

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0121-0596 Lot 36 Forest Ridge

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15364152 thru E15364169

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



February 1,2021

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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loh	Truco		Otr	Dhy	Lot 26 Forest Bidge		
500	TIUSS		Qty	FIY	Lot 30 Polest Ridge		E15364153
J0121-0596	A02	HOWE	12	1	Jah Dafamana (antia	1)	
Comtech. Inc. Favet	teville. NC - 28314.			3.330 s O	ct 7 2020 MiTek Indust	naı) ries. Inc. Sun Jan 31 13	:27:45 2021 Page 1
····, ·, ·,			ID:PtgA9aKCfvmBbR	K6w1bfS5	yA1hk-j9QsZkMAYtVD	0JpvzWrpC2U2SGWmg	bFHcRNDCdzpq3C
	-0-10 ₁ 8 7-11-1 0-10-8 7-11-1	<u> </u>	23-0-1	5	31-0-0	0 31-10-8	
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			4x6				Scale = 1:74.1
		0.00 42	5				
		9.00 12					
				16			
		3x10 / 15	/ \\ >>		2x4 //		
		2x4 \\			3x10 ≫		
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	4x6 18	19 13 12	20	11 10	0 21	4x6	
		4x6 = 3x6 =		3x6 = 4x8	s =		
		0.00					
	10-	5-6 15-6-0 5-6 5-0-10	20-6-10		<u>31-0-0</u> 10-5-6		
Plate Offsets (X,Y) [2	2:0-0-5,0-0-7], [2:0-0-10,0-3-15	, [8:0-0-5,0-0-7], [8:0-0-10,0-3-15]					
I OADING (nsf)	SPACING- 2-0-0	CSI	DEEI ir	(loc)	l/defl l/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.78	Vert(LL) -0.44	11-13	>844 360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.84	Vert(CT) -0.66	8-11	>560 240		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.10	8-11	>999 240	Weight: 165 lb	FT = 20%
			PRACINC				
TOP CHORD 2x4 SP N	No.1		TOP CHORD	Structu	ral wood sheathing di	rectly applied.	
BOT CHORD 2x4 SP 2	2400F 2.0E *Except*		BOT CHORD	Rigid ce	eiling directly applied	or 10-0-0 oc bracing.	
8-10: 2x4 WEBS 2x4 SP M	4 SP No.1 No 2		WEBS	1 Row a	at midpt 5	-11, 5-13	
WEDGE							
Left: 2x4 SP No.3, Right	:: 2x4 SP No.3						
REACTIONS. (size)	2=0-3-8, 8=0-3-8						
Max Hor	z 2=-294(LC 8)						
Max Upi Max Gra	$\pi 2=-103(LC 10), 8=-103(LC 10)$	18)					
		-,					
TOP CHORD 2-3=-20	omp./Max. Ten All forces 25 092/408 3-5=-1990/550 5-7=-	0 (lb) or less except when shown. 1999/549 7-8=-2101/407					
BOT CHORD 2-13=-	151/1769, 11-13=0/1164, 8-11	=-151/1606					
WEBS 5-11=-2	228/1133, 7-11=-530/336, 5-13	=-229/1114, 3-13=-526/335					
NOTES-							
1) Unbalanced roof live l	oads have been considered for	this design.					
2) Wind: ASCE 7-10; Vul MWFRS (envelope) a	n=130mpn (3-second gust) Va nd C-C Exterior(2) -0-10-8 to 3	sa=103mpn; 1CDL=6.0psf; BCDL= -6-5. Interior(1) 3-6-5 to 11-1-3. Ext	b.upst; n=15tt; Cat. II; erior(2) 11-1-3 to 19-1	⊢xp C; E 0-13 Int∉	nciosea; erior(1)		
19-10-13 to 27-5-11, E	Exterior(2) 27-5-11 to 31-10-8	cone;C-C for members and forces &	MWFRS for reactions	s shown;	Lumber		111
DOL=1.60 plate grip E	DOL=1.60	hord live load nonconcurrent with a	ny other live loads			UN'LY CA	ROUL
4) * This truss has been	designed for a live load of 40.0	psf on the bottom chord in all areas	where a rectangle 3-	6-0 tall by	/ 2-0-0 wide	"RIL	- Ling
will fit between the bot	ttom chord and any other mem	bers, with BCDL = 10.0psf.			- t (t, lk)	S 2 ist Book	PN: SIT
5) Provide mechanical co 2=103 8=103	onnection (by others) of truss to	b bearing plate capable of withstand	aing 100 ib uplift at joir	it(s) exce	ept (jt=lb)		W.L.
, 2 .00.					E	CEA	1 1 E
					Ξ	0262	



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4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 2=106.



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Job	Truss	Truss Type	Qty	Ply	Lot 36 Forest Ridge		
							E15364155
J0121-0596	A04	COMMON	3	1	lah Defense of (anti-	1)	
Comtech Inc Eavette	wille NC - 28314			8 330 5 00	Job Reference (option	181) ries Inc. Sun Ian 31 13:1	27:47 2021 Page 1
Conneen, me, rayen	ville, NO - 20014,		ID:PtgA9aKCfv	mBbRX6w	lbfS5yA1hk-fYXc_QOC	4VIxFdzI4xtHHTZO449_	8N9a3lsJGVzpq3A
-	0-10 ₁ 8 7-9-7	15-6-0	23-2-9		31-0-0	32-10-8	
	0-10-8 7-9-7	7-8-9	7-8-9		7-9-7	1-10-8	
			6x6 =				Scale = 1:71.2
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	3x10	3x6 =	4x6 = 8x8	=		site t g	
		0.00	4.0 - 0.0	_			
	9-10-5	19-7-	12		31-0-0	32-10-8	
	9-10-5	9-9-	7		11-4-4	1-10-8	
Plate Offsets (X,Y) [2:	0-3-8,Edge], [2:0-0-10,0-5-12], [2:0-0-5,0-0-7], [4:0-3-0,Edge], [6:0)-3-0,Edge], [10:0-4-	0,0-5-4]			
I OADING (nsf)	SPACING- 2-0-(csi	DEEI i	n (loc)	l/defl l/d	PI ATES	GRIP
TCLL 20.0	Plate Grip DOL 1.1	5 TC 0.77	Vert(LL) -0.3	1 2-12	>758 360	MT20	244/190
TCDL 10.0	Lumber DOL 1.1	5 BC 0.98	Vert(CT) -0.5	4 2-12	>432 240		
BCLL 0.0 *	Rep Stress Incr YES	S WB 0.90	Horz(CT) -0.0	1 9	n/a n/a		FT 000/
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	VVIND(LL) 0.0	8 2-12	>999 240	Weight: 181 lb	FT = 20%
LUMBER-			BRACING-				
TOP CHORD 2x4 SP N	p.1		TOP CHORD	Structur	al wood sheathing dir	ectly applied or 6-0-0 or	c purlins.
BOT CHORD 2x6 SP N	p.1 *Except*		BOT CHORD	Rigid ce	iling directly applied	or 2-2-0 oc bracing.	
2-11: 2x4	SP No.1		WEBS	1 Row a	at midpt 5	-12	
WEBS 2X4 SP N	0.2			2 Rows	at 1/3 pts 5	-10	
Left: 2x6 SP No.1							
REACTIONS. (size)	2=0-5-8, 10=0-3-8, 9=0-4-0						
Max Horz Max Holit	2=291(LC 7)	11) 0- 55/I C 16)					
Max Grav	(2=-67)(10, 10=-117)(10)	(18) 9=82(I C 22)					
	(), (,					
FORCES. (Ib) - Max. Co	mp./Max. Ten All forces 25	0 (lb) or less except when shown.					
TOP CHORD 2-3=-67	9/156, 3-5=-583/323, 5-7=0/1	030, 7-8=-71/844					
WERS 3-12-5	40/090, 10-12=-387/248, 8-10 73/348 5-12265/1118 5-10)=-389/130)1725/258 7-10562/349					
WEB0 012-0	75/540, 5 12- 205/1110, 5 10	5 1125/200, 1 10 - 302/343					
NOTES-							
1) Unbalanced roof live lo	ads have been considered fo	r this design.					
2) Wind: ASCE 7-10; Vult	=130mph (3-second gust) Va	sd=103mph; TCDL=6.0psf; BCDL=5	.0psf; h=15ft; Cat. II;	Exp C; E	nclosed;		
19-10-13 to 26-9-9 Ev	erior(2) 26-9-9 to 31-2-5 zone	eo-o, interior(1) 3-6-5 to 11-1-3, Extension and forces & MM	(ERS for reactions of	10-13, INte	nor(1) iber		
DOL=1.60 plate arip D	DL=1.60		1 10 101 16400015 51	LUII, LUII		minin	11111
 This truss has been de 	signed for a 10.0 psf bottom of	chord live load nonconcurrent with ar	y other live loads.			TH CA	Roille
4) * This truss has been d	esigned for a live load of 40.0	psf on the bottom chord in all areas	where a rectangle 3	-6-0 tall by	2-0-0 wide	NOR	2. 11/1
will fit between the bott	om chord and any other mem	bers, with BCDL = 10.0psf.				FESO	N. Sin

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 9 except (jt=lb) 10=117.



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Edenton, NC 27932





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Job	Truss	Truss Type	Qty	Ply	Lot 36 Forest Ridge	E45004450
J0121-0596	A07	COMMON	1	1		E15364158
Comtech Inc Favette			8	330 s Oc	Job Reference (optiona t 7 2020 MiTek Industrie	II) es Inc. Sun Ian 31 13:27:49 2021 Page 1
		15.0.0	ID:PtgA9aKCfvr	nBbRX6w	1bfS5yA1hk-cxfNP6Pgc	60fUx7gCLwlMuelxuxCcL_tX3LQLOzpq38
	-0 <u>-10-8 7-11-1</u> 0-10-8 7-11-1	<u> </u>		5 5		-0
			1ve			Scale = 1:70.1
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1		17 11 18	10 19	0	20 21	
	4x6 = 10	3x6 =	4x8 =	3x6 =	20 21	3x6 =
	10-5	6 20	-6-10		31-0-0	
	10-5	6 1	0-1-4	-	10-5-6	
Plate Offsets (X,Y) [2:	0-0-0,0-0-3], [8:0-6-0,0-0-5]					
LOADING (psf)	SPACING- 2-0-	CSI.	DEFL. in	(loc)	I/defl L/d	PLATES GRIP
TCDL 10.0	Lumber DOL 1.1	5 BC 0.62	Vert(CT) -0.18	9-11 2-11	>999 240	MT20 244/190
BCLL 0.0 *	Rep Stress Incr YE	S WB 0.69 Matrix-S	Horz(CT) 0.04 Wind(LL) 0.06	8 2-11	n/a n/a	Weight: 188 lb FT - 20%
BODE 10.0		Matrix-5	Wind(LL) 0.00	2-11	2333 240	
LUMBER- TOP CHORD 2x4 SP N	o.1		BRACING- TOP CHORD	Structura	al wood sheathing dire	ctly applied.
BOT CHORD 2x6 SP N	p.1		BOT CHORD	Rigid cei	iling directly applied or	10-0-0 oc bracing.
WEBS 2x4 SP N	0.2					
REACTIONS. (size)	2=0-5-8, 8=0-5-8					
Max Horz Max Uplif	t 2=291(LC 7) t 2=-104(LC 10), 8=-88(LC 1	1)				
Max Grav	2=1617(LC 17), 8=1554(LC	18)				
FORCES. (lb) - Max. Co	mp./Max. Ten All forces 25	0 (lb) or less except when shown.				
TOP CHORD 2-3=-20	98/411, 3-5=-1966/548, 5-7=	1970/556, 7-8=-2099/414				
WEBS 5-9=-23	6/1105, 7-9=-514/343, 5-11=	224/1100, 3-11=-502/324				
NOTES-						
1) Unbalanced roof live lo	ads have been considered fo	r this design.				
 Wind: ASCE 7-10; Vult MWFRS (envelope) an 	=130mph (3-second gust) Va d C-C Exterior(2) -0-10-8 to 3	sd=103mph; TCDL=6.0psf; BCDL=5.0p 3-6-5. Interior(1) 3-6-5 to 11-1-3. Exterio	osf; h=15ft; Cat. II; E or(2) 11-1-3 to 19-10	Exp C; En 0-13, Inte	iclosed; rior(1)	
19-10-13 to 26-4-7, Ex	erior(2) 26-4-7 to 30-9-4 zon	e;C-C for members and forces & MWFF	RS for reactions sho	own; Lum	ber	
3) This truss has been de	signed for a 10.0 psf bottom	chord live load nonconcurrent with any	other live loads.			

4) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 2=104.



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REACTIONS. (size) 12=0-5-8, 8=0-5-8 Max Horz 12=261(LC 9) Max Uplift 12=-54(LC 10), 8=-54(LC 11) Max Grav 12=878(LC 17), 8=878(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-957/256, 3-4=-912/448, 4-5=-912/448, 5-6=-957/256, 2-12=-838/289, 6-8=-837/289

BOT CHORD 11-12=-248/364. 9-11=-7/501

WEBS 4-9=-242/543, 5-9=-404/305, 4-11=-242/543, 3-11=-404/305, 2-11=0/551, 6-9=0/559

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 5-7-3, Exterior(2) 5-7-3 to 14-4-13, Interior(1) 14-4-13 to 16-5-11, Exterior(2) 16-5-11 to 20-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 8.



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- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 5-7-3, Corner(3) 5-7-3 to 14-4-13, Exterior(2) 14-4-13 to 16-5-11, Corner(3) 16-5-11 to 20-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 191 lb uplift at joint 24, 156 lb uplift at joint 14, 124 lb uplift at joint 20, 161 lb uplift at joint 21, 107 lb uplift at joint 22, 287 lb uplift at joint 23, 123 lb uplift at joint 18, 162 lb uplift at joint 17, 111 lb uplift at joint 16 and 276 lb uplift at joint 15.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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Plate Offsets (X,Y	[2:0-1-12,0-1-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-C Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.26 BC 0.52 WB 0.54 Matrix-S	DEFL. in Vert(LL) -0.17 Vert(CT) -0.20 Horz(CT) 0.01 Wind(LL) 0.02	(loc) l/defl L/d 8-10 >999 360 8-10 >999 240 7 n/a n/a 8-10 >999 240	PLATES GRIP MT20 244/190 Weight: 144 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WEBS 2x4 SP No.2			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	ectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing.
REACTIONS.	(size) 11=0-5-8, 7=0-5-8 ax Horz 11=252(LC 7)				

Max Horz 11=252(LC 7) Max Uplift 11=-52(LC 10), 7=-53(LC 10) Max Grav 11=858(LC 17), 7=825(LC 17)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-933/255, 3-4=-888/447, 4-5=-860/447, 5-6=-890/247, 2-11=-818/288,

6-7=-793/230

BOT CHORD 10-11=-262/343, 8-10=-23/467

WEBS 4-8=-242/485, 5-8=-398/322, 4-10=-242/546, 3-10=-404/306, 2-10=0/535, 6-8=-50/580

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 5-7-3, Exterior(2) 5-7-3 to 14-4-13, Interior(1) 14-4-13 to 14-10-13, Exterior(2) 14-10-13 to 19-4-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 11 and 53 lb uplift at joint 7.



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Plate Offsets (X,Y)	[10:0-5-10,0-1-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15	CSI. TC 0.19 BC 0.14	DEFL. ir Vert(LL) -0.00 Vert(CT) -0.00	n (loc) l/defl L/d) 1 n/r 120) 1 n/r 120	PLATES GRIP MT20 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.06 Matrix-R	Horz(CT) -0.01	6 n/a n/a	Weight: 42 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP OTHERS 2x4 SP	No.1 No.1 No.2 No.2		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	ectly applied or 5-9-8 oc purlins, or 10-0-0 oc bracing.

REACTIONS. All bearings 5-9-8.

(lb) - Max Horz 10=305(LC 10)

Max Uplift All uplift 100 b or less at joint(s) 10, 6, 7 except 8=-102(LC 10), 9=-292(LC 10) Max Grav All reactions 250 lb or less at joint(s) 6, 7, 8, 9 except 10=308(LC 10)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-388/326 WEBS 3-9=-258/227

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-9-8, Exterior(2) 3-9-8 to 5-9-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 6, 7 except (jt=lb) 8=102, 9=292.



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ENGINEERING BY

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L		6-0-0)				
I		6-0-0)			Ι	
Plate Offsets (X,Y)	[1:0-1-15,0-1-8], [2:0-0-4,Edge]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.45 BC 0.33 WB 0.03 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.06 2-4 -0.11 2-4 0.00 2 0.13 2-4	l/defl L/d >999 360 >608 240 n/a n/a >548 240	PLATES G MT20 2 Weight: 21 lb	;RIP 44/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 4=-58(LC 7) Max Uplift 2=-127(LC 7), 4=-97(LC 7) Max Grav 2=294(LC 1), 4=223(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWERS (anyelone) and C-C Exterior(2) zone: porch right exposed C-C for members and forces & MWERS for reactions shown:
- MWFRS (envelope) and C-C Exterior(2) zone; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=127.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
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	L		5-0-0							
			5-0-0						1	
Plate Offsets (X,Y	[1:0-1-15,0-1-8], [2:0-0-	4,Edge]								
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.29	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.22	Vert(CT)	-0.05	2-4	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.02	Horz(CT)	0.00	2	n/a	n/a		
BCDL 10.0	Code IRC2015/1	FPI2014	Matrix-P	Wind(LL)	0.06	2-4	>970	240	Weight: 18 lb	FT = 20%
LUMBER-	I			BRACING					-1	

TOP CHORD

BOT CHORD

LUMBER-TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WEBS 2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 4=-49(LC 7) Max Uplift 2=-113(LC 7), 4=-79(LC 7) Max Grav 2=256(LC 1), 4=182(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MW/ERS (available) and C C Exterior (3) zone; parts right expression of the process of the proc
- MWFRS (envelope) and C-C Exterior(2) zone; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=113.



Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.21 BC 0.13 WB 0.00 Matrix-P	DEFL. ir Vert(LL) -0.01 Vert(CT) -0.00 Horz(CT) 0.00	n (loc) l/defi L/d 3 n/r 120 3 n/r 120 2 n/a n/a	PLATES C MT20 2 Weight: 15 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S	P No.1 P No.1 P No.2		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied o	rectly applied or 4-0-0 or or 10-0-0 oc bracing.	c purlins,

REACTIONS. (size) 4=4-0-0, 2=4-0-0 Max Horz 4=-58(LC 7)

Max Uplift 4=-51(LC 11), 2=-91(LC 7) Max Grav 4=148(LC 1), 2=213(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.



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3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=197, 4=197.





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MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=144, 4=144.



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REACTIONS. All bearings 12-11-4.

- (lb) Max Horz 1=349(LC 10)
 - Max Uplift All uplift 100 lb or less at joint(s) 9, 11 except 1=-167(LC 8), 12=-147(LC 10), 13=-144(LC 10), 14=-146(LC 10), 15=-128(LC 10), 10=-118(LC 11)
 - Max Grav All reactions 250 lb or less at joint(s) 9, 11, 12, 13, 14, 15, 10 except 1=321(LC 10)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-468/314, 2-3=-357/239

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 5-1-15, Exterior(2) 5-1-15 to 12-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 11 except (jt=lb) 1=167, 12=147, 13=144, 14=146, 15=128, 10=118.
- 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



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