

Trenco RE: 807186 Master - H&H-NC/Redbud/ 818 Soundside Rd Site Information: Edenton, NC 27932 Project Customer: H and H Project Name: 807186 Lot/Block: C Subdivision: ALL Model: Address: City: Favetteville State: NC General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions): Design Code: IRC2009/TPI2007 Design Program: MiTek 20/20 7.6 Wind Code: ASCE 7-05 Wind Speed: 130 mph Design Method: MWFRS(low-rise)/C-C hybrid Wind ASCE 7-05 Roof Load: 40.0 psf Floor Load: N/A psf Exposure Category: C Mean Roof Height (feet): 25 No. Seal# **Truss Name Date** No. Seal# Truss Name Date 129310936 3/20/17 35 129310970 A01 C01 3/20/17 123456789111111111122222222222333333 29310937 29310938 36 37 38 39 40 29310971 A02 A03 3/20/17 C02 CP01 3/20/17 3/20/17 3/20/17 3/20/17 A04 A05 A05A I29310939 I29310940 CP02 CP03 129310973 3/20/17 129310974 129310940 129310941 129310942 129310943 129310944 129310945 129310975 D01 40 41 42 43 44 129310976 129310976 129310977 A06 A07 /20/17 /20/17 D02 F01 A08 A09 129310978 129310979 3/20/17 3/20/17 129310946 45 129310980 G02 A10 /20/17 46 47 129310947 129310981 H01 A11 3/20/17 129310982 129310983 129310948 A12 101 129310949 A13 48 İŎŻ I29310949 I29310950 I29310951 I29310952 I29310953 A14 A15 129310984 129310985 490123456789 3/20/17 3/20/17 J01 J02 3/20/17 3/20/17 129310986 129310987 A16 A17 3/20/17 129310954 A17A 129310988 Ĵ05 20/17 J06 J07 129310955 A18 129310989 129310956 129310957 129310990 A18A 129310991 J08 A19 3/20/17 129310958 129310959 129310960 A20 A21 A22 129310992 129310993 129310994 J09 3/20/17 3/20/17 J10 3/20/17 ÕÕ 129310961 3/20/17 129310995 61 62 63 129310996 129310997 129310962 J13 /20/17 129310963 J14 129310964 129310965 129310998 J15 64 65 66 67 129310999 J16 20/17 A28 A29 B01 129310966 129311000 J17 20/17

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Sumter,SC.

Truss Design Engineer's Name: Komnick, Chad

0967 Ŏ<u>9</u>68

129310969

B02

My license renewal date for the state of North Carolina is December 31, 2017

17

68

3/20/17

3/20/17

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Komnick, Chad

20/17

March 20,2017

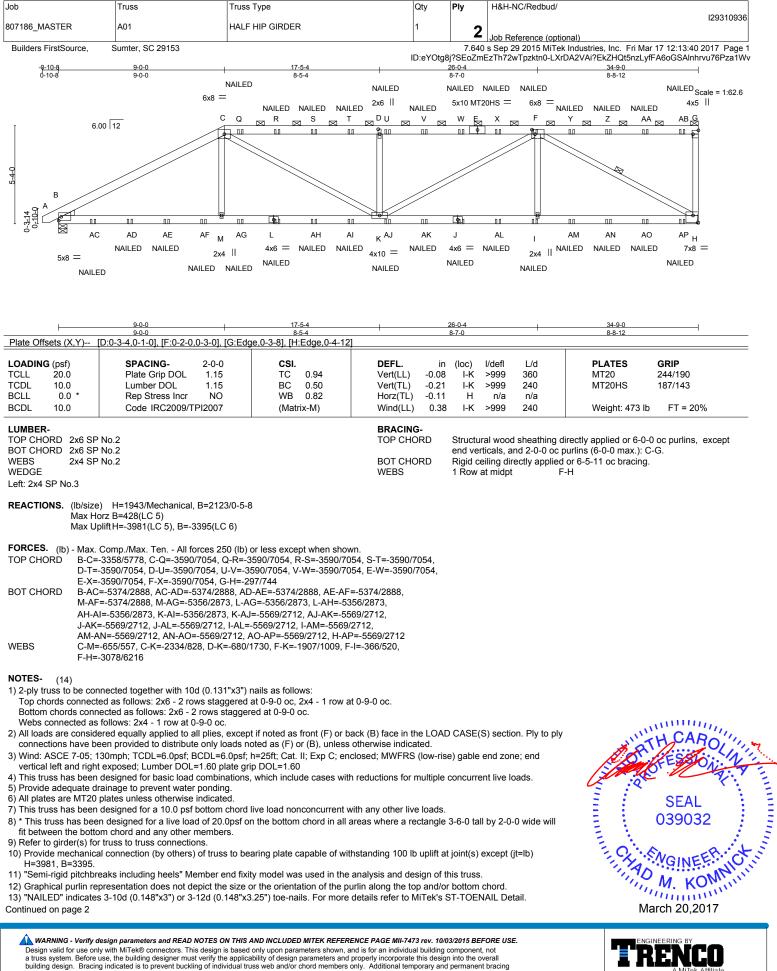
J18 J19

1002

129311003

1 of 2

No.	Seal#	Job ID#	Truss Nan	ne Date
69 70	l29311004 l29311005	807186_Ma 807186_Ma		3/20/17 3/20/17
71	129311006	807186_Ma		3/20/17
72 73	l29311007 l29311008	807186 Ma 807186 Ma	istele istele	3/20/17 3/20/17
74 75	29311009	807186 ⁻ Ma	stel26	3/20/17
75 76	129311010 129311011	807186 Ma 807186 Ma		3/20/17 3/20/17
77	129311012	807186_Ma		3/20/17
78 79	l29311013 l29311014	807186 Ma 807186 Ma		3/20/17 3/20/17
80	129311015	807186 Ma	stel82	3/20/17
81 82	l29311016 l29311017	807186 Ma 807186 Ma	isteb3 isteb4	3/20/17 3/20/17
83 84	129311018	807186 Ma		3/20/17
04 85	l29311019 l29311020	807186 Ma 807186 Ma		3/20/17 3/20/17
86	129311021	807186 ⁻ Ma	steb8	3/20/17
87 88	l29311022 l29311023	807186 Ma 80 <u>7</u> 186 Ma		3/20/17 3/20/17
88 89 90	129311024 129311025	807186_Ma 807186_Ma	iste01	3/20/17
90 91	129311025	807186 Ma		3/20/17 3/20/17
92	129311027	807186 Ma		3/20/17 3/20/17
93 94	l29311028 l29311029	807186 Ma 807186 Ma		3/20/17 3/20/17
95	129311030	807186 <u></u> Ma		3/20/17



besign value to be only with with these contractions. This besign is based only upon parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	H&H-NC/Redbud/
807186 MASTER	A01	HALF HIP GIRDER	1		129310936
-				Z	Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153			7.640	s Sep 29 2015 MiTek Industries, Inc. Fri Mar 17 12:13:41 2017 Page 2

7.640 s Sep 29 2015 MiTek Industries, Inc. Fri Mar 17 12:13:41 2017 Page 2 ID:eYOtg8j?SEoZmEzTh72wTpzktn0-pjPbNOWoTJMbAR?3eVUaUtoLsCchvC1q3Zdhesza1Wu

14) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

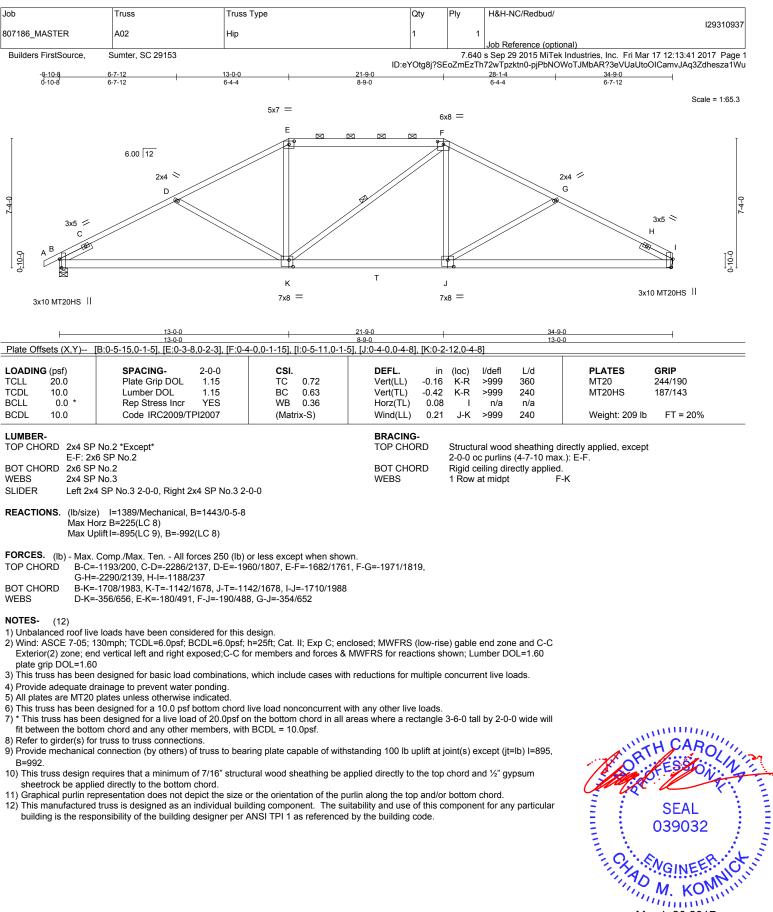
LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: A-C=-60, C-G=-60, H-N=-20

Concentrated Loads (lb)

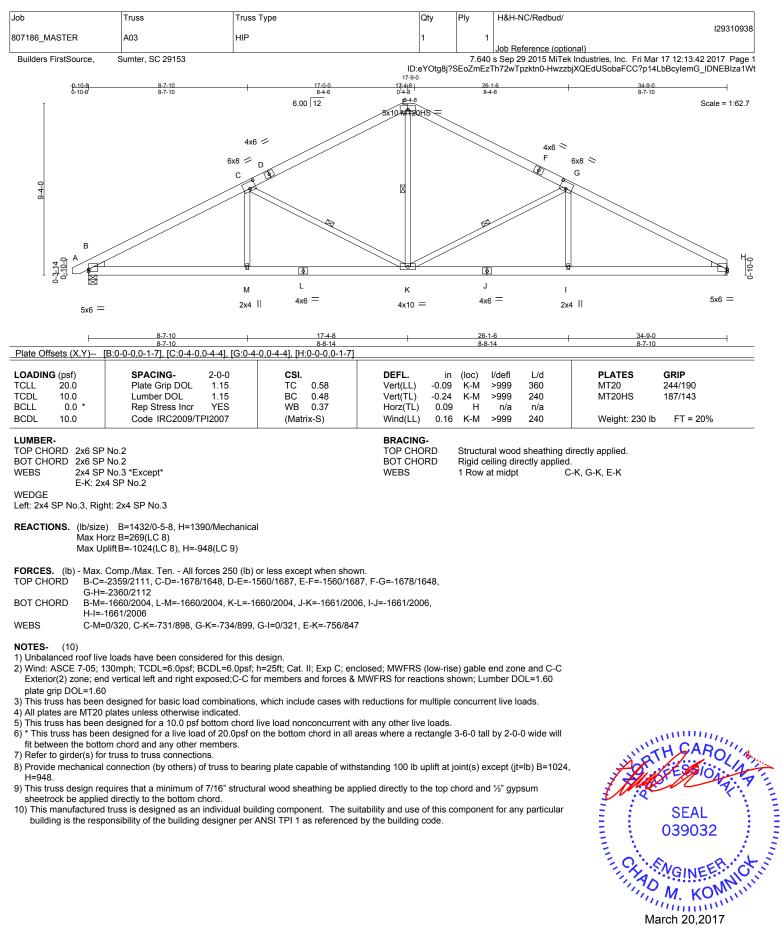
Vert: L=-23(F) I=-23(F) F=-35(F) J=-23(F) Q=-35(F) R=-35(F) S=-35(F) T=-35(F) U=-35(F) V=-35(F) W=-35(F) X=-35(F) Y=-35(F) Z=-35(F) AA=-35(F) AB=-43(F) AC=-125(F) AD=-120(F) AE=-120(F) AF=-122(F) AG=-23(F) AH=-23(F) AI=-23(F)

March 20,2017

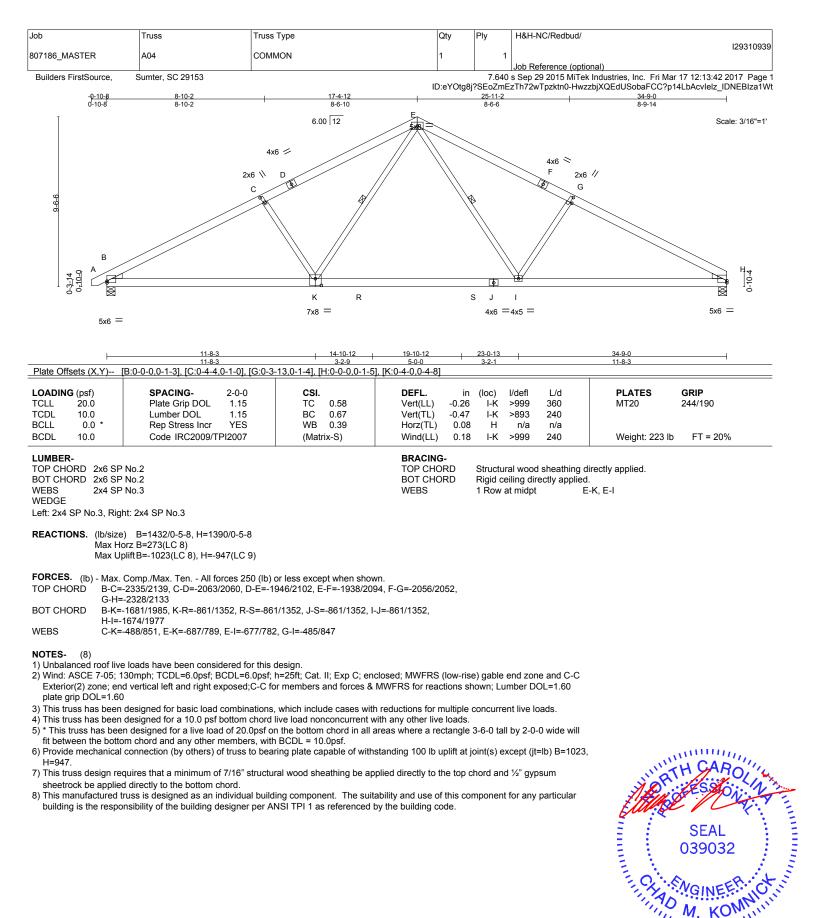
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. This design is based only upon parameters and properly incorporate this design into the overall
 building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoISITPTI Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

TREENCO AMITER Affiliate 818 Soundside Road Edenton, NC 27932



ENGINEERING BY

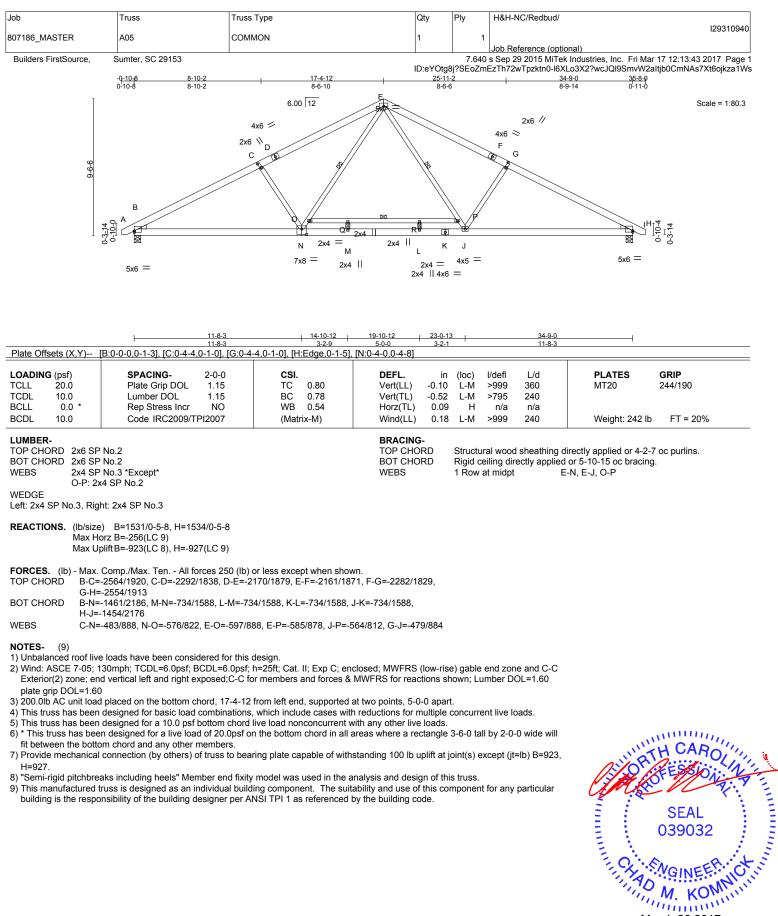
818 Soundside Roa Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. ENGINEERING BY A MITEK Attiliate

Edenton, NC 27932

March 20,2017

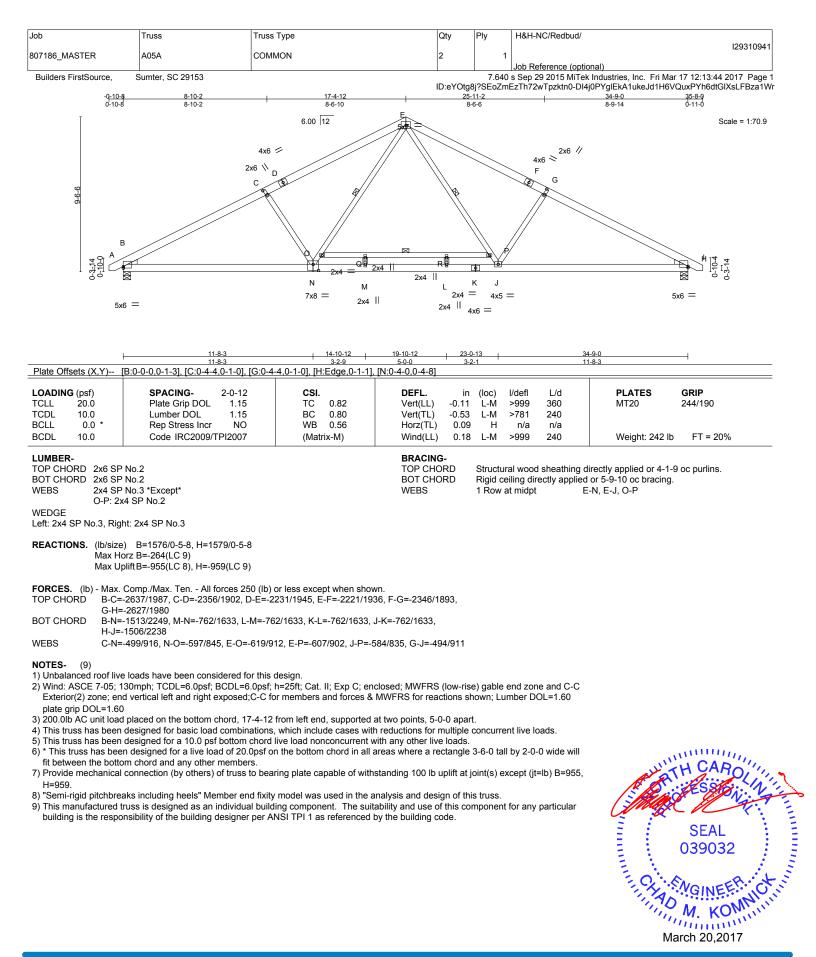


March 20,2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

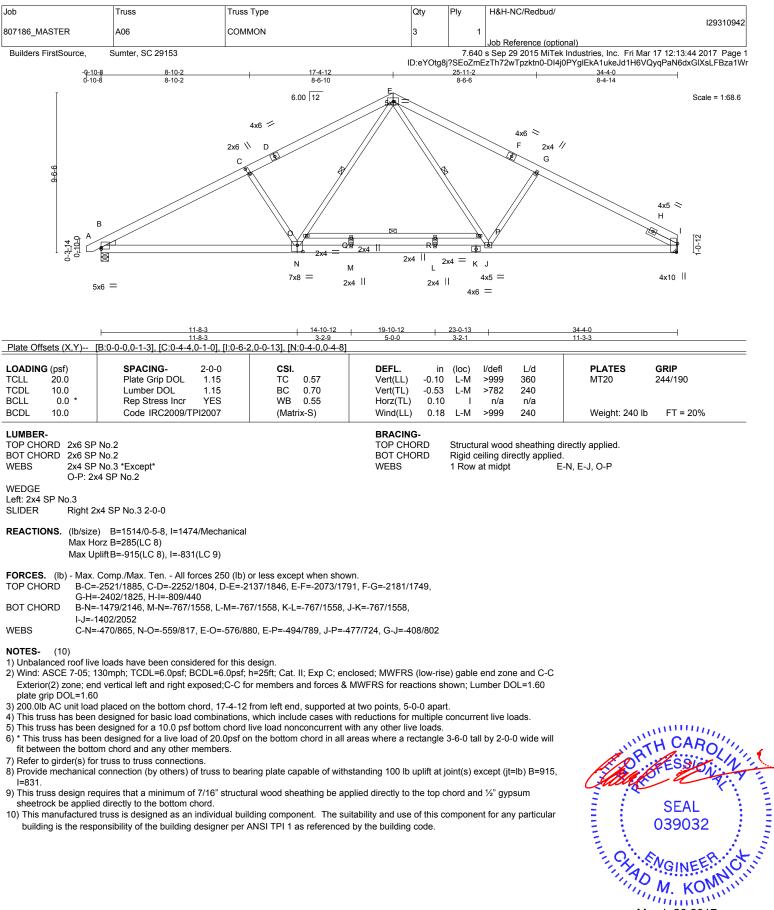


818 Soundside Road Edenton, NC 27932



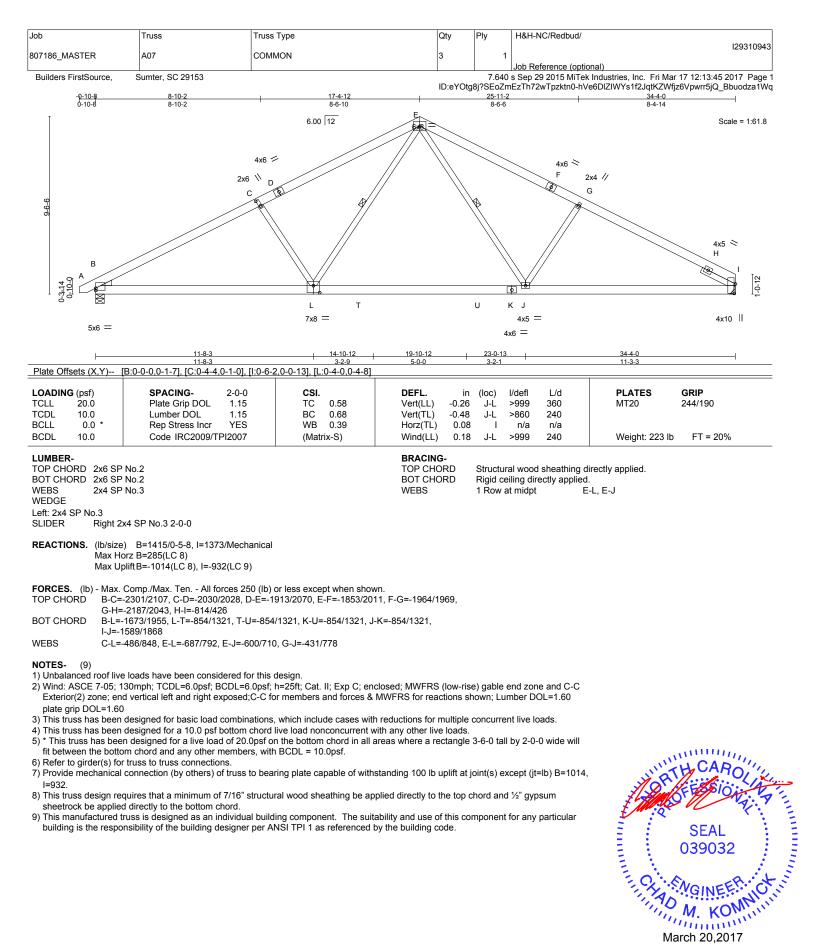
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. This design is based only upon parameters and properly incorporate this design into the overall
 building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoISITPTI Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

ENGINEERING BY A MITEK Affiliate 818 Soundside Road Edenton, NC 27932



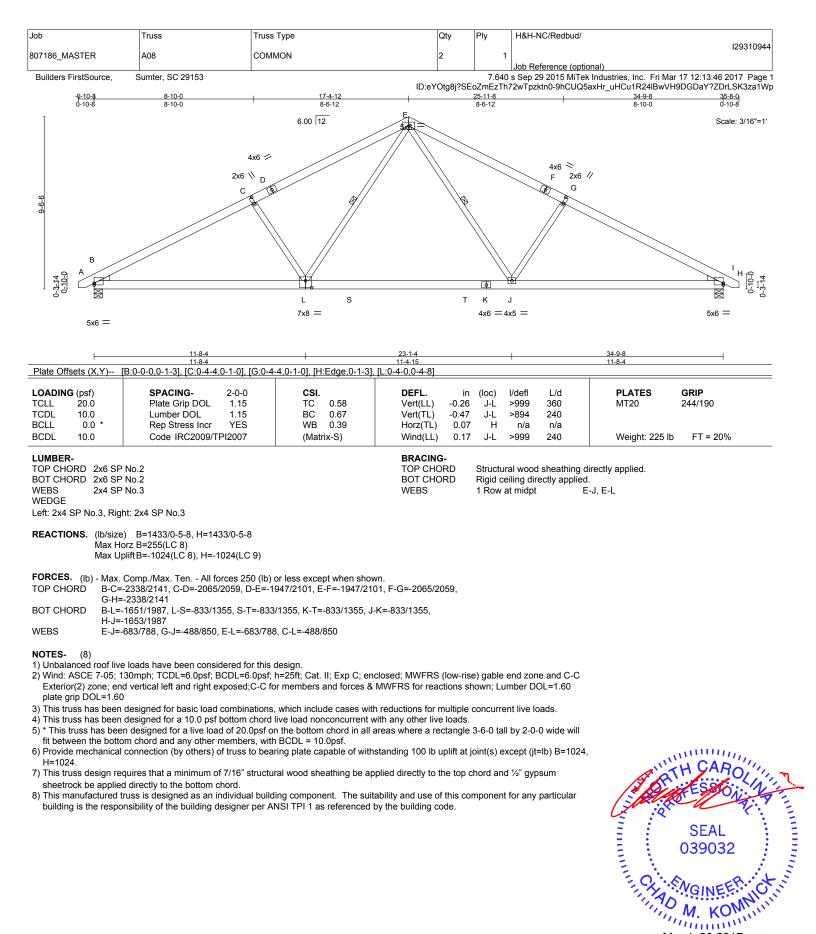
March 20,2017

ENGINEERING BY A MiTek Atfiliate 818 Soundside Road Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. TRENGINEERING BY A MiTek Affiliate

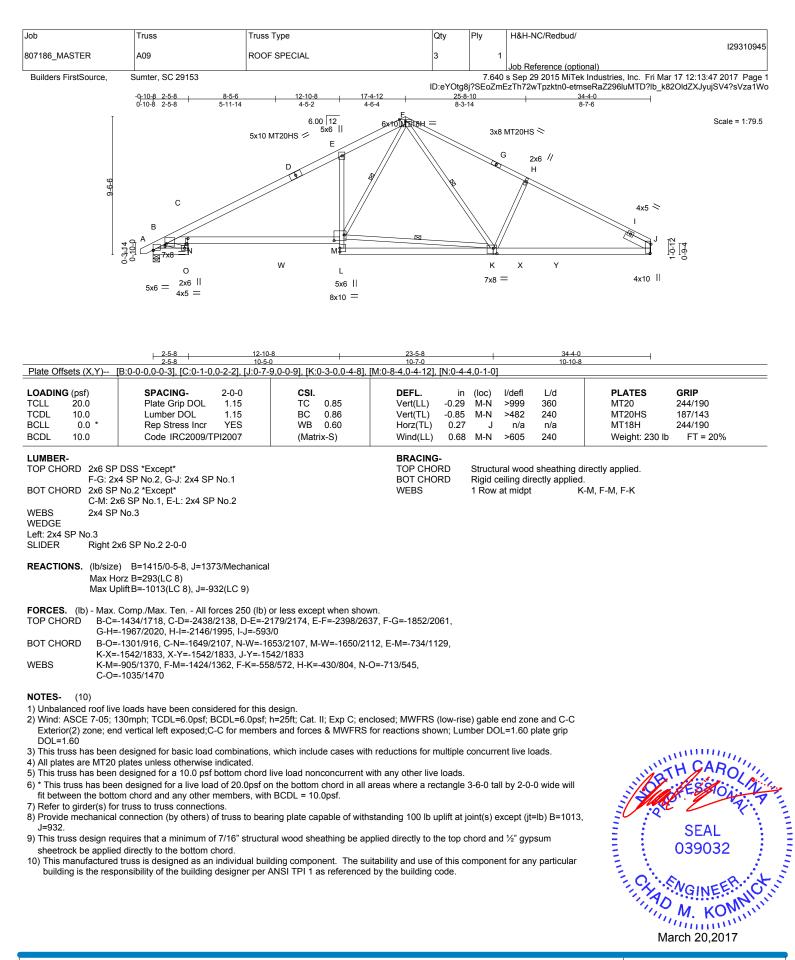
818 Soundside Road Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. ENGINEERING BY

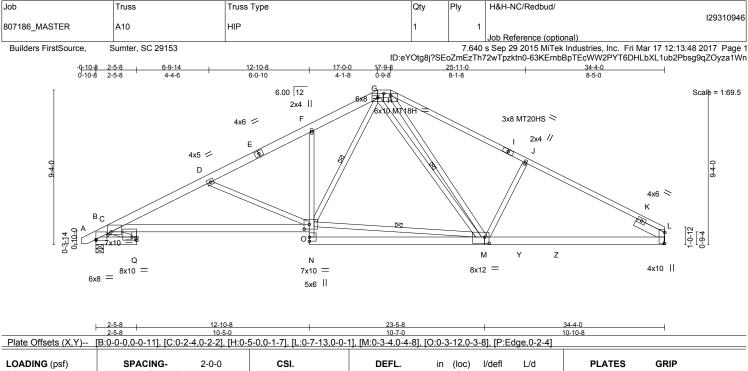
March 20,2017







Edenton, NC 27932



LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.99	Vert(LL)	-0.25	O-P	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.93	Vert(TL)	-0.78	O-P	>527	240	MT20HS	187/143
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.25	L	n/a	n/a	MT18H	244/190
BCDL	10.0	Code IRC2009/T	PI2007	(Matr	ix-S)	Wind(LL)	0.48	0-P	>861	240	Weight: 253 lb	FT = 20%

LUMBER-		BRACING-		
TOP CHORD	2x6 SP No.2 *Except*	TOP CHORD	Structural wood sheathing	directly applied, except
	H-I,I-L: 2x4 SP No.2		2-0-0 oc purlins (5-9-6 ma	ix.): G-H.
BOT CHORD	2x6 SP No.2 *Except*	BOT CHORD	Rigid ceiling directly appli	ed.
	F-N: 2x4 SP No.2	WEBS	1 Row at midpt	M-O, G-O, G-M
	2x4 SP No 3			

REACTIONS. (Ib/size) B=1415/0-5-8, L=1373/Mechanical Max Horz B=286(LC 8) Max UpliftB=-1009(LC 8), L=-928(LC 9)

Right 2x6 SP No.2 2-0-0

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 B-C=-1168/1293, C-D=-2790/2611, D-E=-2198/2027, E-F=-2094/2057, F-G=-2118/2296, G-H=-1411/1790, H-I=-1846/2022, I-J=-1954/1983, J-K=-2145/1991, K-L=-590/0

 BOT CHORD
 B-Q=-824/615, C-P=-2225/2506, O-P=-2225/2506, F-O=-283/535, M-Y=-1541/1832, Y-Z=-1541/1832, L-Z=-1541/1832

WEBS M-0=-759/1227, G-0=-1036/1034, G-M=-388/281, H-M=-296/510, P-Q=-467/394, C-Q=-695/931, J-M=-409/766, D-O=-699/944

NOTES- (12)

SLIDER

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

Provide adequate drainage to prevent water ponding.

5) All plates are MT20 plates unless otherwise indicated.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7)* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=1009, L=928.

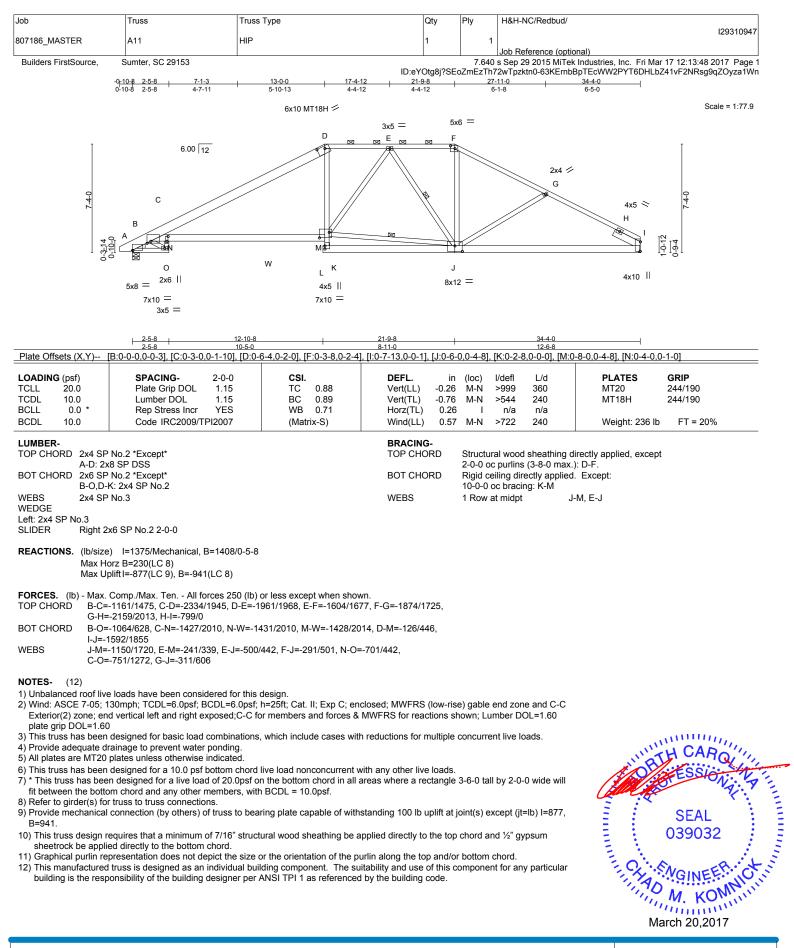
10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

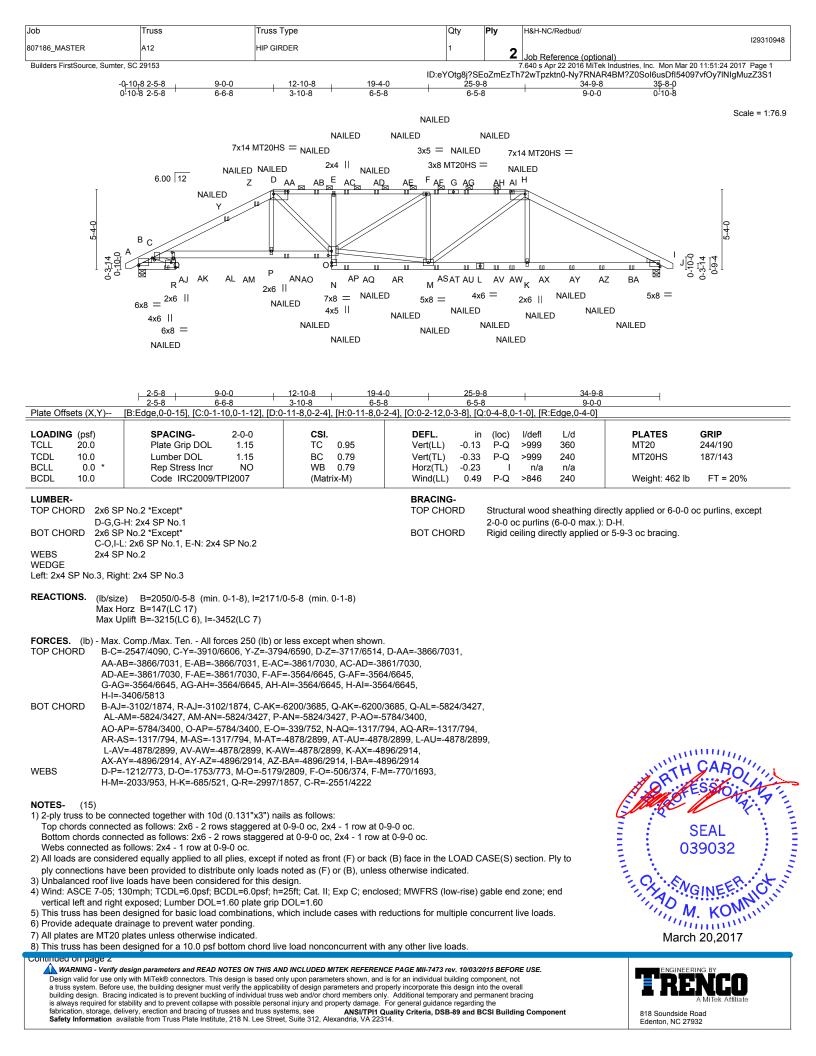
12) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.











Job	Truss	Truss Type	Qty	Ply	H&H-NC/Redbud/
807186_MASTER	A12	HIP GIRDER	1	2	I29310948 Job Reference (optional)
Builders FirstSource, Sumter, SC	29153				.640 s Apr 22 2016 MiTek Industries, Inc. Mon Mar 20 11:51:24 2017 Page 2

ID:eYOtg8j?SEoZmEzTh72wTpzktn0-Ny7RNAR4BM?Z0Sol6usDfl54097vfOy7INIgMuzZ3S1

NOTES- (15)

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

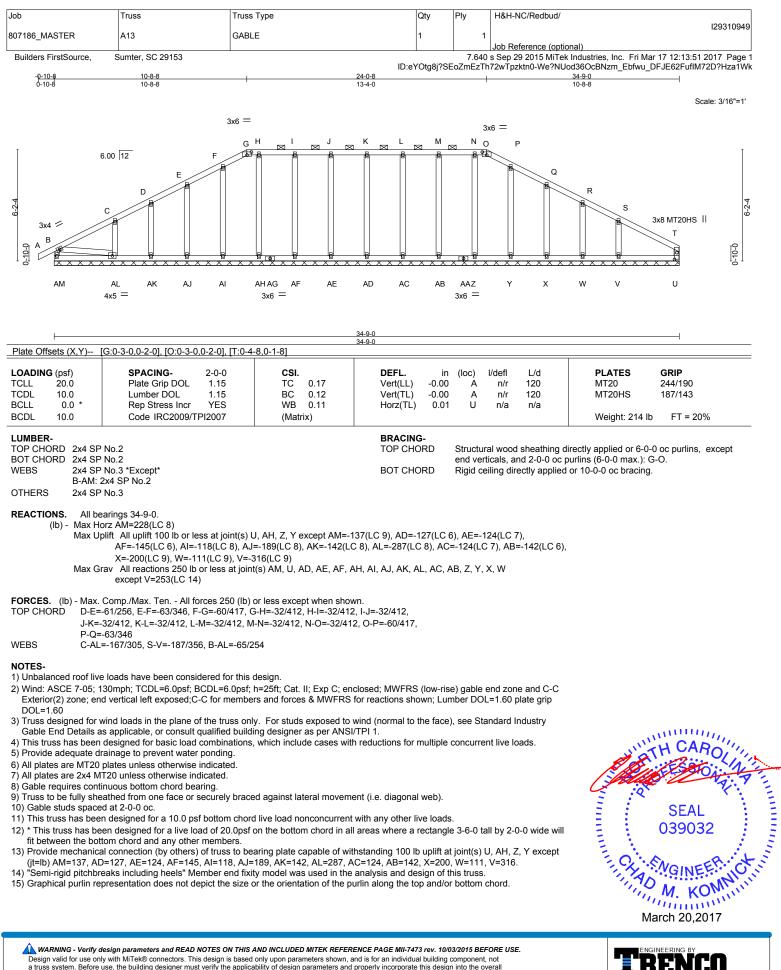
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3215 lb uplift at joint B and 3452 lb uplift at joint I.
- 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails. For more details refer to MiTek's ST-TOENAIL Detail.
- 14) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 44 lb down and 100 lb up at 3-10-8, and 41 lb down and 111 lb up at 5-10-8, and 14 lb down and 65 lb up at 7-10-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 15) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

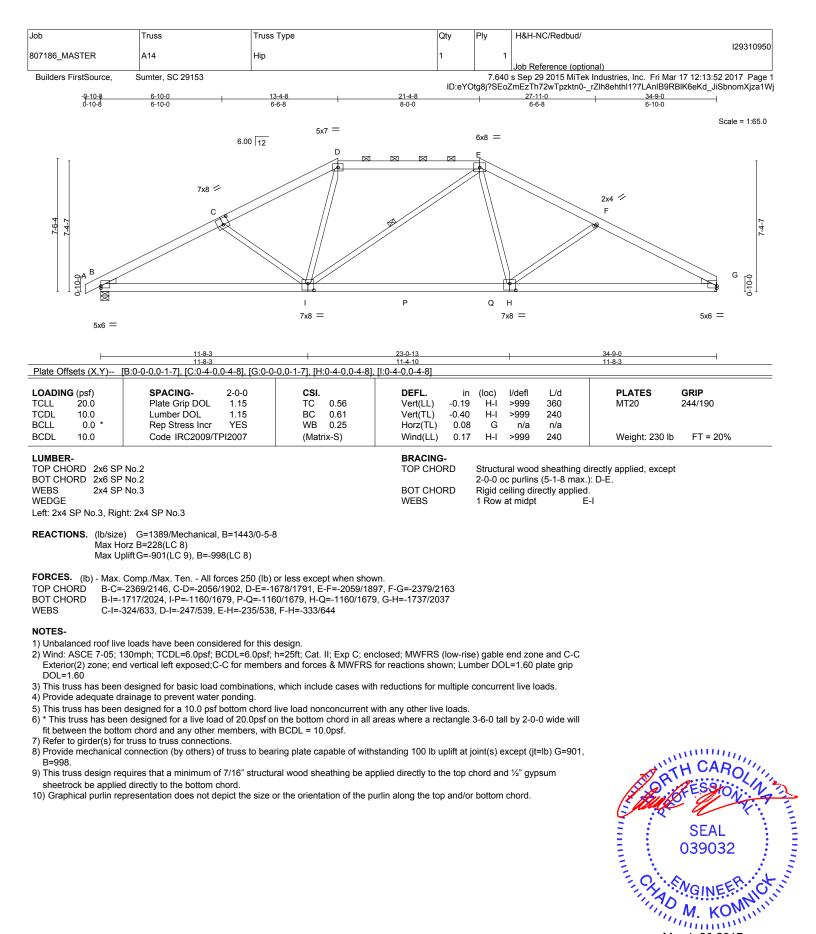
- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
 - Vert: A-D=-60, D-H=-60, H-J=-60, R-S=-20, O-Q=-20, N-V=-20

Concentrated Loads (lb)

Vert: Y=-22(B) Z=-49(B) AC=-35(B) AD=-35(B) AE=-35(B) AF=-35(B) AG=-35(B) AH=-35(B) AI=-35(B) AJ=-125(B) AL=-44 AM=-41 AN=-14 AO=-82(B) AQ=-23(B) AR=-23(B) AR=-23(B) AU=-23(B) AV=-23(B)

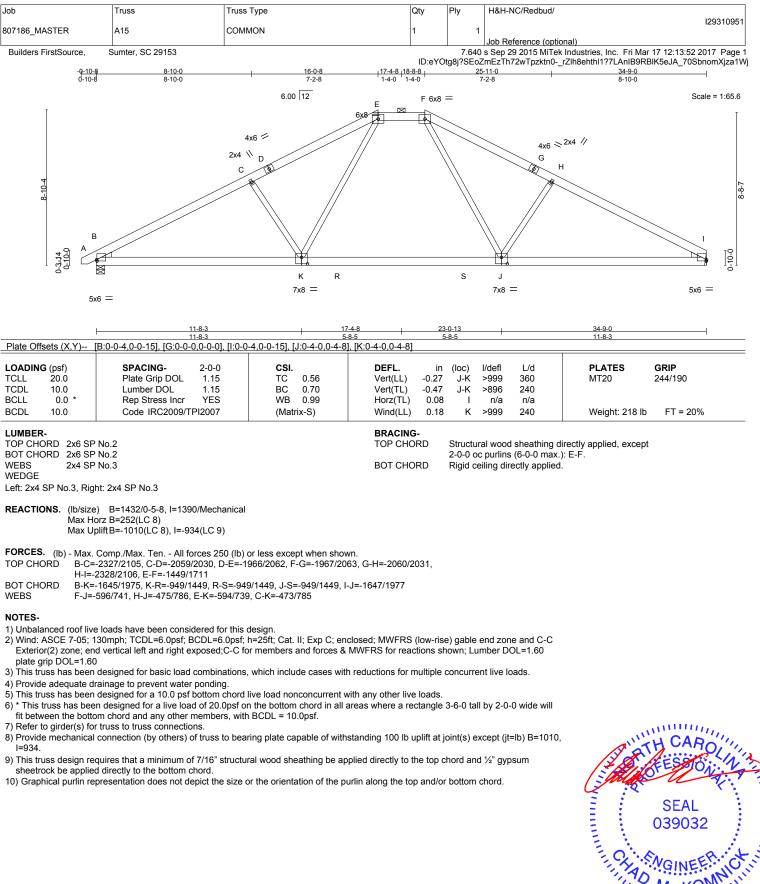
Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. TRENCO

March 20,2017

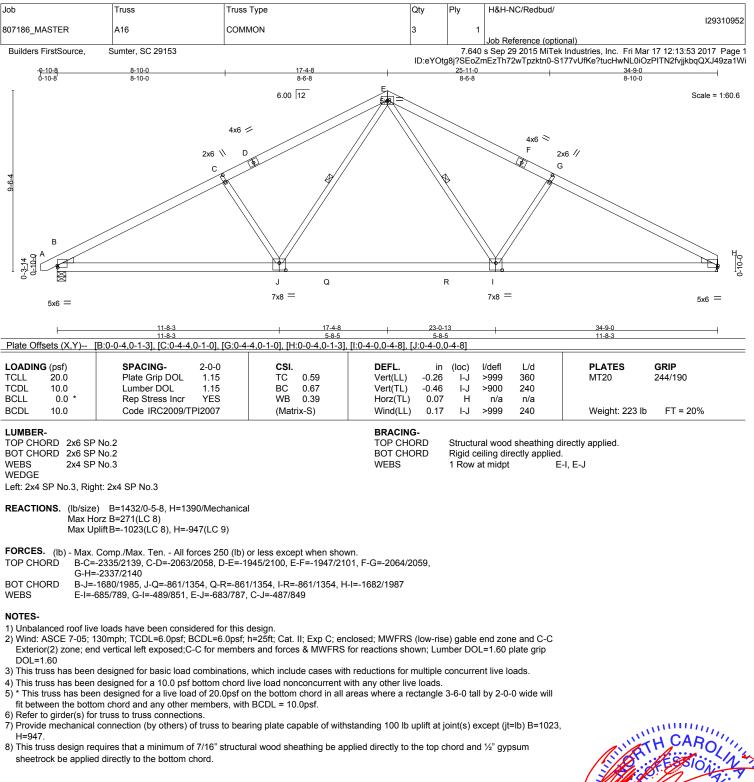
818 Soundside Road Edenton, NC 27932





818 Soundside Road Edenton, NC 27932

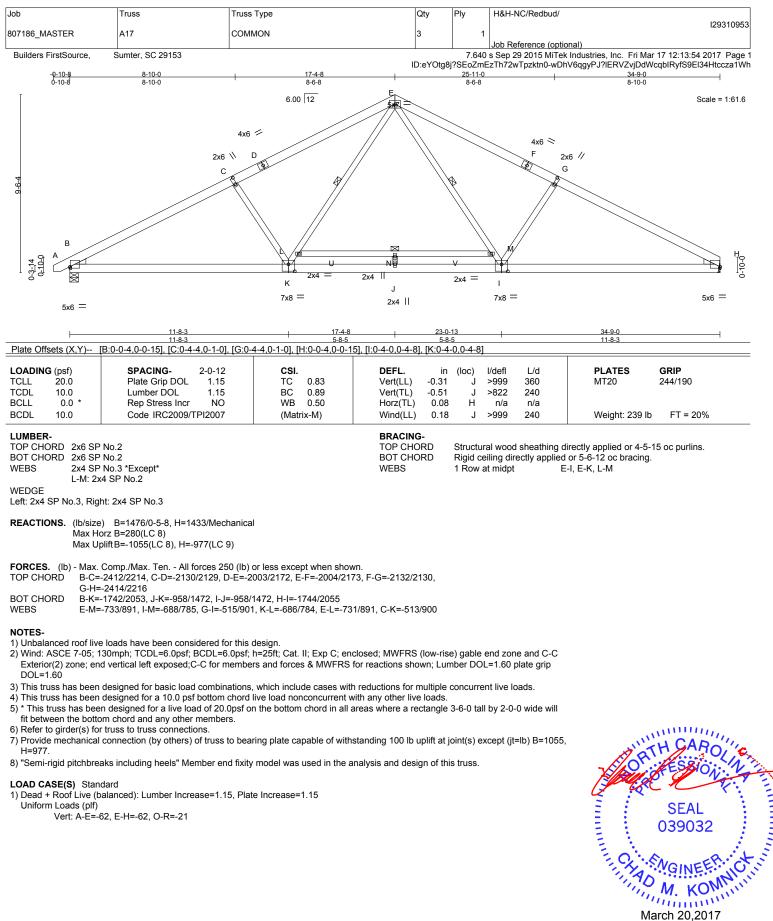
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



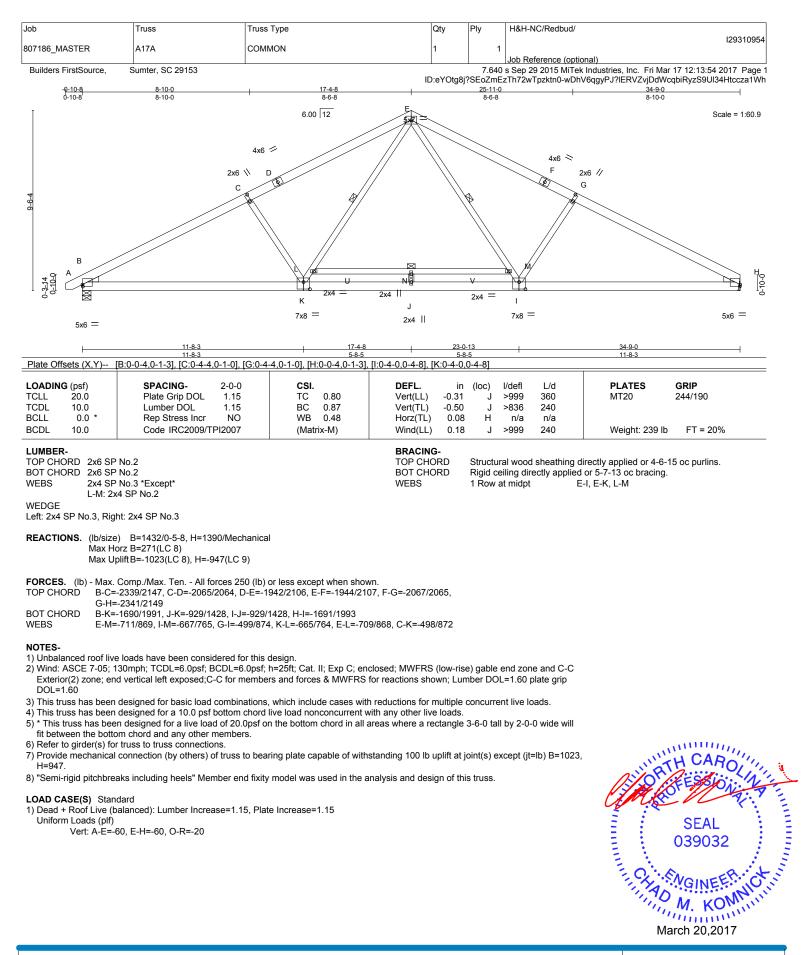


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. This design is based only upon parameters and properly incorporate this design into the overall
 building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoISITPTI Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

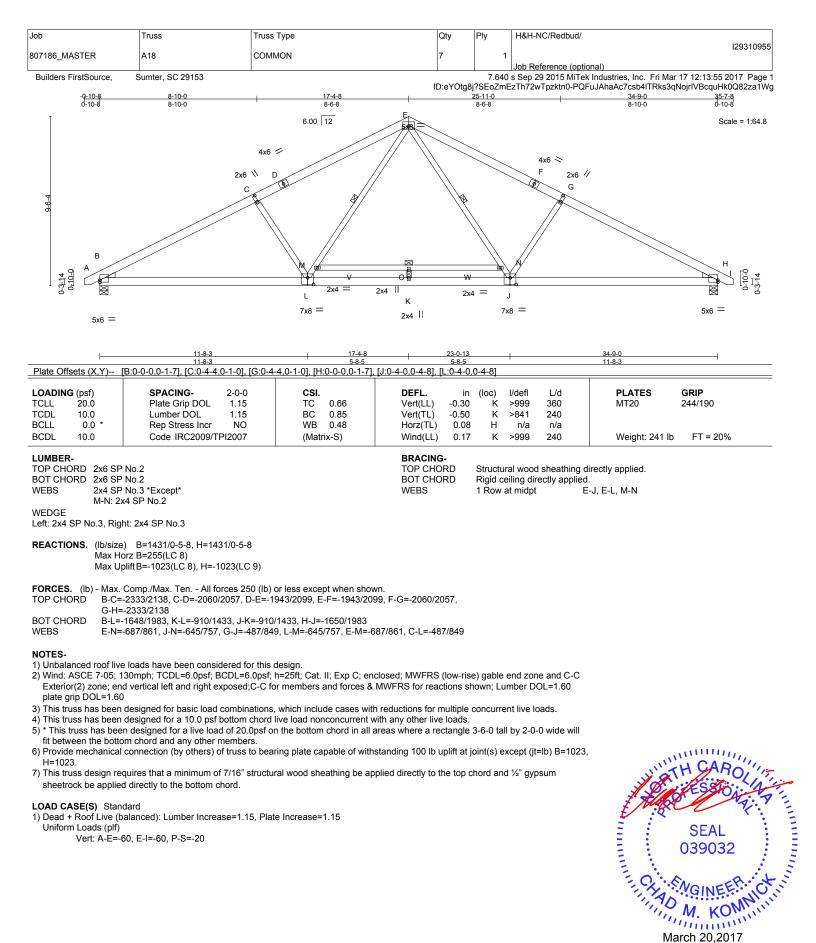
A MiTek A 818 Soundside Road Edenton, NC 27932

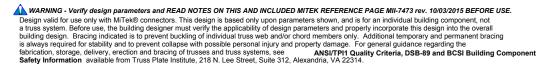


818 Soundside Road Edenton, NC 27932



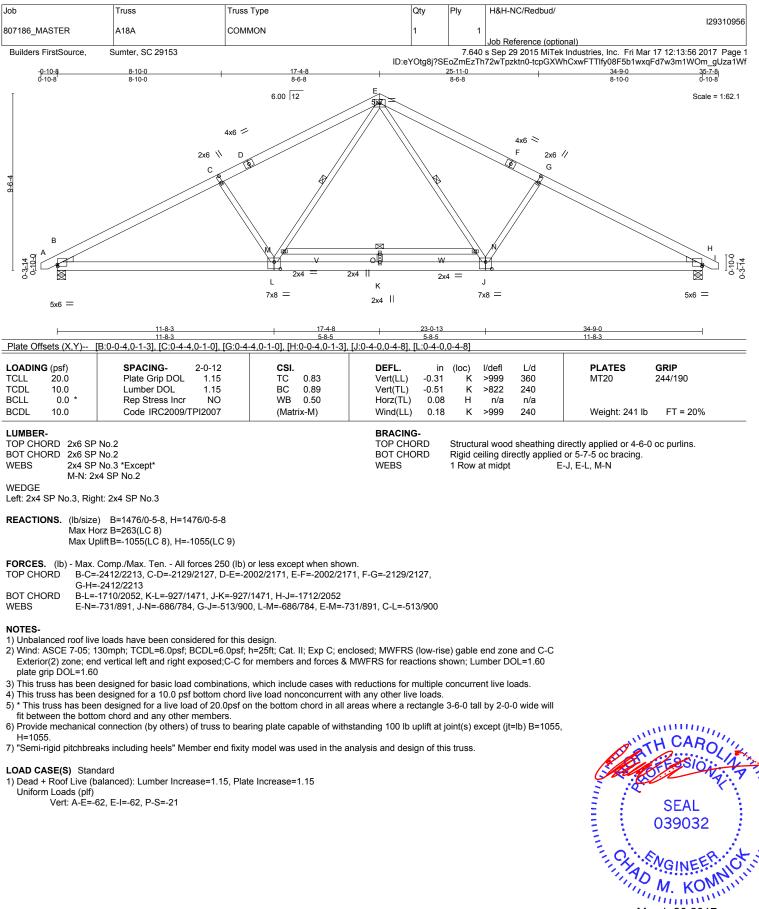






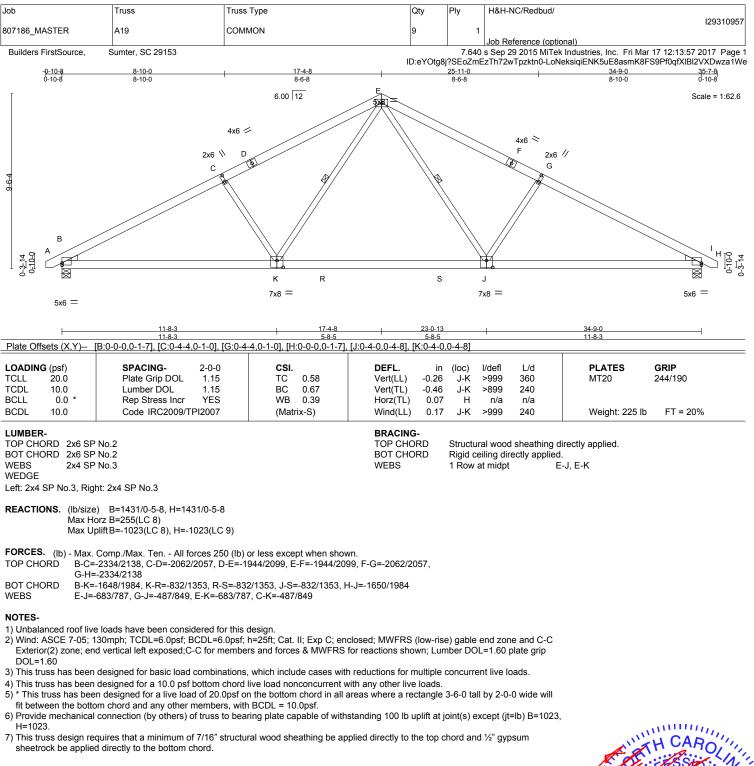


Edenton, NC 27932



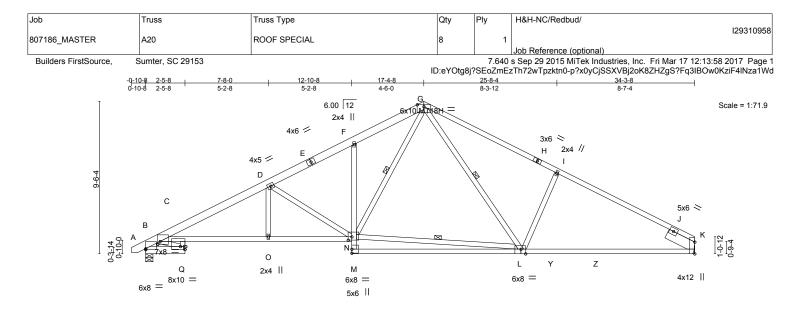
March 20,2017











			7-8-0 5-2-8	<u>12-10-8</u> 5-2-8		23-5-4					<u>34-3-8</u> 10-10-4	
Plate Off	sets (X,Y)	[<u>B:0-0-0,0-0-11], [C:0-1-</u>	12,0-2-2], [K:	Edge,0-0-0], [L:0-3-8,Ed	ge], [N:0-2-12,0-2-8	<u>], [P:Ed</u>	<u>ge,0-2</u>	-4]			
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.92	Vert(LL)	-0.29	L-M	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.92	Vert(TL)	-0.85	L-M	>483	240	MT18H	244/190
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.29	K	n/a	n/a		
BCDL	10.0	Code IRC2009/T	PI2007	(Matr	x-S)	Wind(LL)	0.41	O-P	>999	240	Weight: 216 lb	FT = 20%

BRACING-

WEBS

TOP CHORD BOT CHORD Structural wood sheathing directly applied.

L-N. G-N. G-L

Rigid ceiling directly applied.

1 Row at midpt

L	JM	B	EF	2-
-			_	·-

TOP CHORD	2x6 SP No.2 *Except*
	G-H: 2x4 SP No.2, H-K: 2x4 SP SS
BOT CHORD	2x4 SP No.2 *Except*
	B-Q: 2x6 SP No.2, C-N,K-L: 2x4 SP No.1
WEBS	2x4 SP No.3
SLIDER	Right 2x8 SP DSS 1-11-12

REACTIONS. (lb/size) B=1413/0-5-8, K=1371/Mechanical Max Horz B=293(LC 8) Max UpliftB=-1012(LC 8), K=-930(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD B-C=-1093/1201, C-D=-2793/2503, D-E=-2098/2017, E-F=-1996/2042, F-G=-2010/2220,

G-H=-1830/2041, H-I=-1938/2001, I-J=-2118/1971, J-K=-572/186 BOT CHORD B-Q=-719/534, C-P=-2102/2485, O-P=-2102/2485, N-O=-2102/2485, F-N=-226/371, L-M=-110/390, L-Y=-1510/1791, Y-Z=-1510/1791, K-Z=-1510/1791

WEBS L-N=-760/995, G-N=-936/942, G-L=-584/613, I-L=-400/770, P-Q=-343/294, C-Q=-580/780, D-N=-836/941, D-O=-31/328

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

All plates are MT20 plates unless otherwise indicated.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

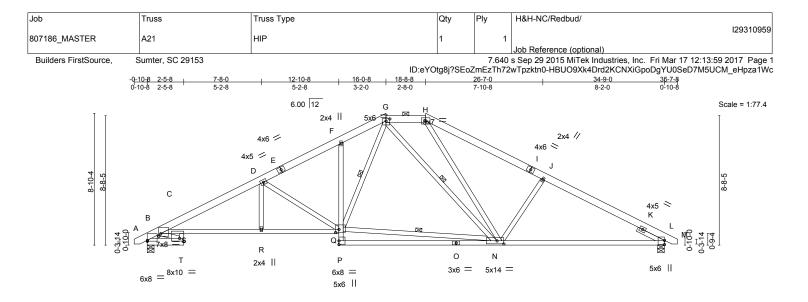
7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=1012, K=930.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.







			-8-0	<u>12-10-8</u> 5-2-8		<u>23-8-0</u> 10-9-8				34-9-1		
Plate Of	fsets (X,Y)	[B:0-0-0,0-0-11], [C:0-1-	8,0-2-2], [G:(<u>0-3-0,0-1-12],</u>	[<u>L:0-3-6,0-0-</u>	<u>1], [N:0-5-8,0-2-8]</u>	<u>[Q:0-2</u>	2-12,0-2	2-8], [S:E	dge,0-2-4]		
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.70	Vert(LL)	-0.25	N-P	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.93	Vert(TL)	-0.71	N-P	>586	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.23	L	n/a	n/a		
BCDL	10.0	Code IRC2009/T	PI2007	(Matr	ix-S)	Wind(LL)	0.38	R-S	>999	240	Weight: 244 lb	FT = 20%
LUMBER	र-					BRACING						
TOP CHORD 2x6 SP No.2					TOP CHOP	RD	Structural wood sheathing directly applied, except					
BOT CH	ORD 2x4 SF	P No.1 *Except*						2-0-0 0	oc purlins	(5-9-8 max.): G-H.	

BOT CHORD

WEBS

Rigid ceiling directly applied.

1 Row at midpt

N-Q, G-Q, G-N

BOT CHORD	2x4 SP No.1 *Except*
	B-T: 2x6 SP No.2, F-P,O-P: 2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Right 2x4 SP No.3 1-11-12

REACTIONS. (Ib/size) B=1431/0-5-8, L=1431/0-5-8 Max Horz B=235(LC 8) Max UpliftB=-1009(LC 8), L=-1009(LC 9)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 B-C=-1110/1249, C-D=-2836/2502, D-E=-2141/2027, E-F=-2106/2052, F-G=-2030/2192, G-H=-1484/1785, H-I=-1934/2022, I-J=-2042/1987, J-K=-2284/2112, K-L=-1073/684

 BOT CHORD
 B-T=-690/544, C-S=-2039/2524, R-S=-2039/2524, Q-R=-2039/2524, F-Q=-132/315, O-P=-106/386, N-O=-106/386, L-N=-1631/1963

WEBS D-R=-28/330, D-Q=-834/922, N-Q=-842/1159, G-Q=-828/832, G-N=-258/192, H-N=-391/602, J-N=-403/747, S-T=-328/298, C-T=-590/748

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

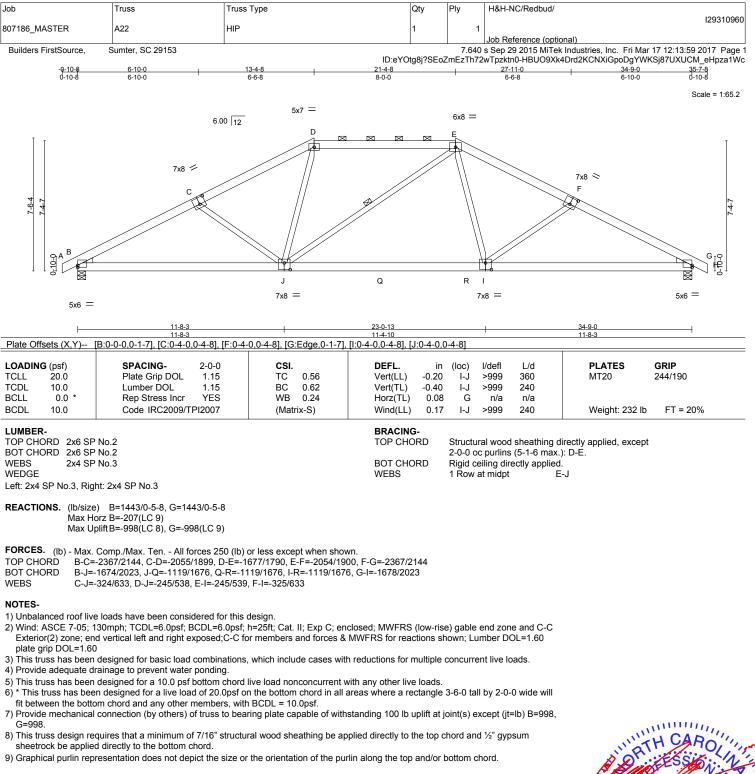
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=1009, L=1009.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

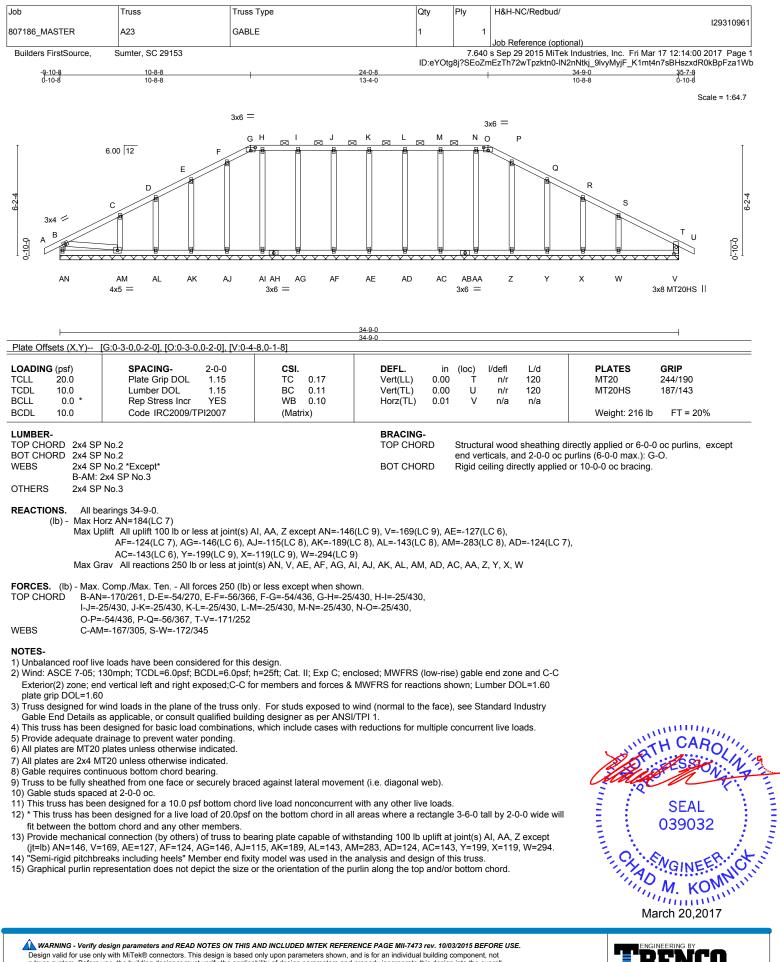




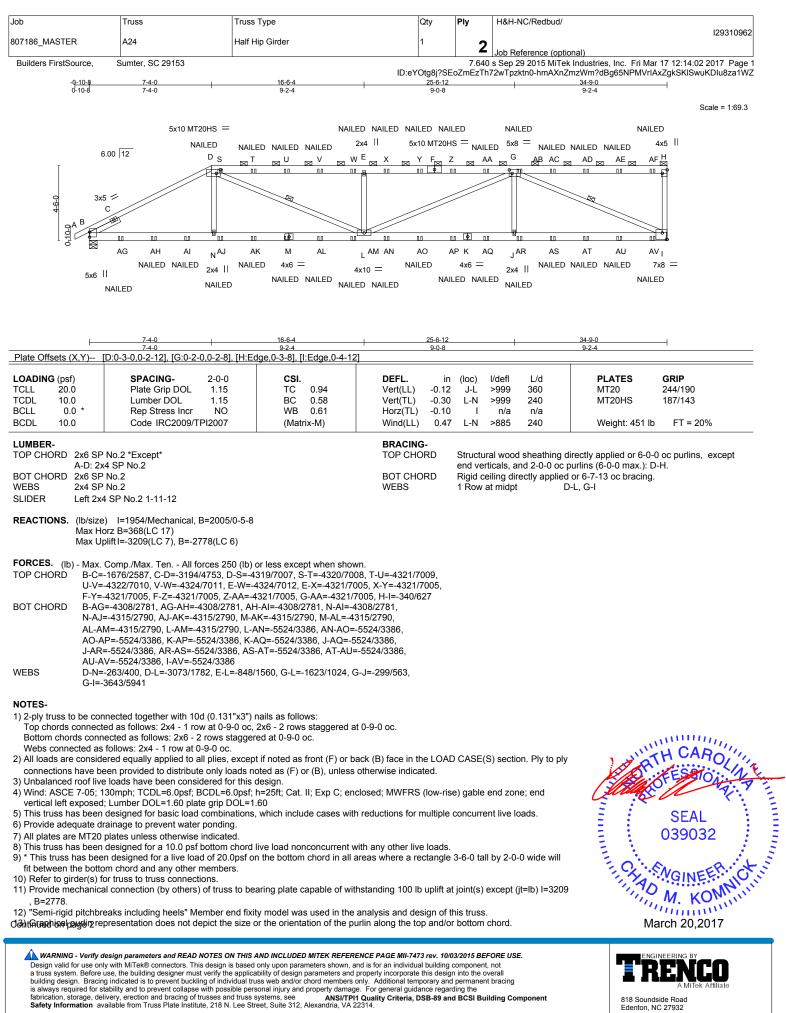








Edenton, NC 27932



Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	H&H-NC/Redbud/
807186_MASTER	A24	Half Hip Girder	1	2	129310962
				–	Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153			7.640	s Sep 29 2015 MiTek Industries, Inc. Fri Mar 17 12:14:02 2017 Page 2
		ID:e	YOtg8j?SE	oZmEzTh	72wTpzktn0-hmAXnZmzWm?dBg65NPMVrIAxZgkSKISwuKDlu8za1WZ

NOTES-

14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails. For more details refer to MiTek's ST-TOENAIL Detail.

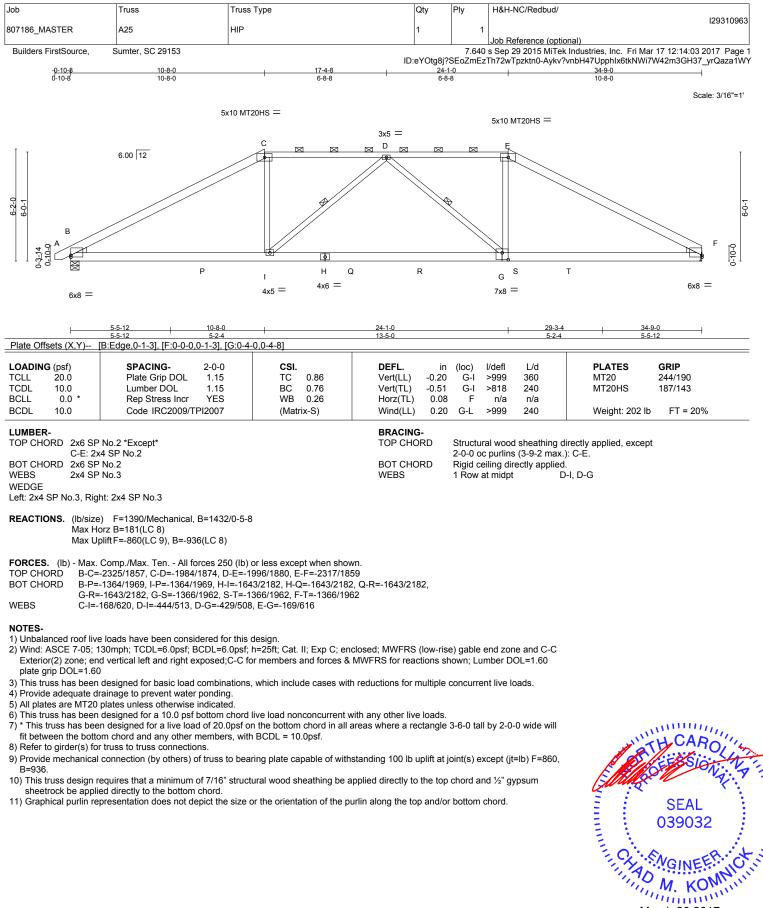
LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: A-D=-60, D-H=-60, I-O=-20

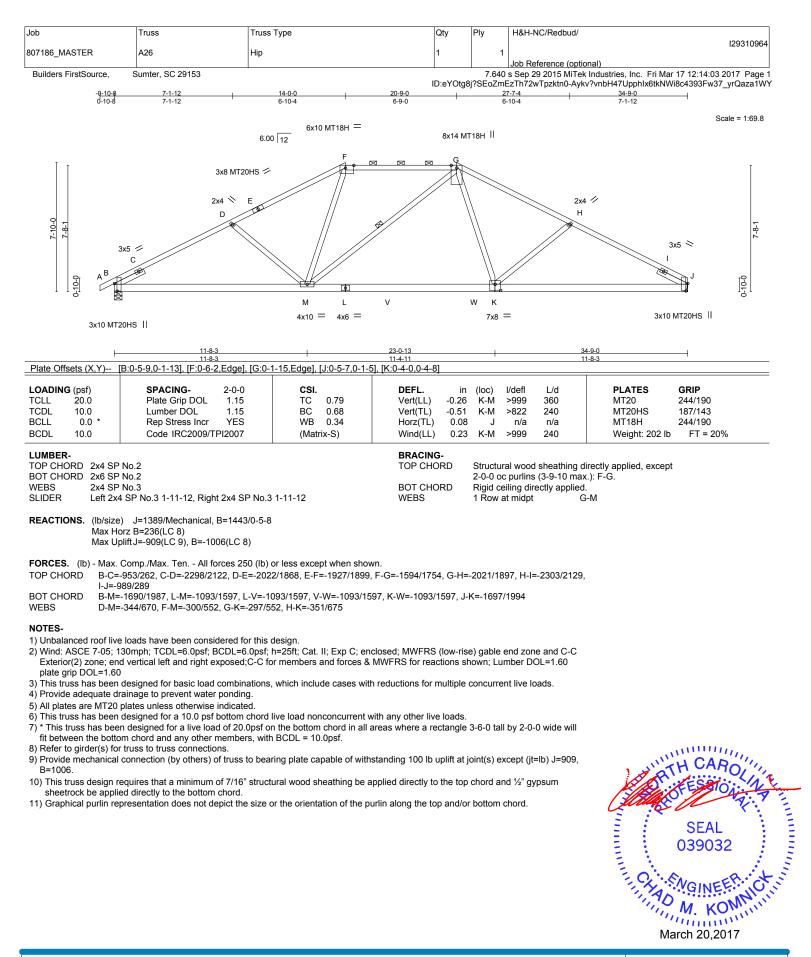
Concentrated Loads (ib) Vert: M=-18(F) S=-46(F) U=-46(F) U=-46(F) W=-46(F) X=-46(F) Y=-46(F) Z=-46(F) AA=-46(F) AB=-46(F) AC=-46(F) AD=-46(F) AE=-46(F) AF=-54(F) AG=-123(F) AH=-39(F) AI=-78(F) AJ=-18(F) AL=-18(F) AM=-18(F) AM=-18(F) AD=-18(F) AD=-18(F) AQ=-18(F) AQ= AV=-21(F)



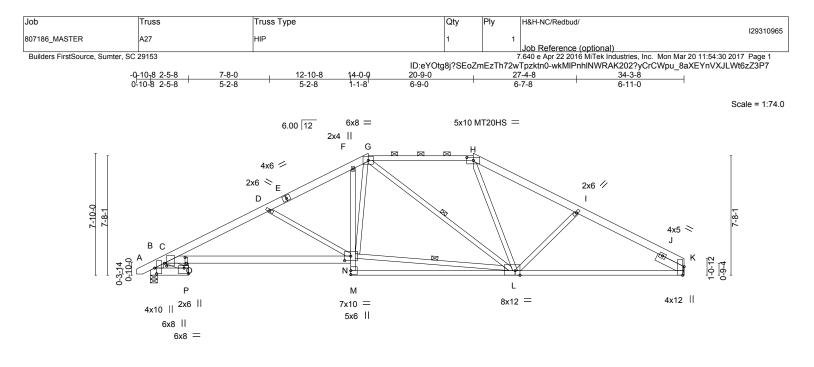


March 20,2017







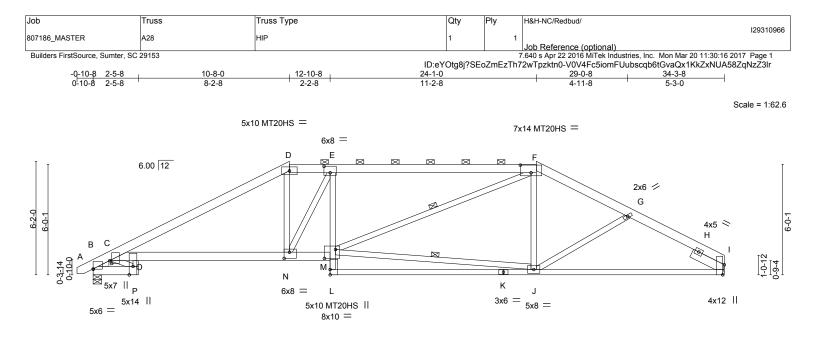


	2-5-8 12-10-		23-5-4		34-3		
Plate Offsets (X,Y)	<u>2-5-8</u> <u>10-5-0</u> [B:0-4-6,0-1-8], [C:0-2-2,0-1-12], [H:0-5		<u>10-6-12</u>], [L:0-3-12,Edge], [N:0-4	I-12,0-3-8], [O:0-4	<u>10-10</u> 4-8,0-1-0], [P:E		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSI. TC 0.89 BC 0.91 WB 0.68 (Matrix-S)	DEFL. ir Vert(LL) -0.27 Vert(TL) -0.76 Horz(TL) 0.25 Wind(LL) 0.47	′ L-Ḿ >999 5 L-M >541	L/d 360 240 n/a 240	PLATES MT20 MT20HS Weight: 238 lb	GRIP 244/190 187/143 FT = 20%
BOT CHORD 2x6 SF F-M,L- WEBS 2x4 SF WEDGE Left: 2x4 SP No.3	x4 SP No.2 P No.2 *Except* M: 2x4 SP No.2, K-L: 2x4 SP No.1		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood 2-0-0 oc purlins Rigid ceiling din 1 Row at midpt	s (4-0-13 max.) rectly applied.	ectly applied, except): G-H. -N, G-L	[PSA]
Max H Max U FORCES. (lb) - Max. TOP CHORD B-C= G-H= BOT CHORD B-P= WEBS D-N=	e) K=1371/Mechanical, B=1413/0-5-8 lorz B=239(LC 8) plift K=-892(LC 9), B=-975(LC 8) Comp./Max. Ten All forces 250 (lb) ol 1474/1492, C-D=-2714/2474, D-E=-21 1533/1711, H-I=-1922/1842, I-J=-2155 -1012/938, C-O=-2208/2596, N-O=-206 i-684/904, L-N=-1149/1632, G-N=-490/5 1038/1014, C-P=-1276/1378, I-L=-241.	· less except when shown 76/1985, E-F=-2141/2012 /1986, J-K=-560/38 9/2437, F-N=-130/301, K- ;18, G-L=-418/276, H-L=-;	, F-G=-1909/2007, ∙L=-1547/1824				
 2) Wind: ASCE 7-05; 1 C-C Exterior(2) zona grip DOL=1.60 3) This truss has been 4) Provide adequate di 5) All plates are MT20 6) This truss has been 7) * This truss has been 8) Provide mechanical joint B. 10) This truss design r sheetrock be applied 	e loads have been considered for this de 30mph; TCDL=6.0psf; BCDL=6.0psf; he e; end vertical left exposed;C-C for men designed for basic load combinations, w rainage to prevent water ponding. plates unless otherwise indicated. designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on oottom chord and any other members. r truss to truss connections. connection (by others) of truss to bearin equires that a minimum of 7/16" structure ed directly to the bottom chord. presentation does not depict the size or dard	25ft; Cat. II; Exp C; encloubers and forces & MWFR which include cases with reload nonconcurrent with the bottom chord in all are ang plate capable of withstational wood sheathing be approximation.	IS for reactions shown; L eductions for multiple co n any other live loads. eas where a rectangle 3- anding 892 lb uplift at joir plied directly to the top ch	umber DOL=1.6 ncurrent live load 6-0 tall by 2-0-0 v nt K and 975 lb u nord and ½" gyps	0 plate ls. vide plift at	OS CHARLEN CHARD M	CAROLINE SEAL 39032 KOMMUNIN

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



March 20,2017



	<u>2-5-8</u> 2-5-8		<u>+ 12-10-8</u> 2-2-8	24-1-0		<u>34-3-8</u> 10-2-8	
Plate Offsets (X,Y) [B:Edge,0-0-3], [C:0-1-14,0-1-4], [E:0-4-0,0-4-4], [F:0-7-0,0-5-0], [I:0-6-9,Edge], [M:0-7-4,0-5-12], [N:0-3-8,0-4-0]							
TCDL ·	(psf) 20.0 10.0 0.0 * 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSI. TC 0.82 BC 0.95 WB 0.70 (Matrix-S)	DEFL. in Vert(LL) -0.34 Vert(TL) -0.94 Horz(TL) 0.23 Wind(LL) 0.44	`J-Ĺ >999 360 J-L >438 240 I n/a n/a	PLATES MT20 MT20HS Weight: 233 lb	GRIP 244/190 187/143 FT = 20%
LUMBER- TOP CHOR BOT CHOR WEBS WEDGE Left: 2x4 SP SLIDER	D 2x4 SP B-P: 2x 2x4 SP	P No.2 *Except* k6 SP No.2, C-M: 2x6 SP No.1, I-K: 2x4 \$	SP No.1	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing directly applied, except 2-0-0 oc purlins (3-3-5 max.): D-F. Rigid ceiling directly applied. 1 Row at midpt J-M, F-M		
REACTIONS. (Ib/size) I=1371/Mechanical, B=1413/0-5-8 (min. 0-1-11) Max Horz B=194(LC 8) Max Uplift I=-843(LC 9), B=-927(LC 8)							
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD B-C=-1726/1656, C-D=-2617/2218, D-E=-2269/2195, E-F=-2483/2385, F-G=-2040/1833, G-H=-2113/1904, H-I=-454/66 BOT CHORD B-P=-1262/1256, C-O=-1878/2460, N-O=-1729/2290, M-N=-1859/2439, E-M=-175/273, K-L=-169/517, J-K=-169/517, I-J=-1483/1770 WEBS D-N=-623/863, E-N=-655/779, J-M=-160/1379, F-M=-670/808, F-J=-27/323, G-J=-196/274, O-P=-1157/1196, C-P=-1710/1718							
 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads. 4) Provide adequate drainage to prevent water ponding. 5) All plates are MT20 plates unless otherwise indicated. 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 8) Refer to girder(s) for truss to truss connections. 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 843 lb uplift at joint I and 927 lb uplift at joint 8. 10) This truss heaign requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord. 							
11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. M. KOM LOAD CASE(S) Standard March 20,2017							

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply H&H-NC/Redbud/	
807186_MASTER	A29	HIP GIRDER	1		129310967
Builders FirstSource, S	umter, SC 29153			2 Job Reference (op 7.640 e Apr 22 2016 M	iTek Industries, Inc. Mon Mar 20 11:36:53 2017 Page 1
	-0_10_8 2-5-8 7-4-0	12-10-8	20-1-12	27-5-0	MukJIG80civaTkzsdXDV3q_4RnwTMp4GtzZ3fe
	0 ⁻ 10-8 2-5-8 ¹ 4-10-8	5-6-8	7-3-4	7-3-4	7-4-0 0 ¹ 10-8
		=	NAILE	Ð	Scale = 1:76.7
	5x10 M	NAILED 1T20HS = NAILED	NAILED	NAILED 5×10 MT20HS	_
		1411 ED 2x4	NAILED 3x5 — AILED 3x8 MT20F	5410 10120113	_
-	6.00 12 NAILED AA				
4-6-0	ВС				3x5 × 0
4	₹ 9 A				XXX JK
l		P <mark>₽⊡</mark> AN ^Q AO AP _{AQ} AR	AS _{ATAU N} M A	^N AW AX AY I AZ BA	BB BC
	S AL AM /	2x6 NAILED = N	AILED $4x6 =$	···· L ···· L ····	NAILED 5x6
	$5x6 = \frac{2x6}{4x5} =$	4x5	$\begin{array}{c} 4x6 \\ \text{NAILED} \\ 5x8 \end{array} =$	NAILED	NAILED
	3x5 =	NAILED	NAILED	NAILED	
	6x8 =	NAILED	NAILE	D NAILED	
	NAILED				
	<u>2-5-8</u> 7-4-0 2-5-8 4-10-8	<u>12-10-8</u> 5-6-8	<u>20-1-12</u> 7-3-4	<u>27-5-0</u> 7-3-4	<u>34-9-0</u> 7-4-0
Plate Offsets (X,Y		-5], [D:0-8-0,0-2-8], [H:0-7-8,0-2-4]			
LOADING (psf)		-0-0 CSI .		n (loc) l/defl L/d	PLATES GRIP
TCLL 20.0 TCDL 10.0	Lumber DOL	1.15 TC 0.97 1.15 BC 0.93	Vert(LL) -0.15 Vert(TL) -0.38	B E >999 240	MT20 244/190 MT20HS 187/143
BCLL 0.0 * BCDL 10.0	Rep Stress Incr Code IRC2009/TPI20	NO WB 0.95 007 (Matrix-M)	Horz(TL) -0.22 Wind(LL) 0.56		Weight: 439 lb FT = 20%
LUMBER-			BRACING-		
	4 SP No.1 *Except* D: 2x6 SP No.2, H-K: 2x4 SP No.	2	TOP CHORD	Structural wood sheathing except	directly applied or 5-11-12 oc purlins, [PSA]
BOT CHORD 2x	6 SP No.2 *Except* O: 2x4 SP No.2	2		2-0-0 oc purlins (5-7-6 max	
WEBS 2x	4 SP No.2		BOT CHORD	Rigid ceiling directly applie	
WEDGE Left: 2x4 SP No.3					
SLIDER Ri	ght 2x4 SP No.2 1-11-12				
	o/size) B=2041/0-5-8 (min. 0-1- ax Horz B=-128(LC 7)	-8), J=2045/0-5-8 (min. 0-1-8)			
	ax Uplift B=-2770(LC 6), J=-2850	(LC 7)			
. ,		250 (lb) or less except when show			
		23, D-AA=-3961/5840, D-AB=-477 5/7369, E-AE=-4725/7276, AE-AF=		ŝ9,	
		/7276, F-G=-4155/6513, G-AH=-4′ 8513, AJ-AK=-4155/6513, H-AK=-⁄			
	H-I=-3245/4765, I-J=-1741/2697 3-AL=-2511/1785, S-AL=-2511/17	785, C-AM=-5531/3851, R-AM=-55	531/3851. R-AN=-5221/36	10.	
	AN-AO=-5221/3610, Q-AO=-522	1/3610, Q-AP=-5249/3637, AP-AQ /3637, E-P=-556/1095, O-AS=-148	=-5249/3637,		
/	AT-AU=-1481/961, AU-AV=-1481	/961, N-AV=-1481/961, M-N=-408 5/2831, AY-AZ=-4085/2831, L-AZ	5/2831, M-AW=-4085/283		AND ALL AND A DECEMBER OF A
l	-BA=-4078/2824, BA-BB=-4078/	2824, BB-BC=-4078/2824, J-BC=-	4078/2824		TH CARO
		N-P=-4951/3246, F-P=-829/619, F R-S=-2461/1781, C-S=-2430/3418			A ANT SON VIS
NOTES-					Concerna
	e connected together with 10d (0. nected as follows: 2x6 - 2 rows st	131"x3") nails as follows: aggered at 0-9-0 oc, 2x4 - 1 row a	t 0-9-0 oc.		SEAL 039032 MGINEER OTION
Bottom chords		s staggered at 0-9-0 oc, 2x4 - 1 ro			039032
2) All loads are co	nsidered equally applied to all plie	es, except if noted as front (F) or b		CASE(S) section. Ply to	
3) Unbalanced roc	f live loads have been considered				NGINEEL
vertical left and	right exposed; Lumber DOL=1.60			gable end zone; end	M. KOM
,	een designed for basic load com te drainage to prevent water pon	binations, which include cases with ding.	h reductions for multiple co	oncurrent live loads.	March 20,2017
7) All plates are M	T20 plates unless otherwise indic				
WARNING - V	erify design parameters and READ NOTE	S ON THIS AND INCLUDED MITEK REFER	ENCE PAGE MII-7473 rev. 10/03/		ENGINEERING BY
a truss system. B building design.	efore use, the building designer must verif Bracing indicated is to prevent buckling of	ign is based only upon parameters shown, a y the applicability of design parameters and individual truss web and/or chord members	properly incorporate this design i only. Additional temporary and p	nto the overall ermanent bracing	I KENCU
is always required fabrication, storage	d for stability and to prevent collapse with p ge, delivery, erection and bracing of trusse	oossible personal injury and property damages and truss systems, see ANSI/TP	ge. For general guidance regardir I1 Quality Criteria, DSB-89 and I	ng the	A MiTek Affiliate 818 Soundside Road
Safety informati	available from Fruss Plate Institute, 21	8 N. Lee Street, Suite 312, Alexandria, VA	22314.		Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	H&H-NC/Redbud/
					129310967
807186_MASTER	A29	HIP GIRDER	1	2	lah Deference (antional)
				-	Job Reference (optional)
Builders FirstSource, Sumter, SC 29153					7.640 e Apr 22 2016 MiTek Industries, Inc. Mon Mar 20 11:36:54 2017 Page 2

ID:eYOtg8j?SEoZmEzTh72wTpzktn0-raYXnivM43O?emH58AFCPq3OES9Dpu13i0YdpJzZ3fd

NOTES-

9)* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2770 lb uplift at joint B and 2850 lb uplift at joint J.

11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

13) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails. For more details refer to MiTek's ST-TOENAIL Detail.

14) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 66 lb down and 103 lb up at 3-10-8, and 72 lb down and 174 lb up at 5-10-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

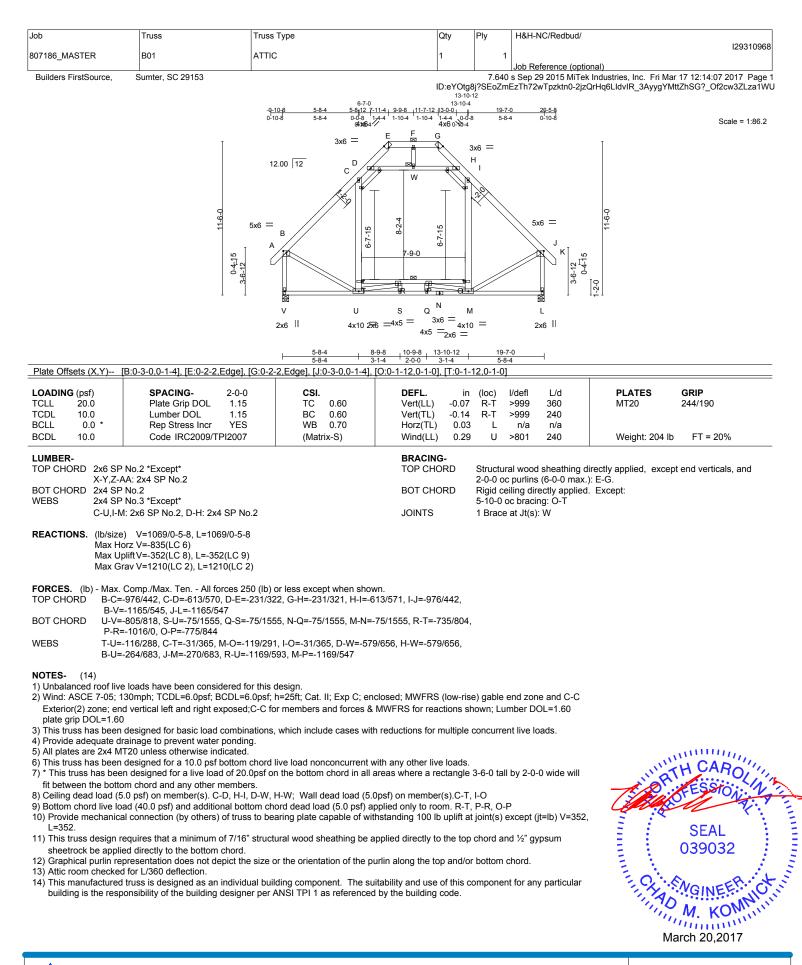
Uniform Loads (plf) Vert: A-D=-60, D-H=-60, H-K=-60, S-T=-20, P-R=-20, O-W=-20

Concentrated Loads (lb)

Vert: F=-46(B) AA=-2(B) AB=-67(B) AC=-67(B) AD=-67(B) AE=-46(B) AF=-46(B) AG=-46(B) AH=-46(B) AI=-46(B) AJ=-46(B) AJ=-46(B) AL=-123(B) AN=-66 AO=-72 AS=-18(B) AT=-18(B) AU=-18(B) AV=-18(B) AV=-18(B) AY=-18(B) AY=-18(B) AZ=-18(B) AZ=-18(

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



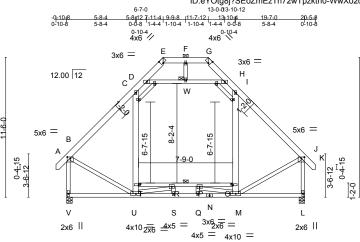


Edenton, NC 27932

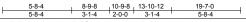
Job	Truss	Truss Type	Qty	Ply	H&H-NC/Redbud/
	D 00		<u> </u>		129310969
807186_MASTER	B02	ATTIC	0	1	Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153				s Sep 29 2015 MiTek Industries, Inc. Fri Mar 17 12:14:08 2017 Page 1

7.640 s Sep 29 2015 MiTek Industries, Inc. Fri Mar 17 12:14:08 2017 Page 1 ID:eYOtg8j?SEoZmEzTh72wTpzktn0-WwXo2crk6cmmwbZFjfTv4ZQ2J5oVkRepHGgc6oza1WT 13-043-10-12

1-6-0



Scale = 1:94.8



	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.15	TC	0.60	Vert(LL)	-0.07	Ř-Ť	>999	360	MT20	244/190
CDL	10.0	Lumber DOL	1.15	BC	0.60	Vert(TL)	-0.14	R-T	>999	240		
CLL	0.0 *	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.03	L	n/a	n/a		
CDL	10.0	Code IRC2009/T	PI2007	(Matr	ix-S)	Wind(LL)	0.29	U	>801	240	Weight: 204 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 *Except*					BRACING TOP CHO		Structu	Ind wood	sheathing d	lirectly applied, except	t and vorticals an	

TOP CHORD	2x6 SP No.2 *Except* X-Y,Z-AA: 2x4 SP No.2		Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): E-G.
BOT CHORD WEBS	2x4 SP No.2 2x4 SP No.3 *Except*		Rigid ceiling directly applied. Except: 5-10-0 oc bracing: O-T
	C-U,I-M: 2x6 SP No.2, D-H: 2x4 SP No.2	JOINTS	1 Brace at Jt(s): W

REACTIONS. (lb/size) V=1069/0-5-8, L=1069/0-5-8 Max Horz V=-835(LC 6) Max UpliftV=-352(LC 8), L=-352(LC 9) Max Grav V=1210(LC 2), L=1210(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD B-C=-976/442, C-D=-613/570, D-E=-231/322, G-H=-231/321, H-I=-613/571, I-J=-976/442,

- B-CE--970442, CD-CF13577, D-CE-2317322, CF1=-2317321, TF1=-013/577, P3--970442, B-VE-1165/545, J-L=-1165/547 BOT CHORD U-V=-805/818, S-U=-75/1555, Q-S=-75/1555, N-Q=-75/1555, M-N=-75/1555, R-T=-735/804, P-R=-1016/0, O-P=-775/844
- WEBS T-U=-116/288, C-T=-31/365, M-O=-119/291, I-O=-31/365, D-W=-579/656, H-W=-579/656, B-U=-264/683, J-M=-270/683, R-U=-1169/593, M-P=-1169/547

NOTES- (14)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

Provide adequate drainage to prevent water ponding.

All plates are 2x4 MT20 unless otherwise indicated.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Ceiling dead load (5.0 psf) on member(s). C-D, H-I, D-W, H-W; Wall dead load (5.0psf) on member(s).C-T, I-O

9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. R-T, P-R, O-P
 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) V=352, L=352.

11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

13) Attic room checked for L/360 deflection.

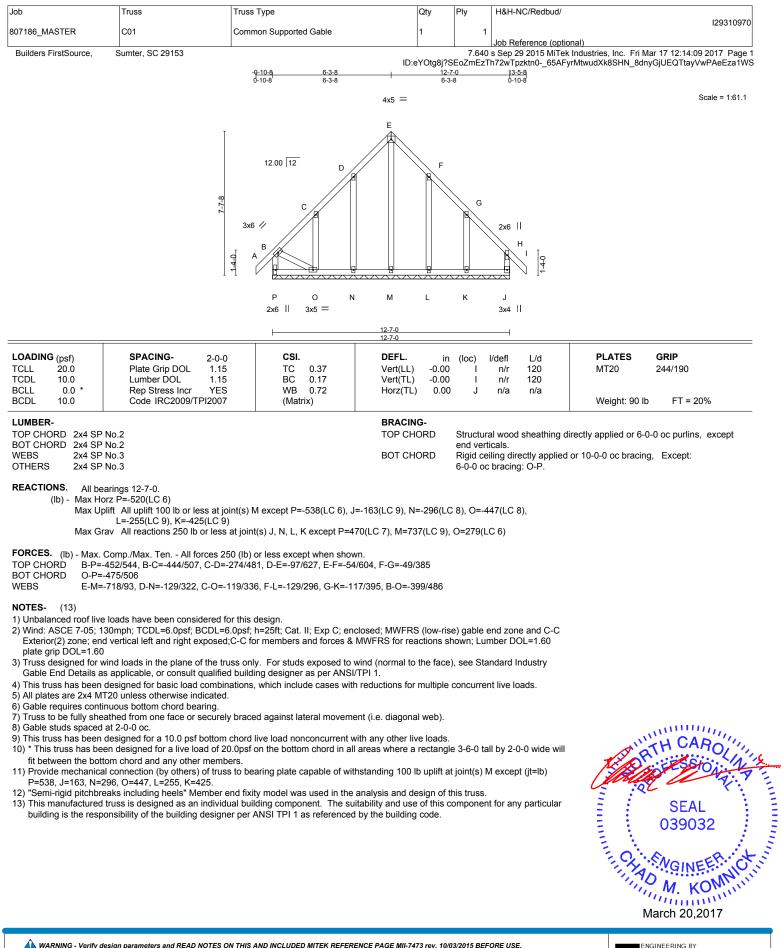
14) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



ENGINEERING BY EREENCO A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 MSIVTP11 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type		Qty	Ply	H&H-NC/Redbud/]
807186_MASTER	C02	Common Girder	1	I	2			129310971
Builders FirstSource,	Sumter, SC 29153				7.640		k Industries, Inc. Fri M	ar 17 12:14:10 2017 Page 1
		<u> 3-3-8</u>	ID:e 6-3-8 9-3-8 3-0-0 3-0-0		SEoZmEz <u>12-7-0</u> 3-3-8	Th72wTpzktn0-SIfZT	Is_eD0U9ujer4VNA_V	/RwuWsCMh5ka9jAgza1WR
			5x6					Scale = 1:59.3
			С					
		12.00 12						
	ω	4x5 1/		$\langle \rangle$	<			
	7-7-8	4x5 */ B			$\langle \rangle$	4x5 ℕ		
		A A			X	E		
			•					
		U P	Q _F R 3x10 MT20HS	ST	U 4	x12		
		4x12	3X10 112015 11	40.7				
Plate Offsets (X,Y)	[A:0-5-12,0-0-10], [E:0-6-12,0-	<u>6-3-8</u> <u>6-3-8</u> 0-10], [F:0-6-4,0-1-8]		12-7-0 6-3-8				
LOADING (psf)	SPACING- 2-0-		DEFL.			/defl L/d	PLATES	GRIP
TCLL 20.0 TCDL 10.0 BCLL 0.0 *	Plate Grip DOL 1.1 Lumber DOL 1.1	5 BC 0.42	Vert(LL) Vert(TL)	-0.04 -0.10	F-I	>999 360 >999 240	MT20 MT20HS	244/190 187/143
3CDL 10.0	Rep Stress Incr No Code IRC2009/TPI200		Horz(TL) Wind(LL)	-0.02 0.08	A F-M	n/a n/a >999 240	Weight: 203 I	b FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x8 SP WEBS 2x4 SP SLIDER Left 2x6	DSS	No 2 2 0 0	BRACING TOP CHC BOT CHC	RD			lirectly applied or 6-0 l or 10-0-0 oc bracing	
REACTIONS. (Ib/size Max Ho	 A=4702/0-5-8, E=4827/0-5 prz A=412(LC 5) pliftA=-2809(LC 7), E=-3094(L 	-8						
TOP CHORD A-B= BOT CHORD A-O= T-U=	-3161/1992, B-C=-3795/2456,	250 (lb) or less except when sh C-D=-3798/2449, D-E=-3126/2 P-Q=-1589/2624, F-Q=-1589/2	2067	624, R-\$	S=-1589/2	2624, S-T=-1589/26	624,	
Top chords connect	nnected together with 10d (0.1 ed as follows: 2x6 - 2 rows sta	ggered at 0-9-0 oc.						
Webs connected as 2) All loads are conside		c. s, except if noted as front (F) or			CASE(S) section. Ply to ply		
3) Unbalanced roof live	e loads have been considered				、			
vertical left and right	t exposed; Lumber DOL=1.60	:6.0psf; h=25ft; Cat. ΙΙ; Exp C; ε plate grip DOL=1.60 inations, which include cases w			, 0			
Áll plates are MT20This truss has been	plates unless otherwise indica designed for a 10.0 psf bottor	ted. n chord live load nonconcurrent	t with any other live	· loads.				CARO MA
fit between the botto	om chord and any other memb						X A	Sig A
E=3094. 10) "Semi-rigid pitchbre	eaks including heels" Member	s to bearing plate capable of wi end fixity model was used in th	ne analysis and des	sign of th	nis truss.	,	Contra	NA. A
1-5-12, 1454 lb dov	wn and 843 lb up at 3-5-12, 1	provided sufficient to support c 454 lb down and 843 lb up at 5 wn and 944 lb up at 11-5-12 ol	5-5-12, 1454 lb dov	in and 8	43 lb up a	at 7-5-12, and 135	3	SEAL 039032
connection device(12) This manufactured	(s) is the responsibility of other I truss is designed as an indivi		suitability and use	of this c			3 3 CT	En Right
LOAD CASE(S) Stan	, , , , ,						and the	M. KOMN
Continued on page 2							N	Aarch 20,2017
A		ON THIS AND INCLUDED MITEK REFE		2 40/0	2/2015 BEE			GINEERING BY

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	H&H-NC/Redbud/
807186 MASTER	C02	Common Girder	1		129310971
	002		•	2	Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153				s Sep 29 2015 MiTek Industries, Inc. Fri Mar 17 12:14:10 2017 Page 2

ID:eYOtg8j?SEoZmEzTh72wTpzktn0-SlfZTIs_eD0U9ujer4VNA_VRwuWsCMh5ka9jAgza1WR

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

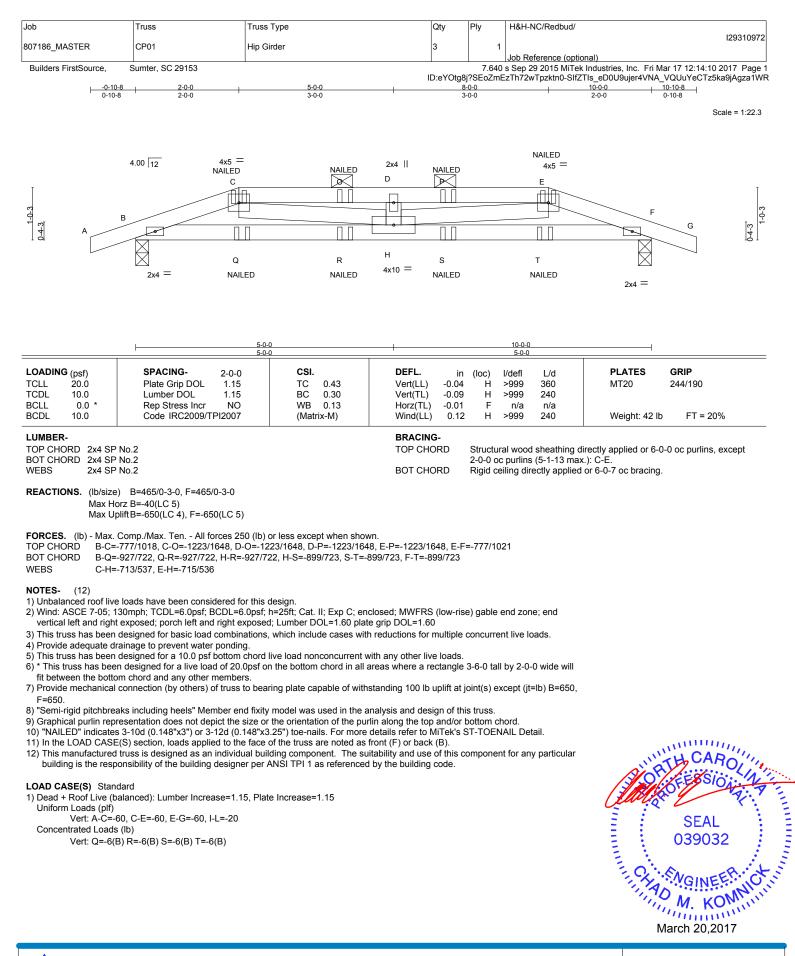
Vert: A-C=-60, C-E=-60, G-K=-20

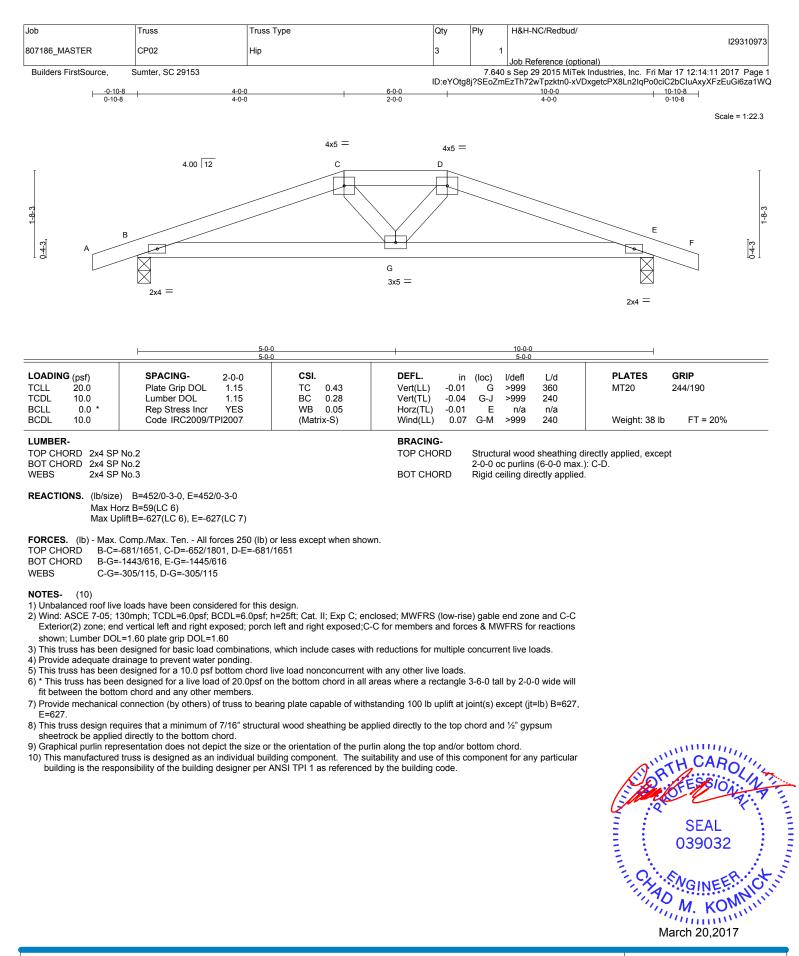
Concentrated Loads (lb)

Vert: O=-1454(B) P=-1454(B) Q=-1454(B) R=-1454(B) T=-1353(B) U=-1353(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

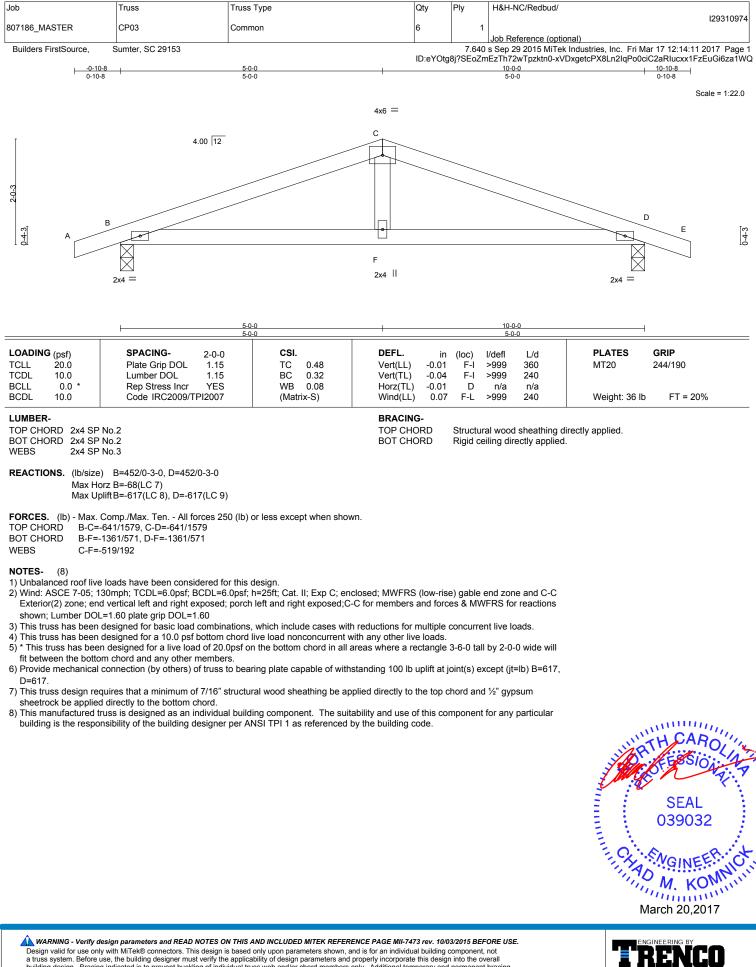




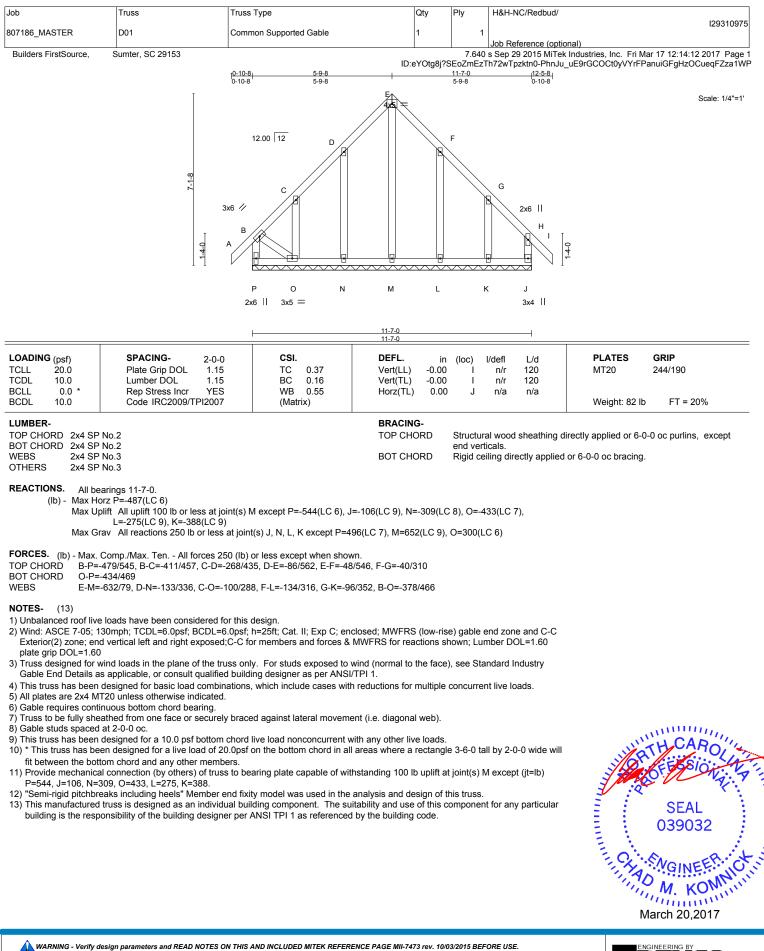


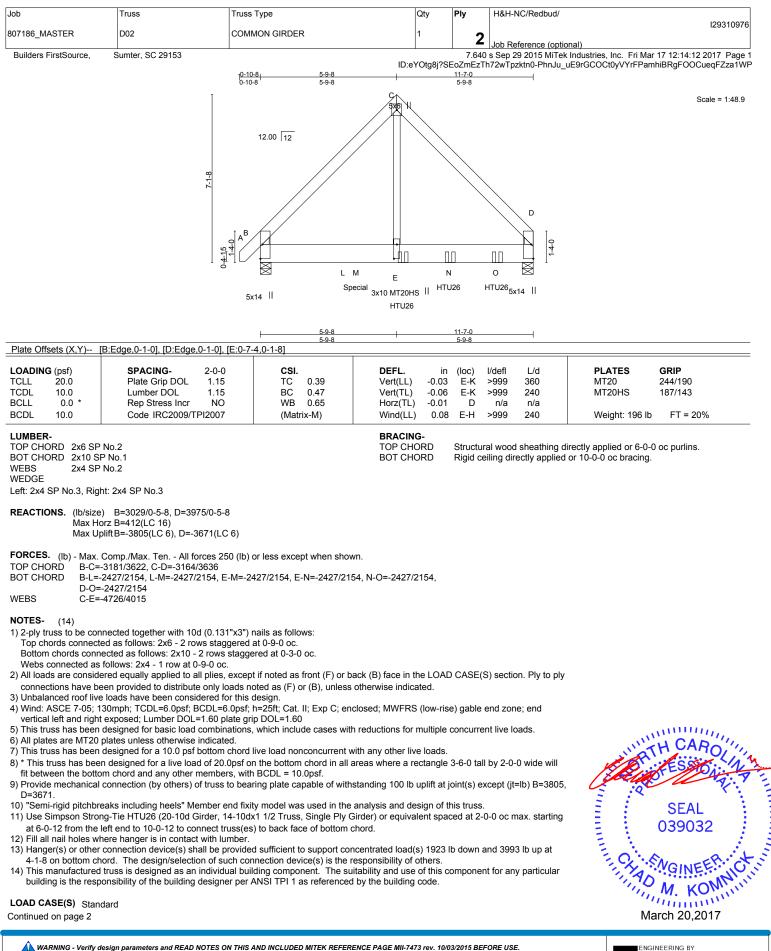






Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. TRENCO A MITEK Affiliate 818 Soundside Road Edenton, NC 27932





Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoISITPI1 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

[Job	Truss	Truss Type	Qty	Ply	H&H-NC/Redbud/
	807186_MASTER	D02	COMMON GIRDER	1	2	129310976
l					-	Job Reference (optional)
	Builders FirstSource, S	Sumter, SC 29153			7.640	s Sep 29 2015 MiTek Industries, Inc. Fri Mar 17 12:14:12 2017 Page 2
			ID:e	YOtg8j?SE	EoZmEzTh	72wTpzktn0-PhnJu_uE9rGCOCt0yVYrFPamhiBRgFOOCueqFZza1WP

LOAD CASE(S) Standard

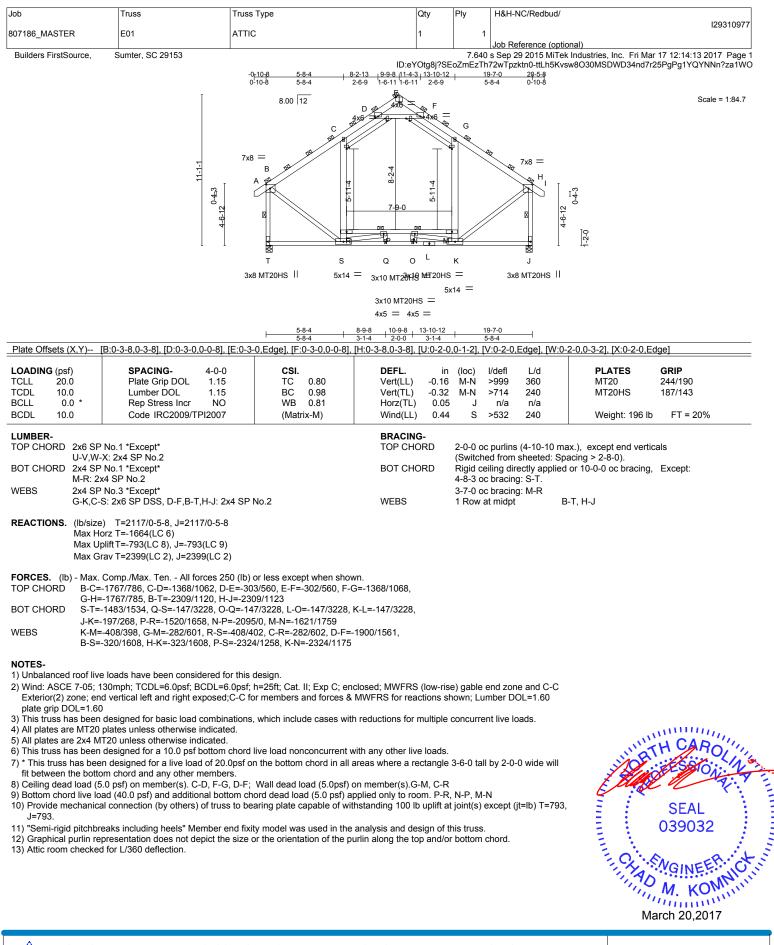
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: A-C=-60, C-D=-60, F-I=-20

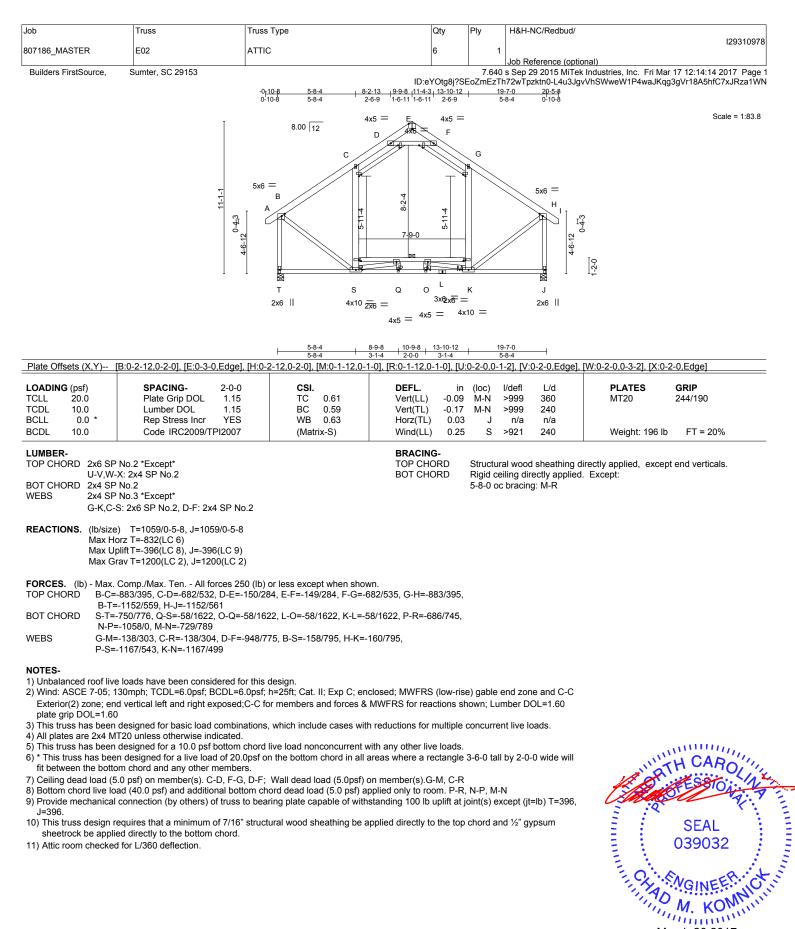
Concentrated Loads (Ib)

Vert: E=-1369(B) M=-1923(B) N=-1370(B) O=-1370(B)

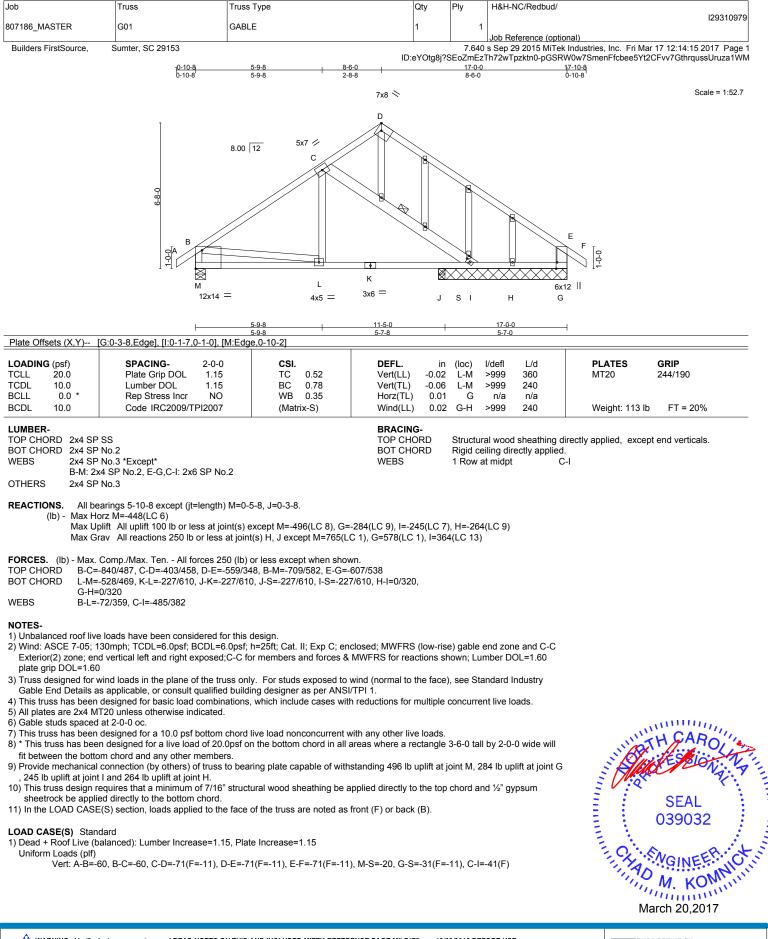
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



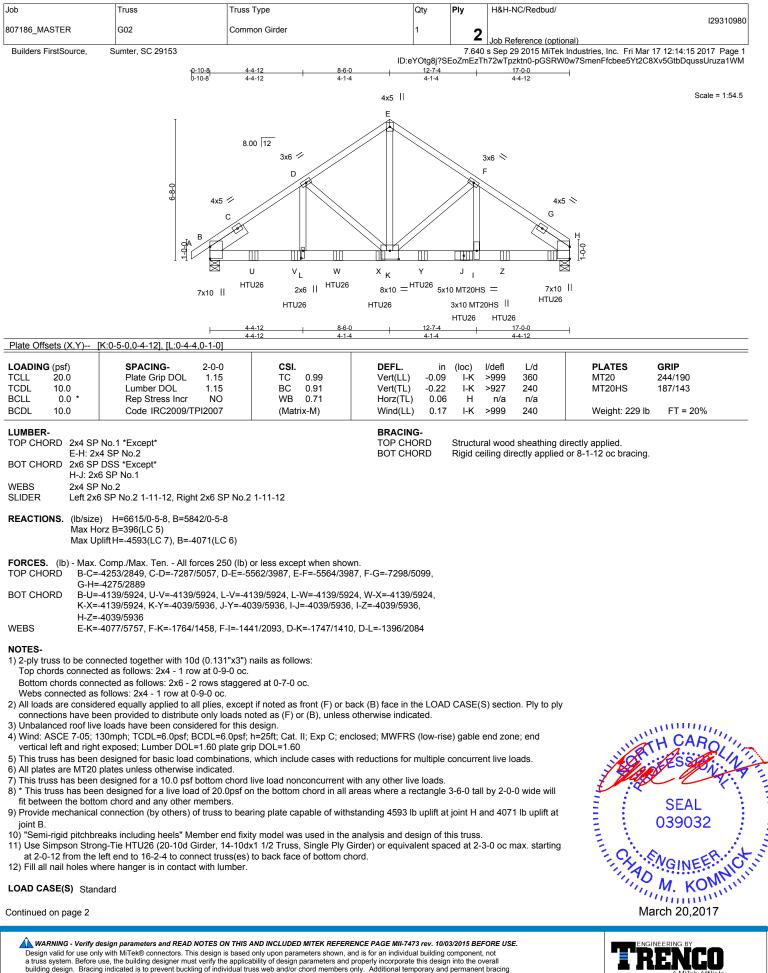








A Millek A 818 Soundside Road Edenton, NC 27932



besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	Truss Type	Qty	Ply	H&H-NC/Redbud/
807186_MASTER	G02	Common Girder	1	2	129310980
				–	Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153	ID:e	YOtg8j?S		s Sep 29 2015 MiTek Industries, Inc. Fri Mar 17 12:14:15 2017 Page 2 72wTpzktn0-pGSRW0w7SmenFfcbee5Yt2C8Xv5GtbDqussUruza1WM

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

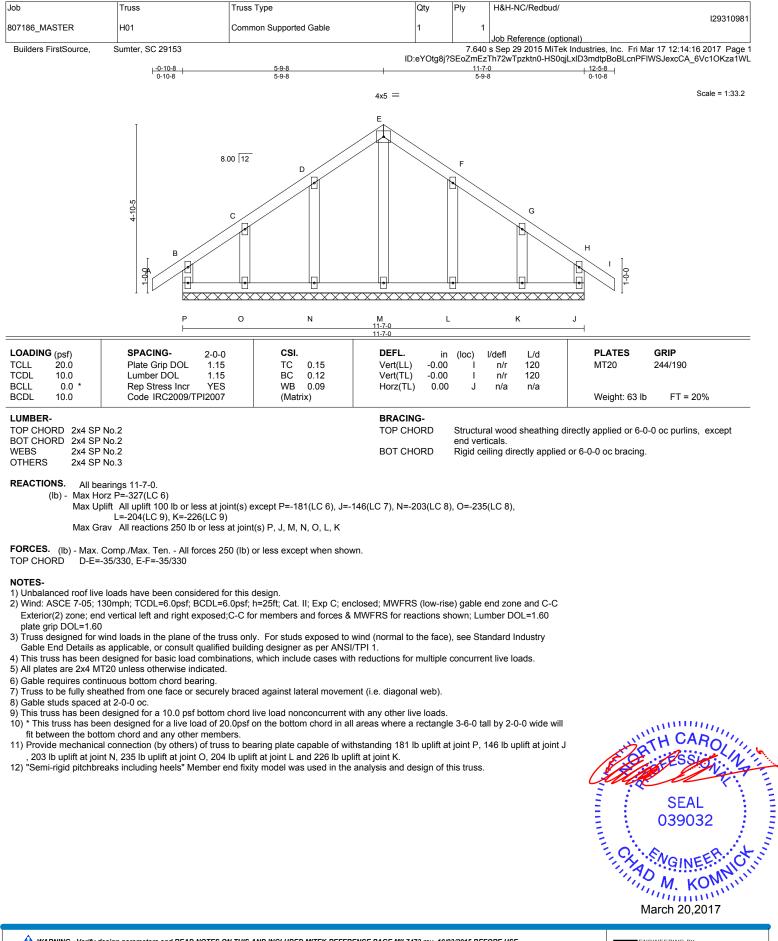
Uniform Loads (plf)

Vert: A-E=-60, E-H=-60, M-Q=-20 Concentrated Loads (lb)

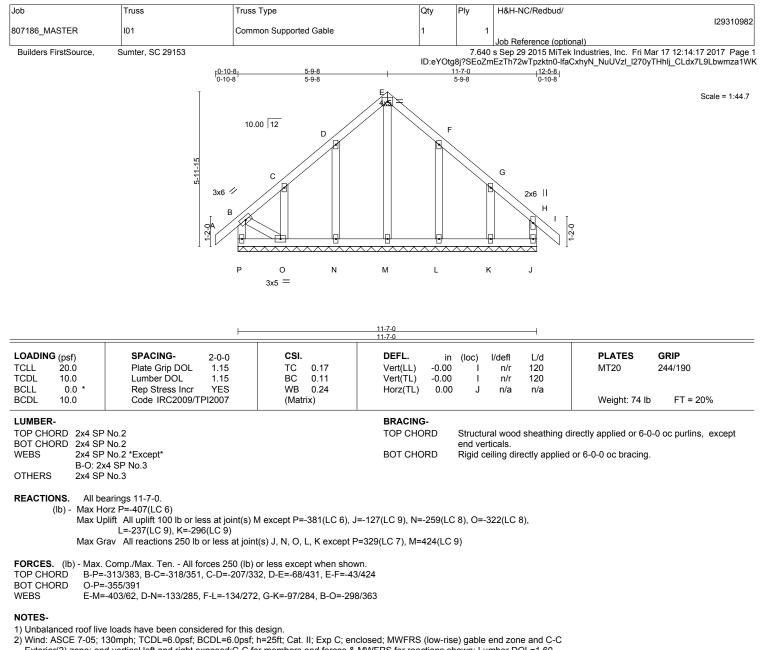
Vert: J=-1370(B) O=-1415(B) U=-1369(B) V=-1370(B) W=-1370(B) X=-1370(B) Y=-1370(B) Z=-1412(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





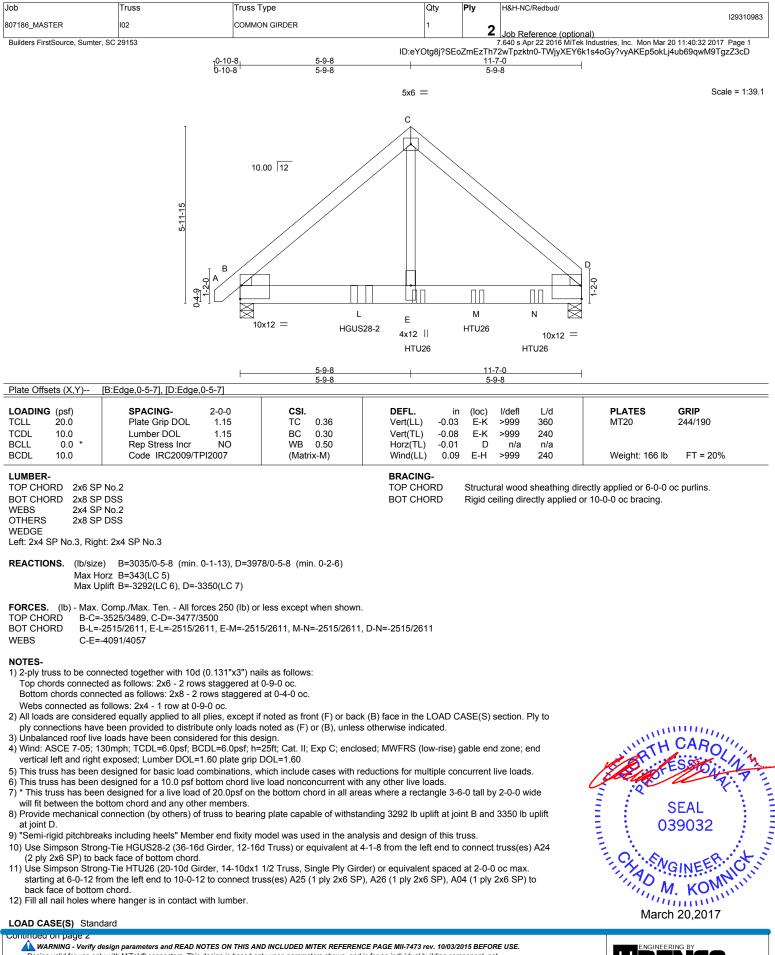
A MiTek Affili 818 Soundside Road Edenton, NC 27932



- Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 8) Gable studs spaced at 2-0-0 oc.9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) M except (jt=lb) P=381, J=127, N=259, O=322, L=237, K=296.
- 12) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.







818 Soundside Roa Edenton, NC 27932

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent uclapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	Truss Type	Qty	Ply	H&H-NC/Redbud/
					129310983
807186_MASTER	102	COMMON GIRDER	1	2	
				_	Job Reference (optional)
Builders FirstSource, Sumter, SC 2	29153				2.640 s Apr 22 2016 MiTek Industries, Inc. Mon Mar 20 11:40:32 2017 Page 2

ID:eYOtg8j?SEoZmEzTh72wTpzktn0-TWjyXEY6k1s4oGy?vyAKEp5okLj4ub69qwM9TgzZ3cD

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

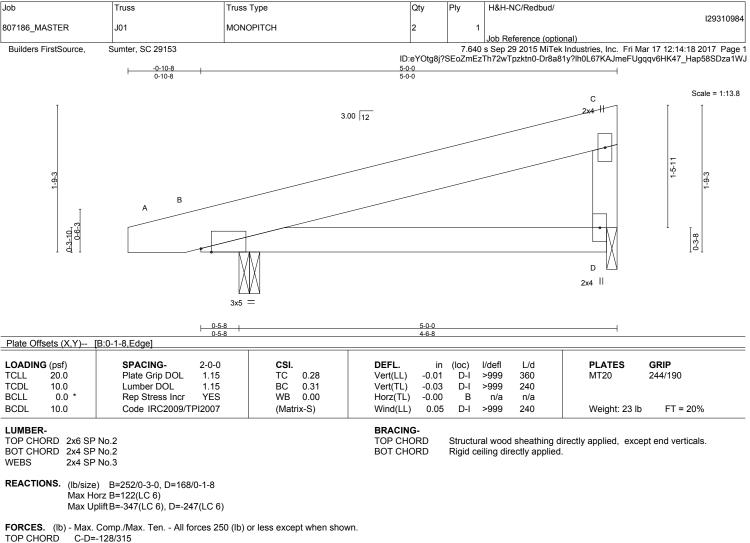
Vert: A-C=-60, C-D=-60, F-I=-20

Concentrated Loads (lb)

Vert: E=-1370(B) L=-1934(B) M=-1369(B) N=-1370(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

NOTES-

1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) D considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) D.

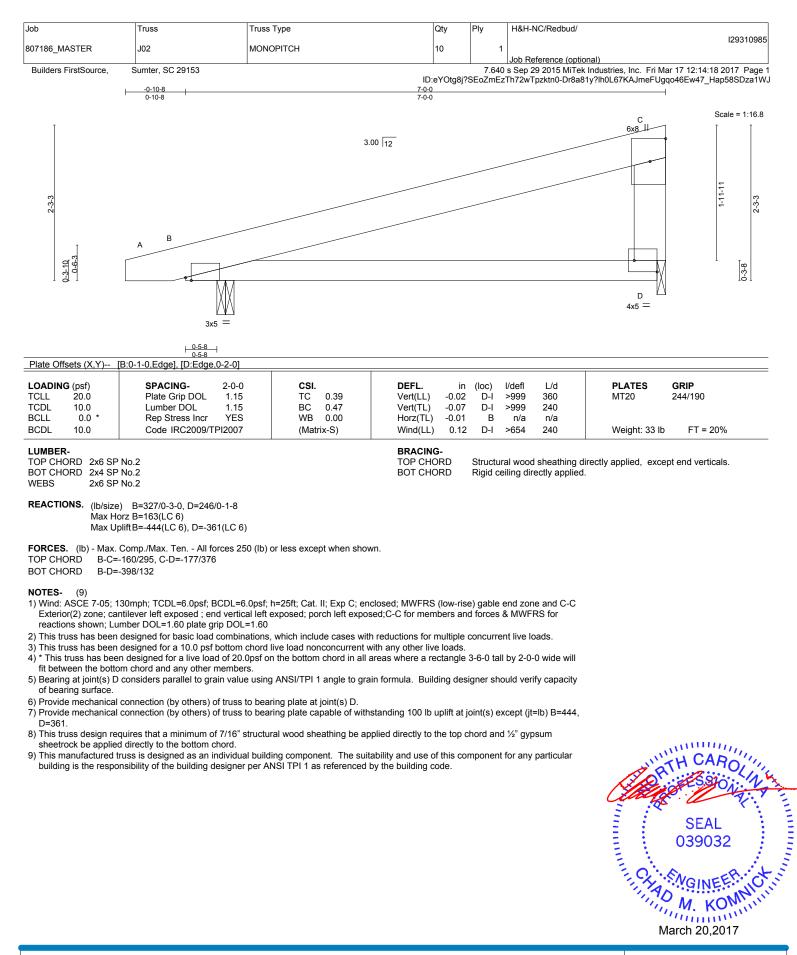
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=347, D=247.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.



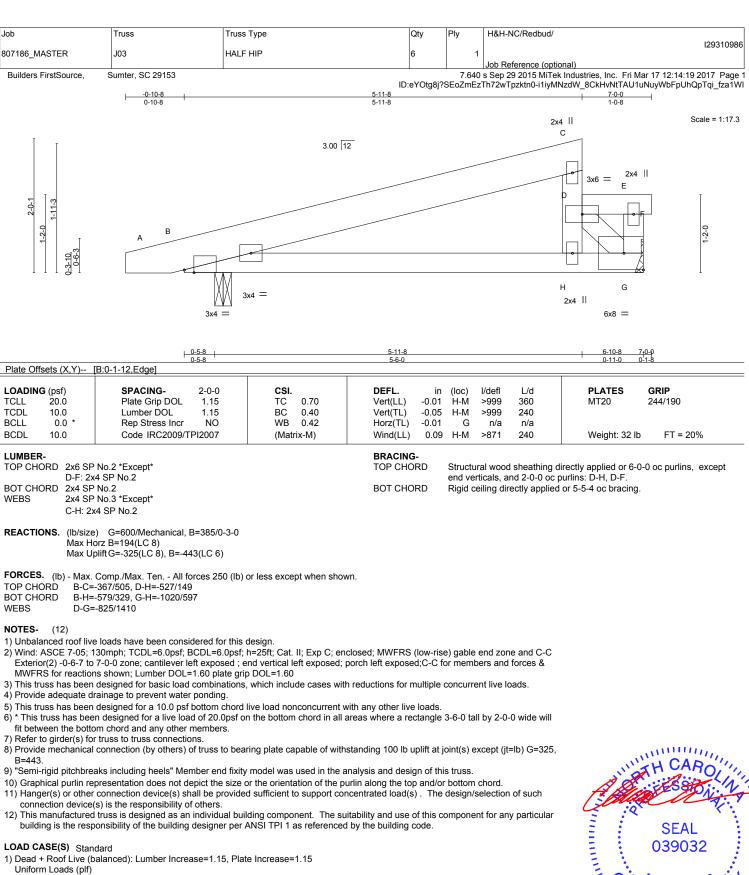
818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



A MITEK Affiliate 818 Soundside Road Edenton, NC 27932

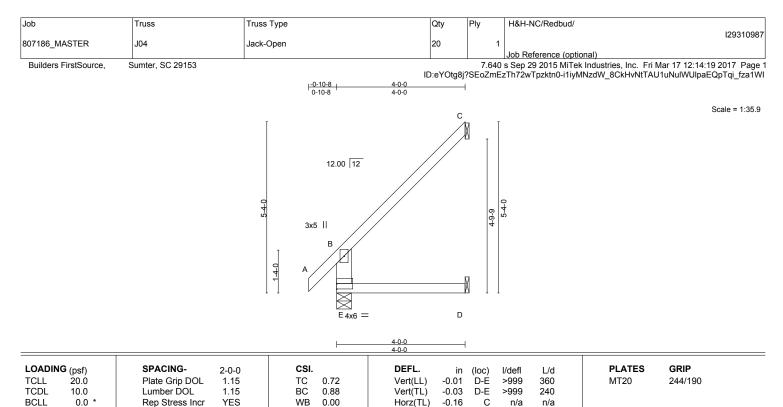
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Vert: A-C=-60, D-E=-60, E-F=-20, G-I=-20 Concentrated Loads (lb) Vert: D=-410

Mananan I. You wanter M. KO MILLININ N March 20,2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



BCDL	10.0	Code IRC2009/TP	12007

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x6 SP No.2

Wind(LL)

0.09

D-F

>523

240

 TOP CHORD
 Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 4-8-4 oc bracing.

Weight: 19 lb

FT = 20%

REACTIONS. (Ib/size) E=224/0-5-8, C=95/Mechanical, D=43/Mechanical Max Horz E=510(LC 8) Max UpliftC=-307(LC 8), D=-91(LC 8) Max Grav E=224(LC 1), C=95(LC 1), D=70(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD \$B-C=-272/52\$

NOTES- (8)

 Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

(Matrix-M)

- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D except (jt=lb) C=307.

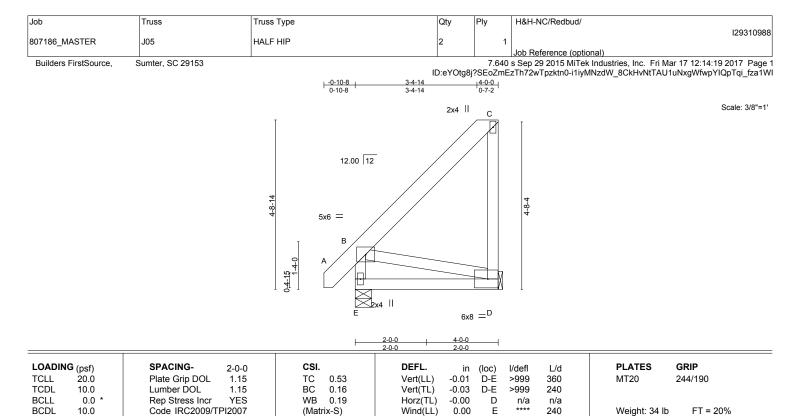
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





LU	MBER-	
----	-------	--

TOP CHORD	2x6 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3 *Except*
	C-D: 2x4 SP No.2

BRACING-

TOP CHORD BOT CHORD

REACTIONS. (lb/size) D=142/Mechanical, E=209/0-5-8 Max Horz E=430(LC 7) Max UpliftD=-303(LC 7), E=-148(LC 6) Max Grav D=245(LC 6), E=209(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD B-C=-445/216, C-D=-183/451, B-E=-241/266

BOT CHORD D-E=-745/273

WEBS B-D=-151/633

NOTES- (8)

 Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) D=303, E=148.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

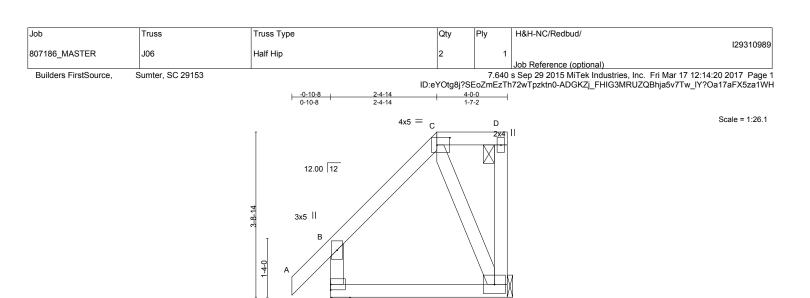


818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoISITPI1 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



=

3x4

4-0-0

E 5x6 =

2-0-0 oc purlins: C-D.

Rigid ceiling directly applied.

Structural wood sheathing directly applied, except end verticals, and

Plate Offsets (X,Y)-- [C:0-3-8,0-2-0]

LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.46	Vert(LL) -0).01 E-F	>999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.25	Vert(TL) -0).02 E-F	>999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.13	Horz(TL) -0	0.00 E	n/a n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0	0.01 E-F	>999 240	Weight: 27 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (Ib/size) F=218/0-5-8, E=140/Mechanical Max Horz F=326(LC 7) Max UpliftF=-154(LC 8), E=-229(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 B-F=-177/310

 BOT CHORD
 E-F=-281/137

 WEBS
 C-E=-112/453

NOTES- (10)

 Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) F=154, E=229.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoISITPI1 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	H&H-NC/Redbud/	1000 4000
807186_MASTER	J07	Half Hip	2	1		12931099
Builders FirstSource,	Sumter, SC 29153			7.640	Job Reference (optic 0 s Sep 29 2015 MiTek	nal) Industries, Inc. Fri Mar 17 12:14:20 2017 Page
		-0-10-8 1-4-14	4-0-	0	h72wTpzktn0-ADGKZj	_FHIG3MRUZQBhja5v9?w0vY0Ba17aFX5za1W
		0-10-8 1-4-14	2-7-	2	·	
	Т	4x5	= c]		Scale = 1:19.9
						
		12.00 12				
		2x6	$\langle \ \ \ \ \ \ \ \ \ \ \ \ \ $			
	2-8-14	в				
	2-6			\mathbf{X}		
	-4-0			//		
	4	A				
					E	
		F			3x5 =	
		2x4				
Diata Offecto (X X)			<u>4-0-0</u> 4-0-0			
Plate Offsets (X,Y)						
LOADING (psf) TCLL 20.0		-0-0 CSI. .15 TC 0.37	DEFL. ir Vert(LL) -0.01		l/defl L/d >999 360	PLATES GRIP MT20 244/190
TCDL 10.0 BCLL 0.0 *		.15 BC 0.11 YES WB 0.08	Vert(TL) -0.02 Horz(TL) -0.00		>999 240 n/a n/a	
BCDL 10.0	Code IRC2009/TPI20		Wind(LL) 0.0		>999 240	Weight: 24 lb FT = 20%
LUMBER-			BRACING-	.		
TOP CHORD 2x4 SP BOT CHORD 2x4 SP			TOP CHORD		ral wood sheathing d ticals, and 2-0-0 oc p	rectly applied or 4-0-0 oc purlins, except urlins: C-D.
WEBS 2x4 SP	No.3		BOT CHORD	Rigid ce	eiling directly applied	or 10-0-0 oc bracing.
REACTIONS. (Ib/size	e) F=218/0-5-8, E=140/Me	chanical				
	orz F=241(LC 7) pliftF=-175(LC 8), E=-179(L	C 7)				
FORCES. (Ib) - Max.	Comp./Max. Ten All force	s 250 (lb) or less except when s	nown.			
	-177/332					
BOT CHORD E-F=	-41/281					
BOT CHORD E-F=	-41/281					

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) F=175, E=179.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

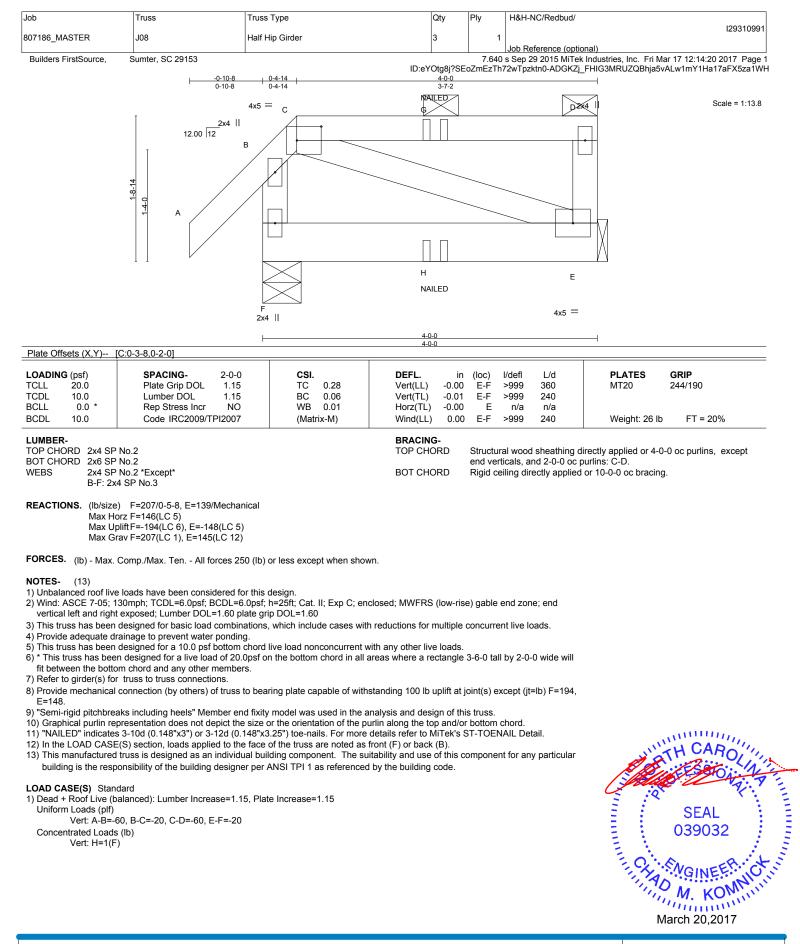
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

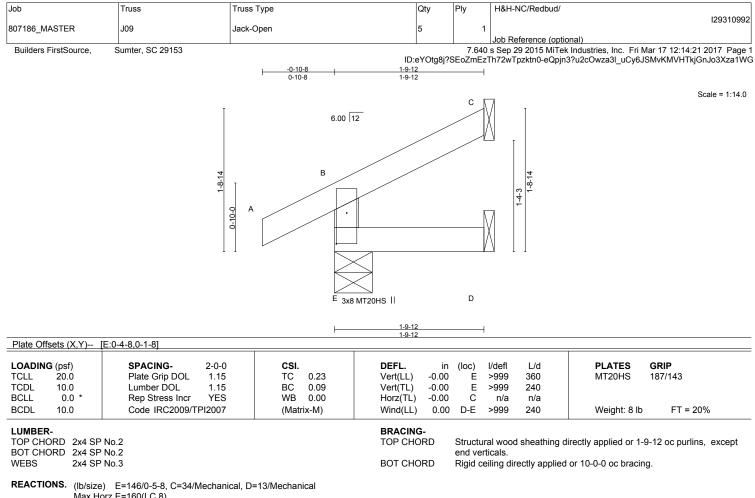


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.









Max Horz E=160(LC 8) Max UpliftE=-137(LC 8), C=-69(LC 8), D=-10(LC 8) Max Grav E=146(LC 1), C=34(LC 1), D=30(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

- Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) C, D except (jt=lb) E=137.

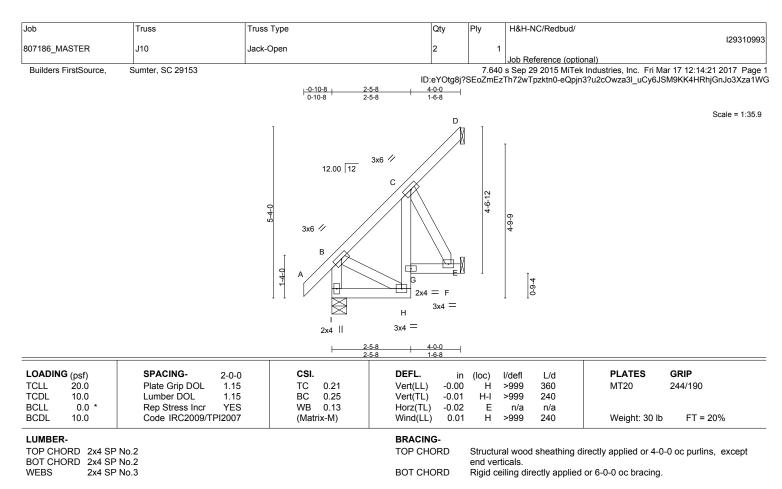
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





REACTIONS. (Ib/size) I=221/0-5-8, D=41/Mechanical, E=102/Mechanical Max Horz I=503(LC 8) Max Uplift D=-118(LC 8), E=-277(LC 8)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 BOT CHORD
 H-I=-499/17

 WEBS
 B-H=0/380, C-F=-114/443

NOTES- (8)

- Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) D=118, E=277.

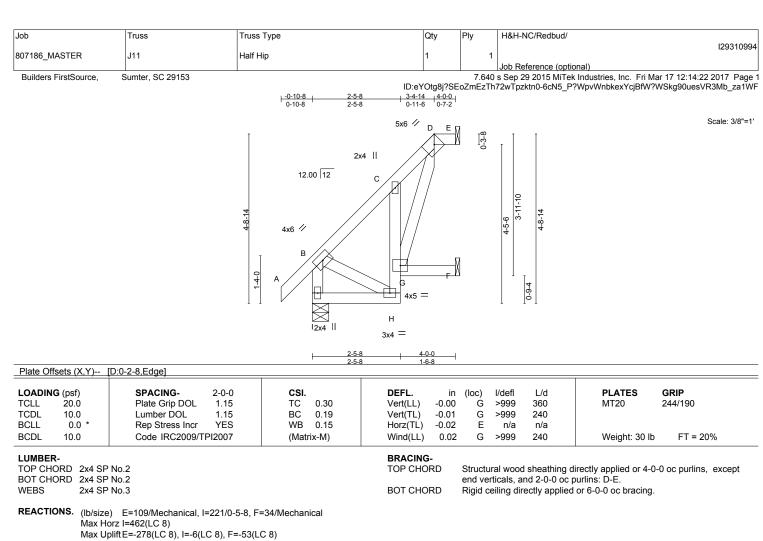
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoISITPI1 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Max Grav E=109(LC 1), I=221(LC 1), F=41(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD C-D=-127/275

BOT CHORD H-I=-409/42, C-G=-144/461

WEBS B-H=-13/258, D-G=-537/148

NOTES-(11)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) I, F except (jt=lb) E=278

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

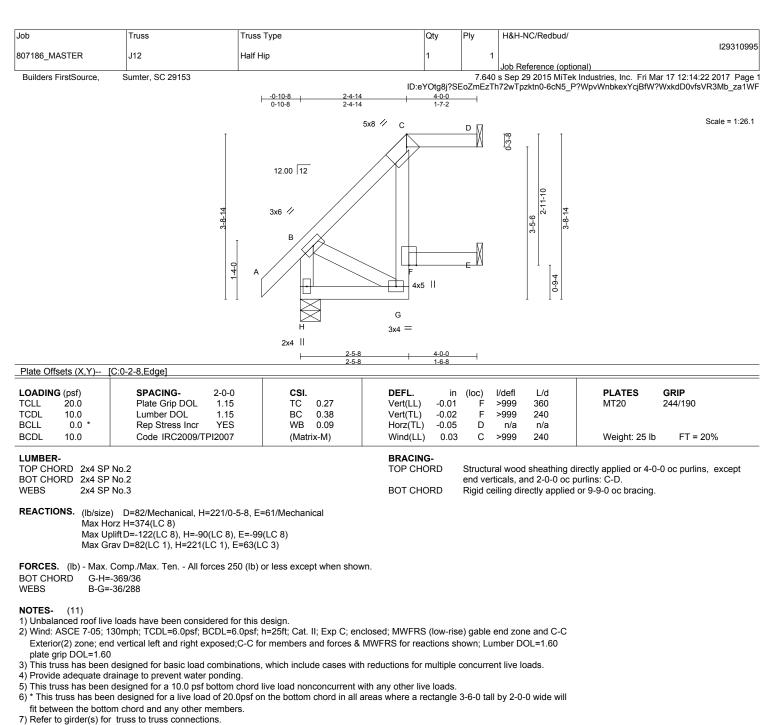
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





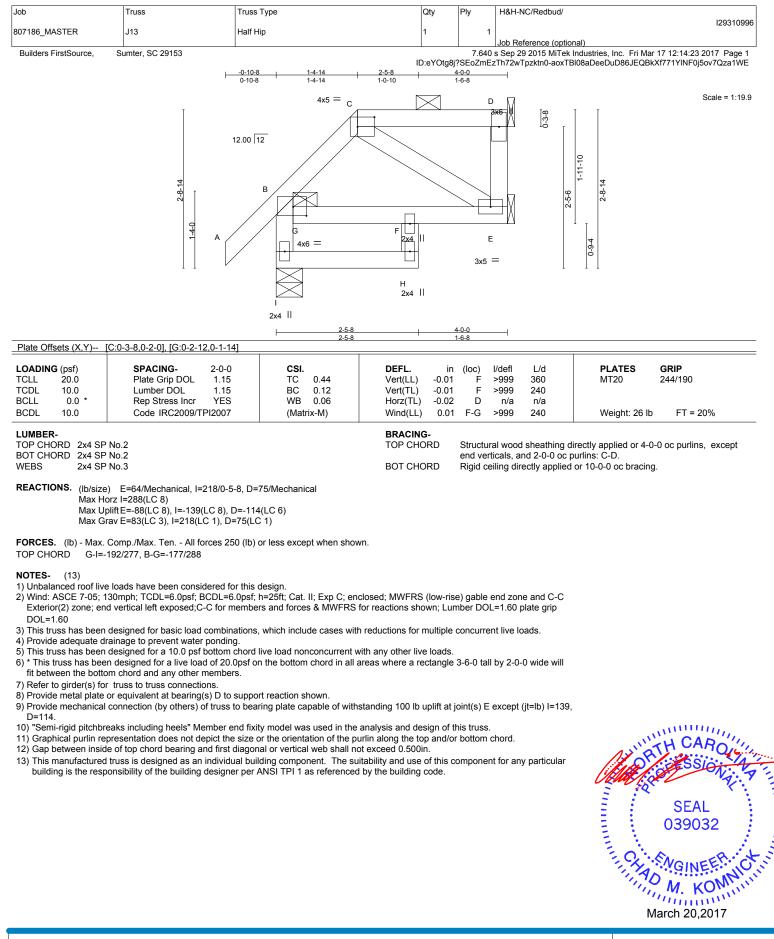
Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) H, E except (jt=lb) D=122.

- 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



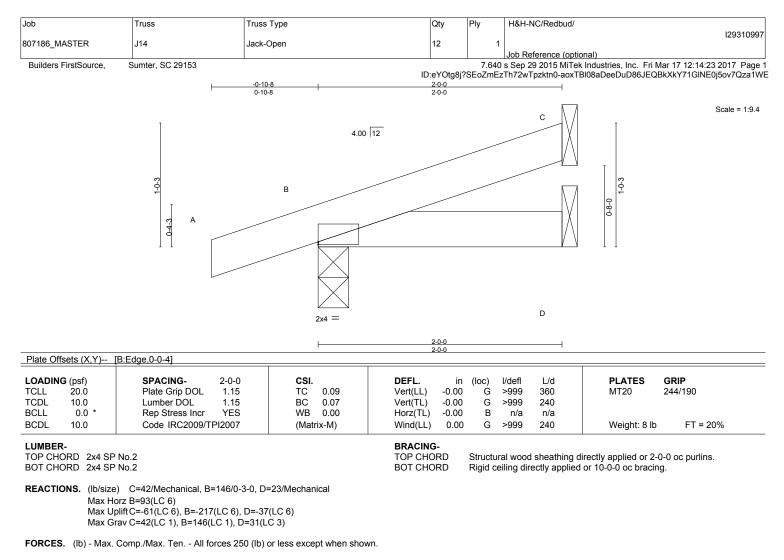
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoISITPI1 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TREERING BY A MI Tek Affiliate 818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



NOTES- (8

- Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

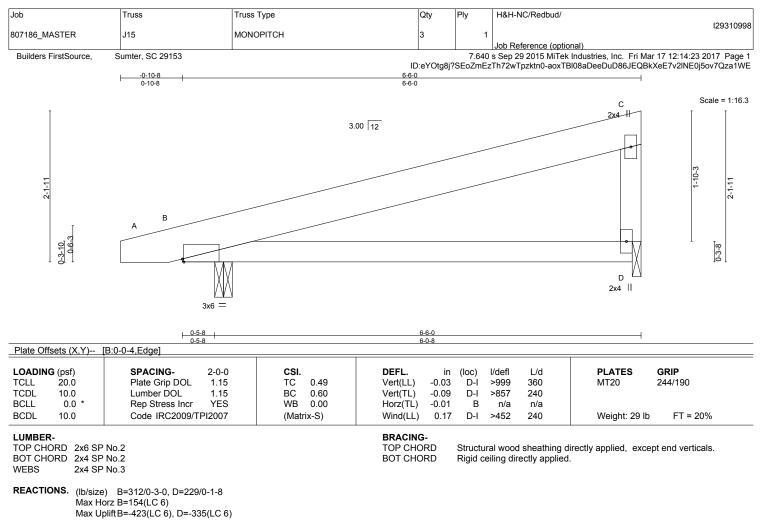
5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) C, D except (jt=lb) B=217.

- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD C-D=-175/408 C-D=-175/408

NOTES-

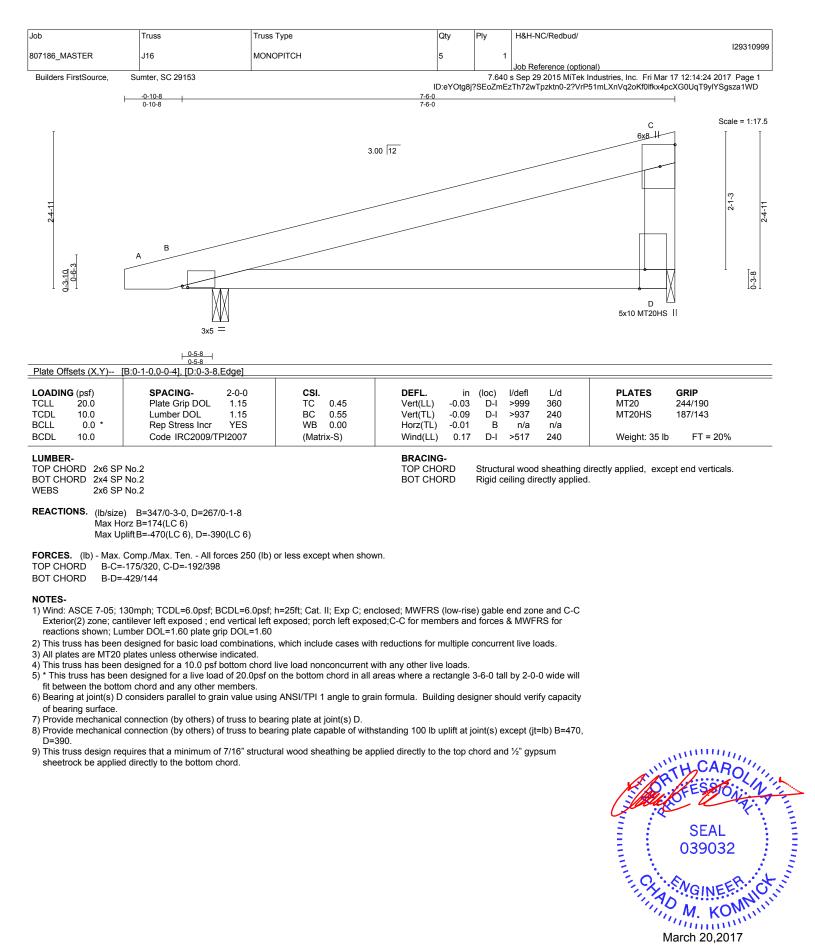
 Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) D considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) D.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=423, D=335.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

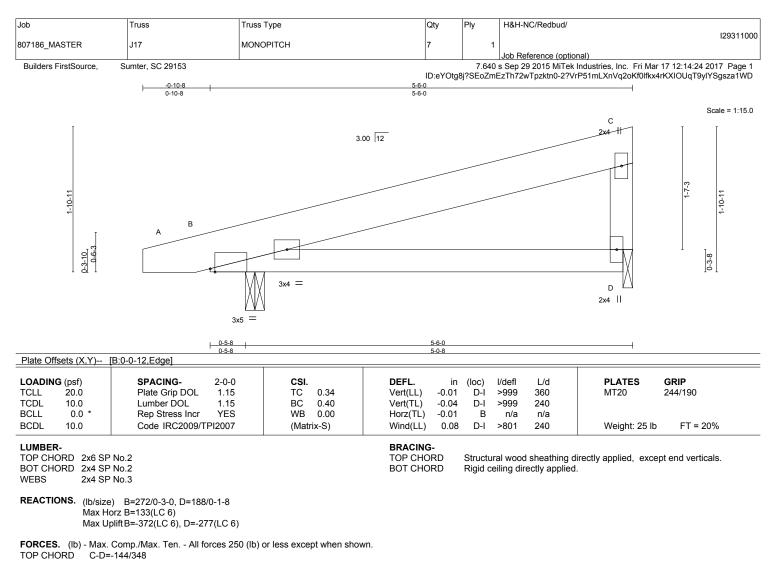










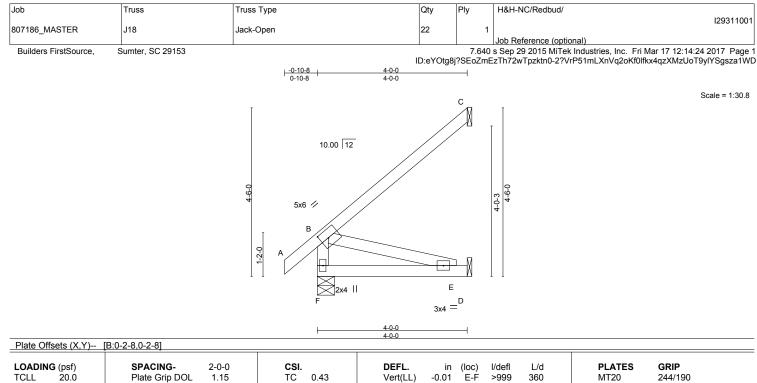


NOTES-

- Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) D considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) D.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=372, D=277.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.







LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.43	Vert(LL) -0.01 E-F >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.17	Vert(TL) -0.03 E-F >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.13	Horz(TL) -0.01 C n/a n/a	
BCDL 10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.01 E-F >999 240	Weight: 22 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

WEBS 2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied, except end verticals.
 Rigid ceiling directly applied.

REACTIONS. (Ib/size) F=221/0-5-8, C=106/Mechanical, D=38/Mechanical Max Horz F=420(LC 8) Max UpliftF=-43(LC 8), C=-230(LC 8), D=-77(LC 8) Max Grav F=221(LC 1), C=106(LC 1), D=76(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. BOT CHORD E-F=-417/15

WEBS B-E=-15/431

NOTES-

 Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

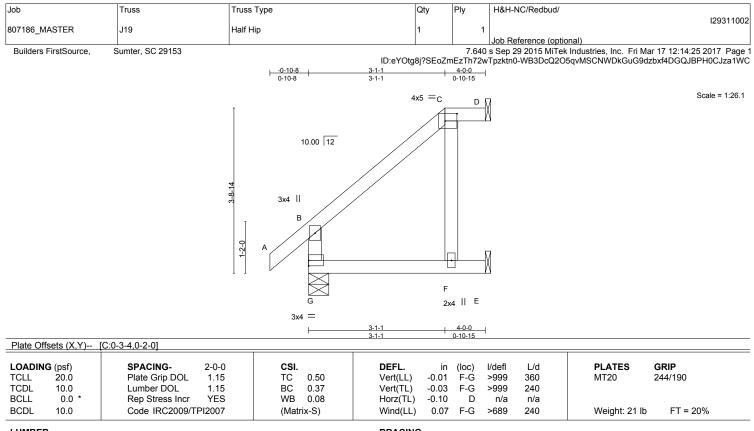
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) F, D except (jt=lb) C=230.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



LUWDER-	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2

```
WEBS 2x4 SP No.3
```

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins: C-D. Rigid ceiling directly applied.

REACTIONS. (lb/size) D=46/Mechanical, G=221/0-5-8, E=98/Mechanical Max Horz G=362(LC 8) Max Uplift D=-40(LC 6), G=-98(LC 8), E=-207(LC 8) Max Grav D=48(LC 14), G=221(LC 1), E=98(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS C-F=-75/282

NOTES-

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

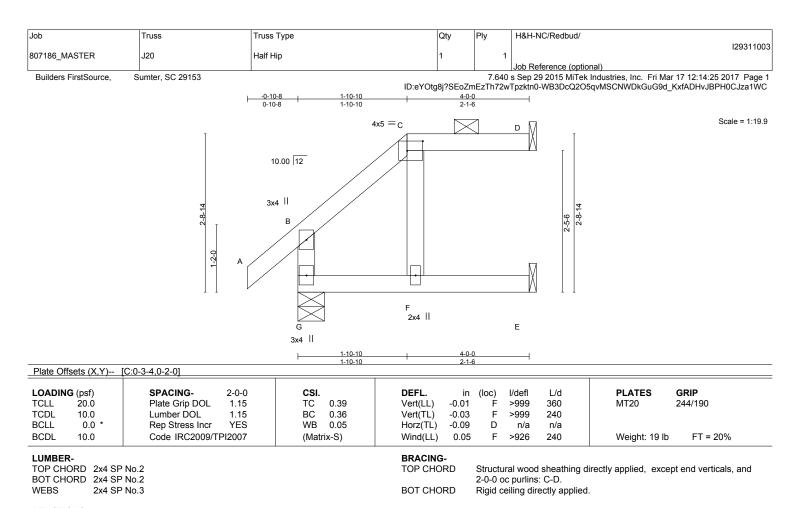
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D, G except (jt=lb) E=207.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







REACTIONS. (Ib/size) D=84/Mechanical, G=221/0-5-8, E=59/Mechanical Max Horz G=275(LC 8) Max Uplift D=-101(LC 7), G=-154(LC 8), E=-57(LC 8) Max Grav D=84(LC 1), G=221(LC 1), E=66(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

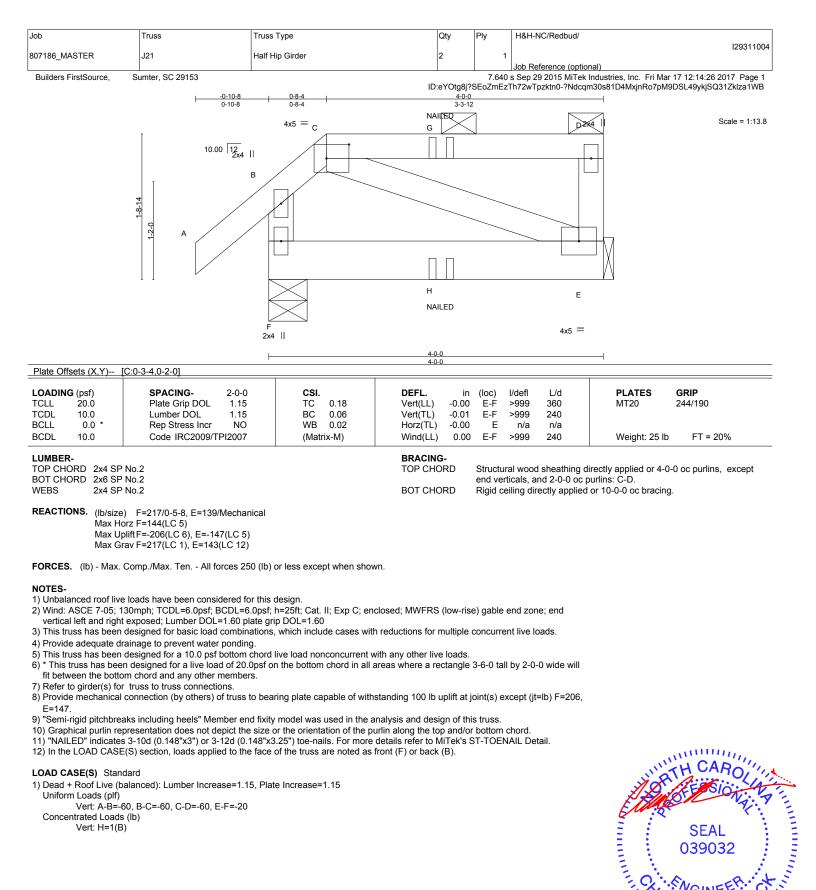
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) E except (jt=lb) D=101, G=154.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



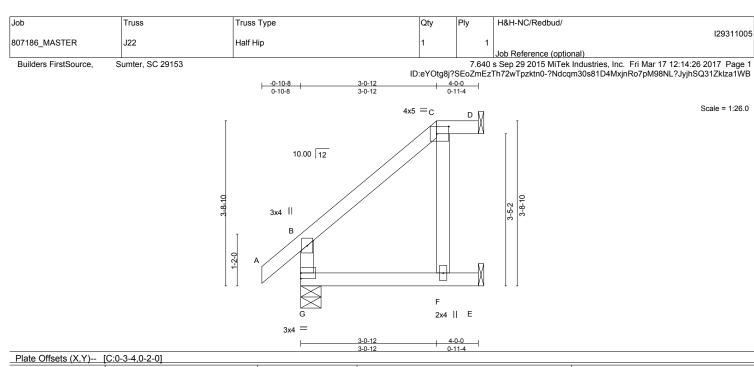




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

March 20,2017

818 Soundside Road Edenton, NC 27932



LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl L/d	PLATES GRIP MT20 244/190
TCLL 20.0	Plate Grip DOL 1.15	TC 0.50	Vert(LL) -0.01	F-G	>999 360	
TCDL 10.0	Lumber DOL 1.15	BC 0.37	Vert(TL) -0.03	F-G	>999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.08	Horz(TL) -0.10	D	n/a n/a	
BCDL 10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.07	F-G	>690 240	Weight: 21 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins: C-D.

 BOT CHORD
 Rigid ceiling directly applied.

REACTIONS. (Ib/size) D=48/Mechanical, G=221/0-5-8, E=96/Mechanical Max Horz G=360(LC 8) Max Uplift D=-41(LC 6), G=-100(LC 8), E=-200(LC 8) Max Grav D=50(LC 14), G=221(LC 1), E=96(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS C-F=-73/276

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

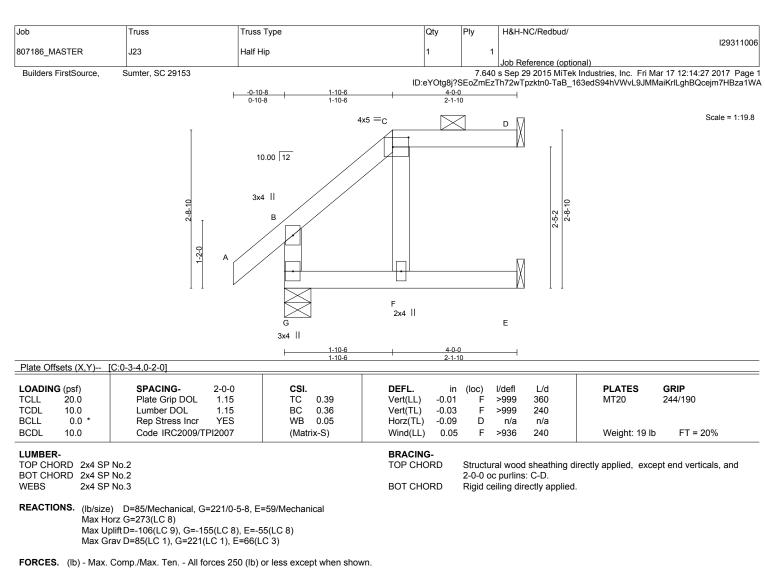
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D, G except (jt=lb) E=200.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







NOTES

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

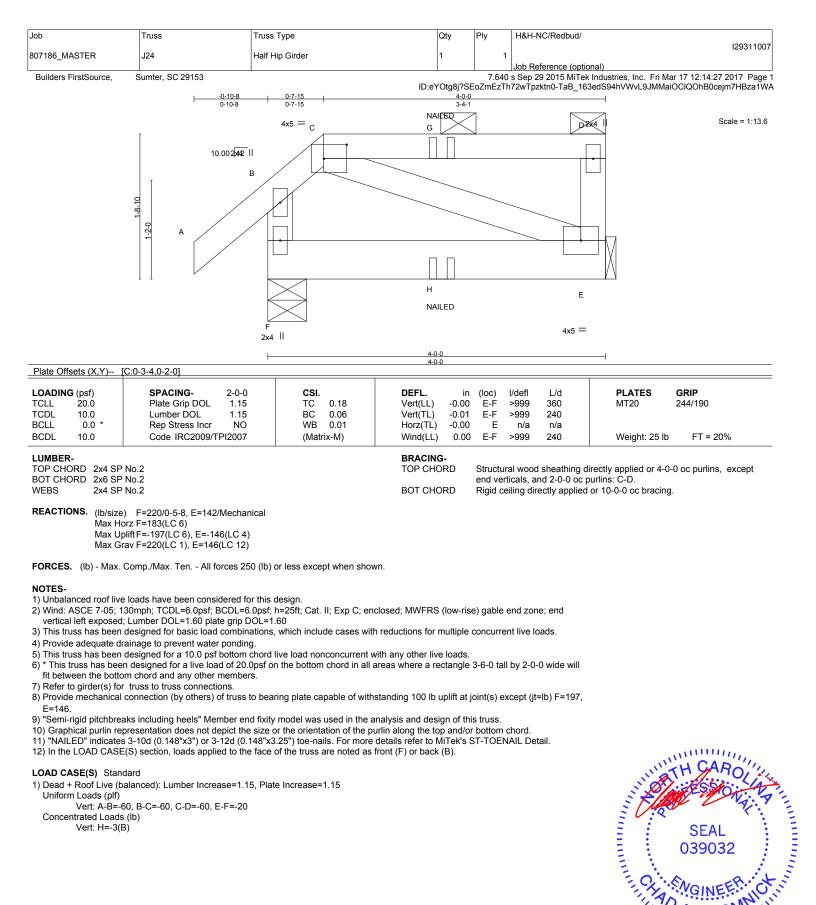
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) E except (jt=lb) D=106. G=155.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

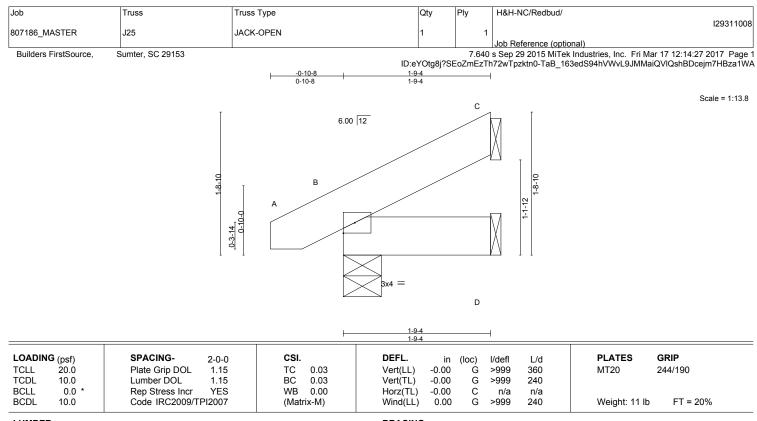






WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. ENGINEERING BY EREENCO A MITEK Atfillate B18 Soundside Road Edenton, NC 27932

March 20,2017



LUMBER-

TOP CHORD2x6 SP No.2BOT CHORD2x6 SP No.2

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 1-9-4 oc purlins.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) C=42/Mechanical, B=119/0-5-8, D=20/Mechanical Max Horz B=118(LC 8) Max UpliftC=-71(LC 8), B=-107(LC 8), D=-8(LC 8) Max Grav C=42(LC 1), B=119(LC 1), D=32(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will

fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

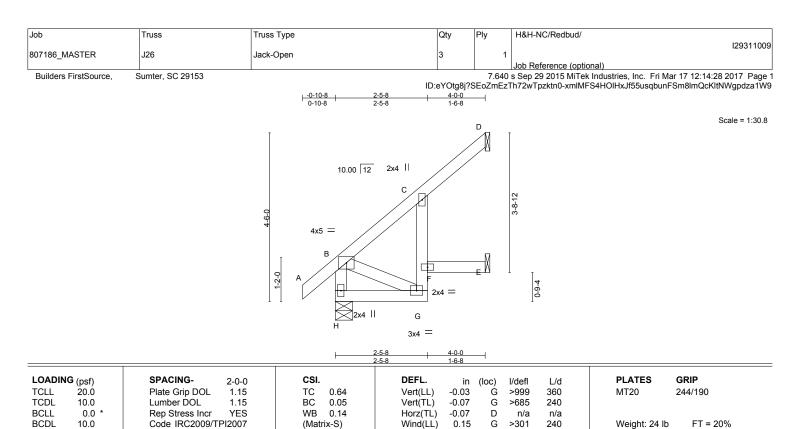
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) C, D except (jt=lb)

B=107.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.







н	IIMBER-	

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS

2x4 SP No.3

REACTIONS. (lb/size) H=221/0-5-8, D=127/Mechanical, E=16/Mechanical Max Horz H=420(LC 8) Max Uplift H=-43(LC 8), D=-317(LC 8) Max Grav H=221(LC 1), D=127(LC 1), E=33(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

BOT CHORD G-H=-417/15 B-G=-17/461 WEBS

NOTES-

1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) H except (jt=lb) D=317

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.



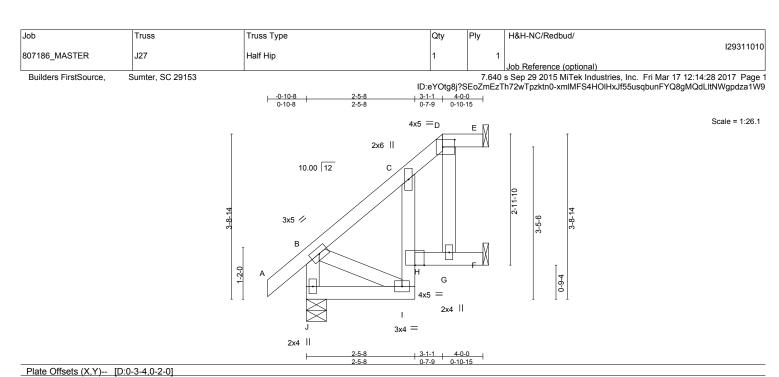
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.



LOADING (psf) SPACING-2-0-0 CSI DEFL in (loc) l/defl L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1.15 тс 0.21 Vert(LL) -0.01 н >999 360 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 BC 0.33 Vert(TL) -0.02 Н >999 240 BCLL 0.0 Rep Stress Incr YES WB 0.07 Horz(TL) -0.04 E n/a n/a BCDL 10.0 Code IRC2009/TPI2007 (Matrix-S) Wind(LL) 0.04 н >999 240 Weight: 27 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

2-0-0 oc purlins: D-E. Rigid ceiling directly applied.

Structural wood sheathing directly applied, except end verticals, and

REACTIONS. (Ib/size) E=51/Mechanical, J=221/0-5-8, F=92/Mechanical Max Horz J=362(LC 8) Max UpliftE=-68(LC 8), J=-98(LC 8), F=-162(LC 8) Max Grav E=51(LC 1), J=221(LC 1), F=92(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. BOT CHORD I-J=-349/40

NOTES-

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

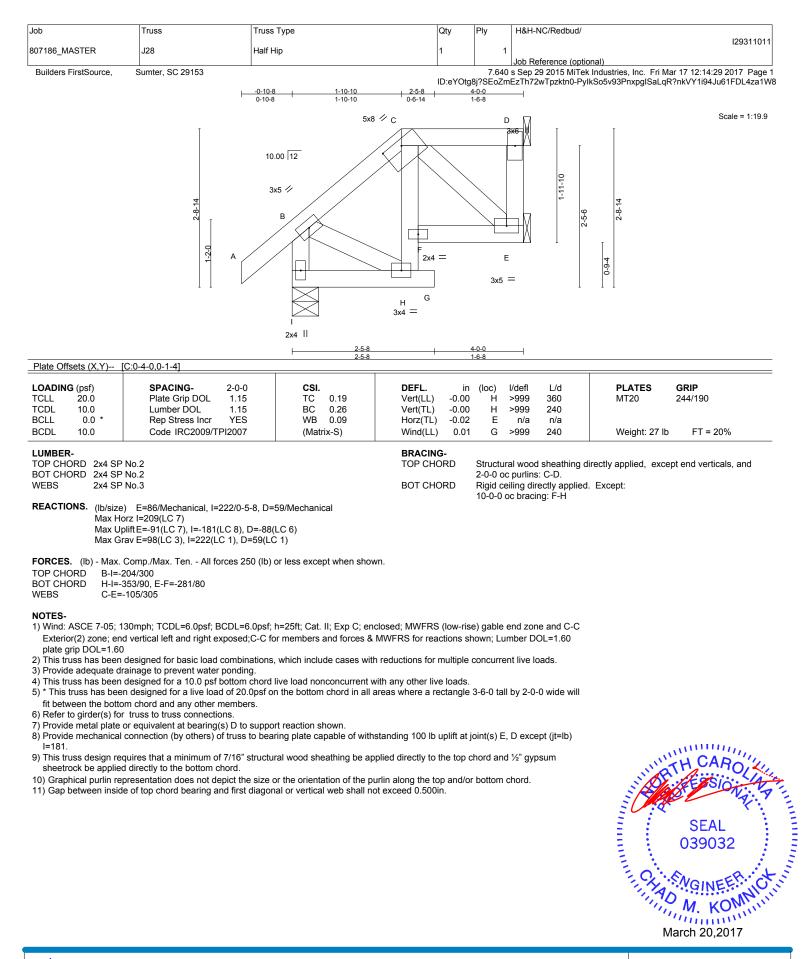
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) E, J except (jt=lb) F=162.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

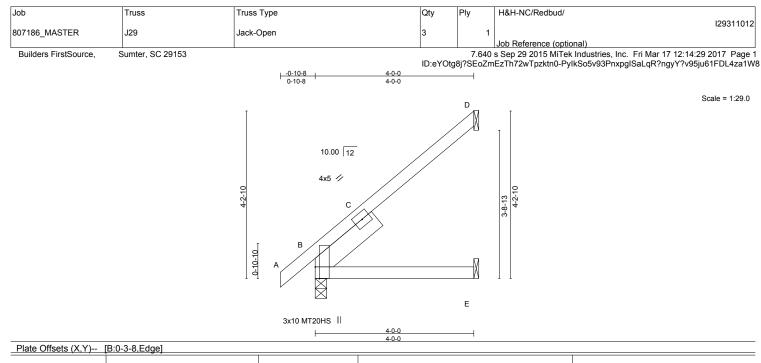
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







r. 10/03/2015 BEFORE USE. Iding component, not design into the overall y and permanent bracing regarding the 89 and BCSI Building Component 818 Soundside Road Edenton, NC 27932



LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.42	Vert(LL)	-0.01	E-H	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.44	Vert(TL)	-0.03	E-H	>999	240	MT20HS	187/143
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL)	-0.03	D	n/a	n/a		
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL)	0.05	E-H	>861	240	Weight: 20 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

SLIDER Left 2x6 SP No.2 1-11-12

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied. Rigid ceiling directly applied.

REACTIONS. (Ib/size) D=101/Mechanical, B=216/0-3-8, E=50/Mechanical Max Horz B=374(LC 8) Max UpliftD=-236(LC 8), B=-67(LC 8), E=-40(LC 8) Max Grav D=101(LC 1), B=216(LC 1), E=71(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) All plates are MT20 plates unless otherwise indicated.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

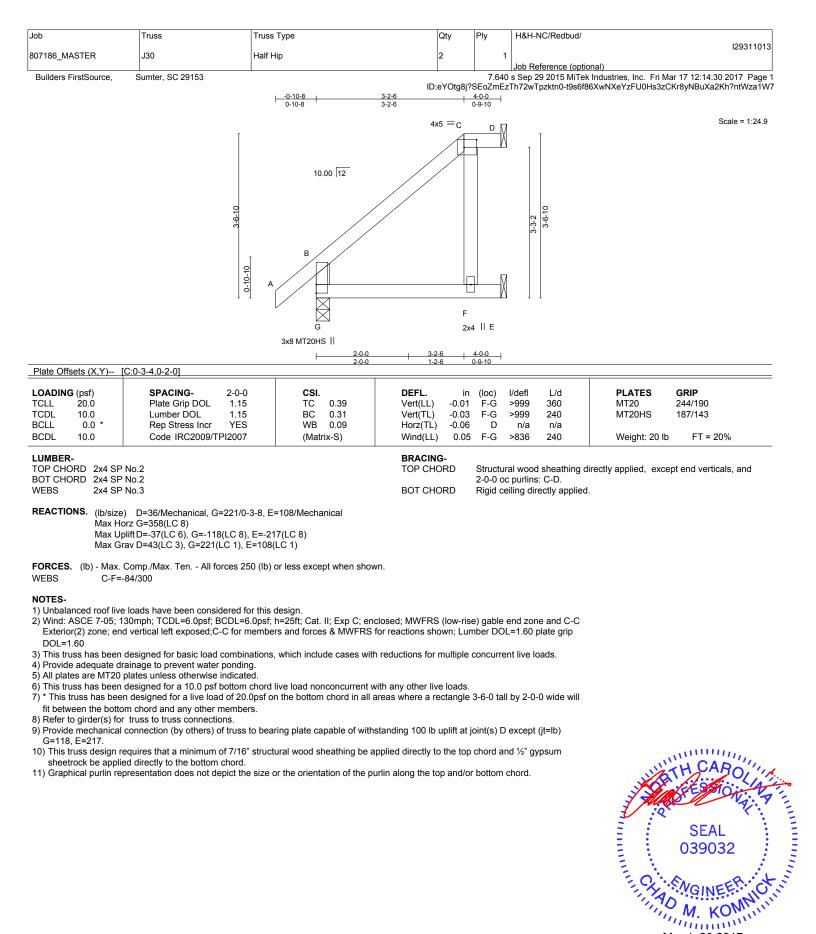
6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) B, E except (jt=lb) D=236.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.



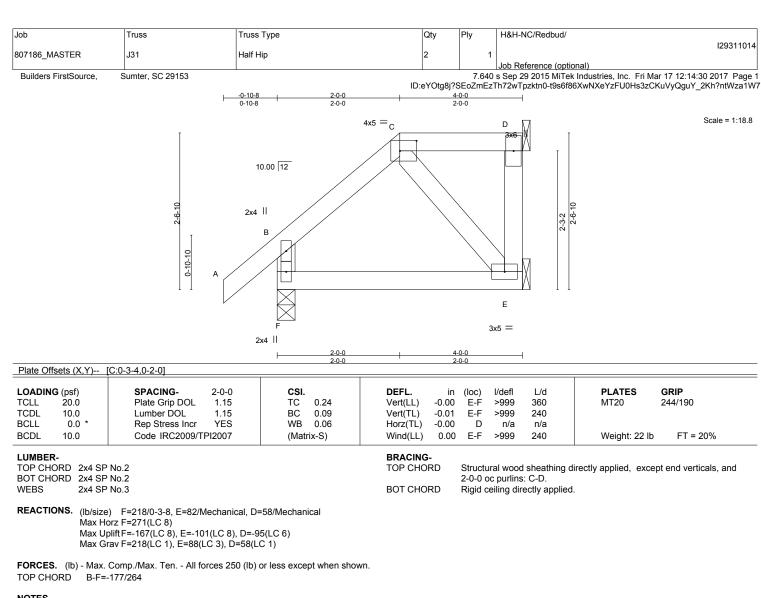




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



March 20,2017



- NOTES-
- 1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

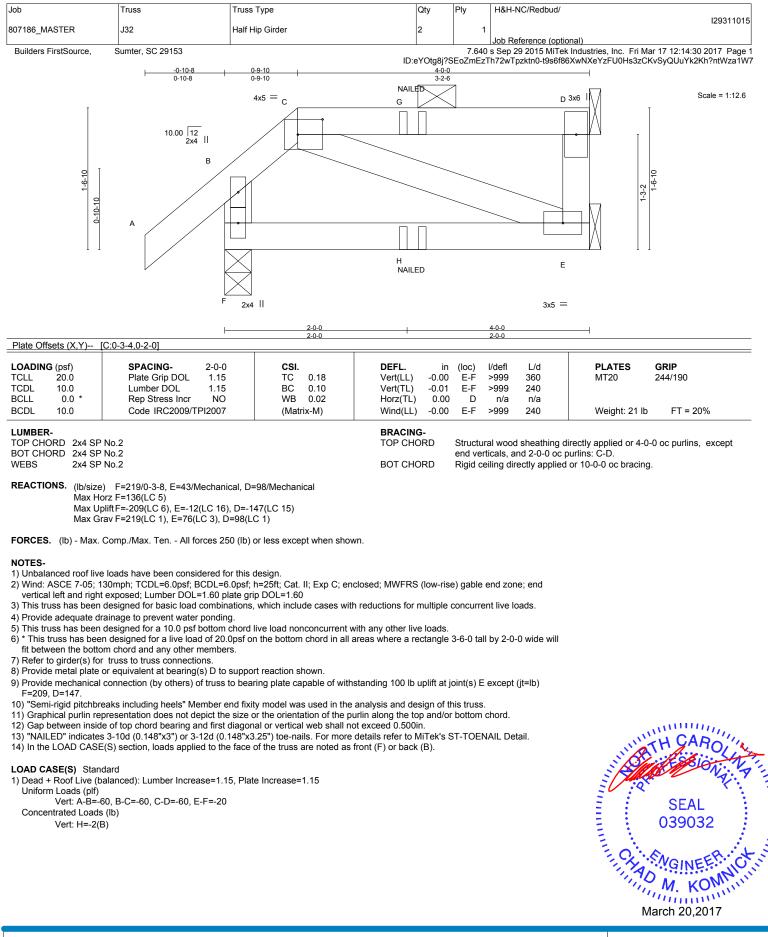
6) Refer to girder(s) for truss to truss connections.

- 7) Provide metal plate or equivalent at bearing(s) D to support reaction shown.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D except (jt=lb) F=167. E=101.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 11) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

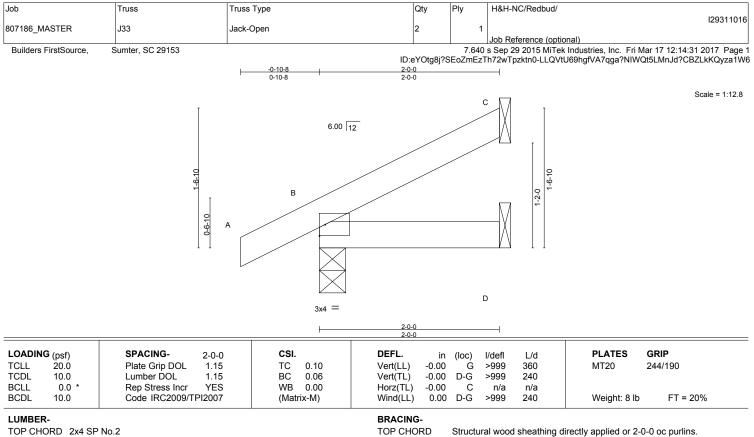


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





AMITER Affiliate B18 Soundside Road Edenton, NC 27932



BOT CHORD 2x4 SP No.2

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (Ib/size) C=48/Mechanical, B=146/0-3-8, D=20/Mechanical Max Horz B=141(LC 8) Max Uplift C=-75(LC 8), B=-148(LC 8) Max Grav C=48(LC 1), B=146(LC 1), D=35(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will

fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) C except (jt=lb)

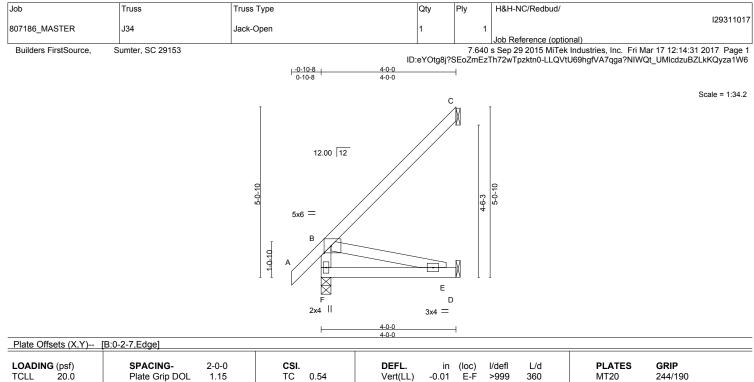
B=148.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.54	Vert(LL) -0.01 E-F	>999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.17	Vert(TL) -0.03 E-F	- >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.15	Horz(TL) -0.01 (C n/a n/a	
BCDL 10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.01 E-F	- >999 240	Weight: 23 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied, except end verticals.
 Rigid ceiling directly applied.

REACTIONS. (Ib/size) F=221/0-3-8, C=106/Mechanical, D=38/Mechanical Max Horz F=491(LC 8) Max UpliftC=-275(LC 8), D=-83(LC 8) Max Grav F=221(LC 1), C=106(LC 1), D=76(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

 TOP CHORD
 B-C=-258/59

 BOT CHORD
 E-F=-487/11

 WEBS
 B-E=-12/500

NOTES-

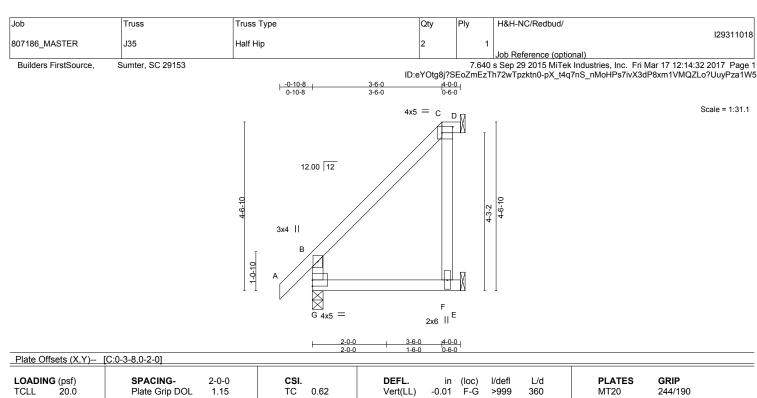
- Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D except (jt=lb) C=275.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.







LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.62	Vert(LL) -0.01 F-G >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.45	Vert(TL) -0.03 F-G >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.19	Horz(TL) -0.10 D n/a n/a	
BCDL 10.0	Code IRC2009/TPI2007	(Matrix-M)	Wind(LL) 0.08 F-G >598 240	Weight: 23 lb FT = 20%
			· · · · · · · · · · · · · · · · · · ·	

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

WEBS 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: C-D. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (Ib/size) D=-32/Mechanical, G=221/0-3-8, E=175/Mechanical Max Horz G=458(LC 8) Max UpliftD=-56(LC 6), G=-30(LC 8), E=-529(LC 8) Max Grav D=221(LC 8), G=221(LC 1), E=175(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS C-F=-149/624

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

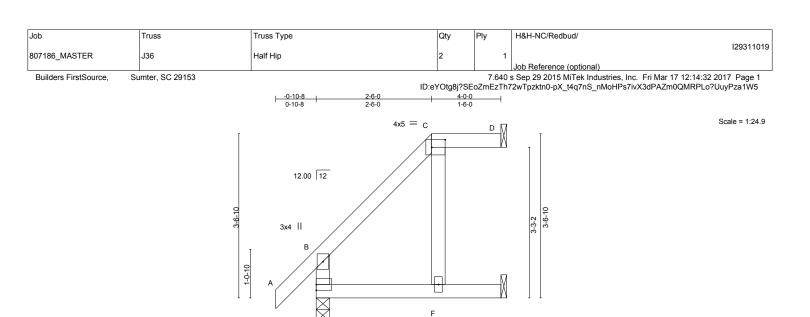
Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D, G except (jt=lb) E=529.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







LUMBER-TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

20.0

10.0

10.0

0.0 *

LOADING (psf)

TCLL

TCDL

BCLL

BCDL

Plate Offsets (X,Y)-- [C:0-3-8,0-2-0]

2x4 SP No.3 WEBS

BRACING-TOP CHORD

DEFL

Vert(LL)

Vert(TL)

Horz(TL)

Wind(LL)

2x4 ||

4-0-0

1-6-0

in (loc)

-0.01

-0.03

-0.12

0.07

Е

F >999

D

F-G

F-G

l/defl

>999

>658

n/a

L/d

360

240

n/a

240

Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: C-D.

PLATES

Weight: 21 lb

MT20

GRIP

244/190

FT = 20%

BOT CHORD

0.46

0.45

0.07

(Matrix-M)

2-6-0

2-6-0

G

3x4 =

CSI

тс

BC

WB

Rigid ceiling directly applied or 6-0-0 oc bracing

REACTIONS. (lb/size) D=70/Mechanical, G=221/0-3-8, E=74/Mechanical Max Horz G=371(LC 8) Max Uplift D=-84(LC 9), G=-110(LC 8), E=-127(LC 8)

SPACING-

Plate Grip DOL

Rep Stress Incr

Code IRC2009/TPI2007

Lumber DOL

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

2-0-0

1.15

1.15

YES

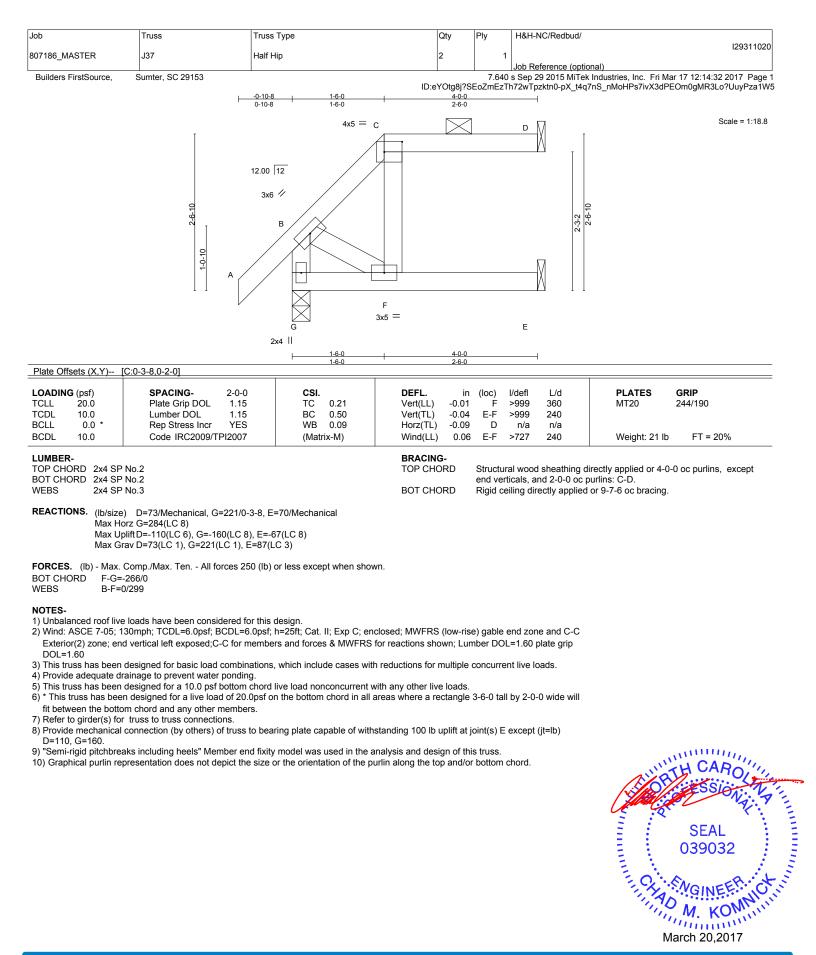
NOTES-

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOI = 1.60
- 3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D except (jt=lb) G=110 F=127
- 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

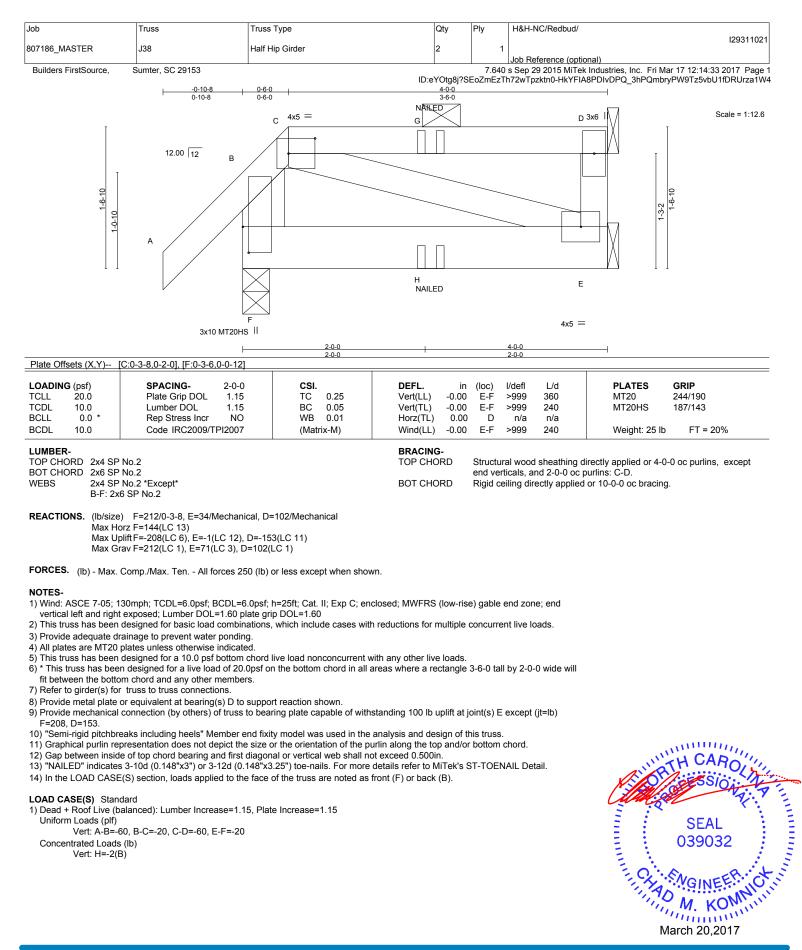


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.











Job	Truss Truss	уре	Qty	Ply H&H-NC/Redbud/	
807186_MASTER	K01 ATTIC		1	1 Job Reference (or	129311022 ptional)
Builders FirstSource, Sumter	, SC 29153	-			MTek Industries, Inc. Mon Mar 20 09:17:07 2017 Page 1 UD9j0yscBJFHQ_BCCOcmNDnMa6fOKxxzZ5ig
	-0 <u>-10-</u> 0-10-i				
			4x6 = 0.11-9		Scale = 1:77.4
			E		
			2x4 2x4	I	
		10.00 12 [P F F	_	
		C dl		G	
	<u>-</u>	2x4	- = T		
	₩ 5x6 =		24	5x6 =	
		3 -6-6	8 F 7-9-0 6	Н	
	04-9 3-6-12				-12 0 4 9
	e S				
		т S	Q O L	K J	1 I 4
			HS =2x4 2x4 5x10		
			4x5 = 3x6 =		
			$\begin{array}{c} 4x5 \equiv \\ 3-9-8 & 10-9-8 & 13-10-12 \\ 3-1-4 & 2-0-0 & 3-1-4 \end{array}$	19-7-0 5-8-4	
Plate Offsets (X,Y)	[<u>B:0-3-0,0-1-8], [E:0-3-0,Edge], [H:0-3-0</u>			J=0=4	
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.61	DEFL. in Vert(LL) -0.09		PLATES GRIP MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.59	Vert(TL) -0.18	M-N >999 240	MT20HS 187/143
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2009/TPI2007	WB 0.66 (Matrix-S)	Horz(TL) 0.03 Wind(LL) 0.26		Weight: 196 lb FT = 20%
LUMBER- TOP CHORD 2x6 SP			BRACING- TOP CHORD	Structurel wood aboathing	
U-V,W-	X: 2x4 SP No.2		BOT CHORD	Rigid ceiling directly applie	g directly applied, except end verticals. ed. Except:
	No.3 *Except*			5-8-0 oc bracing: M-R	
	S: 2x6 SP No.2, D-F: 2x4 SP No.2	(0.5.9) (min $(0.1.9)$			
Max H	e) T=1059/0-5-8 (min. 0-1-8), J=1059 prz T=-843(LC 6)	0-5-8 (min. 0-1-8)			
	olift T=-386(LC 8), J=-386(LC 9) rav T=1200(LC 2), J=1200(LC 2)				
	Comp./Max. Ten All forces 250 (lb) or				
H-J=-	-972/405, C-D=-661/540, F-G=-661/542 1152/541				
N-P=	804/821, Q-S=0/1627, O-Q=0/1627, L-0 1062/0, M-N=-764/837				
	-37/343, C-R=-37/343, D-F=-872/805, E -1180/539	I-S=-158/715, H-K=-163/	715, P-S=-1180/585,		
NOTES-					
2) Wind: ASCE 7-05; 1	loads have been considered for this de 30mph; TCDL=6.0psf; BCDL=6.0psf; h=	25ft; Cat. II; Exp C; enclo			
DOL=1.60 plate grip				,	
	designed for basic load combinations, w plates unless otherwise indicated.	hich include cases with r	eductions for multiple cor	ncurrent live loads.	RTHOROGIC
	Γ20 unless otherwise indicated. designed for a 10.0 psf bottom chord liv	e load nonconcurrent with	h any other live loads.		Ny 7
	n designed for a live load of 20.0psf on t ottom chord and any other members.	he bottom chord in all are	eas where a rectangle 3-6	6-0 tall by 2-0-0 wide	SEAL SEAL
9) Bottom chord live loa	0 psf) on member(s). C-D, F-G, D-F; W ad (40.0 psf) and additional bottom chor	d dead load (5.0 psf) app	lied only to room. P-R, N		SEAL 039032
10) Provide mechanica joint J.	I connection (by others) of truss to bear	ng plate capable of withs	tanding 386 lb uplift at jo	int T and 386 lb uplift at	
11) This truss design re	equires that a minimum of 7/16" structured directly to the bottom chord.	al wood sheathing be app	blied directly to the top ch	ord and 1/2" gypsum	I OF NGINEER OT
12) Attic room checked					
LOAD CASE(S) Stand	lard				March 20 2017

LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

ENGINEERING BY RENCO 818 Soundside Road Edenton, NC 27932

March 20,2017

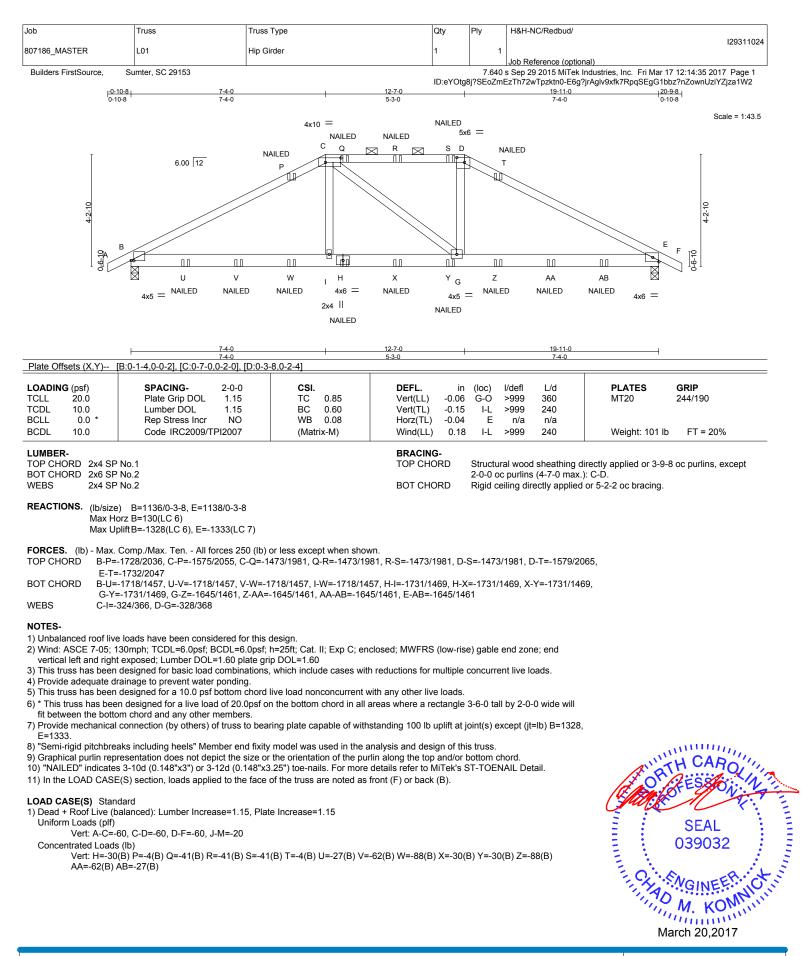
Job	Truss	Туре	Qty Ply	/ H&H-NC/Redbud/	129311023
807186_MASTER	K02 ATTIC		6	1 Job Reference (optio	nal)
Builders FirstSource, Sumter	, SC 29153		ID:eYOtg8j?SE		k Industries, Inc. Mon Mar 20 09:20:17 2017 Page 1 PsigTXf3k38iS9izhF4gSCdqkiPXoDQNcwzZ5fi
	-0 <u>-10</u> 0-10	- <u>8 5-8-4 6-10-</u> -8 5-8-4 1-2-4	B 9-9-8 11-8-15 13-10-12 1-11-7 1-11-7 1-2-4	<u>19-7-0</u> 2ρ-5-8 5-8-4 0-10-8	
			4x6 = 0.11-9		Scale = 1:77.4
	Ţ		E		
			2x4 2x4 F		
				C C	
		c		G	
		2x4	 =2x4	R. A.	
	다. 성 단 5x6 =	в	8-2-4	5x6 =	
	A	B to the second	·····································	H	თ
	04.9 3.6-12			3-6-12	640
	, w				1-2-0
		T S	Q O ^L K	J	
		5x10 MT20	HS =2x4 2x4 5x10 MT2	20HS =	
			4x5 = 3x6 =		
			$\begin{array}{c} 4x5 \equiv \\ 3-9-8 + 10-9-8 + 13-10-12 \\ 3-1-4 + 2-0-0 + 3-1-4 \end{array}$	19-7-0 5-8-4	
Plate Offsets (X,Y)	[B:0-3-0,0-1-8], [E:0-3-0,Edge], [H:0-3-	0,0-1-8], [M:0-1-12,0-1-0],	[R:0-1-12,0-1-0]		
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.61		loc) l/defl L/d M-N >999 360	PLATES GRIP MT20 244/190
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr YES	BC 0.59 WB 0.66	Vert(TL) -0.18 M Horz(TL) 0.03	∕I-N >999 240 J n/a n/a	MT20HS 187/143
BCDL 10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.26	S >874 240	Weight: 196 lb FT = 20%
LUMBER- TOP CHORD 2x6 SF	No.2 *Except*		BRACING- TOP CHORD St	ructural wood sheathing di	rectly applied, except end verticals.
	-X: 2x4 SP No.2		BOT CHORD Ri	gid ceiling directly applied. 8-0 oc bracing: M-R	
WEBS 2x4 SF	No.3 *Except* S: 2x6 SP No.2, D-F: 2x4 SP No.2		Ũ		
,	e) T=1059/0-5-8 (min. 0-1-8), J=105	9/0-5-8 (min. 0-1-8)			
Max H	orz T=-843(LC 6) plift T=-386(LC 8), J=-386(LC 9)				
	rav T=1200(LC 2), J=1200(LC 2)				
	Comp./Max. Ten All forces 250 (lb) c -972/405, C-D=-661/540, F-G=-661/54				
H-J=	1152/541				
N-P=	-804/821, Q-S=0/1627, O-Q=0/1627, L- -1062/0, M-N=-764/837				
	37/343, C-R=-37/343, D-F=-872/805, -1180/539	B-S=-156/715, H-K=-163/	715, P-S=-1180/585,		
NOTES-					
2) Wind: ASCE 7-05; 1	loads have been considered for this d 30mph; TCDL=6.0psf; BCDL=6.0psf; h	=25ft; Cat. II; Exp C; enclo			
DOL=1.60 plate grip				,	N'SH CAP
4) All plates are MT20	designed for basic load combinations, plates unless otherwise indicated.	which include cases with r	eductions for multiple concu	rrent live loads.	A DE LESIO IN
6) This truss has been	T20 unless otherwise indicated. designed for a 10.0 psf bottom chord li				
will fit between the b	n designed for a live load of 20.0psf on ottom chord and any other members.		-	tall by 2-0-0 wide	SEAL
9) Bottom chord live lo	.0 psf) on member(s). C-D, F-G, D-F; N ad (40.0 psf) and additional bottom cho	rd dead load (5.0 psf) app	lied only to room. P-R, N-P,		SEAL 039032
joint J.	I connection (by others) of truss to bea			·	「美人」「人美
sheetrock be applied	equires that a minimum of 7/16" structu ed directly to the bottom chord.	al wood sheathing be app	blied directly to the top chord	and ½" gypsum	I GINEER OT
12) Attic room checked	for L/360 deflection.				M. KOMPILIT
LOAD CASE(S) Stan	dard				March 20 2017

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

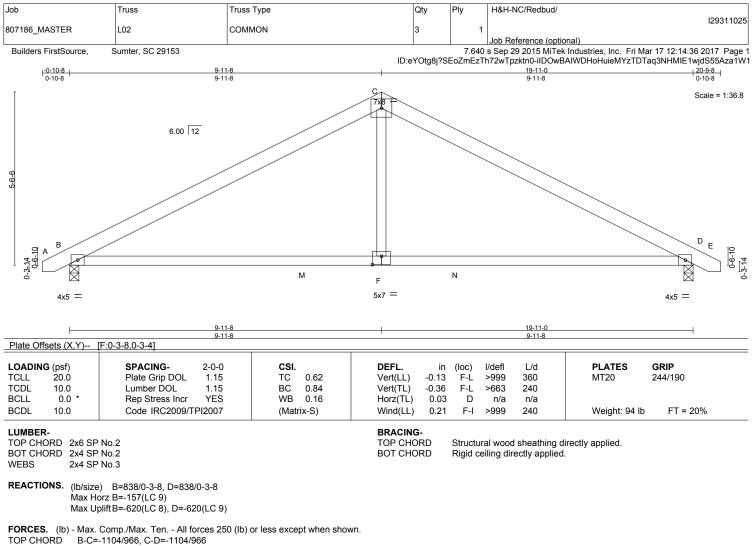
ENGINEERING BY RENCO

March 20,2017

818 Soundside Road Edenton, NC 27932







BOT CHORD B-M=-588/931, F-M=-588/931, F-N=-588/931, D-N=-588/931

WEBS C-F=0/408

NOTES-

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

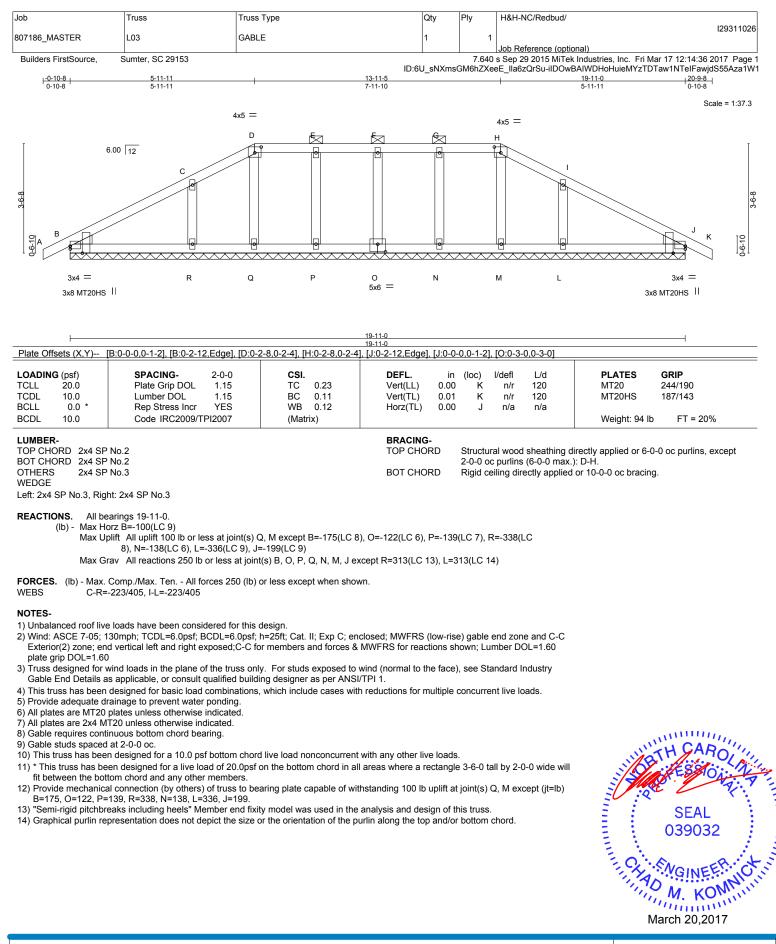
3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will
- fit between the bottom chord and any other members, with BCDL = 10.0psf. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=620, D=620.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.



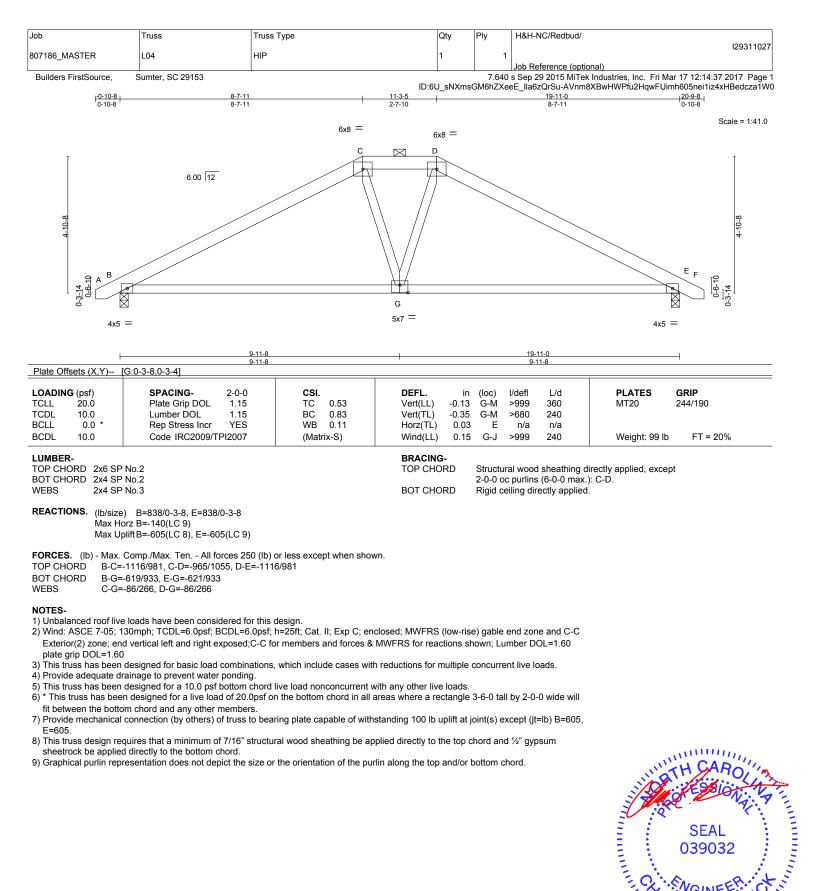
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. This design is based only upon parameters and properly incorporate this design into the overall
 building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoISITPTI Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932





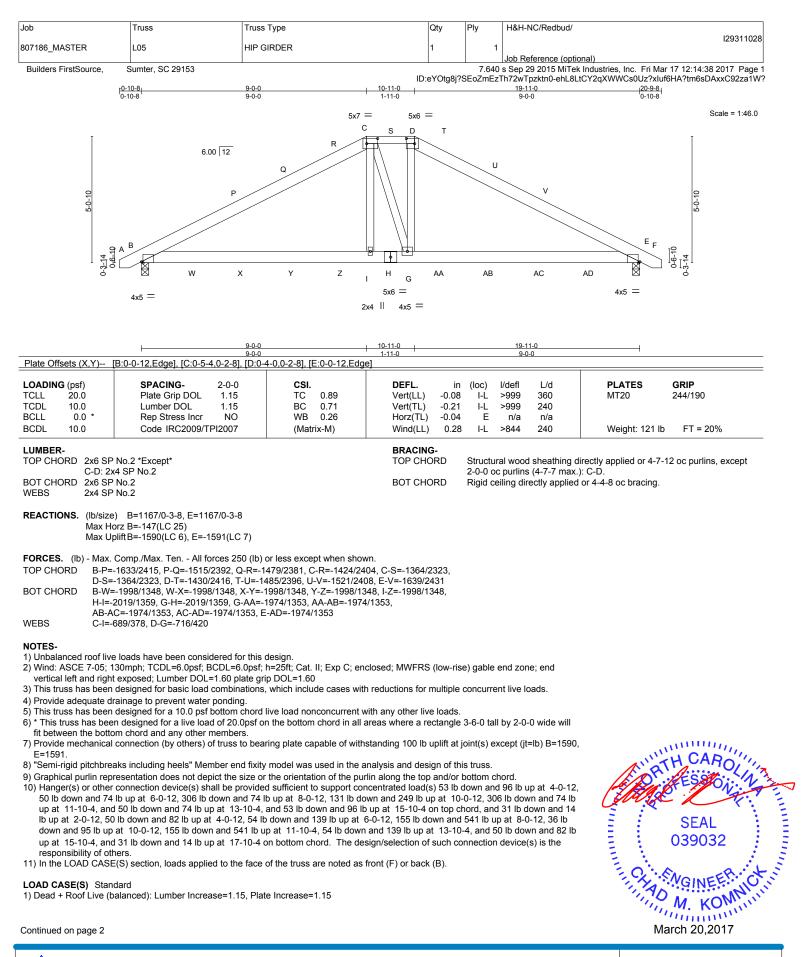
Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



March 20,2017



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	H&H-NC/Redbud/
807186 MASTER	L05	HIP GIRDER	1	1	129311028
··· ·· <u> </u>		-			Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153				s Sep 29 2015 MiTek Industries, Inc. Fri Mar 17 12:14:38 2017 Page 2

ID:eYOtg8j?SEoZmEzTh72wTpzktn0-ehL8LtCY2qXWWCs0Uz?xluf6HA?tm6sDAxxC92za1W?

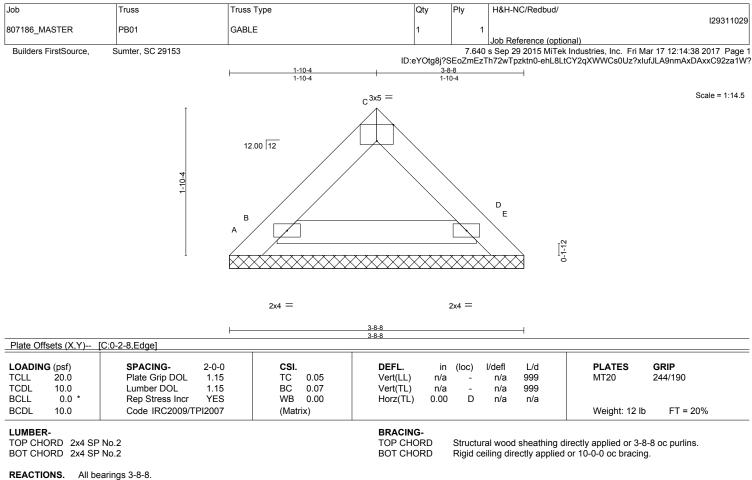
LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: A-C=-60, C-D=-60, D-F=-60, J-M=-20 Concentrated Loads (lb)

Vert: H=-18(B) P=-13(B) Q=-11(B) S=-46(B) U=-11(B) V=-13(B) W=-14(B) X=-50(B) Y=-54(B) Z=-155(B) AA=-155(B) AB=-54(B) AC=-50(B) AD=-14(B)





(lb) - Max Horz A=-108(LC 6)

Max Uplif All uplif 100 lb or less at joint(s) A, E except B=-169(LC 8), D=-137(LC 9) Max Grav All reactions 250 lb or less at joint(s) A, E, B, D

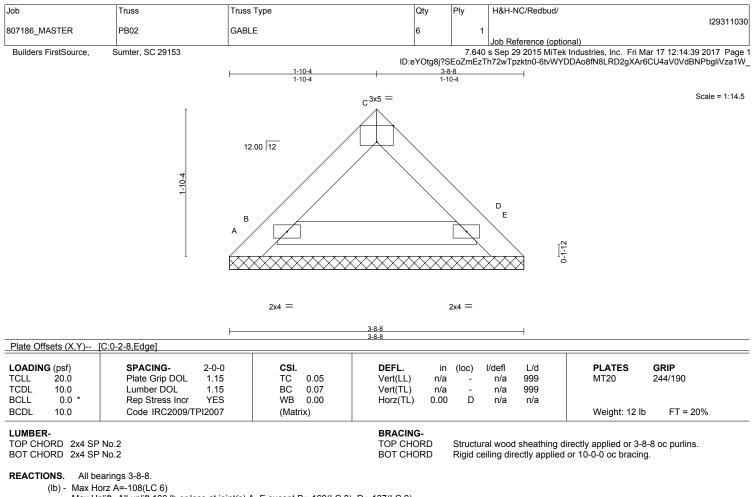
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (13)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) A, B considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, E except (jt=lb) B=169, D=137.
- 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 13) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



818 Soundside Road Edenton, NC 27932



Max Uplift All uplift 100 lb or less at joint(s) A, E except B=-169(LC 8), D=-137(LC 9) Max Grav All reactions 250 lb or less at joint(s) A, E, B, D

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (13)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) A, B considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, E except (jt=lb) B=169, D=137.
- 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 13) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



818 Soundside Road Edenton, NC 27932

