDL    | 10.0  | Lumber DOL 1.15      | BC 0.02   | Vert(CT) -0.00 7 >999 180 |                        |
| BCLL    | 0.0 * | Rep Stress Incr YES  | WB 0.00   | Horz(CT) 0.00 3 n/a n/a   |                        |
| BCDL    | 10.0  | Code IRC2015/TPI2014 | Matrix-MP |                           | Weight: 16 lb FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=67(LC 12)

Max Uplift 3=-35(LC 12), 2=-28(LC 12)

Max Grav 3=62(LC 1), 2=176(LC 1), 4=46(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 2-8-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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| LOADING (psf)   TCLL 20.0   TCDL 10.0   BCLL 0.0   BCDL 10.0 | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014 | <b>CSI.</b><br>TC 0.35<br>BC 0.21<br>WB 0.07<br>Matrix-P | DEFL. ir<br>Vert(LL) n/a<br>Vert(CT) n/a<br>Horz(CT) 0.00 | n (loc) l/defl L/d<br>a - n/a 999<br>a - n/a 999<br>) n/a n/a | PLATES GRIP   MT20 244/190   Weight: 30 lb FT = 20% |
|--|--|--|---|---|---|
| LUMBER-  |  |  | BRACING-  |   |   |
| BOT CHORD 2x4 SP   | No.3<br>No.3   |  | TOP CHORD   | Structural wood sheathing dir<br>except end verticals.        | rectly applied or 6-0-0 oc purlins,                 |
| WEBS 2x4 SP  | No.3   |  | BOT CHORD   | Rigid ceiling directly applied of                             | or 10-0-0 oc bracing.                               |
| OTHERS 2x4 SP  | N0.3   |  |   |   |   |
| REACTIONS. (size   | e) 1=7-8-0, 4=7-8-0, 5=7-8-0   |  |   |   |   |
| Max H  | orz 1=134(LC 12)   |  |   |   |   |
| Max U  | plift 4=-37(LC 12), 5=-106(LC 12)  |  |   |   |   |
| Max G  | rav 1=83(LC 1), 4=122(LC 1), 5=350(LC  | ; 1)   |   |   |   |

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 2-5=-262/241

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.







| LOADING (p<br>TCLL 20<br>TCDL 10<br>BCLL 10<br>BCDL 10 | psf)<br>0.0<br>0.0<br>0.0 *<br>0.0   | SPACING-<br>Plate Grip DOL<br>Lumber DOL<br>Rep Stress Incr<br>Code IRC2015/TP | 2-0-0<br>1.15<br>1.15<br>YES<br>12014 | CSI.<br>TC (<br>BC (<br>WB (<br>Matrix-F | 0.24<br>0.15<br>0.06<br>P | DEFL.<br>Vert(LL)<br>Vert(CT)<br>Horz(CT) | in<br>n/a<br>n/a<br>0.00 | (loc)<br>-<br>-                  | l/defl<br>n/a<br>n/a<br>n/a | L/d<br>999<br>999<br>n/a                   | PLATES<br>MT20<br>Weight: 21 lb                | <b>GRIP</b><br>244/190<br>FT = 20% |
|--|--------------------------------------|--|---------------------------------------|--|---------------------------|---|--------------------------|----------------------------------|-----------------------------|--|--|------------------------------------|
| LUMBER-<br>TOP CHORD<br>BOT CHORD<br>WEBS              | D 2x4 SP N<br>D 2x4 SP N<br>2x4 SP N | No.3<br>No.3<br>No.3   |                                       |  |                           | BRACING-<br>TOP CHORI<br>BOT CHORI        | D<br>D                   | Structur<br>except e<br>Rigid ce | al wood and vertice         | sheathing dire<br>cals.<br>ctly applied or | ectly applied or 5-8-8<br>r 10-0-0 oc bracing. | oc purlins,                        |

**REACTIONS.** (size) 1=5-8-0, 4=5-8-0, 5=5-8-0

Max Horz 1=96(LC 12)

Max Uplift 4=-33(LC 12), 5=-84(LC 12) Max Grav 1=45(LC 12), 4=109(LC 1), 5=276(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.







| LOADING (psf)   TCLL 20.0   TCDL 10.0   BCLL 0.0 *   BCDL 10.0 | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014 | <b>CSI.</b><br>TC 0.26<br>BC 0.17<br>WB 0.00<br>Matrix-P | DEFL. ir<br>Vert(LL) n/a<br>Vert(CT) n/a<br>Horz(CT) 0.00 | n (loc) l/defl L/d<br>a - n/a 999<br>a - n/a 999<br>) n/a n/a | <b>PLATES</b><br>MT20<br>Weight: 12 lb | <b>GRIP</b><br>244/190<br>FT = 20% |
|--|--|--|---|---|--|------------------------------------|
| LUMBER-<br>TOP CHORD 2x4 S<br>BOT CHORD 2x4 S                  | P No.3<br>P No.3   |  | BRACING-<br>TOP CHORD                                     | Structural wood sheathing dir except end verticals.           | ectly applied or 3-8-8                 | oc purlins,                        |
| WEBS 2x4 S   | P No.3   |  | BOT CHORD   | Rigid ceiling directly applied of                             | or 10-0-0 oc bracing.                  |                                    |
| REACTIONS. (siz  | ze) 1=3-8-0, 3=3-8-0<br>Horz 1=57(LC 12)   |  |   |   |  |                                    |

Max Uplift 1=-7(LC 12), 3=-36(LC 12)

Max Grav 1=117(LC 1), 3=117(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.







| 0- <u>0-8</u><br>0-0-8                                       | 2-6-8<br>2-6-0  |  | 10-6-8<br>8-0-0   | 13-1-0<br>2-6-8                                     |
|--|---|--|---|---|
| LOADING (psf)   TCLL 20.0   TCDL 10.0   BCLL 0.0   BCDL 10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014 | <b>CSI.</b><br>TC 0.34<br>BC 0.12<br>WB 0.06<br>Matrix-P | <b>DEFL.</b> in (loc) I/defl L/d<br>Vert(LL) n/a - n/a 999<br>Vert(CT) n/a - n/a 999<br>Horz(CT) 0.00 5 n/a n/a | PLATES GRIP   MT20 197/144   Weight: 45 lb FT = 20% |
| LUMBER-<br>TOP CHORD 2x4 SF                                  | 2 No.3  |  | BRACING-<br>TOP CHORD Structural wood sheathing of  | lirectly applied or 6-0-0 oc purlins.               |

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

| TOP CHORD | 2x4 SP No.3                 |
|-----------|-----------------------------|
| BOT CHORD | 2x4 SP No.2 or 2x4 SPF No.2 |
| WEBS      | 2x4 SP No.3                 |
| OTHERS    | 2x4 SP No.3                 |

2x4 SP N0.3

REACTIONS. All bearings 13-0-0.

(lb) - Max Horz 1=49(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-109(LC 12), 6=-109(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=261(LC 1), 8=314(LC 1), 6=314(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.



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| 0-0<br>0-0  | -8<br>-8  |  | <u>9-1-0</u><br>9-0-8   |                                       |
|---|---|--|---|---------------------------------------|
| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 * | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES | <b>CSI.</b><br>TC 0.49<br>BC 0.26<br>WB 0.04 | DEFL. in (loc) l/defl L/d   Vert(LL) n/a - n/a 999   Vert(CT) n/a - n/a 999   Horz(CT) 0.00 3 n/a n/a | <b>PLATES GRIP</b><br>MT20 244/190    |
| BCDL 10.0   | Code IRC2015/TPI2014  | Matrix-P                                     |   | Weight: 29 lb FT = 20%                |
| LUMBER-<br>TOP CHORD 2x4 SI                           | P No.3  |  | BRACING-<br>TOP CHORD Structural wood sheathing (   | directly applied or 6-0-0 oc purlins. |

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3OTHERS2x4 SP No.3

**REACTIONS.** (size) 1=9-0-0, 3=9-0-0, 4=9-0-0

Max Horz 1=-33(LC 13)

Max Uplift 1=-36(LC 12), 3=-43(LC 13), 4=-1(LC 12)

Max Grav 1=158(LC 1), 3=158(LC 1), 4=310(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.







2x4 💋

2x4 📚

| Plate Offsets (X,Y) [   | 0- <u>ρ-8</u><br>0-0-8<br>2:0-2-0,Edge]  |   | <u>5-1-0</u><br>5-0-8   |   |
|---|--|---|---|---|
| OADING (psf)   CLL 20.0   CDL 10.0   iCLL 0.0   * 3000000000000000000000000000000000000 | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014 | CSI.<br>TC 0.10<br>BC 0.31<br>WB 0.00<br>Matrix-P | DEFL. in (loc) l/defl L/d   Vert(LL) n/a - n/a 999   Vert(CT) n/a - n/a 999   Horz(CT) 0.00 3 n/a n/a | PLATES GRIP   MT20 244/190   Weight: 14 lb FT = 20% |
| UMBER-<br>OP CHORD 2x4 SP<br>30T CHORD 2x4 SP   | No.3<br>No.3   |   | BRACING-<br>TOP CHORD Structural wood sheathing<br>BOT CHORD Rigid ceiling directly applied           | directly applied or 5-1-0 oc purlins.               |

REACTIONS. (size) 1=5-0-0, 3=5-0-0 Max Horz 1=16(LC 12)

Max Uplift 1=-18(LC 12), 3=-18(LC 13) Max Grav 1=153(LC 1), 3=153(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.







| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0 *<br>BCDL 10.0              | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014 | CSI.<br>TC 0.23<br>BC 0.19<br>WB 0.09<br>Matrix-P | DEFL. ii<br>Vert(LL) n/a<br>Vert(CT) n/a<br>Horz(CT) 0.00 | n (loc) l/defl L/d<br>a - n/a 999<br>a - n/a 999<br>0 n/a n/a                           | PLATES GRIP   MT20 197/144   Weight: 56 lb FT = 20%          |
|---|---|---|---|---|--|
| LUMBER-<br>TOP CHORD 2x4 SF<br>BOT CHORD 2x4 SF<br>WEBS 2x4 SF<br>OTHERS 2x4 SF | P No.2 or 2x4 SPF No.2<br>P No.2 or 2x4 SPF No.2<br>P No.3<br>P No.3                                    |   | BRACING-<br>TOP CHORD<br>BOT CHORD                        | Structural wood sheathing di<br>except end verticals.<br>Rigid ceiling directly applied | rectly applied or 6-0-0 oc purlins,<br>or 10-0-0 oc bracing. |

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REACTIONS. All bearings 12-8-0.

(lb) - Max Horz 1=231(LC 12)

Max UpLift All uplift 100 lb or less at joint(s) 5 except 6=-104(LC 12), 7=-110(LC 12) Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 6=374(LC 2), 7=359(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-277/117

WEBS 3-6=-256/199, 2-7=-270/210

## NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.







#### TOP CHORD 2x4 SP No.3

Structural wood sheathing directly applied or 6-0-0 oc purlins, 2x4 SP No.3 BOT CHORD except end verticals. 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WFBS OTHERS 2x4 SP No.3

REACTIONS. All bearings 10-8-0.

(lb) -Max Horz 1=192(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 5, 7 except 6=-110(LC 12) Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 6=364(LC 1), 7=265(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-260/105

WEBS 3-6=-273/225

## NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.







| LOADING (psf) SPACING   TCLL 20.0 Plate Grip   TCDL 10.0 Lumber D   BCLL 0.0 * Rep Stress   BCDL 10.0 Code IRC | - 2-0-0 C:<br>DOL 1.15 TC<br>OL 1.15 BR<br>s Incr YES W<br>:2015/TPI2014 M | SI. DEFL.   C 0.44 Vert(LL)   C 0.27 Vert(CT)   /B 0.08 Horz(CT)   atrix-P Horz(CT) | in (loc) l/defl<br>n/a - n/a<br>n/a - n/a<br>0.00 n/a       | L/d <b>PLATE</b><br>999 MT20<br>999<br>n/a Weight:                          | S GRIP<br>244/190<br>35 lb FT = 20% |
|--|--|---|---|---|-------------------------------------|
| LUMBER-<br>TOP CHORD 2x4 SP No.3<br>BOT CHORD 2x4 SP No.3<br>WEBS 2x4 SP No.3<br>OTHERS 2x4 SP No.3            |  | BRACING<br>TOP CHO<br>BOT CHO   | RD Structural wood<br>except end ver<br>RD Rigid ceiling di | d sheathing directly applied c<br>ticals.<br>rectly applied or 10-0-0 oc br | r 6-0-0 oc purlins,<br>acing.       |
| REACTIONS. (size) 1=8-8-0, 4=8<br>Max Horz 1=154(LC 12<br>Max Uplift 4=-35(LC 12                               | 3-8-0, 5=8-8-0<br>2)<br>), 5=-121(LC 12)                                   |   |   |   |                                     |

Max Grav 1=125(LC 1), 4=113(LC 1), 5=397(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-5=-297/263

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.







| LOADING (psf)   TCLL 20.0   TCDL 10.0   BCLL 0.0 *   BCDL 10.0             | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014 | CSI.<br>TC 0.32<br>BC 0.20<br>WB 0.07<br>Matrix-P | DEFL. i<br>Vert(LL) n/i<br>Vert(CT) n/i<br>Horz(CT) 0.00 | n (loc) l/defl L/d<br>a - n/a 999<br>a - n/a 999<br>0 n/a n/a                           | PLATES GRIP   MT20 244/190   Weight: 26 lb FT = 20%          |
|--|---|---|--|---|--|
| LUMBER-   TOP CHORD 2x4 SP No.3   BOT CHORD 2x4 SP No.3   WEBS 2x4 SP No.3 |   |   | BRACING-<br>TOP CHORD<br>BOT CHORD                       | Structural wood sheathing di<br>except end verticals.<br>Rigid ceiling directly applied | rectly applied or 6-0-0 oc purlins,<br>or 10-0-0 oc bracing. |

**REACTIONS.** (size) 1=6-11-0, 4=6-11-0, 5=6-11-0

Max Horz 1=120(LC 12) Max Uplift 4=-38(LC 12), 5=-99(LC 12)

Max Grav 1=57(LC 21), 4=125(LC 1), 5=325(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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| LOADING (psf)   TCLL 20.0   TCDL 10.0   BCLL 0.0 *   BCDL 10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014 | CSI.<br>TC 0.57<br>BC 0.36<br>WB 0.00<br>Matrix-P | DEFL. ii<br>Vert(LL) n/a<br>Vert(CT) n/a<br>Horz(CT) 0.00 | n (loc) l/defl L/d<br>a - n/a 999<br>a - n/a 999<br>) n/a n/a                              | PLATES GRIP   MT20 244/190   Weight: 17 lb FT = 20%           |
|--|---|---|---|--|---|
| LUMBER-<br>TOP CHORD 2x4 S<br>BOT CHORD 2x4 S<br>WEBS 2x4 S    | P No.3<br>P No.3<br>P No.3  |   | BRACING-<br>TOP CHORD<br>BOT CHORD                        | Structural wood sheathing dir<br>except end verticals.<br>Rigid ceiling directly applied o | rectly applied or 4-11-8 oc purlins,<br>or 10-0-0 oc bracing. |

REACTIONS. (size) 1=4-11-0, 3=4-11-0 Max Horz 1=81(LC 12)

Max Holz 1=61(LC 12) Max Uplift 1=-10(LC 12), 3=-51(LC 12) Max Grav 1=167(LC 1), 3=167(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.



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2x4 💋

2x4 ||

| LOADING (psf)<br>TCLL 20.0<br>TCDL 10.0<br>BCLL 0.0<br>BCDL 10.0           | SPACING-<br>Plate Grip DOL<br>Lumber DOL<br>* Rep Stress Inc<br>Code IRC2015 | 2-0-0<br>1.15<br>1.15<br>YES<br>/TPI2014 | <b>CSI.</b><br>TC 0.14<br>BC 0.09<br>WB 0.00<br>Matrix-P                                   | DEFL. ii<br>Vert(LL) n/a<br>Vert(CT) n/a<br>Horz(CT) 0.00   | n (loc) l/defi L/d<br>a - n/a 999<br>a - n/a 999<br>o n/a n/a | PLATES GRIP   MT20 244/190   Weight: 10 lb FT = 20% |
|--|--|--|--|---|---|---|
| LUMBER-   TOP CHORD 2x4 SP No.3   BOT CHORD 2x4 SP No.3   WEBS 2x4 SP No.3 |  | BRACING-<br>TOP CHORD<br>BOT CHORD       | Structural wood sheathing dir<br>except end verticals.<br>Rigid ceiling directly applied c | ectly applied or 2-11-8 oc purlins,<br>r 10-0-0 oc bracing. |   |   |
| REACTIONS.   | (size) 1=2-11-0, 3=2-11-   | 0  |  |   |   |   |

Max Horz 1=42(LC 12)

Max Uplift 1=-5(LC 12), 3=-27(LC 12) Max Grav 1=87(LC 1), 3=87(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Gable requires continuous bottom chord bearing.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.





