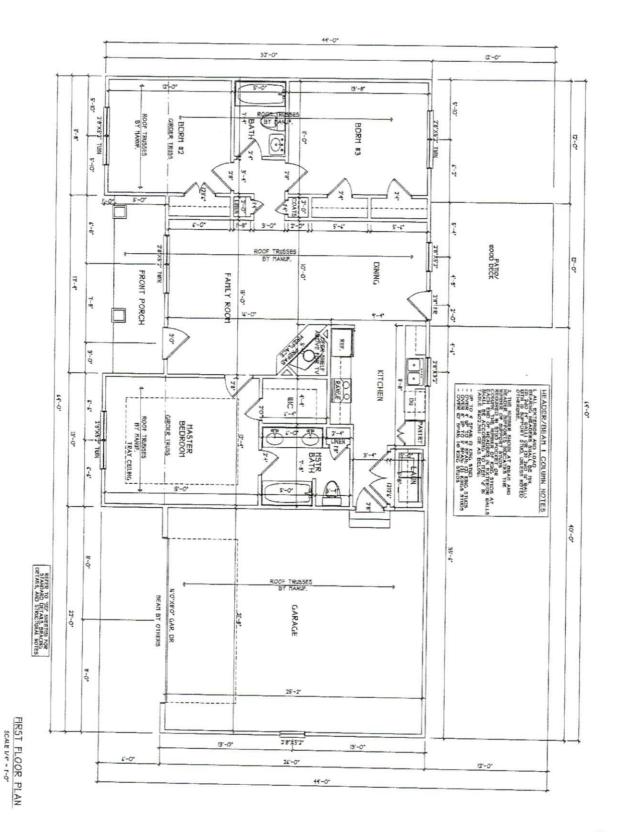


F	1 510	ATE: 02/25/2	ART DEVIATION OF THE SPECIFIC REASONING OR DIRECTOR YOUR H SONARED HOTE DESIGN INCLUDING HAS SEEN CRASH THE PLAN HAS SEEN CRASH	UJ3	H SQUARED HOME	HEATHER HALL. 185 HEATHERSTONE CT	FRONT PORCH	= 1240 = 88	#1940	"THE DAKO"	
3	7	020	N ACCOMPANCE BITH MORTH CARDINA STATE RESCRIPTIAL EMBLING CODES 200 EDITION	JU	DESIGN, INC.	BENSON NC 27504 (9(8) 207-1403	PATIO/WOOD DECK GARAGE	= 144 = 572	"1240	JRT MANG.	PROP



H SQUARED HEATHER HALL HOME HOME DESIGN, INC.

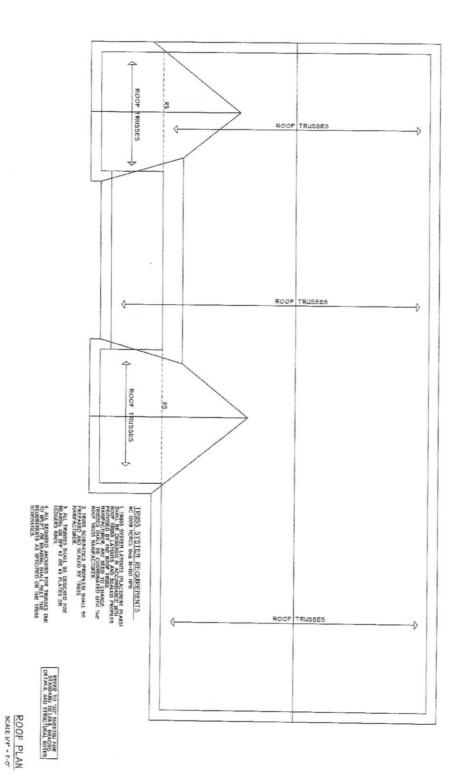
HOME DESIGN, INC.

HEATHER HALL HEATHER HOME SOURCE HOME HEATHER HALL HOME BENSON NC 27504 (989 207-1409)

HEATHER HALL HEATHER MOTAGE: FIRST FLOOR = 1240 FIRST FORCH = 86 PATIO//WOOD DECK = 144 GARAGE = 572

HEATHER HALL HEATHER MOTAGE: FIRST FLOOR = 1240 FIRST HAND GARAGED

JRT MANG. PROP.



H SQUARED HEATHER HALL SOURCE OF DECEMBER 1997 THE DAKOTA II"

HEATHER HALL ISS HEATHERSTONE CT BENSON NC 27504

DESIGN, INC.

HEATHER HALL ISS HEATHERSTONE CT BENSON NC 27504

GRANGE 207-H03

GRANGE 207-H03

JRT MANG. PROP.

#### STRUCTURAL NOTES

- ALL CORSTRUCTION SHALL CONFORM TO THE LAYEST REQUIRERIES OF THE MORTH CAROLINA STATE RESIDENTIAL CODE 2028 EDITION, PRISS ALL LOCAL CODES AND REGILATIONS. THE STRUCTURES HEAVIERS OR DESIGNES IN NOT RESPONSIBLE FOR, AND LILL NOT HAVE CONTROL OF, CONSTRUCTION HEAVIER, METHODS. TECHNORIES, SHOWERES OR PROCEDURES, OR FOR AMERIT PREVAINTIONS AND PROCEDURES OR PROCEDURES, OF FOR AMERIT PREVAINTIONS AND PROCEDURES ARE REPORTED FOR THE CONSTRUCTION FAIRER TO CONCEPT AND ARE TO CONTROL OF THE CONTROL OF A PALIES TO CONCEPT AND THE CONTROL OF A PALIES TO CONCEPT AND THE CONTROL OF A PALIES TO CONCEPT AND THE PROCEDURE OF THE CONTROL OF A CONTRACT. ALL MEDIES SHALL BE FRANCE, ARKORDED, TED AND BRACED IN ACCORDANCE WITH COOL CONSTRUCTION PRACTICE. AND THE BRACING CODE.
- 2) DESIGN LOADS (R3OL4) ROOMS OTHER THAN SLEEPING ROOMS 40
  SLEEPING ROOMS 30
  ATTIC UTTH PERHAMENT STAM 40
  ATTIC UTH OUT STORAGE 50
  ATTIC UTH OUT STORAGE 50
  STAMS 40
  EXTEROR BALCORES 40
  DECKA 40 DECKS GUARDRANS AND HANDRALS PASSENGER VEHICLE GARAGES HRE ESCAPES SNOW 1/360 1/340
- UND LOAD (BASED ON 15/20 MPH UND YELOCITY I EXPOSURE B) 3) WALL BRACKG, BRACED WALL PAKELS SHALL BE CONSTRUCTED ACCORDING TO
- A SECTION PROCESS.

  SECTION PROCESS.

  THE HENDRY AND LOCATION OF BRANKES SHALL COPING THE TABLE BROODS.

  THE LIKETH AND LOCATION OF BRANKES SHALL COPING THE TABLE BROODS.

  THE LIKETH OF BRACED PARELS SHALL BE DEFERRIBED BY SECTION BROODS.

  ATTERAL BRACES SHALL BE STATISHED FOR PERFINDO 3 DY CONTINUOUSLY

  SHALTHMO BRALLS SHIT STROCTURAL SHALL BHAD SECTION BROODS.

  SHALTHMO BRACES SHALL BE SHALLED BAY STREED HE SECTION BROODS.

  CONNECTE SHALL SHALL BE ADARDED BAY STREED HO 3 SOOP PM AND A

  CONNECTE SHALL BROOD BAY SHALL BE SHALL BOY SHALL BY A SPECTROL.

  CONNECTE SHALL BROOD BAY SHALL BY SHALL BY A SHALL BE SHALL BY A SPECTROL.

  CONNECTE SHALL BROOD BAY SHALL BY SHALL BY A SHALL BY A SPECTROL.

  BAY SHALL BROOD BAY SHALL BE ADARDED HE SHALL BY ON THE PARTACLE BY A SHALL BY

- 4) ALL FRANKG LIMBER SHALL BE 5PF #2 PF = 815 P50 URLESS NOTED OTHERWISE (UNO). ALL TREATED LIMBER SHALL BE 51F #2 CP6=115 P50, PLATE MATERIAL MAY BE 5PF #3 OR 5YP #3 (Pdf4m) = 425 P51 MNI.
- TO ALL BOODER BEAMS AND READERS SHALL HAVE THE POLICINING BND SUPPORTS.

  D 3rd STND COURS FOR F-OF PAX. BEAM SPAS SHALL FAVOR ST.

  SPAS OREATER THAN (\*-OF PAX. BEAM SPAS SHALL FAX. STDDS FOR BEAM
  SPAS OREATER THAN (\*-OF PAX. BEAM SPAS SHALL FOR SHALL SHALL BE LARKATED VENERE HISSEN F->1000 PM, F->100 PM, F-
- 9 ALL ROOT TRIDS AND 1-X3ST LATOUTS SHALL BE PREPARED IN ACCORDANCE UTH ANY SHALED STRICTURAL DRADGES, TRUSSES AND 1-X0STS SHALL BE INSTALLED ACCORDING TO THE HARMACURE'S SPECIMENTORS, ANY CHANGE IN TRUSS OR 1-X0ST LAYOUT SHALL BE COOKDRATED UTH DESIGNER OR ENGINEER.
- TRUSS OR 1-KGST LATOUT SHALL BE COOKDRATED BITH DESCRIPE OR EXCITED ALL STREUMAL STEEL BASING, STEEL BASING SHALL BE SUPPORTED AT EACH EXD STILL BE SUPPORTED AT EACH EXD STILL AND REAL THACE BOTH PROVIS SCALD BEAKING FROM BASIN SUPPORT TO SCRIPE AND REAL THACE BOTH PROVISE STATE BEAKING FROM BASING SUPPORT TO SCRIPE STREET AT LONG, LATERAL SUPPORT IS CONCEINED LAG SCREEN OF TO EACH STREET A "4" LONG, LATERAL SUPPORT IS CONCEINED AND SOLE FLATE IS MADED ON THE STREET AND SOLE FLATE IS
- D ROBAR SHALL BE DEFORMED STEEL, ASTMIS, GRADE 10.
- 13 FUTCH BEARS SHALL BE BOLTED TOGETHER USING (3) ROLS OF 1/2" DIAMETER BOLTS (ASTIT AND ILLI'NE USINESS PLACED UNDER THE THERADED END OF BOLT, BOLTS SHALL BE SPACED AT 2" CO. CHANA, AND STAGEMEN AT THE TOP AND BOTTON OF BEAR 1/2" EDGE DISTANCE, UTH 2 BOLTS LOCATED AT 4" FROM EACH END.
- (3) BRICK ENTELS SHALL BE 3 1/2'-3 1/2'-4/4' STEEL ANGLE FOR UP TO 6'-0' SPAN AND 6'-4'-5/4' STEEL ANGLE WITH 6' LEG VERTICAL FOR SPANS UP TO 4'-0' ONO).
- H) THE POSTIVE AND NEGATIVE DESIGN PRESSURE FOR DOORS AND EXHOUS FOR A MEAN ROOF HEIGHT OF 35 FEET OR LESS SHALL BE 25 PSF.
- b) THE POSITIVE AND REGATIVE DESIGN PRESSURES REQUIRED FOR ANY ROOF OR WALL CLADDING APPLICATION NOT SPECIFICALLY ADDRESSED IN THE NORTH CARCIDAS STATE RESIDENTIAL CODE 20'S EDITION SHALL BE AS FOLLOUS.
- ROOM; 45.4 PSP 2.2542 PTCH OR LESS 34.8 PSF 2.2542 TO 1/2 PTCH 31 PSF 1/2 TO 12/2 PTCH
- STALL ST. 211 PSF - UALLS

HEADER/BEAM ( COLUMN NOTES

L ALL EXTERIOR AND LOAD BEARING HEADERS SHALL BE MIN. (2) 240 (4" WALL) OR (3) 240 (4" WALL) WITH (8 SUPPORT STUD, UNLESS NOTED OTHERWISE.

7. THE NUMBER SHOWN AT BEAH AND HEALER SUPPORTS NOTCATES THE RUNDING BY STATE OF THE RUNDING BY STATE OF THE RUNDING BY STATE OF THE ST

UP TO 4" SPAN, II) KING STUD OVER 4" UP TO 8" SPAN, (2) KING STUDS OVER 8" UP TO 8" SPAN, (3) KINGS STUDS OVER 8" SPAN, (4) KING STUDS

FOUNDATION STRUCTURAL NOTES:

(T) (3) 260 SYP #2 OR SPEEZ GROEF TYPICAL ING

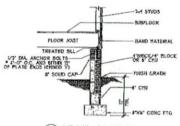
(1) CONCRETE BLOCK PER SEZ SHALL BE 5ZE HOLLOS HASONRY SOLID HASONRY 3 × K UP 10 32 HOCH UP 10 1-0 HOCH CI × K UP 10 32 HOCH UP 10 1-0 HOCH K × K UP 10 44 HOCH UP 10 1-0 HOCH K × K UP 10 44 HOCH UP 10 1-0 HOCH K × K UP 10 45 HOCH UP 10 1-0 HOCH K × K UP 10 45 HOCH UP 10 1-0 HOCH K × K UP 10 45 HOCH UP 10 1-0 HOCH K × K UP 10 45 HOCH UP 10 HOCH K × K UP 10 45 HOCH UP 10 HOCH K × K UP 10 45 HOCH UP 10 HOCH UP 10 HOCH K × K UP 10 45 HOCH UP 10 HOCH

( HALL FOOTING AS POLLOWS DEPTH: 6' - UP TO 2-0'2 STORY

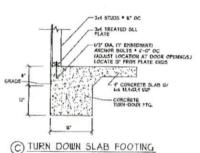
UOTH SIDNG 10R EQUAL)
- %" - W" TO 2-U2 STORY
- 20" - 3 STORY
- 18" - 1 STORY
- %" - 1 STORY
- 20" - 7 STORY
- 20" - 7 STORY

FOR FOUNDATION BULL HEIGHT AND BACKFEL RECURERENTS, REFER TO NORTH CARGINA RESOURTED CODE TABLE RECULT THE 40 AND THE ASSUMED BOX DEARRIG CAPACITY = 7000 PSF. COVITACION NUSI VERFY STE COUNTIONS AND CONTACT SOLS SHOWNER F HANGMAN OR USES AND THE SOLS SHOWNERS FOR THE SOLS SHOWNERS SHOWNERS FOR THE SOLS SHOWN

- (1) to 240 SYP#2 OR SPE#2 GIROUR
- (5) (2) LTSX1.25 LVL OR LSL GROER
- (1) IN LYEXASS LVL OR USL GIRDER
- "M" DESIGNATES A SIGNIFICANT POINT LOAD TO HAVE SOLID BLOCKING TO PIER. SOLID BLOCK ALL BEAM BEARING POINTS NOTED TO HAVE THREE OR HORE STUDS TO FKD, TYPICAL.
- e. ABBREVIATIONS: "N" = SNOLE JOIST "DJ" = DOUBLE JOIST "TJ" = TRIPLE JOIST



(A) CRAWL SECTION



- WALL STUDS PRATED SILL 1/- 1' STEP GRADE 1 CONCRETE CONCRETE TURN-DOLL

N FORDATI

(B) GARAGE SLAB

B' FOUNDATION MALL
SEE FOUNDATION PLAN
FOR FOOTING SIZE

STLL PLATE

GRADE

\$" SOLID CAP

4'BRICK/4" BLOCK

" F-O" O.C. AND BITHN D' OF PLATE ENDS SEMBED YO

-4' CONC. SLAB LITH FIBERHESH OR LITERESH ON & ML. VAPOR BARRER ON 4' CRUSHED STONE

(D) TURN DOWN SLAB & GARAGE

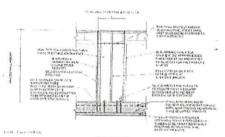


FIGURE PROZEST PART PARE

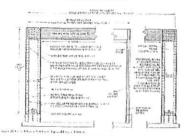
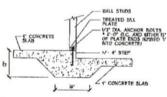
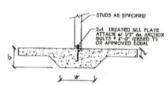


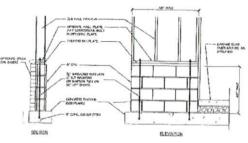
FIGURE POLICES A



(E) THICKENED SLAB & GARAGE



(F) TYPICAL THICKENED SLAB



GARAGE WING WALL' REINFORCING



SHEET

DETAIL

BUILDING ASIC B

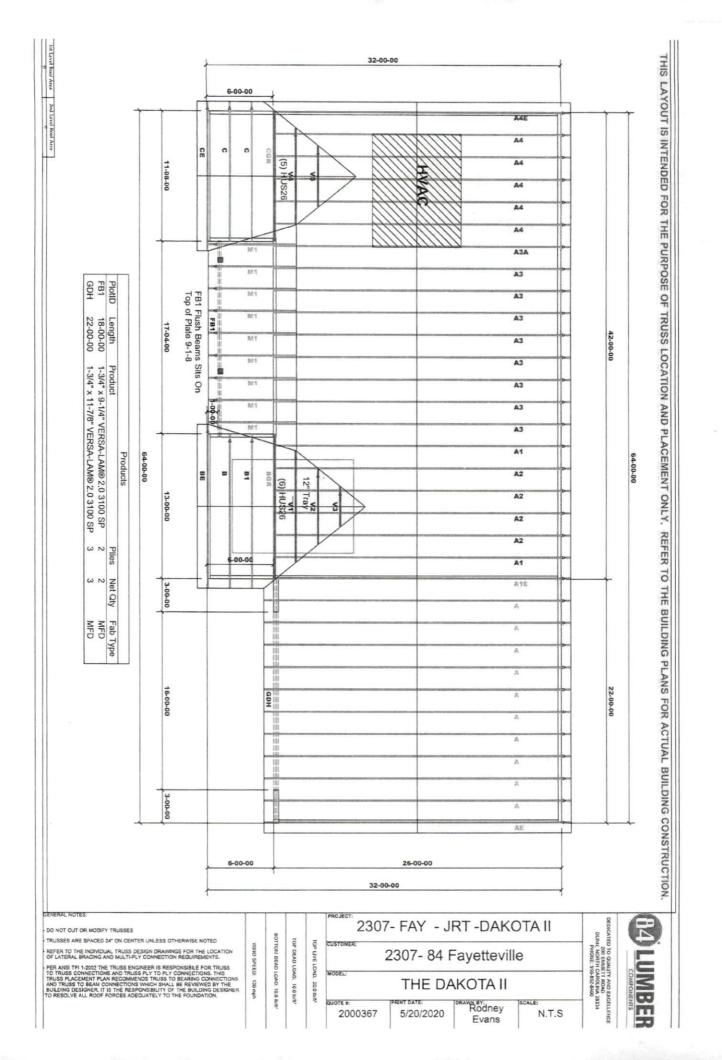
NO P APPLY NOTE A \*PLEASE ALL DET

5 TONE C 27504 HEATHERSTO BENSON NC 2 (BIBI 207-14 92

SQUARED HOME INC DESIGN, H 71

THE FLAN WE MEN CRAM N ACCORDANCE BTH KOCH CAROLM STATE RECORDAN SALING COOLS NO SETTON WIT DEVALUE OF THE SERVICE NAME OF THE SERVICE

PER





**BC CALC® Member Report** 



# Double 1-3/4" x 9-1/4" VERSA-LAM® 2.0 3100 SP

PASSED

# Roof\Dropped Beams\FB1() (Dropped Beam)

Dry | 3 spans | No cant.

May 20, 2020 13:51:46

**Build 7493** 

Job name: Address: City, State, Zip: Customer:

Code reports:

File name:

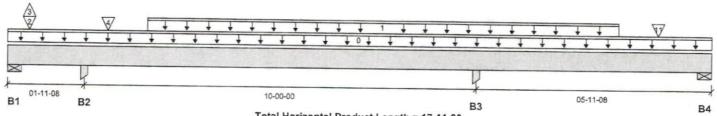
2000367A.mmdl

Description:

Roof\Dropped Beams\FB1()

Specifier: Designer:

Company:



Total Horizontal Product Length = 17-11-00

Reaction Summary (Down / Unlift) (lbs)

ESR-1040

Bearing	Live	Dead	Snow	Wind	Roof Live	
B1, 3-1/2"		0 / 242		290 / 77	71 / 380	
B2, 5-1/2"		697 / 0		190 / 702	811 / 54	
B3, 5-1/2"		602 / 0		161 / 594	653 / 0	
B4, 3-1/2"		93 / 0		21 / 74	163 / 67	

Loa	ad Summary						Live	Dead	Snow	Wind	Roof	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	Live 125%	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	17-11-00	Тор		9			12070	00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	03-06-12				57			72	n\a
2	M1(c1)	Conc. Pt. (lbs)	L	00-06-12	00-06-12	Top		64			50	n\a
3	M1(c1)	Conc. Pt. (lbs)	L	00-06-12	00-06-12						-21	n\a
4	M1(c1)	Conc. Pt. (lbs)	L	02-06-12	02-06-12			126			168	n\a
11	M1(c1)	Conc. Pt. (lbs)	L	16-06-12				102			130	n\a

Controls Summary	Value	% Allowable	Duration	Case	Location
Pos. Moment	779 ft-lbs	4.7%	125%	4	06-06-12
Neg. Moment	-1155 ft-lbs	7.1%	125%	5	01-11-08
End Shear	576 lbs	7.5%	125%	4	01-00-12
Cont. Shear	689 lbs	9.0%	125%	5	00-11-08
Total Load Deflection	L/999 (0.022")	n\a	n\a	4	07-00-12
Live Load Deflection	L/999 (0.012")	n\a	n\a	312	07-02-04
Total Neg. Defl.	L/999 (-0.003")	n\a	n\a	4	13-10-15
Max Defl.	0.022"	n\a	n\a	4	07-00-12
Span / Depth	13.0				

Beari	ng Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Wall/Plate	3-1/2" x 3-1/2"	29 lbs	0.6%	0.3%	Spruce-Pine-Fir
B1	Uplift		622 lbs			
B2	Column	5-1/2" x 3-1/2"	1508 lbs	10.8%	10.4%	Unspecified
B3	Column	5-1/2" x 3-1/2"	1255 lbs	9.0%	8.7%	Unspecified
B4	Wall/Plate	3-1/2" x 3-1/2"	256 lbs	4.9%	2.8%	Spruce-Pine-Fir

### Cautions

Uplift of -622 lbs found at bearing B1.





# Double 1-3/4" x 9-1/4" VERSA-LAM® 2.0 3100 SP

## Roof\Dropped Beams\FB1() (Dropped Beam)

Dry | 3 spans | No cant.

May 20, 2020 13:51:46

PASSED

BC CALC® Member Report

Build 7493 Job name:

Address:

City, State, Zip: Customer:

Code reports:

ESR-1040

File name:

2000367A.mmdl

Description:

Roof\Dropped Beams\FB1()

Specifier:

Designer:

Company:

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Design meets arbitrary (0.75") Maximum live load deflection criteria.

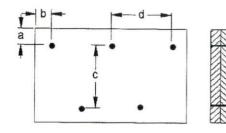
Calculations assume unbraced length of Top: 01-10-08, Bottom: 01-10-08.

BC CALC® analysis is based on IBC 2012.

Wind loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

## Connection Diagram: Full Length of Member



a minimum = 2"

c = 5-1/4"

b minimum = 3"

d = 24"

Connectors are: 3-1/4 in. Pneumatic Gun Nails

### Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





# Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

B2

May 20, 2020 13:51:46

Roof\Dropped Beams\GDH(i19) (Dropped Beam)

BC CALC® Member Report

**Build 7493** Job name:

Address: City, State, Zip:

Customer:

Code reports:

ESR-1040

Dry | 1 span | No cant.

File name:

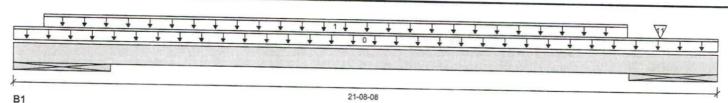
2000367A.mmdl

Description:

Roof\Dropped Beams\GDH(i19)

Specifier: Designer:

Company:



Total Horizontal Product Length = 21-08-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind	Roof Live
B1, 36" B2, 32-1/2"		2891 / 0		1374 / 1950	2804 / 0
DZ, 32-1/2		2878 / 0		1383 / 1999	2811 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Roof	Tributary
Tag		Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	Live 125%	•
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	21-08-08	Top		18	11070	10070	12070	00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L		18-11-04			268			279	n\a
11	A(c1)	Conc. Pt. (lbs)	L	19-11-04	19-11-04	Тор		557			600	n\a

Value	% Allowable	Duration	Case	Location
17173 ft-lbs	44.1%		1	11-11-04
3425 lbs	23.1%	# 100 F00 F00 F00 F00 F00 F00 F00 F00 F00	1	03-11-14
L/356 (0.544")	67.5%		1	10-11-04
L/718 (0.27")	50.2%		10.100.000	10-11-04
0.544"	54.4%		1	10-11-04
16.3		1110	•	10-11-04
1107 lbs	8.0%	100%		
1157 lbs	8.4%	100%		
	17173 ft-lbs 3425 lbs L/356 (0.544") L/718 (0.27") 0.544" 16.3 1107 lbs	17173 ft-lbs 44.1% 3425 lbs 23.1% L/356 (0.544") 67.5% L/718 (0.27") 50.2% 0.544" 54.4% 16.3 1107 lbs 8.0%	17173 ft-lbs 44.1% 125% 3425 lbs 23.1% 125% L/356 (0.544") 67.5% n\a L/718 (0.27") 50.2% n\a 0.544" 54.4% n\a 16.3 1107 lbs 8.0% 100%	17173 ft-lbs 44.1% 125% 1 3425 lbs 23.1% 125% 1 L/356 (0.544") 67.5% n\a 1 L/718 (0.27") 50.2% n\a 116 0.544" 54.4% n\a 1 16.3 1107 lbs 8.0% 100%

Bear	ing Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Wall/Plate	36" x 5-1/4"	5695 lbs	4.2%	4.0%	Unspecified
B2	Wall/Plate	32-1/2" x 5-1/4"	5689 lbs	7.8%	4.4%	Spruce-Pine-Fir

#### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Design meets arbitrary (0.75") Maximum live load deflection criteria.

Calculations assume unbraced length of Top: 01-10-08, Bottom: 01-10-08.

BC CALC® analysis is based on IBC 2012.

Wind loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.





# Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

Roof\Dropped Beams\GDH(i19) (Dropped Beam)

Dry | 1 span | No cant.

May 20, 2020 13:51:46

BC CALC® Member Report Build 7493

Job name:

Address: City, State, Zip:

Customer:

Code reports:

ESR-1040

File name:

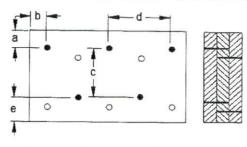
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Description: Roof\Dropped Beams\GDH(i19)

Specifier:

Designer: Company:

# Connection Diagram: Full Length of Member



a minimum = 2" b minimum = 3" c = 7 - 7/8" d = 24"

e minimum = 3"

Nailing applies to both sides of the member Connectors are: 3-1/4 in. Pneumatic Gun Nails

#### Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

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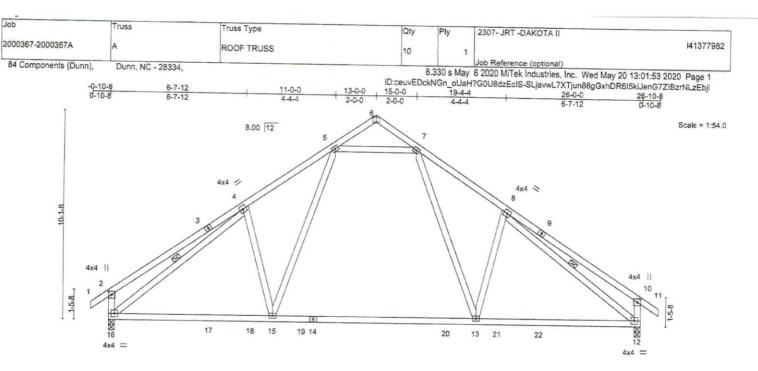


Plate Offsets (X,Y)-	8-0-0 8-0-0 [2:0-2-0,0-1-12], [6:0-2-0,Edge], [10:0-2-	0,0-1-12]	18-0-0 10-0-0			26-0-0 8-0-0	4
COADING (psf) FCLL 20.0 FCDL 10.0 BCLL 0.0	SPACING-         2-0-0           Plate Grip DOL         1.15           Lumber DOL         1.15           Rep Stress Incr         YES	CSI. TC 0.64 BC 0.76 WB 0.44		13-15 >798 13-15 >551	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
CDL 10.0	Code IRC2015/TPI2014	Matrix-S				Weight: 164 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.1

2x4 SP No.3

(size) 16=0-3-8, 12=0-3-8

Max Horz 16=273(LC 11)

Max Uplift 16=-126(LC 12), 12=-126(LC 13) Max Grav 16=1165(LC 20), 12=1165(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-381/239, 4-5=-1323/338, 7-8=-1323/338, 8-10=-381/239, 2-16=-403/228,

10-12=-402/228

**BOT CHORD** 

15-16=-130/1191, 13-15=-11/927, 12-13=-82/1056

WEBS

7-13=-152/617, 8-13=-265/281, 5-15=-151/617, 4-15=-265/281, 4-16=-1197/24,

8-12=-1196/24, 5-7=-791/286

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 3x4 MT20 unless otherwise indicated.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16 and 12. This connection is for uplift only and does not consider lateral forces.

7) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



Structural wood sheathing directly applied or 4-9-12 oc purlins.

4-16, 8-12

Rigid ceiling directly applied or 10-0-0 oc bracing.

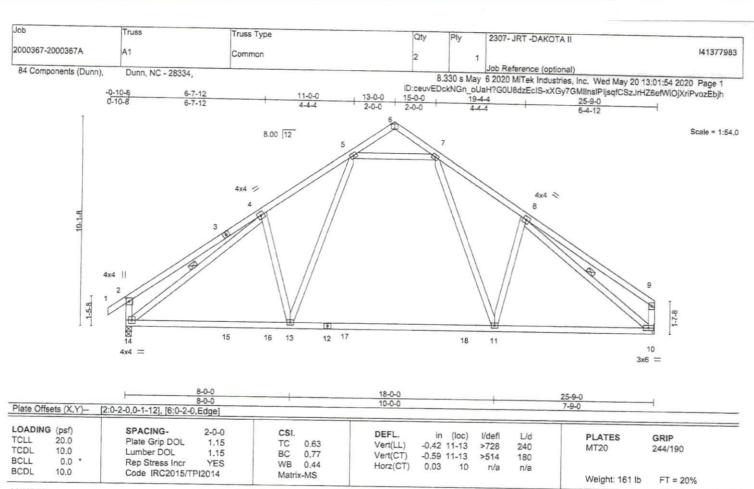
except end verticals.

1 Row at midnt

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property dange. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSVTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Edenton, NC 27932



BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No.1

2x4 SP No.3

(size) 14=0-3-8, 10=Mechanical

Max Horz 14=270(LC 11) Max Uplift 14=-125(LC 12), 10=-103(LC 13) Max Grav 14=1142(LC 19), 10=1049(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-369/232, 4-5=-1291/330, 7-8=-1237/329, 8-9=-255/166, 2-14=-392/223,

9-10=-267/148

**BOT CHORD** 13-14=-144/1158, 11-13=-43/883, 10-11=-123/975

WEBS

4-13=-262/277, 5-13=-144/626, 7-11=-140/526, 8-11=-236/271, 4-14=-1174/29, 8-10=-1218/107, 5-7=-762/287

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 3x4 MT20 unless otherwise indicated.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Refer to girder(s) for truss to truss connections.

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=103
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 4-8-2 oc purlins,

4-14, 8-10

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty Ply 2307- JRT -DAKOTA II 2000367-2000367A 141377984 A1E Common Supported Gable Job Reference (optional) 84 Components (Dunn), Dunn, NC - 28334 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:01:57 2020 Page 1 ID:ceuvEDckNGn\_oUaH?G0U8dzEcIS-L6y5IIOdaiEKGIRRVnm9byTtJJq7j7k9Dpx3W7zEbje 4x4 = Scale = 1:65 6 Q 8.00 12 10 6x6 / 12 10-1-8 13 3x4 | 15 16 32 31 30 29 28 27 26 25 23 22 21 20 19 18 17 3x4 | 3x4 = LOADING (psf) SPACING. 2-0-0 CSI. DEFL I/defl L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.28 Vert(LL) -0.00 n/r 120 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 BC 0.15 Vert(CT) -0.00 n/r 90 BCLL 0.0 Rep Stress Incr YES WB 0.14 Horz(CT) -0.00 17 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-R Weight: 191 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 **WEBS** 2x4 SP No.3

OTHERS 2x4 SP No 3 BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals. BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 31-32,30-31,29-30.

WEBS 1 Row at midot 9-24, 8-25, 10-23

REACTIONS. All bearings 25-9-0

(lb) - Max Horz 32=270(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 25, 27, 28, 29, 30, 23, 22, 21, 20, 19 except 32=-281(LC 8),

17=-302(LC 11), 31=-282(LC 9), 18=-268(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 25, 27, 28, 29, 30, 23, 22, 21, 20, 19 except 32=335(LC 9),

17=306(LC 8), 24=270(LC 13), 31=332(LC 10), 18=349(LC 11)

TOP CHORD

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 7-8=-221/274, 8-9=-265/324, 9-10=-265/324, 10-11=-222/274

WEBS 9-24=-291/175

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 32, 17, 25, 27, 28, 29, 30, 31, 23, 22, 21, 20, 19, and 18. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Edenton, NC 27932 Job Truss Truss Type 2307-JRT -DAKOTA II Qty 141377985 2000367-2000367A A2 Roof Special lob Reference (optional) 84 Components (Dunn), Dunn, NC - 28334. 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:01:58 2020 Page 1 ID:ceuvEDckNGn\_oUaH?G0U8dzEcIS-pJWTyePFL?MBtv0e3UHO89?\_6j?6SU3JSTgc2ZzEbjd 18-8-8 5-8-8 13-0-0 25-9-0 7-0-8 4x6 || Scale = 1:62.7 5 8.00 12 2x4 || 4x4 / 10-1-8 4x4 | 8x8 = 15 16 9 6x6 = 3x6 = 5x12 = 3x4 || Plate Offsets (X,Y)--[2:0-2-0,0-1-12], [7:0-3-13,0-0-4], [7:0-0-3,0-0-2], [7:Edge,0-3-2], [8:0-5-8,0-4-0], [10:0-6-0,0-3-0] LOADING (psf) SPACING-2-0-0 CSI (loc) DEFL I/defl Ľď **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.54 Vert(LL) -0.17 10-11 >999 240 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 BC 0.87 Vert(CT) -0.34 9-10 >917 180 BCLL 0.0 Rep Stress Incr YES WB 0.52 Horz(CT) 0.03 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Weight: 161 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 4-3-8 oc purlins, 2x4 SP No.2 \*Except\* **BOT CHORD** except end verticals. 6-9: 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS 2x4 SP No.3 WERS 1 Row at midpt

WEDGE Right: 2x4 SP No.3

REACTIONS.

(size) 11=0-3-8, 7=Mechanical

Max Horz 11=-250(LC 10)

Max Uplift 11=-125(LC 12), 7=-104(LC 13) Max Grav 11=1087(LC 1), 7=1023(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-380/197, 4-5=-1106/324, 5-6=-1426/447, 6-7=-1459/270, 2-11=-403/196 BOT CHORD

10-11=-185/1016, 6-8=-422/297, 7-8=-119/1127 WEBS

4-10=-280/266, 5-10=-111/458, 8-10=-39/659, 5-8=-257/776, 4-11=-990/85

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

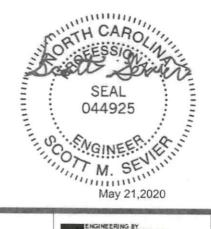
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed: MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=104.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 11. This connection is for uplift only and does not consider lateral forces.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer. Bracking indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracking is always required for stability and to prevent occliapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracking of trusses and truss systems, see 
ANSITTPI Quality Criteria, DSB-69 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Ro

Job Truss Truss Type Qty 2307- JRT -DAKOTA II 2000367-2000367A A3 141377986 **ROOF TRUSS** lob Reference (optional) 84 Components (Dunn), Dunn, NC - 28334. 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:00 2020 Page 1 ID:ceuvEDckNGn\_oUaH?G0U8dzEcIS-lheENJRWtdcv7DA0BvJsDa5JNXiFwPSbvn9j7RzEbjb 15-0-0 19-4-4 26-0-0 6-7-12 3x4 = 8.00 12 3x4 / 3x4 < 4x4 0 4x4 10-1-8 4x4 | 4x4 || 9 1-5-8 15 13 16 17 12 4x4 = 3x4 = 3x4 = 3x4 = 4x4 = Plate Offsets (X,Y)- [2:0-2-0,0-1-12], [6:0-2-0,Edge] LOADING (psf) SPACING-CSI. DEFL L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.61 Vert(LL) -0.39 11-13 >798 240 MT20 244/190 TCDL 100 Lumber DOL 1.15 BC 0.76 Vert(CT -0.56 11-13 >551 180 BCLL 0.0 Rep Stress Inc. YES WB 0.46 Horz(CT) 0.03 10 n/a BCDL 100 Code IRC2015/TPI2014 Matrix-MS Weight: 162 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 4-9-9 oc purlins. BOT CHORD 2x4 SP No.1 except end verticals. WEBS 2x4 SP No 3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS 1 Row at midpt 4-14, 8-10 REACTIONS. (size) 14=0-3-8, 10=0-3-8 Max Horz 14=268(LC 9) Max Uplift 14=-126(LC 12), 10=-105(LC 13) Max Grav 14=1166(LC 20), 10=1106(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

2-4=-371/232, 4-5=-1320/333, 7-8=-1329/336, 8-9=-291/175, 2-14=-394/223,

9-10=-292/156

**BOT CHORD** WEBS

13-14=-144/1184, 11-13=-37/920, 10-11=-123/1053 7-11=-148/619, 8-11=-269/279, 5-13=-145/607, 4-13=-257/277, 4-14=-1203/32,

8-10=-1250/97, 5-7=-795/288

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 10. This connection is for uplift only and does not consider lateral forces.

6) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE U.S. Design valid for use only with MITek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITP!1 Quality Criteria, DSB-69 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty Ply 2307- JRT -DAKOTA II 2000367-2000367A A3A ROOF TRUSS 141377987 ob Reference (optional) 84 Components (Dunn), Dunn, NC - 28334. 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:01 2020 Page 1 ID:ceuvEDckNGn\_oUaH?G0U8dzEcIS-DuCcbfR8ewkmkMiDkdq5modU7w0Lft8i8QvHfuzEbja 6-7-12 15-0-0 19-4-4 26-0-0 Scale = 1:54.8 8.00 12 4x4 / 4x4 10-1-8 4x4 || 1-5-8 0-10-12 19 17 18 15 14 13 20 12 11 4x6 | 4x6 || 10 4x6 || 3x6 = 4x6 | 4x6 | 3x6 = Plate Offsets (X,Y)-[2:0-2-0,0-1-12], [6:0-2-0,Edge] LOADING (psf) SPACING-CSI. DEFL. I/defl L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.61 Vert(LL) -0.17 15-16 >999 240 MT20 TCDL 244/190 10.0 Lumber DOL 1.15 BC 0.84 Vert(CT) -0.24 15-16 >999 180 BCLL 0.0 Rep Stress Incr YES WB 0.44 Horz(CT) 0.03 10 n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Weight: 187 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 4-10-6 oc purlins, BOT CHORD 2x4 SP No.2 \*Except\* except end verticals. 12-14: 2x8 SP No.2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS 2x4 SP No.3 WEBS 1 Row at midpt 4-16, 8-10 REACTIONS.

(size) 16=0-3-8, 10=0-3-8

Max Horz 16=268(LC 9)

Max Uplift 16=-126(LC 12), 10=-105(LC 13) Max Grav 16=1159(LC 20), 10=1099(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown 2-4=-384/226, 4-5=-1282/338, 7-8=-1291/341, 8-9=-319/169, 2-16=-412/219. TOP CHORD

9-10=-307/152

15-16=-147/1155, 11-15=-40/897, 10-11=-126/1025

BOT CHORD WEBS

7-11=-149/596, 8-11=-271/278, 5-15=-146/583, 4-15=-260/275, 4-16=-1133/44,

8-10=-1179/109, 5-7=-772/291

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 3x4 MT20 unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16 and 10. This connection is for uplift only and does not consider lateral forces.

7) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type		4. Te				
2000367-2000367A	A4	ROOF TRUSS	5	ity i	Ply 1	2307- JRT -DAKO	TA II	141377988
84 Components (Dunn),	Dunn, NC - 28334,			200		Job Reference (opt	ional)	
			ID:ceuvEDci	8.33 NGn pU	0 s May	6 2020 MiTek Indus	tries, Inc. Wed May 20 1 EscMWKPIKMKI?AfcKN	3:02:02 2020 Page 1
	-9 <u>-10-8</u> 6-7-12 0-10-8 6-7-12		13-0-0 15-0-0	- 10		25-9	-0	JOKquM4eqBKzEbjZ
			2-0-0	4-	4-4	6-4-1	12	
			3x4 =					Scale = 1:63.1
	4x4    4x4    5x4    5x5    5x	17 18 15 14 13	6 x6    4x6    4	3x4 > 7  19 12  4x6	2 11 3x4 =	4x4 11 8	3x4    9 10 3x6 =	
	8-0-		18-0-0			25-9-0		
Plate Offsets (X,Y)- [2	:0-2-0,0-1-12], [6:0-2-0,Edge]	0	10-0-0			7-9-0		
TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-         2-0-0           Plate Grip DOL         1.15           Lumber DOL         1.15           Rep Stress Incr         YES           Code IRC2015/TPI2014	CSI. TC 0.63 BC 0.79 WB 0.41 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in ( -0.17 15 -0.26 15 0.03	5-16 >9 5-16 >9	defl L/d 999 240 999 180 n/a n/a	PLATES MT20 Weight: 186 lb	GRIP 244/190 FT = 20%
12-14: 2x	o.2 *Except* 8 SP No.2	13)	BRACING- TOP CHORI BOT CHORI WEBS	ex Ri	cept end	verticals.  g directly applied	rectly applied or 4-9-10 or 10-0-0 oc bracing.	

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-4=-381/226, 4-5=-1247/335, 7-8=-1193/334, 8-9=-265/160, 2-16=-408/219, TOP CHORD

9-10=-273/144

15-16=-147/1125, 11-15=-46/856, 10-11=-127/943

4-15=-264/275, 5-15=-145/599, 7-11=-142/499, 8-11=-239/269, 4-16=-1095/41, WEBS

8-10=-1140/119, 5-7=-757/290

#### NOTES-

BOT CHORD

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=103.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16. This connection is for uplift only and does not consider lateral forces.

8) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

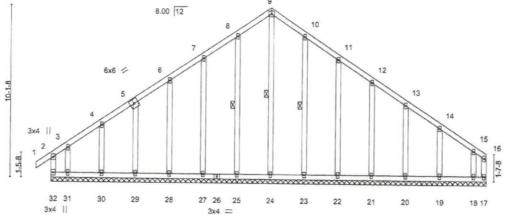


MARNING - Verily design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MT6480 connectors. This design is based only upon parameters who, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property beginning. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITP! Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty 2307- JRT -DAKOTA II 141377989 2000367-2000367A A4E Common Supported Gable ob Reference (optional) 84 Components (Dunn), Dunn, NC - 28334. 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:03 2020 Page 1 ID:ceuvEDckNGn\_oUaH?G0U8dzEcIS-AGJM?LTOAY\_T\_gvbs2tZrDjuoktX7rD2bkONkmzEbjY 4x4 = Scale = 1:65.6 9 8.00 12 10



LOADING (psf) SPACING-2-0-0 CSI DEFL. (loc) in VdefI PLATES L/d GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.28 Vert(LL) -0.00 n/r 120 MT20 244/190 TCDL 10.0 Lumber DOI 1.15 BC 0.15 Vert(CT -0.00n/r 90 BCLL 0.0 Rep Stress Incr YES WB 0.14 Horz(CT) -0.00 17 n/a n/a BCDI 10.0 Code IRC2015/TPI2014 Matrix-R Weight: 191 lb FT = 20%

LUMBER-TOP CHORD 2x4 S BOT CHORD 2x4 S

2x4 SP No.2 2x4 SP No.2 2x4 SP No.3

WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3 BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly

Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 31-32,30-31,29-30.

WEBS 1 Row at midpt 9-24, 8-25, 10-23

REACTIONS. All bearings 25-9-0.

(lb) - Max Horz 32=270(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 25, 27, 28, 29, 30, 23, 22, 21, 20, 19 except 32=-281(LC 8),

17=-302(LC 11), 31=-282(LC 9), 18=-268(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 25, 27, 28, 29, 30, 23, 22, 21, 20, 19 except 32=335(LC 9), 17=306(LC 8), 24=270(LC 13), 31=332(LC 10), 18=349(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 7-8=-221/274, 8-9=-265/324, 9-10=-265/324, 10-11=-222/274

WEBS 9-24=-291/175

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSTPT1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty 2307- JRT -DAKOTA II 2000367-2000367A AE GABLE 141377990 84 Components (Dunn), lob Reference (optional) Dunn, NC - 28334. 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:05 2020 Page 1 D:ceuvEDckNGn\_oUaH?G0U8dzEcIS-6fR7Q1Vei9EBD\_2\_zTv1weoEXYZ2blhL32tUofzEbjW 26-10-8 0-10-8 4x4 = Scale = 1:65.6 8.00 12 6x6 / 12 6x6 < 10-1-8 13 3x4 3x4 11 15 16 1-5-8 33 32 30 29 28 27 26 25 23 22 21 20 19 3x4 || 3x4 = 3x4 || LOADING (psf) SPACING-2-0-0 CSI DEFL (loc) I/def L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.27 Vert(LL) -0.00 17 n/r 120 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 BC 0.15 Vert(CT) -0.0017 n/r 90 BCII 0.0 Rep Stress Incr YES WB 0.14 Horz(CT) 0.01 18 n/a n/a BCDI 10.0 Code IRC2015/TPI2014 Matrix-R Weight: 194 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, BOT CHORD 2x4 SP No.2 except end verticals WEBS 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. 2x4 SP No.3 WEBS

OTHERS

1 Row at midpt 9-25, 8-26, 10-24

REACTIONS. All bearings 26-0-0

(lb) - Max Horz 33=273(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 26, 28, 29, 30, 31, 24, 23, 22, 21, 20 except 33=-260(LC 8),

18=-209(LC 9), 32=-255(LC 9), 19=-216(LC 8)

All reactions 250 lb or less at joint(s) 18, 26, 28, 29, 30, 31, 24, 23, 22, 21, 20 except 33=303(LC

11), 25=269(LC 13), 32=320(LC 10), 19=279(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

WEBS

7-8=-218/276, 8-9=-262/327, 9-10=-262/327, 10-11=-218/276

9-25=-294/172

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For study exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 2-0-0 oc.

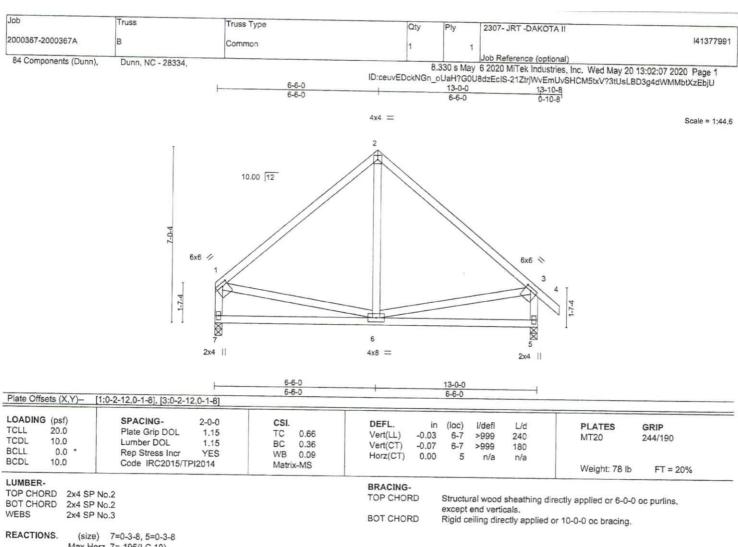
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

11) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandra, VA 22314.





Max Horz 7=-195(LC 10)

Max Uplift 7=-42(LC 12), 5=-61(LC 13) Max Grav 7=506(LC 1), 5=572(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-473/120, 2-3=-480/127, 1-7=-449/126, 3-5=-515/169 TOP CHORD

**BOT CHORD** 

6-7=-187/265

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 5. This connection is for uplift only and does not consider lateral forces.



▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a russ system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty 2307- JRT -DAKOTA II 2000367-2000367A B1 141377992 Roof Special | Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:08 2020 Page 1 84 Components (Dunn), Dunn, NC - 28334. ID:ceuvEDckNGn\_oUaH?G0U8dzEcIS-WE7F32XX?4cm4RnZfbTkYGQmOIR4o65ni058P\_zEbjT 10-8-8 13-10-8 4x6 || Scale = 1:44.6 3 10.00 12 3x4 || 3x4 || 7-0-4 3x6 3x6 N 1-7-4 13 4x8 14 4x8 11 2x4 | 3x6 = 3x6 = 2x4 || LOADING (psf) SPACING-2-0-0 CSI. DEFL I/defi L/d **PLATES** TCLL GRIP 20.0 Plate Grip DOL 1.15 TC 0.23 Vert(LL) -0.20 9-10 >762 240 MT20 TCDL 244/190 10.0 Lumber DOL 1.15 BC 0.76 Vert(CT) -0.42 9-10 >362 180 BCLL 0.0 Rep Stress Incr YES WB 0.17 Horz(CT) 0.11 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Weight: 85 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, BOT CHORD 2x4 SP No.2 except end verticals. 2x4 SP No.3 WEBS BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. REACTIONS. (size) 12=0-3-8, 7=0-3-8 Max Horz 12=-195(LC 8)

Max Uplift 12=-42(LC 12), 7=-61(LC 13) Max Grav 12=506(LC 1), 7=572(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD

1-2=-449/110, 2-3=-823/271, 3-4=-730/257, 4-5=-442/111, 1-12=-538/102, 5-7=-601/151

BOT CHORD

2-10=-375/218, 9-10=-31/338, 4-9=-357/202 WEBS

3-9=-153/441, 3-10=-172/523, 1-11=-79/358, 5-8=-48/353

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12 and 7. This connection is for uplift only and does not consider lateral forces.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTex® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty 2307- JRT -DAKOTA II 2000367-2000367A BE 141377993 Common Supported Gable 84 Components (Dunn), b Reference (optional) Dunn, NC - 28334 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:09 2020 Page 1 ID:ceuvEDckNGn\_oUaH?G0U8dzEcIS-\_QgdGOY9mNkdibMlCl\_z4UzzY9yXXaKw\_grixQzEbjS 13-0-0 4×4 = Scale = 1:46.3 10.00 12 7-0-4 4x6 / 4x6 N 8 9 16 15 12 3x6 = 4x4 = 4x4 =3x6 = Plate Offsets (X,Y)- [2:0-2-14,0-2-0], [8:0-2-14,0-2-0] LOADING (psf) SPACING-2-0-0 CSI DEFL I/def PLATES GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.07 Vert(LL) -0.00 9 n/r 120 MT20 TCDL 10.0 244/190 Lumber DOL 1.15 BC 0.05 Vert(CT) -0.009 n/r 90 BCLL 0.0 Rep Stress Inc. YES WB 0.10 Horz(CT) 0.00 11 n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Weight: 95 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

OTHERS 2x4 SP No.3 BRACING-

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 13-0-0.

(lb) - Max Horz 16=-202(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 12 except 15=-169(LC 12), 11=-165(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty Ply 2307- JRT -DAKOTA II 2000367-2000367A BGR Roof Special Girder 141377994 2 84 Components (Dunn), | \( \bigsize \) Job Reference (optional)
8.330 s May 6 2020 MITek Industries, Inc. Wed May 20 13:02:12 2020 Page 1 Dunn, NC - 28334. ID:ceuvEDckNGn\_oUaH?G0UBdzEcIS-P?MmuQa22I7CZ35KuRXgi6bRNMmNkm5Mge3MYlzEbjP 13-0-0 4x4 || Scale = 1:44.6 10.00 12 3x6 // 3x6 N P-0-2 3x6 1 3x6 1-7-4 to 8 9 15 16 17 5x12 5x12 = LUS26 13 8x8 = LUS26 LUS26 4x4 244 2x4 4x4 = LUS26 LUS26 Plate Offsets (X,Y)-[9:0-4-0,0-4-8] LOADING (psf) SPACING-2-0-0 CSI. DEFL (loc) in I/def **PLATES** TCII 20.0 GRIP Plate Grip DOL 1.15 TC 0.29 Vert(LL) -0.04 9-10 >999 240 MT20 TCDL 10.0 244/190 Lumber DOL 1.15 BC 0.87 Vert(CT) -0.099-10 >999 180 BCLL 0.0 Rep Stress Incr NO WB 0.74 Horz(CT) 0.06 BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Weight: 197 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, BOT CHORD 2x6 SP No.2 \*Except\* except end verticals. 2-11,4-7: 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing WERS 2x4 SP No.3 REACTIONS. (size) 12=0-3-8, 6=0-3-8 Max Horz 12=178(LC 11) Max Uplift 12=-413(LC 12), 6=-362(LC 13) Max Grav 12=3735(LC 1), 6=3288(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-4156/562

1-2=-4156/562, 2-3=-3092/450, 3-4=-3092/450, 4-5=-4254/553, 1-12=-3176/425,

5-6=-3255/423

BOT CHORD 10-11=-44/458, 2-10=-171/1009, 9-10=-468/3281, 8-9=-453/3338, 4-8=-154/1095 WEBS 3-9=-449/3582, 4-9=-1141/267, 2-9=-1078/274, 1-10=-380/3096, 5-8=-414/3223

### NOTES-

1) 2-ply truss to be connected together with 10d (0.120"x3") nails as follows:

Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc, 2x4 - 1 row at 0-7-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 6) \* This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 12, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12 and 6. This connection is for uplift only and does not consider lateral forces.
   Use Simpson Strong-Tie LUS26 (4-10d Girder, 4-10d Truss) or equivalent spaced at 2-0-0 oc max. starting at 1-0-12 from the left

end to 9-0-12 to connect truss(es) to back face of bottom chord.

10) Fill all nail holes where hanger is in contact with lumber.

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 997 lb down and 123 lb up at 10-10-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEX REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITTP11 Quality Criteria, DSB-69 and BCSI Building Component Safety Information available from Truss Piate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Job	Truss	T T				
	iruss	Truss Type	Qty	Ply	2307- JRT -DAKOTA II	
2000367-2000367A	BGR	Roof Special Girder	1	_		141377994
84 Components (Dunn)	Duno MC 20224			2	Job Reference (optional)	

8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:12 2020 Page 2 ID:ceuvEDckNGn\_oUaH?G0U8dzEcIS-P?MmuQa22I7CZ35KuRXgi6bRNMmNkm5Mge3MYIzEbjP

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-60, 3-5=-60, 11-12=-20, 8-10=-20, 6-7=-20 Concentrated Loads (lb)

Vert: 8=-997(B) 13=-998(B) 14=-1003(B) 15=-1003(B) 16=-1003(B) 17=-1003(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE U.S.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N, Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	-					
	11033	Truss Type	Qty	Ply	2307- JRT -DAKOTA I	1	
2000367-2000367A	c	Common	2	1			141377995
84 Components (Dunn),	Dunn, NC - 28334,				Job Reference (optional	()	
	20004,		ID:FD-1410	8.330 s May	6 2020 MiTak Industrias	Ina Madal - 00 10	:02:13 2020 Page 1
	-0-1		ID.CEUVEDCKNG	n_oUaH?G0l 11-8-0	J8dzEcIS-tBw86mbgpcF	3ACgWR82vFK7ZtmF	TNIWulpv4BzEbjO
	0-1	0-8' 5-10-0		5-10-0			
			4x4 =				
			484 —				Scale = 1:42.7
			3				
		10.00 12					
				//			
	65.5				\		
	4x4	, //			4x4 1		
	1	///			4		
	1, 1	1/2					
	47-1	No.					
	17				1-7-4		
	1 1	8					
		7			5		
			6		5		
	2	x4	4x8 =		2x4		
		F 10.0					
DI		5-10-0 5-10-0		11-8-0 5-10-0			
Plate Offsets (X,Y)- [2:	0-0-12,0-1-8], [4:0-0-12,0-1-8]			5-10-0			
LOADING (psf)	SPACING- 2-0-0	CSI.			T		
TCLL 20.0	Plate Grip DOL 1.15	TC 0.50			/defl L/d		RIP
TCDL 10.0	Lumber DOL 1.15	BC 0.28	Vert(LL) -0.00 Vert(CT) -0.00		999 240 999 180	MT20 2	44/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES	WB 0.08	Horz(CT) 0.0		n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MS				Weight: 71 lb	FT = 20%
LUMBER-			BRACING-				
TOP CHORD 2x4 SP No			TOP CHORD	Structural	wood choothing discou		
BOT CHORD 2x4 SP No			. Or Oriono	except en	wood sheathing direct d verticals.	ly applied or 6-0-0 oc	purlins,
WEBS 2x4 SP No	0.3		BOT CHORD		ng directly applied or 1	0-0-0 oc bracing	
REACTIONS. (size)	7=0-3-8, 5=0-3-8						
May No.	7 4040 0 0						

Max Horz 7=181(LC 9) Max Uplift 7=-56(LC 12), 5=-37(LC 13) Max Grav 7=519(LC 1), 5=452(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-422/118, 3-4=-415/112, 2-7=-468/160, 4-5=-401/117

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 5. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty Ply 2307- JRT -DAKOTA II 2000367-2000367A CF 141377996 Common Supported Gable 84 Components (Dunn), lob Reference (optional) Dunn, NC - 28334. 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:14 2020 Page 1 ID:ceuvEDckNGn\_oUaH?G0U8dzEclS-LOUWJ6clawNwoMFi?rZ8nXgqHAfqCqzf7yYTcdzEbjN 11-8-0 Scale = 1:42.7 10.00 12 6-2-9 4x6 4x6 📏 1-7-4 16 15 14 13 11 10 6x6 = 6x6 = Plate Offsets (X,Y)-- [2:0-2-14,0-2-0]. [8:0-2-14,0-2-0] LOADING (psf) SPACING-2-0-0 CSI DEFL (loc) I/defl L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.07 Vert(LL) -0.00 9 n/r 120 TCDL MT20 244/190 10.0 Lumber DOL 1.15 BC 0.04 Vert(CT) -0.00 9 n/r 90 BCLL 0.0 Rep Stress Incr YES WB 0.08 Horz(CT) 0.00 10 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Weight: 82 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

**OTHERS** 2x4 SP No.3 except end verticals.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 11-8-0.

(lb) - Max Horz 16=188(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 12 except 15=-159(LC 12), 11=-154(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIJ-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITP! Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

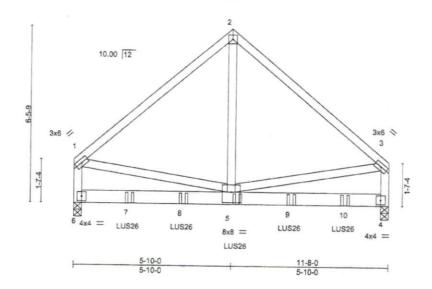


Job Truss Truss Type Qty 2307- JRT -DAKOTA II 2000367-2000367A CGR 141377997 Common Girder 2 Job Reference (optional) 84 Components (Dunn), Dunn. NC - 28334

8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:16 2020 Page 1 ID:ceuvEDckNGn\_oUaH?G0U8dzEcIS-HmcHkodY6Xdd2gO57GccsyI?8z89gdkybG1ZhWzEbjL

4x4 =

Scale = 1:40.9



COADING CCLL CCDL BCLL	(psf) 20.0 10.0 0.0	SPACING-         2-0-0           Plate Grip DOL         1.15           Lumber DOL         1.15           Rep Stress Incr         NO	CSI. TC 0.82 BC 0.88 WB 0.57	DEFL. Vert(LL) -0.0 Vert(CT) -0.1 Horz(CT) 0.0	5-6	I/defl >999 >999	L/d 240 180	PLATES MT20	GRIP 244/190
BCDL	10.0	Code IRC2015/TPI2014	Matrix-MS	11012(01) 0.0	, 4	11/4	n/a	Weight: 156 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No 2 BOT CHORD 2x6 SP No.2

WEBS 2x4 SP No.3

REACTIONS.

(size) 6=0-3-8, 4=0-3-8 Max Horz 6=-164(LC 31)

Max Uplift 6=-315(LC 12), 4=-333(LC 13) Max Grav 6=2867(LC 1), 4=3028(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

1-2=-2517/350, 2-3=-2518/350, 1-6=-2052/302, 3-4=-2050/302

**BOT CHORD** 5-6=-211/468, 4-5=-110/387

WEBS

2-5=-265/2762, 1-5=-198/1516, 3-5=-200/1508

#### NOTES-

- 1) 2-ply truss to be connected together with 10d (0.120"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-7-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 6, 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 4. This connection is for uplift only and does not consider lateral forces.

  9) Use Simpson Strong-Tie LUS26 (4-10d Girder, 4-10d Truss) or equivalent spaced at 2-0-0 oc max. starting at 2-0-12 from the left
- end to 10-0-12 to connect truss(es) to back face of bottom chord.

10) Fill all nail holes where hanger is in contact with lumber.

Continued on page 2

May 21,2020

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-59 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Truss Truss Type Qty Ply 2307- JRT -DAKOTA II 2000367-2000367A CGR 141377997 Common Girder 2 Job Reference (optional) 84 Components (Dunn), Dunn, NC - 28334.

8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:16 2020 Page 2 ID:ceuvEDckNGn\_oUaH?G0U8dzEcIS-HmcHkodY6Xdd2gO57Gccsyl?8z89gdkybG1ZhWzEbjL

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 4-6=-20

Concentrated Loads (lb)

Vert: 5=-997(B) 7=-997(B) 8=-997(B) 9=-997(B) 10=-997(B)



b	Truss	Truss Type	Qty	DI.	Teach 122	
00367-2000367A	M1	P.C.	Qty	Ply	2307- JRT -DAKOTA II	
		Monopitch	9	1		141377998
34 Components (Dunn),	Dunn, NC - 28334,			220 - 11-	Job Reference (optional)	
	-0-10-8		ID:ceuvEDckN0	n oUaH26	6 2020 MiTek Industries, Inc. Wed May 20 1 GOU8dzEcIS-HmcHkodY6Xdd2gO57Gccsyl7d:	3:02:16 2020 Page 1
1	0-10-8	5-3-8 5-3-8			7-1-8	ziAglfybG1ZhWzEbjL
		5-5-6			1-10-0	
					4	Scale = 1:17.1
		3.00 12			2x4	Ţ
					3	
						T
4						
2.1.						
						2-1-4
	2				1 1	
p.3.14					4	2-5-0
l d					1	
L	3x4 =				5 2x4 =	1 1
	$\times$			4	284 =	
				0-9-7		0.94
					6	1 1
		VERTICAL LEGS ARE NOT DESIGN LOADS IMPOSED BY SUPPORTS (E	ED FOR LATER	AL	$\bowtie$	
		SOLD BY SOFFORTS (E	DEARINGS).			

OADING (psf)  CLL 20.0  CDL 10.0  BCLL 0.0 *	SPACING-         2-0-0           Plate Grip DOL         1.15           Lumber DOL         1.15           Rep Stress Incr         YES	CSI. TC 0.28 BC 0.24 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.03 -0.06 0.00	(loc) 5-9 5-9	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
ICDL 10.0	Code IRC2015/TPI2014	Matrix-MP				100	100	Weight: 23 lb	FT = 20%

BRACING-

TOP CHORD

**BOT CHORD** 

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS

2x4 SP No.3

(size) 2=0-3-8, 6=0-3-8

Max Horz 2=92(LC 11) Max Uplift 2=-58(LC 8), 6=-96(LC 12)

Max Grav 2=240(LC 1), 6=343(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. 5-6=-343/231, 3-5=-283/242 TOP CHORD

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 5-3-8 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N, Lee Street. Suite 312, Alexandria, VA 22314.



Joh Truss Truss Type Qty 2307- JRT -DAKOTA II 2000367-2000367A V1 Valley 141377999 lob Reference (optional) 84 Components (Dunn), Dunn, NC - 28334, 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:17 2020 Page 1 D:ceuvEDckNGn\_oUaH?G0U8dzEcIS-Iz9fy7eAtrIUfqzHg\_7rPAll1NeqPBt6pwn7DyzEbjK 4-11-11 4x4 = Scale = 1:27.7 2 10.00 12 0-0 0-0-4 2x4 / 2x4 || 9-11-2 LOADING (psf) SPACING-2-0-0 CSI DEFL. in I/defl TCLL **PLATES** GRIP 20 0 Plate Grip DOL 1.15 TC 0.30 Vert(LL) n/a n/a 999 TCDL MT20 10.0 244/190 Lumber DOL 1.15 BC 0.21 Vert(CT) BCLL n/a n/a 999 0.0 Rep Stress Incr YES WB 0.07 Horz(CT) BCDL 0.00 3 n/a n/a 10.0 Code IRC2015/TPI2014 Matrix-S Weight: 38 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. 2x4 SP No.2 **BOT CHORD** BOT CHORD OTHERS 2x4 SP No.3 Rigid ceiling directly applied or 10-0-0 oc bracing. REACTIONS. (size) 1=9-10-13, 3=9-10-13, 4=9-10-13 Max Horz 1=-94(LC 8)

Max Uplift 1=-32(LC 13), 3=-43(LC 13), 4=-7(LC 12)

Max Grav 1=194(LC 1), 3=194(LC 1), 4=344(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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Job Truss Truss Type Qty 2307- JRT -DAKOTA II 2000367-2000367A V2 Valley 141378000 84 Components (Dunn), Dunn, NC - 28334, lob Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:18 2020 Page ID:ceuvEDckNGn\_oUaH?G0U8dzEcIS-E9j19Tfoe8tLH\_YUEhe4yNqVsn0y8fjF2aWgmPzEbjJ 4x4 = Scale = 1:20.0 2 10.00 12 0-0-0 0-0-4 2x4 // 2x4 || 6-8-1 LOADING (psf) SPACING-2-0-0 CSI DEFL in (loc) I/defl TCLL 20.0 L/d PLATES GRIP Plate Grip DOL 1.15 TC BC 0.16 Vert(LL) n/a n/a 999 TCDL MT20 10.0 244/190 Lumber DOL 1.15 0.09 Vert(CT) BCLL n/a n/a 999 0.0 Rep Stress Incr YES WB 0.03 Horz(CT) 0.00 BCDL 3 n/a n/a 10.0 Code IRC2015/TPI2014 Matrix-P Weight: 25 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD 2x4 SP No.2 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. **OTHERS** 2x4 SP No.3 REACTIONS. (size) 1=6-8-6, 3=6-8-6, 4=6-8-6

Max Horz 1=-61(LC 8)

Max Uplift 1=-29(LC 13), 3=-36(LC 13)

Max Grav 1=136(LC 1), 3=136(LC 1), 4=203(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a trust system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty 2307- JRT -DAKOTA II 2000367-2000367A V3 Valley 141378001 1 84 Components (Dunn), lob Reference (optional) Dunn, NC - 28334. 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:19 2020 Page 1 ID:ceuvEDckNGn\_oUaH?G0U8dzEclS-iLHPMpfRPS?Cv77goP9JUbNhiBLDt6POHEGDIrzEbjI 3x4 = 2 Scale = 1:10.2 10.00 12 1-5-12 3 0-0-0 0-0-4 2x4 // 2x4 \

Plate Offsets (X,Y)-	2:0-2-0,Edge]		3-6-5 3-6-5					3-6 <sub>0</sub> 10 0-0-5	
OADING (psf) CLL 20.0 CDL 10.0 CLL 0.0 * CDL 10.0	SPACING-         2-0-0           Plate Grip DOL         1.15           Lumber DOL         1.15           Rep Stress Incr         YES	CSI. TC 0.03 BC 0.09 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
10.0	Code IRC2015/TPI2014	Matrix-P						Weight: 11 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

(size) 1=3-6-0, 3=3-6-0

Max Horz 1=-28(LC 8) Max Uplift 1=-10(LC 12), 3=-10(LC 13)

Max Grav 1=110(LC 1), 3=110(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

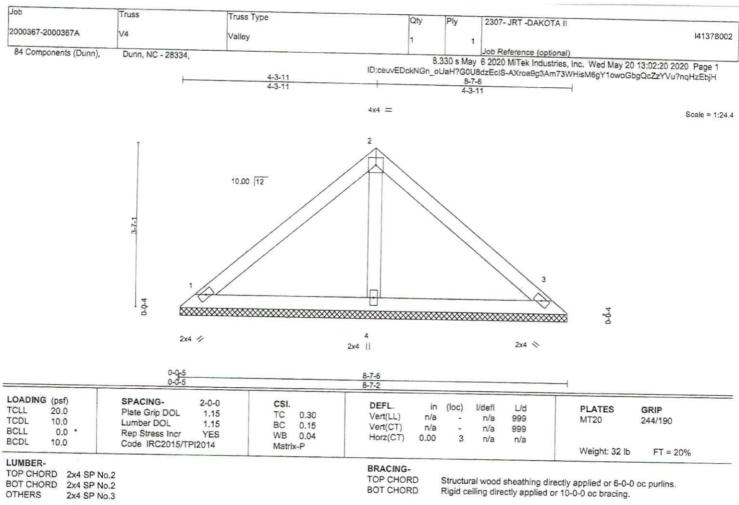


Structural wood sheathing directly applied or 3-6-10 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suits 312, Alexandria, VA 22314.





REACTIONS.

(size) 1=8-6-13, 3=8-6-13, 4=8-6-13

Max Horz 1=-80(LC 8)

Max Uplift 1=-37(LC 13), 3=-47(LC 13)

Max Grav 1=179(LC 1), 3=179(LC 1), 4=266(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty 2307- JRT -DAKOTA II 2000367-2000367A V5 Valley 141378003 84 Components (Dunn), ob Reference (optional) Dunn, NC - 28334, 8.330 s May 6 2020 MiTek Industries, Inc. Wed May 20 13:02:20 2020 Page 1 ID:ceuvEDckNGn\_oUaH?G0U8dzEcIS-AXroa9g3Am73WHisM6gY1owrRbi\_cZLYVu?nqHzEbjH 4x4 = Scale = 1:17.1 2 10.00 12 0-0-4 0-0-4 2x4 / 2x4 || 2x4 N 0-0-5 LOADING (psf) SPACING-2-0-0 CSI DEFL in I/defl TCLL (loc) L/d **PLATES** 20.0 GRIP Plate Grip DOL 1.15 TC 0.09 Vert(LL) n/a 999 TCDL n/a MT20 10.0 244/190 Lumber DOL 1.15 BC 0.05 Vert(CT) n/a BCLL 0.0 n/a 999 Rep Stress Incr YES WB 0.02 Horz(CT) 0.00 3 BCDL 10.0 n/a n/a Code IRC2015/TPI2014 Matrix-P Weight: 19 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 5-5-0 oc purlins. BOT CHORD 2x4 SP No.2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. **OTHERS** 2x4 SP No.3

REACTIONS.

(size) 1=5-4-6, 3=5-4-6, 4=5-4-6

Max Horz 1=-48(LC 8)

Max Uplift 1=-22(LC 13), 3=-28(LC 13)

Max Grav 1=106(LC 1), 3=106(LC 1), 4=157(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Barcing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

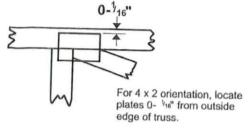


# Symbols

# PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth



This symbol indicates the required direction of slots in connector plates.

\* Plate location details available in MiTek 20/20 software or upon request.

#### PLATE SIZE

 $4 \times 4$ 

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

### LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

#### BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

#### Industry Standards:

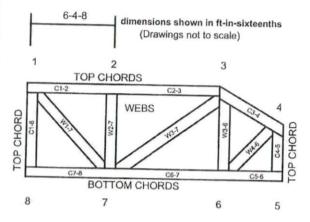
ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.

DSB-89: BCSI:

Building Component Safety Information, Guide to Good Practice for Handling. Installing & Bracing of Metal Plate

Connected Wood Trusses.

# **Numbering System**



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

#### PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015



# General Safety Notes

## Failure to Follow Could Cause Property Damage or Personal Injury

- 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- 2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- 3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- 6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- 10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- 16. Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- 18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- 19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.

20. Design assumes manufacture in accordance with

ANSI/TPI 1 Quality Criteria.