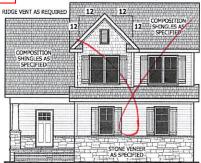
Gaston II.aec

Development Company, Inc\200128B Gaston II\200128B

RIDGE VENT AS REQUIRED



FRONT ELEVATION WITH SIDE LOAD GARAGE

SCALE 1/8" = 1'-0"

PLANS DESIGNED TO THE 2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE

Mean Roof Height 25'-8	HEIGHT TO RIDGE 30'-0"				
CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A		
FENESTRATION U-FACTOR	0.35	0.35	0.35		
SKYLIGHT U-FACTOR	0.55	0.55	0.55		
GLAZED FENESTRATION SHGC	0.30	0.30	0.30		
CEILING R-VALUE	38 or 30cl	38 or 30d	38 or 30d		
WALL R-VALUE	15	15	19		
FLOOR R-VALUE	19	19	30		
 BASEMENT WALL R-VALUE 	5/13	10/15	10/15		
** SLAB R-VALUE	0	10	10		
 CRAWL SPACE WALL R-VALUE 	5/13	10/15	10/19		

" "10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION ** INGLETION DEPTH WITH MONOLITHIC SLAS 24" OR FROM INSPECTION GAS TO SOTTOM OF FOOTING; INSULTION DETH WITH STEM WALL SUG 24" OR TO BOTTOM OF FOUNDATION WALL
DESIGNED FOR WIND STEED OF 120 MPM, 3 SECOND GUST 193 FASTEST MLLE BOOGUSE 18"

COMPONENT & CLADDING DESIGNED FOR THE FOLLOWING LOADS

COMPONENT	aur	CALLAC	DEDIG	HED IT	K I DE	TULLU	SHILLE	LUMUS
MEAN ROOF	UP 1	O 30'	30'-1"	TO 35'	35'-1"	TO 40"	40'-1"	TO 45'
ZONE 1	14.2	-15.0	14.9	-15.8	15.5	-16.4	15.9	-16.8
ZONE 2	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	-21.8	17.4	-22.4
ESIGNED FOR WIN								
COMPONENT								
MEAN ROOF	UP T	O 30'	30'-1"	TO 35'	35'-1"	TO 40"	40'-1"	TO 45'
ZONE 1	16.7	-18.0	17.5	-18.9	18.2	-19.6	18.7	-20.2
ZONE 2	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 3	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 4	18.2	-19.0	19.1	-20.0	19.8	-20.7	20.4	-21.3
ZONE 5	18.2	-24.0	19.1	-25.2	19.8	-26.2	20.4	-26.9

AIR LEAKAGE

N1102.4.1 Building thermal envelope. The building thermal envelope shall be durably sealed with an air barrier system to limit Inflitration. The sealing methods between dissimilar materials shall allow for differential expansion and contraction. For all homes, allow for differential expansion and contraction. For all nomes, where present, the following shall be caulked, gasketed, weather stripped or otherwise sealed with an air barrier material or solid material consistent with Appendix E-2.4 of this code:

inate and considers, with appendix e.z.4 of this code:

1. Blocking and sealing floor/ceiling systems and under knee walls open to unconditioned or exterior space.

2. Capping and sealing shafts or chases, including flue shafts.

3. Capping and sealing soffit or dropped ceiling areas.

ROOF VENTILATION

SECTION REDG

R306.1 Ventilation required. Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or snow. Ventilation openings shall have a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Ventilation openings having a least dimension larger than 1/4 Inch (6.4 mm) shall be provided with corrosion-resistant wire cloth screening, hardware cloth, or similar material with openings having a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm)

PER CODE

ameniand of your liquid in mining members shall conform to the requirements of Section R802.7. R806.2 Mininguish area. The total net free verblating area shall not be less than 1/150 of the area of the space ventilated except that reduction of the total rate to 1/130 is permitted provided that a least 50 percent and not total rate to 1/130 is permitted provided that a least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above the eave or comice vents with the balance of the required ventilation provided by eave or cornice vents. As an alternative, the net free cross-ventilation area may be reduced to 1/300 when a Class I or II vapor retarder is installed on the warm-in-winter side of the celling

 Enclosed attic/rafter spaces requiring less than 1 square foot (0.0929 m2) of ventilation may be vented with continuous soffit ventilation only. 2. Enclosed attic/rafter spaces over unconditioned space may be vented with continuous soffit vent only.

SQUARE FOOTAGE OF ROOF TO BE VENTED = 1558 SQ.FT. NET FREE CROSS VENTILATION NEEDED:

WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 10.39 SQ.FT.
WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE: OR WITH CLASS LOR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 5.16 SQ.FT.

GUARD RAIL NOTES

SECTION R312

R312.1 Where required. Guards shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or grade below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect

screening shall not be considered as a guard.

R312.2 Height. Required guards at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914). mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads

1. Guards on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the

treads.

2. Where the top of the *quard* also serves as a handrail on the open sides of stairs, the top of the guard shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting ding edges of the treads.

R312.3 Opening limitations. Required guards shall not have openings from the walking surface to the required guard height which allow passage of a sphere 4 inches (102 mm)in diameter.

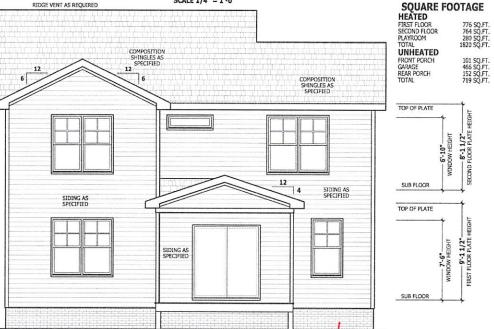
The triangular openings at the open side of a stair, formed by the riser, tread and bottom rall of a guard, shall not allow passage of a sphere 6 inches (153

mm) in diameter.

2. Guards on the open sides of stairs shall not have openings which allow passage of a sphere 4 3/8 inches (111 mm) in diameter.



FRONT ELEVATION SCALE 1/4" = 1'-0"



REAR ELEVATION

SCALE 1/4" = 1'-0"

HAYNES HOME PLANS, INC.
ASSUMES NO LIABILITY FOR
CONTRACTIONS PRACTICES AND
PROCEDURES.
CODES AND CONDITIONS MY
WAYN WITH LOOTHON. A LOCAL
DESIGNER, ARCHITECT OR
NOTINEED RECORD BE CONSILTED
BEFORE CONSTRUCTION.
THISSE DRAWNING AND
LISTER PRAYING AND
AS SUCH SYMILE PRAYING AND
AS SUCH SYMIL

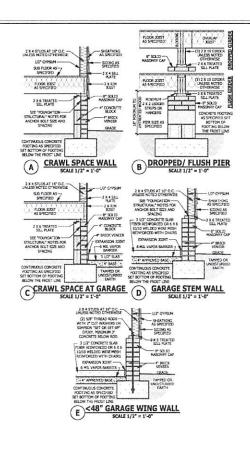
REAR ELEVATION H GASTON రా

H FRONT

SQUARE FOOTAGE 776 SQ F1 764 SQ F1 280 SQ F1 1820 SQ F1 PLAYROOM TOTAL UNHEATED FRONT PORCH GAPAGE REAR RORCH TOTAL 101 SQ.FT 466 SQ.FT 152 SQ.FT 719 SQ.FT

© Copyright 2016 5/19/2020 181035B

PAGE 1 OF 8



FOUNDATION STRUCTURAL

115 to 130 mph wind zone (1 1/2 to 2 1/2 story)
CONTINUOUS POOTING: 15" wide and 8" thick minimum, 20" wide minimum at brick veneer. Must extended 2" to either side of supported wail. GIRDERS: (3) 2 X 10 girder unless noted otherwise.

PIERS: 16" X 16" piers with 8" solid masonry cap on 30" X 30" X 10" concrete footing with maximum pier height of 64" with hollow masonry and

160" with solid masonry.

POINT LOADS:

Other toal measuring.

POINT LOADS:

Other toal to pier, girder or foundation wall.

115 and 120 MPH ANCHORS BOLTS: 1/2" diameter anchor boits embedded

minimum 7°, maximum 6°0° on center, within 12° of plate ends, and minimum two anchor botts per plate.

130 MPH ANCHORS BOLTS: 1/2° diameter anchor botts embedded minimum 15°, maximum 4°0° on center, within 12° of plate ends, and minimum two

12 , mountain 4-0 or center, would 12 or pace exis, and management to anchor botts per plate.

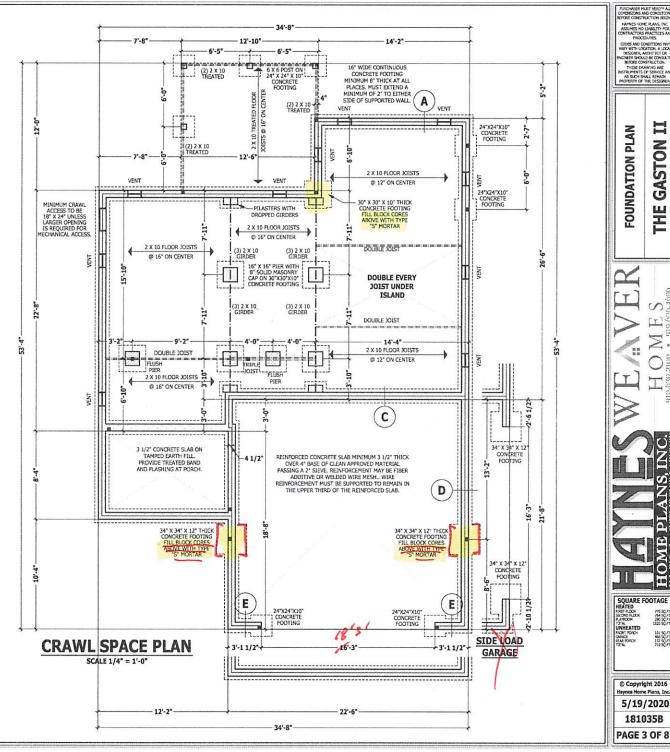
CONCRETE: Concrete shall have a minimum 28 day strength of 3000 psi and a maximum 5" slump. Air entrained per table 402.2. All concrete shall be in accordance with ACI standards. All samples for pumping shall be taken from the exit end of the pump.

SQILS: Allowable soil bearing pressure assumed to be 2000 PSF. The South Allowable so in bearing pressure assumed to be 2000 Far. The contractor must contact a geotechnical engineer and a structural engineer if unsatisfactory subsurface conditions are encountered. The surface area adjacent to the foundation wall shall be provided with adequate drainage, and shall be graded so as to drain surface water away from foundation walls.

CLOSED CRAWL PER R409 OR WALL VENTED CRAWL SPACE

UNDER-FLOOR SPACE (SECTION R408)

SQUARE FOOTAGE OF FOUNDATION TO BE VENTED = 735 SQ.FT. WITHOUT CROSS VENTILATION AREA OF VENTING NEEDED = 4.9 SQ.FT. WITH CROSS VENTILATION AREA OF VENTING NEEDED = 0.49 SO.FT. NOTE: NUMBER OF VENTS NEED WILL VARY DEPENDING ON VENTS USED AND CROSS VENTILATION.



GASTON

H

776 SQ.FT. 784 SQ.FT. 290 SQ.FT. 1820 SQ.FT.

101 SQ FT 466 SQ FT 157 SQ FT 719 SQ FT

STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, price all local codes and regulations. This document in no way shall be construed to supersede the code.

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no

Bability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconles and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		
Guardrail in-fill components	50		
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS:

ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x106 PSI

Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI

Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-Joist layouts shall be

prepared in accordance with this document. Trusses and I-joist layouts shall be installed according to the manufacture's coordinate. installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing. ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick. CONCRETE AND SOILS: See foundation notes.

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer falls to meet or exceed designated heel heights, finished knee wall heights, or finished coiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics.

BEARING. All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights

and floor system thicknesses.

EXTERIOR HEADERS

(2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE

KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16' KING STUD(S) 1 2 3 5 6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END NON LOAD BEARING HEADERS TO BE LADDER FRAMED

BRACE WALL PANEL NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise

GYPSUM: All interior sides of exterior walls and both sides Interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1. REQUIRED LENGTH OF BRACING: Required brace wall length

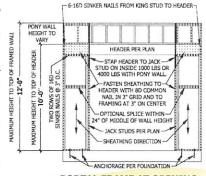
for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length.

HD: 800 lbs hold down hold down device fastened to the edge

of the brace wall panel closets to the corner.

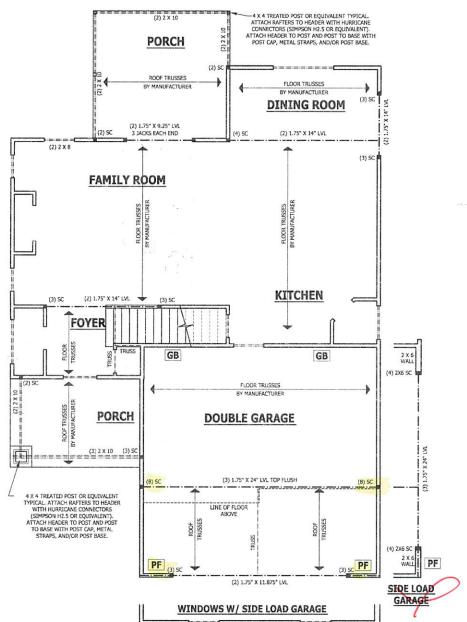
Methods Per Table R602.10.1 CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at Intermediate supports with 6d common nails or Bd(2 1/2" long x 0.113" diameter). CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing

GB: Interior walls show as GB are to have minimum 1/2' gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws. PF: Portal fame per figure R602.10.1



PORTAL FRAME AT OPENING

(METHOD PF PER FIGURE AND SECTION R602.10.1) SCALE 1/4" = 1'-0"



FIRST FLOOR STRUCTURAL

HAYNES HOME PLANS, INC.

THESE DRAWING ARE INSTRUMENTS OF SERVICE AN AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER

STRUCTURAL

GASTON FLOOR H FIRST

SQUARE FOOTAGE HEATED 776 SQ.FT 764 SQ.FT 280 SQ.FT 1820 SQ.FT UNHEATED

© Copyright 2016 Haynes Home Plans, Inc. 5/19/2020

181035B

PAGE 5 OF 8

OB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and

the building code.			
DESIGN LOADS	LIVE LCAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(U)
Attics without storage	10	10	L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		-
Guardrall in-fill components	50		-
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Chaire	40		1/360

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise

ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2500 PSL Fv=265 PSL E=1.9x106 PSL Parallel strand lumber (PSL) = F5=2900 PSI, Fv=290 PSI, E=2,0x106 PSI Laminated strand lumber (LSL) Fb=2230 PSI, Fv=400 PSI, E=1,55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-101ST MEMBERS: All mof truss and I-tolet layouts shall be prepared in accordance with this document.
Trusses and I-loists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9°-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" boths at 2°-0" on center for spans up to 18°-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist specing, minimum **S/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing.

**ROOF SHEATHING: OSB or CDX roof sheathing minimum

SECTION R612

SECTION R807

R807.1 Attic access. An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have to auto, areas of a Ecceed was open a feet (37.10 mg) and have a vertical height of 60 inches (1524 mm) or greater. The net clear opening shall not be less than 20 inches by 30 inches (508 mm by 762 mm) and shall be located in a hallway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

Exceptions:

1. Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. not required to have access.

EXTERIOR WINDOWS AND DOORS

R612.1 General. This section prescribes performance and construction requirements for exterior windows and doors installed in walls. Windows and doors shall be installed and flashed in accordance with the fenestration manufacturer's written installation instructions Window and door openings shall be flashed in accordance with Section R703.8. Written

Installation instructions shall be provided by the fenestration manufacturer for each window R612.2 Window sills. In dwelling units, where the opening of an operable window is

located more than 72 inches (1829 mm) above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches (610 mm) above the finished floor of the room in which the window is located. Operable sections of

where such openings are located within 24 inches (610 mm) of the finished floor.

guards, where provided, shall comply with the requirements of ASTM F 2090.

windows shall not permit openings that allow passage of a 4 inch (102 mm) diameter sphere

3. Openings that are provided with fall prevention devices that comply with ASTM F 2090. Windows that are provided with opening limiting devices that comply with Section R612.4.

R612.3 Window fall prevention devices. Window fall prevention devices and window

2. Pull down stair treads, stringers, handralls, and hardware may protrude into the net dear opening.

EXTERIOR HEADERS

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE KING STUDS EACH END PER TABLE BELOW

HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16' KING STUD(S) 1 2 3 5 6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE NON LOAD BEARING HEADERS TO BE LADDER FRAMED

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finis colling integrits shown on ordere crawings the infished square hootage vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. BEARING. All trusses shall be designed for bearing on SPF #2 plates or

WALL THICKNESSES

Exterior walls and walls adjacent to a garage area are drawn as 4" or as noted 2 X 6 are drawn as 6" to include 1/2" sheathing or gypsum. Subtract 1/2" for stud face

Interior walls are drawn as 3 1/2" or as noted 2 X 6 are drawn as 5 1/2", and do not include gypsum.

with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins.

KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights shown on these drawings the finished square footage may reasonability of the truss manufacturer.

ledgers unless noted otherwise.

Plate Heights & Hoor Systems. See elevation page(s) for plate heights

and floor system thicknesses.

2'-10 2'-8" X 5'-2" TWIN ROOF TRUSSES BY MANUEACTURE 4'-0" X 1'-0" MASTER BEDROOM 2'-8" X 5'-2" TWIN TRANSOM (2) 2 × 10 (2) 2 X 10 GIRDER TRUSS BY MANUFACTURER 2 JACKS EACH END 2 JACKS EACH END (4) 50 14'-2" 14'-2" 2'-4' PLAY ROOM W.I.C. LINEN 2 X 6 WALL 2'-6" 4'-0" X 1'-0" **MASTER** TRANSON SHOWER BATH HEADER HEIGHT 2'-6" | 2'-6" 10'-10 4'-0" X 1'-0" RAIL OR HALF WALL LINEN TRANSOM SET AT 7'-5" HEADER HEIGHT BATH STORAGE HATCHED WALLS -INDICATED LOAD BEARING AND EXTERIOR 6'-6 1/2 WALLS WHERE HEADERS 2'-3 1/2" BEDROOM #2 BEDROOM #3 10'-10" GIRDER TRUSS (3) SC (3) SC BY MANUFACTURER -ROOF TRUSSES 2'-8" ¥ 5'-2" TWIN 2'-8" X 5'-2" TWIN (2) 2 X 12 2 JACKS FACH END BRACING NOT SHOWN - 11'-2' 11'-4" ON UPPER STORY PER R602.10.3.2 (5) & (6) 12'-2" - 22'-6"

CODES AND CONDITIONS MA VARY WITH LOCATION, A LOC DESCRIES, ARCHITECT OR

NEER SHOULD BE CONSULTED BY CONSULTED BEFORE CONSTRUCTION.
THESE DRAWING ARE TRUMENTS OF SERVICE A AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNE

PLAN GASTON SECOND FLOOR H

SQUARE FOOTAGE 776 SQ F1 764 SQ F1 290 SQ F1 1820 SQ F1 UNHEATED FRONT PORCH 101 SQ.F1 466 SQ.F1 157 SQ.F1 719 SQ.F1

© Copyright 2016 5/19/2020 181035B

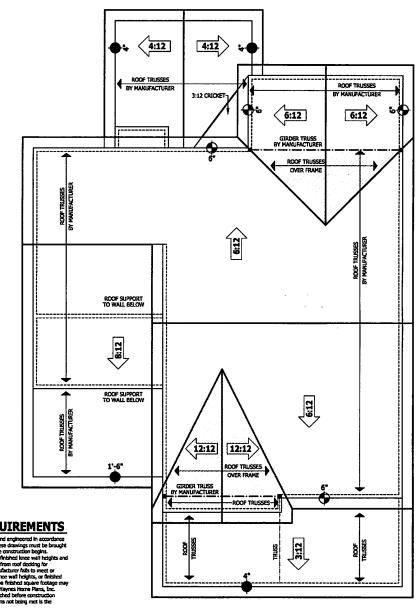
PAGE 6 OF 8

SECOND FLOOR PLAN

CONCRETE AND SOILS: See foundation notes. ATTIC ACCESS

II\200128B

Gaston



ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any varietion with these drawings must be brought to letynes Home Plan, Inc. attention before construction begins for DICE WALL AND CELLING HISTORIES. All finished hone wall heights and celling heights are shown hurred down 10° from roof deciding for instaction. If for any reasons the tours manufacture risks to meet or exceed designated here heights, finished hone wall heights, or finished circling heights shown on these dewrings the finished square forbage may vary. Any disorgancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these canditions not being met is the reasonability of the bruss manufacturer.

AMCHORAGE. All required enchors for trusses due to upfit or boering stall meet the requirements as specified on the truss schematics.

shall meet the requirements as specified on the truss schematics. BEARING. All trusses shall be designed for bearing on SPF #2 plates or ledges unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

HEEL HEIGHT ABOVE FIRST FLOOR PLATE

HEEL HEIGHT ABOVE SECOND FLOOR PLATE

ROOF PLAN SCALE 1/4" = 1'-0"

HAYNES HOME PLAKS, INC. ASSUMES NO LIMITLITY PO CONTRACTORS PRACTICES A

COOIS AND CONCETTORS MAY WITH LOCATION, A LOCA DISSIBLE, AND FIT OF SCHOOL CONSTRUCTION. THESE DRAWING ARE DISTRIBUTED OF SERVICE AND AS SUCH SHALL REMAIN MOPERTY OF THE DESSERGE.

GASTON **ROOF PLAN** THE



SQUARE FOOTAGE PREST PLOCE SECOND PLOCE PLATECOM TOTAL URCHEATED PROST PORCH GRACE PLATECOM TOTAL MASSIT. MASSIT. MASSIT.

© Copyright 2016 Haynes Home Plens, Inc. 5/19/2020 181035B

PAGE 7 OF 8

CP TOTAL 931

CI BIO DU

CECLION VATOR

OTTIMY MOLLOSS

CELLIDERS SA DMITTOOP

- 4" APPROVED BASE

RELIEVAS ROPAV JIM 8

EXPANSION JOINT-

EÎNFORCED WITH CHAIRS

TO/ TO AVELDED WITH MISSH FIRER REINFORCED OR 6 X 6

SWCING

CINA SIZIZ TJOB ROHDWA

STRUCTURAL" NOTES FOR

-NOTTACHUNGT 332

NLESS NOTED OTHERWISE

A

BELOW THE PROST LINE

SET BOTTOM OF FOOTING

FOOTING AS SPECIFIED

CONTINUOUS CONCRETE

CNA BSLT SIZE AND

STRUCTURAL" NOTES FOR

SIL PLATE

4-24 HOOTH BUS

WUSAYD "S/I

•

DECK BKACING

rdon aun se suadiums modeles

gayswised bolt with rut and weater at each end of each brading member per Figure ANLOS.3. ANLOSA.1.5. For embedment of piles in Costal Region

the standard decise or parallel for the structure at the shall expect of the standard decise. The 2 x 6x shall be expect of the posts with free for the posts with free first posts with the first chosen.

ANT 199.1.4. 2 x 6 dagonal vertical cross bracing may be provided in two perpendicular cirections for

9 X 9 150 2k 6.0. 3.-9. 1.-8. 4 X 4 48 2k 4.0. 5.-9. 1.-0.

STRUMO HITEO THORN TWANTED TOO

S. 201 MA sugil cities as stated with the provided by S. 201 MA sugil cities sometimes of provided by S. 201 MA sugil cities sometimes of provided by S. 201 MA sugil cities of the sug

on 2010, 1.2. 4 x 4 wood trace braces may be provided on each column in both directions. The lorse braces shell for each post as a point not less than 1/3 of the post

shows finished grade per Figure AM109 and the dock is accided to the structure in strotcose with Section API 1094, istawi brading in not required. API 1094, 8,4 4 wood loste braces may be strucked or

AN1.09.1 Deads bracing, Decks shall be braced to provide therein statistics, the (drowing are acceptable means to provide thereis statistics.) The result of the provide there is subtility and the deck front height is being than 4-0. ABLIDEALLY WITH a well to deck from ARLIDE statistics to the property of the SEGMENT of the statistics of the the segment of the segment of the second of the segment of the segment of the second of second o

We from before the sense for the stock becomes to the stock for the security that some security that the security that the security that some security that security the security that

ANTIOLS Sains shall be constructed per Rigura ANTIO. Surface types call be no greated whan 7 look spen between Surface typers and persons the properts possible the properts property of the p

2CVTE 3\4.. = 1.-0..

GARAGE STEM WALL

2CVTE 3\4. = 1.-0.

CRAWL SPACE WALL

HIM

#CIVAL:

NZONKA CVO - B. ZOITD

SPECIFIED AS

AS SPECIFIED

WUSAYD "S/I

BOVES

SCOOLS STOCK

HYM

trinimin 3 I/2 inches between step cut and back of strik

DECK STAIR NOTES

the the statement france. The exterior latin shall cover and terminate on the **2CVTE 3\4.** = 1,-0. ect to toristice ects of elists of this **MEED SCREED** bne zeene bayaq ayoda (min IZ) tanbri baqqari wolla lilw tarti aqyi a to ad llariz

 to muminism 6 boosig ad literiz boorge S no ritnes anti avode (mm 501) zarbni CURING CHADE strail be provided at or below the foundation plate line on exterior stud walls in accordance with ASTM C 926. The weep in accordance with ASTM C 926. The weep HOR FOUNDATION NOTTACHOOM 333 INDIMENSA OIL CHOUND OR 2" (mm e8) earbri 5\te to sensi insmitsatt OT " MUMINIM weep screed, with a minimum vertical mm) (No. 26 galvanized sheet gage), comosion-resistant weep screed or plastic OSSESS ASSESSED MEDIANA ROWN -HIVI CELLIDARS SA and the report and the January from the strong con-active control and the January of the strong con-trol control and the property of the control behavior. In the property of the property of the control and set between the property of the control and set between the property of set between the property of AS SPECIFIED STONE VEENER

2.0) riori-610.0 muminim A - 1.5.0.6019 Installed per manufactures instructions and per the 2012 North Carolina Residential Building code. of or reene and stone veneer to be

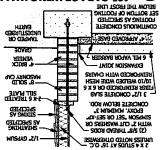
SUZGREDECK DRUGE OF THE WEED SCREED.

MEED SCREEDS

2CVTE 3/4. = 1.-0. TYPICAL DECK STAIR DETAIL FIGURE AM110

OF STEPS FOR BEARING DONCIGETE MAD AT BOTTOM PROVIDED 3 1/2" THICK TREATED 2 X 10 OR TREATED 2 X 4 PLATE FOR SEARING HALATED FLOOR JOIST SIZED FER FOUNDATION PLAN MUMINIM BELINEEN DECKLING HAUTHUR TREATED DECIGNG MINIMUM 1/4" GAP

2CVFE 3\4. = 1.-0. E <48" GARAGE WING WALL

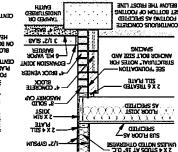


2CVTE 3\4. = 1.-0. DROPPED/ FLUSH PIER SMLI IOUN'I SMI WOLES ENTROOP CELLED BAS NEED AND A STREEN NU NUTTUR CONCRETE HOOLING SYEEDWAH STRUSSOR MASONRY CAP S X S LEDGER GDOS -8 → - HOMENIN SILL PLATE CETASHT & X S-

FLUSH GIRDER OTHERWISE CHILDING SV DIAFTERS MOLLED TELOOR SOUTH (a) 5 x 10 CHOR SIL PLATE WYZONSK CVb B. ZOND SIMMENTO CHILDRAS SY GVERLAP T210t R00.H

MANAGE PXS-DISTRIBUTED CONCENSES

2CVFE 3\4. = 1.-0. CCRAWL SPACE AT GARAGE



LEGW SITE CATA CRUSTS SELECT REST DESTRUCTIVES MACHINES BLOGGEOUS SHOWS SUSIES DE HERRES uni nozimbal pontin tiptin patno irrawis fizimalizamon e montu. beta-percentain of likely acrosses made in 40 yra (gebeal scholdred in the stockholdred in the security of the security of the security of the beta beta from the beta contained as the security of contrast of years of which the security of the beta security of the years of the security of the security of the security of the exemblers as posited that security of the percentage of the likel beta-percent of which percentage of the percentage of the percentage of which percentage of the percentage of DEST TERMOLENE TWO DRIVES

nicitive ballisteni ad ot bastupan zi mnala silome ano nariti anom naci akam instaled on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one fall story below the upper level.

paccase, there is no state that the chart is 3. On each additional stray of the dwelling, including besenand

to window staborum ant in some gringsole edentities which for To the secon specifical recent :5100

Exception: Where tensing slaving are provided meeting the requirements of Section R314.4. R334.3 Location. Smoke slaving the installed in the lots

USM SCHROLOTOR LE COMEZURAN DO DUP LICORES ÉLISARISÉES : brisketes, it shall become a permanent that is of the occupancy and brind by the homeowner. The system state to the homeowner is a brind to homeowner. as fast, games sooms not notes about to barupon as abstrars ame and showing text metary made and burdeauch and Ladzimou will notice about the branch and burdeauch as made to Defector at matry grainers and burdeauch a madel's make a close colsustions and the such insertation to noticulation a groun bas vocacance and to autaff insertation a service Each of Albacham .C. Arith to ancievory theoretics amainya mela sril blorbaucht amainya notibotob sitowa E.A.E.G , amais oxioma obubri zeut S. Arith rithe sonabracas et battaria sohob notisalikon sidducia bins sobasta alema kin notianidimo a anti tuta samalia entrora un notica setti et berna se battaria.

ad tests armets saloms IA andissalition bus notineab salome LAMER rithw constructes in balletten bus V.I.S. JU rithw constructes in basel grainew shift blorisecon's still bus abox sint to anoishors; still the salom sale blorisecon's still bus abox sint to anoishors; still the salom sale blorisecon's still bus abox sint to anoishors; still bus salom sint to anoisho PERMINDER

SMOKE ALARMS

© DECK ATTACHMENT CELEBOR SA MOGULE X S GENERAL NO 2CVTS 1\3. = 1.-0.

EITTED BORCH SECTION WITH VENT XXXX a imomo si x a JIGA ST X S AD AMOUNT STEP OA 2006 TIAN LES XXXX CLASSE

2CVTE 3/4" = 1'-0" TAPERED COLUMN

PORCH HEADER WITH **МВ СОГЛИИ** WITH POST CAP CENTER LIKE OF HEADER. ATTACHED TO HEADER **BASONRY BASE** MENO WINULOO CEMENATI HEADGR AS DESIRED ON BOTH SIDES & UNDER L SUCHORISM BLOCKING INSTALLED-INSTALLED PER HANUFACTURES TITHOR SIGRAH SO JYNIV CEALLER OF COLLIMIN BASE PORCH HEADER PER — PLAN INSTALLED OVER ADSAH BUZ 8 X S Z

ноти яод

10001 335

PLAN OR NOTTON

ZI

LITER SOUTHING AST -

CHEATHING AS SPECIFIED

CHINGLES AS SPECIFIED

MOUTH HUNGH NO GEOGE -4

antidoring between a wal-mounted handral and a guardetinener, the 3. Two or more separate rafe shall be considered continuous If the the continuous of the rafe course within 6 inches (\$25) and other. If

Handrais Shall be permitted to be interrupted by a newel post.
 The use of a voice, furnord, starting existing or starting newel shall be showed over the lowest bread.

EMERGIA DES MAS SING DES PENCHASAS. The formation of the continuous of the continuous of because of because of the continuous of the conti

A. When handras fittings or bendings are used to provide continuous bandras between flights, the transition from handrast to quadrast, or used to the start of a flight, the handrast height at the fittings or bendings shall be the start of a flight, the handrast height at the fittings or bendings shall be a set to the second or the se

1. The use of a volute, furnout or starting easing shell be allowed over the

futur cost) south or their area for the property of their costs of their costs of continuous run of treads or flight with four or more visions. The depet is a first status of reads in the supper continuous or first supplied to the first supplied of ramp stops, or first surface of ramp stops, or first surface of ramp stops. these to sole and read its no between all select elisabeth aliabath C.T.L.E.F.

bon and (mm 91) richt Plu, north ezzel bon groupe A. (mm 91) man J. (mm 91) man bon at (mm 91) inch ezee with sold exemples (mm 32) section Plu I next exemples (mm 32) section Plu I next exemples (mm 32) section (Mm 91) inch exemples (mm 91) initing with the (Minn SOL) of Hindhold (SOL) with the board print.

**ALTCH The India The India of Caravatare at the colors as all the present of the India of Caravatare at the colors of the India of planes of the forenest projection of adjacent breats and at a right angle to plant of the forenest projection of adjacent breat shad when similaring a benefit is a point at a right and of 9 man) and estable (30 mm) and set of 9 man, and make the plant of 9 man and of estable and project and project plant of 9 man and plant of 9 man and 9 man an

min). The trest deput shall be measured nonzonialy between the vertical the adjacent treats.

**CALS The depth. The minimum bread depth study be 9 brokes (CAS)

**CALSTITUTE of the beautiful to the study of (210 mm). The riser shall be measured vertically between leading odges of

art is souther and not no prizer bear the floor surface of the yearliers out to make portion of the security or practice and the the security was and read from Sair treath one securities N.V. LECH Assimpted the state of the statement head toom in all pasts of the stateway from the state of th

STAIRWAY NOTES

7.1101

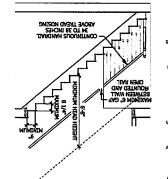
SUCCOUNTED INSCRIPTION REGULATIONS cross governments is either slevel select beungsched 1940 zonochood its ni skibus with previous as basel select emisse sobsonom nookes nodeste select. Decob ert bras slots siet rithe sonstrause ni balastel sel filest be Occorom nocies benkepen self achsementapen emails E.Z.EE

11.216 by the skinn metabolizars, metabolings, in existing diversings, where SMLLS Where required in consisting diversing a control of metabolizars of a control of metabolizars of metabolizars of control of metabolizars of metab

\$25.5 Gwiton monoside alemas. In new construction, dwelling units strat be provided with an approved carbon monoside alem installed outside of each acquarite aleeping area in the instruction with oil the becinoon(s) as directed

CARBON MONOXIDE ALARMS

8 40 8 **3**549 2CVIE 1/4. = 1,-0. TYPICAL STAIR DETAIL 2\16\5050



2CVTE 3\4. = 1.-0. TYPICAL WALL DETAIL

BALLY TROOF SHT WOLLS FOOTING AS SPECIFIED RET BOTTING SET BOTTING THE CONTINUOUS CONCRETE SMITHUS GNA SIZE TJOS ROHOWA STRUCTURAL" NOTES FOR BLOOK CONCRETE OSTABIT 8 X S STAPI LII2 WYZOKIU CVD CENTURAL SA TRIOX SOOJA WIN X Z TIAM. -SV HOOTH ROS SIDING AS WINSWYD "Z/T X X 4 STUDS AT 16" O.C. -**STWEENTO** NOTE ON ELEVATION PAGES BOOD BES ENOZ BLYWITD

WALL INSULATION PER

- I/S. CUSON -

CENTRACE SA

NOTHERS . NO STADIA & Y T TWOTIAN -ELEVATION PAGES SMILINEA LILLIOS -NO ELLON BOOD BES 11406 SER CLIMATE ZONE VIDSV4 8 X T WUSAYD "S/I 3) 2 X 4 TOP PLATE ELFFAS MOTTAJUZKIT ELEVATION PAGES NO ELON EGGG EES SHEATHING AS SPECIFIED

DATESS NOTED

16" ON CENTER

TA SOUTS P X S

401 + X 2 (Z)

CELLIDEAS SV DATOIS

APPLICATION AS SPECIFIED

NOTTALIZENT TOOR PER CLIMATE ZONE -12% BUILDING FELT SHINGLES AS SPECIFIED OK ELEVATIONS

PITCH PER ROOF PLAN

TYPICAL DETAILS m GAS ᆼ Ž

BSEOTRT

alone some same tur

1250

© Copyright 2016

SOUASE FOOTAGE

MOLIN DRAW HOORNAS HOORNAS AND ISTASKINU HONON INDR HONON INDR HONON INDR

METALOGICA DE CONTRE O SENCIA CONCENCACION SENCIA CONCENCACION SENCIA CONCENCACION SENCIA CONTRE O SENC



RE: J0620-2873

Trenco

818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Lot/Block: Project Name: J0620-2873

Model:

Address:

Subdivision:

City:

State

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2009/TPI2007

Design Program: MiTek 20/20 8.3

Wind Code: N/A

Wind Speed: N/A mph

Roof Load: N/A psf

Floor Load: 55.0 psf

This package includes 15 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E14484726	F1	6/24/2020
2	E14484727	F3	6/24/2020
3	E14484728	F4	6/24/2020
4	E14484729	F5	6/24/2020
5	E14484730	F6	6/24/2020
6	E14484731	F7	6/24/2020
7	E14484732	F8	6/24/2020
8	E14484733	F9	6/24/2020
9	E14484734	F10	6/24/2020
10	E14484735	F11	6/24/2020
11	E14484736	F12	6/24/2020
12	E14484737	KW	6/24/2020
13	E14484738	KW1	6/24/2020
14	E14484739	KW2	6/24/2020
15	E14484740	KW3	6/24/2020

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



June 24, 2020

Job	Truss	Truss Type	Qty	Ply		2 Marine 1922 1920 1920 1920 1920 1920 1920 1920
J0620-2873	F1	FLOOR	8	1		E14484726
		(100 to	1850		Job Reference (optional)	

Comtech, Inc, Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:24 2020 Page 1 ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-MeuOvwf1TRjrPwe_ay0FyewWIMXNXFoP0BDBVcz8N6v

Structural wood sheathing directly applied or 6-0-0 oc purlins,

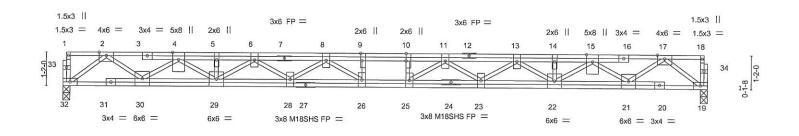
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

0-1-8 H 1-3-0

1-5-0

0-1-8 Scale = 1:38.6



2-9-0		14-6-8	19-8-0	22-5-0
		6-8-0	5-1-8	2-9-0
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 1-7-3 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014		in (loc) I/defl L/d -0.30 25-26 >889 480 -0.41 25-26 >646 360 0.06 19 n/a n/a	PLATES GRIP MT20 244/190 M18SHS 244/190 Weight: 164 lb FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP 2400F 2.0E(flat) **BOT CHORD** 2x4 SP 2400F 2.0E(flat)

WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 32=0-3-0, 19=0-3-0

Max Grav 32=970(LC 1), 19=970(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-4=-2267/0, 4-5=-4172/0, 5-6=-4172/0, 6-8=-5247/0, 8-9=-5785/0, 9-10=-5785/0, 10-11=-5785/0, 11-13=-5247/0, 13-14=-4172/0, 14-15=-4172/0, 15-17=-2267/0 30-32=0/1227, 29-30=0/3336, 28-29=0/4843, 26-28=0/5628, 25-26=0/5785, 23-25=0/5628, **BOT CHORD**

22-23=0/4843, 21-22=0/3336, 19-21=0/1227

WEBS 17-19=-1536/0, 2-32=-1536/0, 17-21=0/1317, 2-30=0/1317, 15-21=-1329/0,

4-30=-1329/0, 15-22=0/1021, 4-29=0/1021, 13-22=-819/0, 6-29=-819/0, 13-23=0/501,

6-28=0/501, 11-23=-483/0, 8-28=-483/0, 11-25=-216/559, 8-26=-216/559

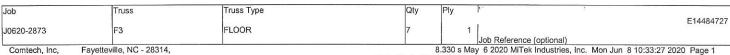
NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 3x6 MT20 unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.
- 5) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



June 8,2020





Fayetteville, NC - 28314, Comtech, Inc,

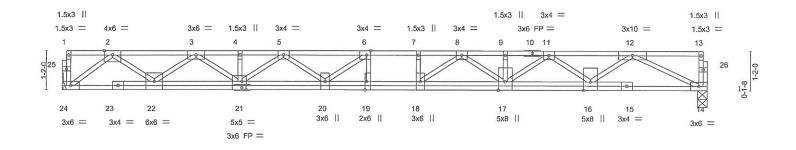
ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-nDaXXyhvmM5QHNMZF5ZyaGYypaXrkbori9Rs6wz8N6s

0-1-8

H | 1-3-0

1-4-8

2-0-0 0-1-8 Scale = 1:33.3



-	10-3-12 10-3-12									19-4-8 8-0-0		
Plate Offsets (X,Y)	[6:0-1-8,Edge], [19:0-3-0,	1-0-12 8-0-0			-0							
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.00 1.00 YES PI2014	CSI. TC BC WB Matrix	0.39 0.32 0.61 ×-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.28 -0.38 0.05	(loc) 19 19 14	I/defl >826 >602 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 120 lb	GRIP 244/190 FT = 20%F, 11%E	

LUMBER-

TOP CHORD 2x4 SP 2400F 2.0E(flat) 2x4 SP 2400F 2.0E(flat) **BOT CHORD**

2x4 SP No.3(flat) WEBS

BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD

REACTIONS. (size) 24=Mechanical, 14=0-3-8

Max Grav 24=1046(LC 1), 14=1046(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2347/0, 3-4=-4043/0, 4-5=-4040/0, 5-6=-4852/0, 6-7=-5063/0, 7-8=-5063/0, TOP CHORD

8-9=-4353/0, 9-11=-4353/0, 11-12=-2866/0

BOT CHORD 22-24=0/1335, 21-22=0/3312, 20-21=0/4601, 19-20=0/5063, 18-19=0/5063, 17-18=0/4812,

16-17=0/3736, 14-16=0/1964

2-24=-1673/0, 2-22=0/1285, 3-22=-1226/0, 3-21=0/897, 5-21=-713/0, 5-20=0/445,

6-20=-593/162, 6-19=-277/217, 12-14=-2182/0, 12-16=0/1147, 11-16=-1105/0,

11-17=0/769, 8-17=-605/0, 8-18=-83/605

NOTES-

WEBS

- Unbalanced floor live loads have been considered for this design.
 Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Design valid for use only with MTick® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	
J0620-2873	F4	FLOOR	3	1	E14484728
					Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:28 2020 Page 1 ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-FP8vIliXXgDHuXxlpo5B7U423_I5T5n_xpBPeNz8N6r

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

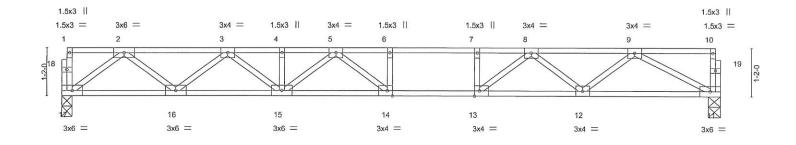
except end verticals.

0 - 1 - 8

H | 1-3-0







i L	9-3-8						10-7-0			15-11-8	
	9-3-8					1-3-8			5-4-8		
Plate Offse	ets (X,Y)	[13:0-1-8,Edge], [14:0-1-	8,Edge]								
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.74	Vert(LL)	-0.24 14-15	>785	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.90	Vert(CT)	-0.33 14-15	>570	360	21750U (2.1047250)	
BCLL	0.0	Rep Stress Incr	YES	WB	0.44	Horz(CT)	0.05 11	n/a	n/a		
BCDL	5.0	Code IRC2015/TF	PI2014	Matrix	-S					Weight: 80 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 17=0-3-0, 11=0-3-8

Max Grav 17=858(LC 1), 11=858(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1774/0, 3-4=-2887/0, 4-5=-2887/0, 5-6=-3157/0, 6-7=-3157/0, 7-8=-3157/0,

8-9=-2067/0

16-17=0/1070, 15-16=0/2453, 14-15=0/3153, 13-14=0/3157, 12-13=0/2674, 11-12=0/1453 2-17=-1340/0, 2-16=0/916, 3-16=-884/0, 3-15=0/554, 5-15=-340/0, 5-14=-241/390, 9-11=-1641/0, 9-12=0/799, 8-12=-790/0, 8-13=0/814, 7-13=-365/0 **BOT CHORD**

WEBS

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see __ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply		
J0620-2873	F5	FLOOR	6	1		E14484729
30020-2013		Control and the State of State			Job Reference (optional)

Comtech Inc. Favetteville, NC - 28314.

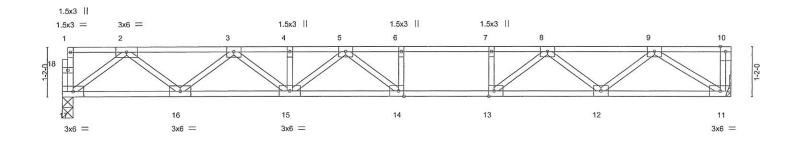
8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:29 2020 Page 1 $ID: BoL? hgXgIYpqwdOiyUmcQyz41fz-jbhHyej9I_L8WhWxNVcQfhdDVN4BCYD8ATwyApz8N6q\\$

0-1-8

H | 1-3-0

2-0-0

Scale = 1:26.0



T.	9-1-12						10-3-8	_1		15-8-0		
Г	9-1-12					1-1-12			5-4-8			
Plate Off	sets (X,Y)	[13:0-1-8,Edge], [14:0-1-	8,Edge]									
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL	40.0	Plate Grip DOL	1.00	TC	0.76	Vert(LL)	-0.24 14-15	>776	480	MT20	244/190	
TCDL	10.0	Lumber DOL	1.00	BC	0.90	Vert(CT)	-0.33 14-15	>566	360			
BCLL	0.0	Rep Stress Incr	YES	WB	0.42	Horz(CT)	0.05 11	n/a	n/a			
BCDL	5.0	Code IRC2015/TI	PI2014	Matri	x-S					Weight: 79 lb	FT = 20%F, 11%E	

LUMBER-

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

WEBS

2x4 SP No.3(flat)

BRACING-TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

(size) 17=0-3-0, 11=Mechanical Max Grav 17=842(LC 1), 11=848(LC 1) REACTIONS.

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1733/0, 3-4=-2808/0, 4-5=-2808/0, 5-6=-3022/0, 6-7=-3022/0, 7-8=-3022/0, TOP CHORD

BOT CHORD

16-17=0/1049, 15-16=0/2394, 14-15=0/3053, 13-14=0/3022, 12-13=0/2502, 11-12=0/1241

2-17=-1313/0, 2-16=0/891, 3-16=-861/0, 3-15=0/528, 5-15=-314/0, 5-14=-267/352,

9-11=-1463/0, 9-12=0/821, 8-12=-822/0, 8-13=0/841, 7-13=-375/0

NOTES-

WEBS

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

MSVIPPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	F444	10.4700
J0620-2873	F6	FLOOR	1	1	E144	184730
		1 (1) (1) (1) (1) (1) (1) (1) (1) (1) (1			Job Reference (optional)	

Comtech, Inc, Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:30 2020 Page 1 $ID: BoL? hgXgIYpqwdOiyUmcQyz41 fz-BoFfA_ko3HT? 8r58wD7fCvANAnQ_x?GHO7gWiFz8N6parter for the control of the co$

0-1-8

HI-3-0

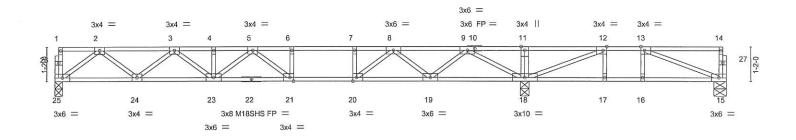
2-0-0

1-9-12 2-6-0 1-1-12 2-6-0

Structural wood sheathing directly applied or 5-8-12 oc purlins,

Rigid ceiling directly applied or 2-2-0 oc bracing.

except end verticals.



1	9-3-6 9-3-6	10-6-12 1-3-6	15-9-12 5-3-0			22-7-0 6-9-4	
Plate Offsets (X,Y)	- [12:0-1-8,Edge], [13:0-1-8,Edge], [20:0	-1-8,Edge], [21:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.83 BC 0.93 WB 0.44	DEFL. in (loc) Vert(LL) -0.24 21-23 Vert(CT) -0.33 21-23 Horz(CT) 0.04 15	l/defl >783 >568 n/a	L/d 480 360 n/a	PLATES MT20 M18SHS	GRIP 244/190 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S				Weight: 113 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

WEBS

TOP CHORD 2x4 SP No.1(flat) 2x4 SP No.1(flat) **BOT CHORD**

2x4 SP No.3(flat)

(size) 25=0-3-0, 18=0-3-8, 15=0-5-0

Max Uplift 15=-52(LC 3)

Max Grav 25=802(LC 10), 18=1452(LC 1), 15=308(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1634/0, 3-4=-2617/0, 4-5=-2617/0, 5-6=-2689/0, 6-7=-2689/0, 7-8=-2689/0,

8-9=-1399/0, 9-11=0/1071, 11-12=0/1075, 12-13=-444/311

BOT CHORD 24-25=0/996, 23-24=0/2251, 21-23=0/2810, 20-21=0/2689, 19-20=0/2082, 18-19=0/726,

17-18=-311/444, 16-17=-311/444, 15-16=-311/444 2-25=-1248/0, 2-24=0/830, 3-24=-803/0, 3-23=0/467, 5-21=-347/212, 9-18=-1758/0,

9-19=0/906, 8-19=-935/0, 8-20=0/916, 7-20=-404/0, 12-18=-1130/0, 13-15=-468/336

NOTES-

WEBS

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 1.5x3 MT20 unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.



June 8,2020

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters and properly design parameters and properly design parameters and properly design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucallapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/PH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply		
J0620-2873	F7	FLOOR	3	1		E14484731
					Job Reference (optional)	

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:31 2020 Page 1 ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-f_p1NJIQpbbrl?gKUweul6igDBqcgTNRdnP3Fiz8N6o

Structural wood sheathing directly applied or 6-0-0 oc purlins,

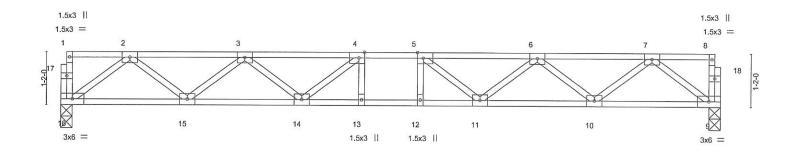
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

0-1-8

H | 1-3-0 | 1-2-0

0-1-8 Scale = 1:24.3



			14-5-0					1
			14-5-0					
Plate Offsets (X,Y)	[4:0-1-8,Edge], [5:0-1-8,Edge]							
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	BC 0.	0.59 Vert(CT) -	-0.12 12-13 >	/defl L -999 48 -999 36	30 50	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S	-S				Weight: 73 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

WEBS 2x4 SP No.3(flat)

(size) 16=0-3-0, 9=0-3-0

Max Grav 16=773(LC 1), 9=773(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1570/0, 3-4=-2405/0, 4-5=-2647/0, 5-6=-2405/0, 6-7=-1570/0

BOT CHORD 15-16=0/955, 14-15=0/2151, 13-14=0/2647, 12-13=0/2647, 11-12=0/2647, 10-11=0/2151,

9-10=0/955 WEBS 7-9=-1195/0,

7-9=-1195/0, 7-10=0/801, 6-10=-756/0, 6-11=0/386, 5-11=-454/0, 2-16=-1195/0,

2-15=0/801, 3-15=-756/0, 3-14=0/386, 4-14=-454/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





818 Soundside Road Edenton, NC 27932 Job Truss Truss Type Qty E14484732 J0620-2873 F8 FLOOR GIRDER 2 | **Z** | Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:32 2020 Page 1

Comtech, Inc. Fayetteville, NC - 28314,

ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-7ANPbfl2avjiN8FW2e97HKFr?bCVPpuasR9cn8z8N6n

0-1-8 H - 1-3-0

0-11-0

0-1-8 Scale = 1:24.7

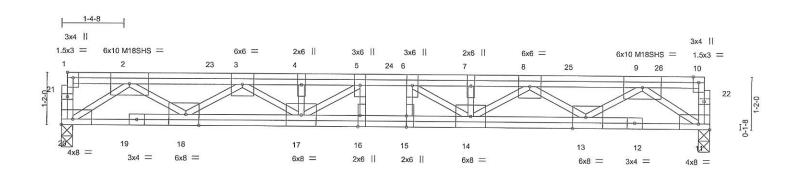


Plate Offsets (X,Y)	9-0-8 9-0-8 [1:Edge,0-1-8], [11:Edge,0-1-8], [13:0-3		, [16:0-3-0,Edge], [18:0-3	14-5-0 5-4-8 3:0-3-8.Edge], [20:Edge,0-1-8]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.30 BC 0.48 WB 0.81 Matrix-S	DEFL. in Vert(LL) -0.19 Vert(CT) -0.26 Horz(CT) 0.06	(loc) I/defl 15 >903	L/d 480 360 n/a	PLATES MT20 M18SHS Weight: 221 lb	GRIP 244/190 244/190 FT = 20%F, 11%E	

LUMBER-

TOP CHORD 2x4 SP 2400F 2.0E(flat) **BOT CHORD** 2x4 SP 2400F 2.0E(flat) WEBS

2x4 SP No.3(flat) *Except* 2-18,9-13: 2x4 SP No.2(flat) BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 20=0-3-0, 11=0-3-0

Max Grav 20=4019(LC 1), 11=4153(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

10-11=-255/0, 2-3=-9312/0, 3-4=-14910/0, 4-5=-14910/0, 5-6=-15908/0, 6-7=-14837/0, 7-8=-14837/0, 8-9=-9126/0

BOT CHORD 18-20=0/5647, 17-18=0/12930, 16-17=0/15908, 15-16=0/15908, 14-15=0/15908, 13-14=0/12661, 11-13=0/5549

2-20=-6930/0, 2-18=0/4544, 3-18=-4487/0, 3-17=0/2416, 4-17=-649/0, 5-17=-1297/0,

9-11=-6793/0, 9-13=0/4437, 8-13=-4384/0, 8-14=0/2654, 7-14=-690/0, 6-14=-1406/0

NOTES-

WEBS

- 1) Fasten trusses together to act as a single unit as per standard industry detail, or loads are to be evenly applied to all plies.
- 2) Unbalanced floor live loads have been considered for this design.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1026 lb down at 1-4-8, 1026 lb down at 3-4-8, 1026 lb down at 5-4-8, 971 lb down at 7-4-8, 1026 lb down at 9-4-8, and 1026 lb down at 11-4-8, and 1027 lb down at 13-4-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 11-20=-10, 1-10=-100

Concentrated Loads (lb)

Vert: 2=-946(F) 4=-946(F) 7=-946(F) 23=-946(F) 24=-946(F) 25=-946(F) 26=-953(F)



June 8,2020

818 Soundside Road

ob		Truss	Truss Type		Qty	Ply	F1448	84733
0620-2873		F9	FLOOR		3	1	Job Reference (optional)	
Comtech, Inc,	Fayettev	ville, NC - 28314,		ID:BoL	.8 hgXgIYp?۔	330 s May qwdOiyUn	6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:33 2020 Page ncQyz41fz-bNxoo?mgLCrZ?lqicLgMqXo?_?c88QBj44uAJaz8N6	im
	0-1-8	2-6-0		0-10-8	-1		Scale =	= 1:12.4
			3x4 =	=			50 m W	
	11.5x3	1	2		3 3x4 =		4 3x4	mat
1-2-0		3x4 =		•				1-2-0
	8		7 1.5	ix3 II	1.5x3 6		5	
	3x6 =	=					3x6 =	

Ply

Qty

Plate Offsets (X,Y)	[2:0-1-8,Edge], [3:0-1-8,Edge], [9:0-1-8,	0-1-8]		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.35 BC 0.20 WB 0.17 Matrix-S	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) -0.03 5-6 >999 480 MT20 244/190 Vert(CT) -0.03 5-6 >999 360 Horz(CT) 0.01 5 n/a n/a Weight: 35 lb FT = 20	%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1(flat) TOP CHORD BOT CHORD 2x4 SP No.1(flat)

2x4 SP No.3(flat) WEBS

REACTIONS. (size) 8=0-5-0, 5=Mechanical

Max Grav 8=344(LC 1), 5=351(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-578/0

BOT CHORD 2-8=-613/0, 3-5=-620/0 **WEBS**

NOTES-

7-8=0/578, 6-7=0/578, 5-6=0/578

- 1) Unbalanced floor live loads have been considered for this design.
- Plates checked for a plus or minus 1 degree rotation about its center.
 Refer to girder(s) for truss to truss connections.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss well and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see MSI/TPI fuality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932

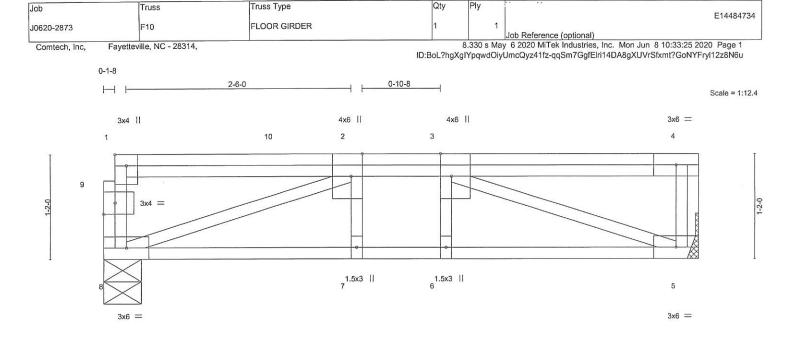


Plate Off	sets (X,Y)	[1:Edge,0-1-8], [2:0-3-0,E	dge], [3:0-3-0	,Edge], [9:0-	1-8,0-1-8]	6-7-8						
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.22	Vert(LL)	-0.02	7-8	>999	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	ВС	0.22	Vert(CT)	-0.03	7-8	>999	360		
BCLL	0.0	Rep Stress Incr	NO	WB	0.22	Horz(CT)	0.01	5	n/a	n/a		
BCDL	5.0	Code IRC2015/TF	PI2014	Matri	x-S						Weight: 44 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

6-7-8

LUMBER-

REACTIONS.

2x4 SP No.1(flat)

TOP CHORD **BOT CHORD** 2x4 SP No.1(flat)

WEBS 2x4 SP No.3(flat)

(size) 8=0-5-0, 5=Mechanical

Max Grav 8=440(LC 1), 5=403(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-773/0

7-8=0/773, 6-7=0/773, 5-6=0/773 **BOT CHORD**

2-8=-813/0, 3-5=-822/0 **WEBS**

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 149 lb down at 1-11-8, and 101 Ib down at 3-1-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 5-8=-10, 1-4=-100

Concentrated Loads (lb)

Vert: 2=-73(B) 10=-75(B)



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

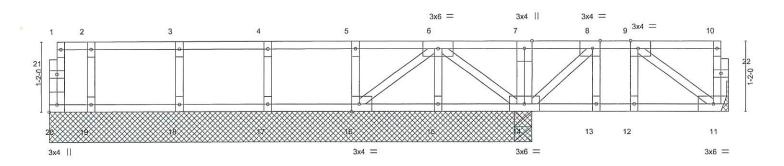
<u>k</u> WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTel® connectors. This case only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer. Bracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Flate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	G	lty	Ply		E14484735
J0620-2873	F11	FLOOR	1	fi S	1	Job Reference (optional)	£14404760
Comtech, Inc, Fayet	teville, NC - 28314,		ID:Bol		330 s May	y 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:2 UmcQyz41fz-J108KchH?3zZfDnMhN2j23?stAGi?H	
0-1-8							
⊢ I ⁰⁻⁴⁻⁸ I ⊢	1-4-0 1-4	-0 1-4-0	-l 1-3-0	4			0 ₁ 1-8 Scale = 1:18.4



—				7-10-8 7-10-8						-0-0 -1-8	11-3-0 3-3-0	
Plate Offse	ets (X,Y)	[8:0-1-8,Edge], [9:0-1-8,E	Edge], [16:0-1-		Edge,0-1-8]							
LOADING TCLL TCDL BCLL	(psf) 40.0 10.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.00 1.00 YES	CSI. TC BC WB	0.08 0.06 0.04	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 0.00	(loc) 12 12	l/defl >999 >999 n/a	L/d 480 360 n/a	PLATES MT20	GRIP 244/190
BCDL	5.0	Code IRC2015/TR		Matri		11012(01)	0.00		IIIa	11/4	Weight: 59 lb	FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.1(flat)

2x4 SP No.1(flat) BOT CHORD

2x4 SP No.3(flat) **WEBS**

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 8-0-0 except (jt=length) 11=Mechanical.

(lb) - Max Uplift All uplift 100 lb or less at joint(s) 20

Max Grav All reactions 250 lb or less at joint(s) 11, 15, 16, 17, 18, 19 except 14=278(LC 15), 14=265(LC 1)

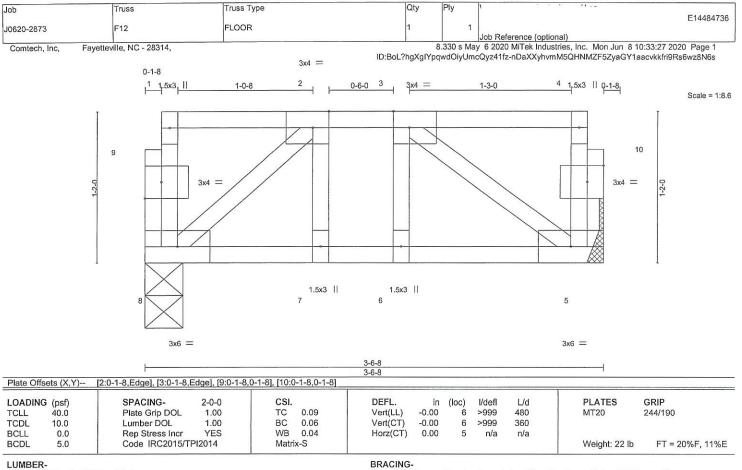
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 1.5x3 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.



818 Soundside Road Edenton, NC 27932



TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1(flat) **BOT CHORD** 2x4 SP No.1(flat)

WEBS 2x4 SP No.3(flat)

> (size) 8=0-3-8, 5=Mechanical Max Grav 8=175(LC 1), 5=175(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





Structural wood sheathing directly applied or 3-6-8 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

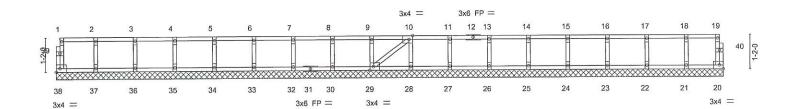
Job	Truss	Truss Type	Qty	Ply		E14484737
J0620-2873	кw	FLOOR SUPPORTED GABL	1	1	Job Reference (optional)	
					COCCATTAL Laboration Laboration Col	

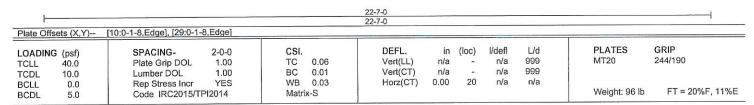
Fayetteville, NC - 28314, Comtech, Inc,

0-118

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:34 2020 Page 1 ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-4ZVA?LnI6WzQcSPv93CbMIKEEO?OtvbtJkejs0z8N6I

Scale = 1:37.6





LUMBER-

TOP CHORD 2x4 SP No.1(flat)

BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.3(flat) WEBS

2x4 SP No.3(flat) OTHERS

BRACING-

Structural wood sheathing directly applied or 6-0-0 oc purlins, TOP CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD**

REACTIONS. All bearings 22-7-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 38, 20, 37, 36, 35, 34, 33, 32, 30, 29, 28, 27, 26, 25, 24, 23, 22, 21

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/PH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Roa Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E14484738
J0620-2873	KW1	FLOOR SUPPORTED GABL	1	1	Job Reference (optional)

Comtech, Inc,

Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:35 2020 Page 1 ID: BoL? hgXgIYpqwdOiyUmcQyz41fz-YI3YDhowtq5HEcz5jmjqvytPxoLdcMq0YONHOTz8N6k

Scale: 3/8"=1"

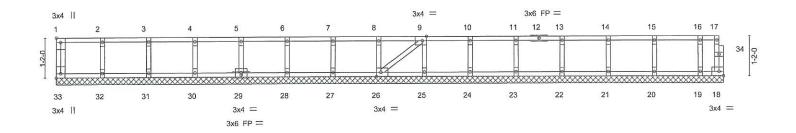


Plate Offsets (X,Y)	[1:Edge,0-1-8], [9:0-1-8,Edge], [26:0-1-	6,Eugej, [55.Euge,0-1-6]							
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.01	Vert(CT)	n/a	2	n/a	999		
BCLL 0.0	Rep Stress Incr YES	WB 0.03	Horz(CT)	0.00	18	n/a	n/a	1	
3CDL 5.0	Code IRC2015/TPI2014	Matrix-S						Weight: 84 lb	FT = 20%F, 11%E

2x4 SP No.1(flat)

BOT CHORD **WEBS** 2x4 SP No.3(flat)

2x4 SP No.3(flat) OTHERS

except end verticals. **BOT CHORD**

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 19-4-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 33, 18, 32, 31, 30, 29, 28, 27, 26, 25, 24, 23, 22, 21, 20, 19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see _ ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	E14484739
J0620-2873	KW2	FLOOR SUPPORTED GABL	1	1	L14404703
20020 20.0		TOURNA COSCIO SEA CROME TOUGHT STATE OF THE CONTROL OF MATERIAL AND			Job Reference (optional)

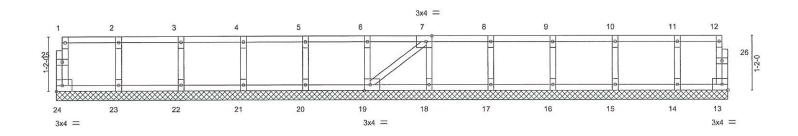
Comtech, Inc,

Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:37 2020 Page 1 ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-U8AleNpBPRL?Tw7UrBII_NylUc154GKJ?isNSLz8N6i

0-1-8

0-1-8 Scale: 1/2"=1"



	5		14-5-0						
Plate Offsets (X,Y)	[7:0-1-8,Edge], [19:0-1-8,Edge]		14-5-0						
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.06 BC 0.01 WB 0.03	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 13	I/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
3CDL 5.0	Code IRC2015/TPI2014	Matrix-S	J. 202					Weight: 63 lb	FT = 20%F, 11%l
LUMBER- TOP CHORD 2x4 SF	P No.1(flat)		BRACING- TOP CHORD			al wood		rectly applied or 6-0-0	oc purlins,

WEBS 2x4 SP No.3(flat)

2x4 SP No.3(flat) **OTHERS**

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 14-5-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 24, 13, 23, 22, 21, 20, 19, 18, 17, 16, 15, 14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

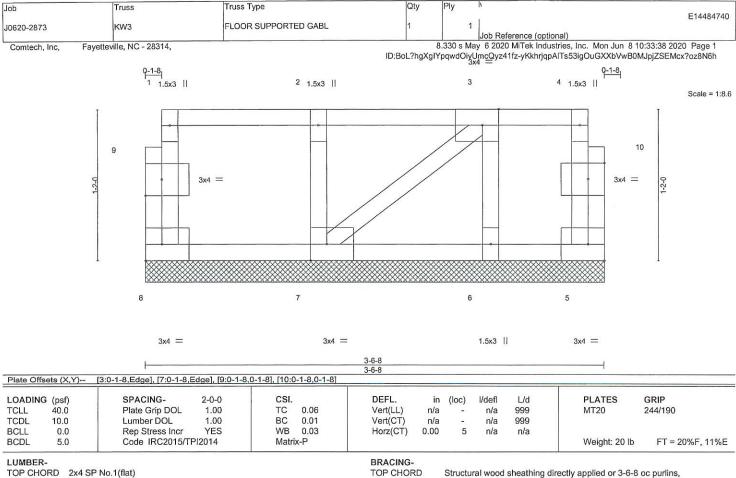
- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTeke connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/dor chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.1(flat) **BOT CHORD** 2x4 SP No.1(flat) **WEBS** 2x4 SP No.3(flat)

OTHERS 2x4 SP No.3(flat)

REACTIONS. All bearings 3-6-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Plates checked for a plus or minus 1 degree rotation about its center.
- 2) Gable requires continuous bottom chord bearing.
- 3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 4) Gable studs spaced at 1-4-0 oc.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



🔼 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

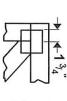
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly anage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



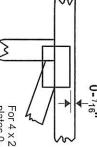
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.

Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

œ

6

This symbol indicates the required direction of slots in connector plates.

Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

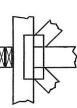
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only

Industry Standards:

ANSI/TPI1: National Design Specification for Metal
Plate Connected Wood Truss Construction.
Design Standard for Brazing

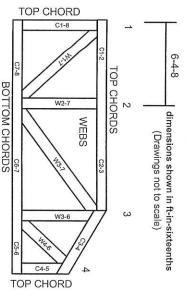
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information,

Guide to Good Practice for Handling,

Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

© 2012 MiTek® All Rights Reserved





MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

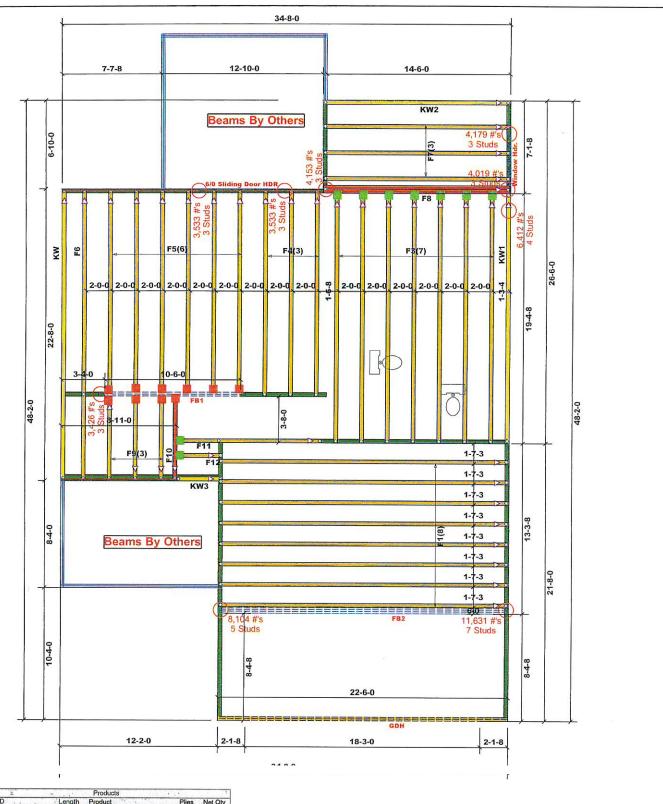
General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- . Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.

4

- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANS/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



		Products		Sand and
PlotID	Length	Product	Plies	Net Qty
6/0 Sliding Door HDR	7-0-0	1-3/4"x 9-1/4" LVL Kerto-S	2	2
GDH	23-0-0	1-3/4"x 14" LVL Kerto-S	2	2
FB1	12-0-0	1-3/4"x 14" LVL Kerto-S	2	2
Window Hdr.	7-0-0	1-3/4"x 14" LVL Kerto-S	2	2
FB2	23-0-0	1-3/4"x 23-7/8" LVL Kerto-S	3	3

■= USP HUS410 2x Hanger

■ = USP MSH422 2x Strap Hanger

Truss Placement Plan SCALE: NTS

A = Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

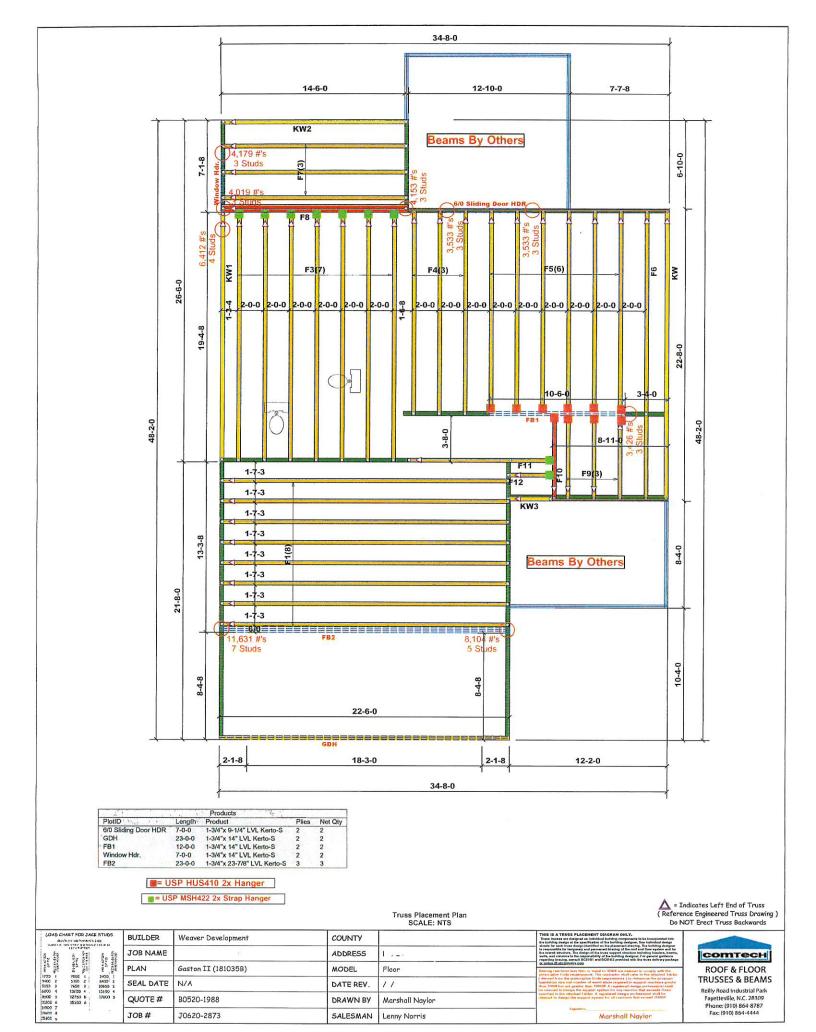
LOAD CHART FOR JACK STUDS	BUILDER	Weaver Development	COUNTY	
ECHOCOCOTOLS	JOB NAME	Lot 2 St. Andrews	ADDRESS	
A STATE OF S	PLAN		MODEL	Floor
1700 1 2000 1 3400 1 3400 2 5500 2 5800 2 5100 3 7650 3 10200 5 6800 4 10700 4 11600 4	SEAL DATE	N/A	DATE REV.	11
81900 5 12759 6 17000 6 10200 6 15500 6	QUOTE#	B0520-1988	DRAWN BY	Marshall Naylor
1360 7 1360 4	JOB#	J0620-2873	SALESMAN	Lenny Norris

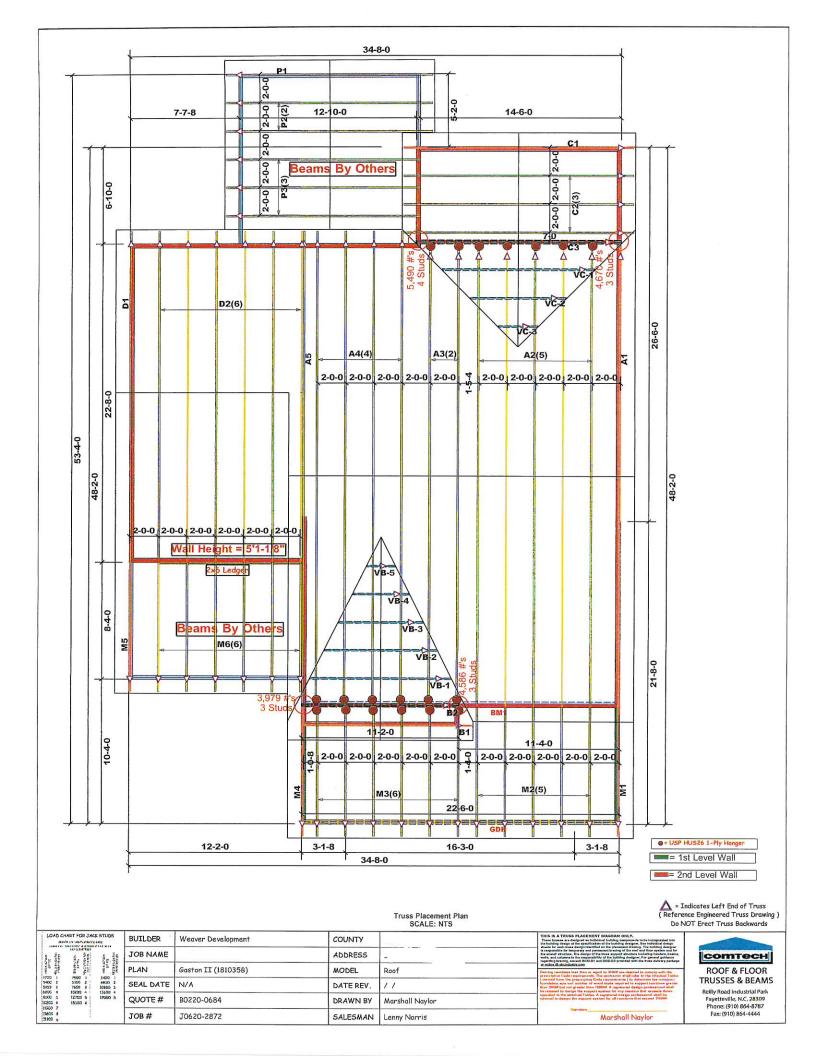
_	THIS IS A TRUES PLACEMENT DIAGRAM ON LY. These breases on designed as bendered substrage components to be incorporated who there in the components of the incorporated with the components of the incorporated with the components of the confidence of the components of the components of the components of the components of the confidence of the components of the components of the components of the components of the confidence of the components o
_	Bening treatment less throw or squal in 2009 in exhemat in comply soft the preceptive facilities requirements. He remote that facilities the schedule filled (chieved from the principles Codes (approximated) has determine the minimum formation from the principles Codes (approximated) has determine the own desire facilities for approximate the principles to approximate the principles (approximate approximate the principles (approximated) approximate the desire of the principles (approximated approximated appr

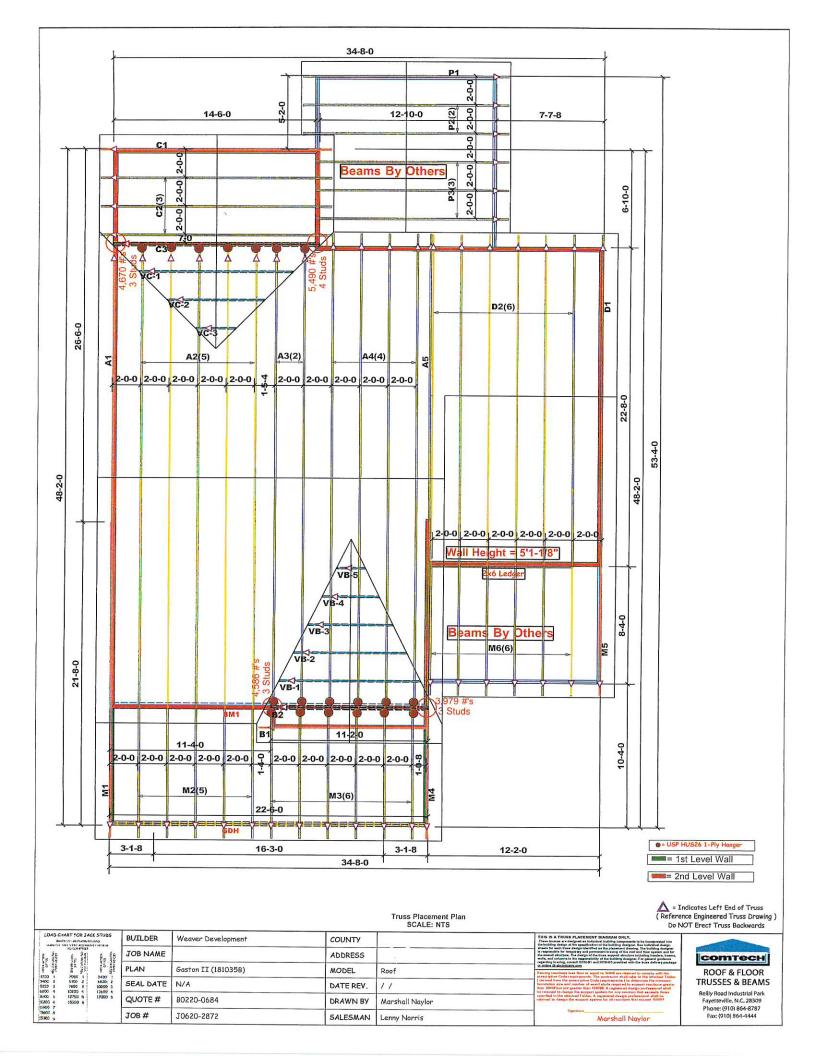
Marshall Naylor

ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444







isDesign

Client: Project:

Address:

Weaver Development

Gaston II (181035B)

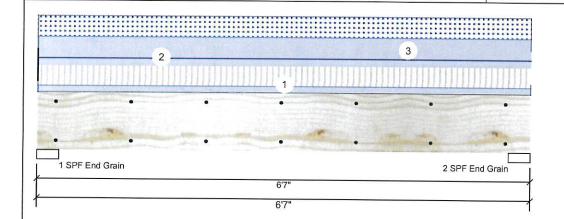
Date: 6/24/2020

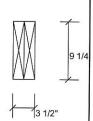
Input by: Christine Shivy Job Name: 6/0 Sliding Door HDR

Project #:

6/0 Sliding Door HDR Kerto-S LVL 1.750" X 9.250"

2-Ply - PASSED





Page 1 of 1

Member	Information
--------	-------------

Type: Girder Plies: 2 Moisture Condition: Dry

Deflection LL: 480 Deflection TL: 360 Importance:

Temperature:

Normal Temp <= 100°F

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions	UNPAT	TERNED	lb	(Uplift)	
-----------	-------	--------	----	----------	--

Brg	Live	Dead	Snow	Wind	Const
1	1060	1903	1113	0	0
2	1060	1903	1113	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5033 ft-lb	3'3 1/2"	14423 ft-lb	0.349 (35%)	D+0.75(L+S)	L
Unbraced	5033 ft-lb	3'3 1/2"	10451 ft-lb	0.482 (48%)	D+0.75(L+S)	L
Shear	2459 lb	1'	7943 lb	0.310 (31%)	D+0.75(L+S)	L
LL Defl inch	0.042 (L/1741)	3'3 1/2"	0.153 (L/480)	0.280 (28%)	0.75(L+S)	L
TL Defl inch	0.092 (L/803)	3'3 1/2"	0.204 (L/360)	0.450 (45%)	D+0.75(L+S)	L

Bearings

ı	Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
	1 - SPF End Grain	3.500"	33%	1903 / 1629	3533	L	D+0.75(L+S)
	2 - SPF End Grain	3.500"	33%	1903 / 1629	3533	L	D+0.75(L+S)

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

2 Refer to last page of calculations for fasteners required for specified loads.
3 Girders are designed to be supported on the bottom edge only.
4 Top loads must be supported equally by all plies.
5 Top broad at bearings

	The string of table backs on	onigic pry width.								
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	108 PLF	322 PLF	0 PLF	0 PLF	0 PLF	
2	Uniform			Тор	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Load
3	Uniform			Тор	338 PLF	0 PLF	338 PLF	0 PLF	0 PLF	
	Self Weight				7 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation

 LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





Client:

Weaver Homes

Project:

Address:

Gaston II (181035B)

Date:

6/24/2020

Level: Level

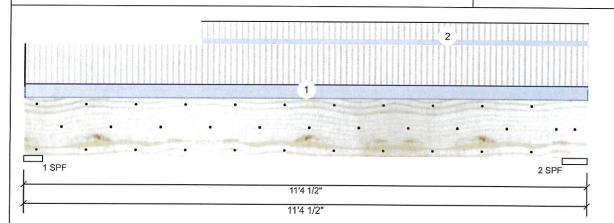
Input by: Christine Shivy Job Name:

Project #:

Kerto-S LVL FB₁

1.750" X 14.000"

2-Ply - PASSED



Floor

ASD

No

IBC 2012

Not Checked



Page 1 of 1

Member I	nformat	ion
----------	---------	-----

Girder Type: Plies: 2

Moisture Condition: Dry Deflection LL: 480 Deflection TL:

Importance: Temperature:

360 Normal

Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const	
1	2129	771	0	0	0	
2	2523	904	0	0	0	

Bearings

Bearing	Length	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.	
1-SPF	4.500"	43%	771 / 2129	2899	L	D+L	
2-SPF	6.000"	38%	904 / 2523	3426	L	D+L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8168 ft-lb	5'9 3/16"	26999 ft-lb	0.303 (30%)	D+L	L
Unbraced	8168 ft-lb	5'9 3/16"	10258 ft-lb	0.796 (80%)	D+L	L
Shear	2941 lb	9'9 1/4"	10453 lb	0.281 (28%)	D+L	L
LL Defl inch	0.090 (L/1419)	5'8 3/16"	0.266 (L/480)	0.340 (34%)	L	L
TL Defl inch	0.122 (L/1044)	5'8 3/16"	0.354 (L/360)	0.340 (34%)	D+L	L

Application:

Design Method:

Building Code:

Load Sharing:

Deck:

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	106 PLF	318 PLF	0 PLF	0 PLF	0 PLF	F5
2	Part. Uniform	3-6-8 to 11-4-8		Тор	44 PLF	132 PLF	0 PLF	0 PLF	0 PLF	F9
	Self Weight				11 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loade.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be out or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood

301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 2/26/2023



Client:

Weaver Homes

Project:

Address: Gaston II (181035B) Date:

6/24/2020

Page 1 of 1

Marshall Naylor Input by:

Job Name: BM1 Project #:

Kerto-S LVL FB2

1.750" X 24.000"

3-Ply - PASSED

Level: Level

Reactions UNPATTERNED Ib (Uplift)

Cap. React D/L lb

73% 6536 / 5095

51% 4429 / 3676

Total Ld. Case

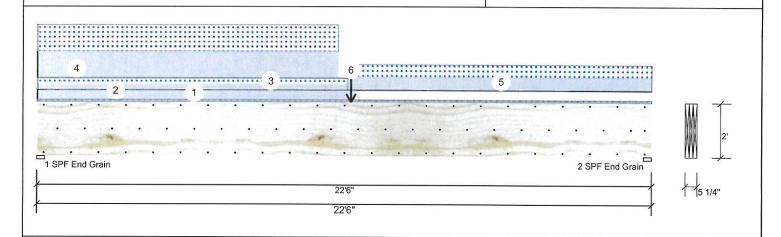
11631 L

8104 L

Ld. Comb.

D+S

D+S



							(-)		
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const
Plies:	3	Design Method:	ASD	1	225	6536	5095	0	0
Moisture Condition:	Dry	Building Code:	IBC 2012	2	225	4429	3676	0	0
Deflection LL:	480	Load Sharing:	Yes					ä	Ü
Deflection TL:	360	Deck:	Not Checked	1					
Importance:	Normal								
Temperature:	Temp <= 100°F								

Bearings Bearing Length

End Grain 2-SPF 3.500"

End Grain

1-SPF 3.500"

Analysis Re	Analysis Results										
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case					
Moment	65477 ft-lb	11'5 3/4"	131295 ft-lb	0.499 (50%)	D+S	L					
Unbraced	65477 ft-lb	11'5 3/4"	65903 ft-lb	0.994 (99%)	D+S	L					
Shear	10093 lb	2'2 5/8"	30912 lb	0.327 (33%)	D+S	L					
II Define	0.206 (1.4474)	4414 44 /4 (1)	0.550 (1.1400)	0 440 (440()	_						

LL Defl inch 0.226 (L/1171) 11'1 11/16" 0.552 (L/480) 0.410 (41%) S TL Defl inch 0.501 (L/528) 11' 7/8" 0.735 (L/360) 0.680 (68%) D+S

Design Notes

Member Information

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 3'11 5/8" o.c.
- 6 Bottom braced at bearings.

	/ Lateral siende	rness ratio based on s	ingle ply width.									
	ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	_
	1	Tie-In	0-0-0 to 22-6-0	0-6-0	Far Face	15 PSF	40 PSF	0 PSF	0 PSF	0 PSF	1' Floor	
:	2	Part. Uniform	0-0-0 to 11-7-8		Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall	
,	3	Part. Uniform	0-0-0 to 11-4-0		Near Face	79 PLF	0 PLF	79 PLF	0 PLF	0 PLF	M2	
4	4	Part. Uniform	0-0-0 to 11-0-0		Тор	341 PLF	0 PLF	341 PLF	0 PLF	0 PLF	A2	
!	5	Part. Uniform	11-4-0 to 22-6-0		Near Face	164 PLF	0 PLF	164 PLF	0 PLF	0 PLF	МЗ	
(6	Point	11-5-12		Тор	2293 lb	0 lb	2293 lb	0 lb	0 lb	B2	
		Self Weight				28 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Identifing & Installation

 LVI beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





Front GDH

Client:

Weaver Homes

Project:

Address:

Gaston II (181035B)

Date:

6/24/2020

Page 1 of 1

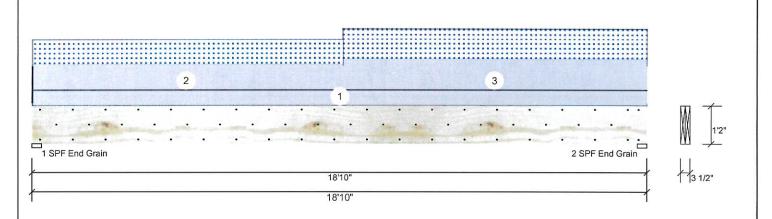
Input by: Christine Shivy Job Name: Front GDH

Project #:

1.750" X 14.000" Kerto-S LVL

2-Ply - PASSED

Level: Level



Member	Information
--------	-------------

Type: Girder Plies: 2

Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360

Importance: Temperature: Normal

Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const	
1	0	1619	951	0	0	
2	0	1720	1053	0	0	
1						

Floor

ASD

No

IBC 2012

Not Checked

Application:

Design Method:

Building Code:

Load Sharing:

Deck:

Analysis Results

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	11981 ft-lb	9'8 7/8"	31049 ft-lb	0.386 (39%)		L
	Unbraced	11981 ft-lb	9'8 7/8"	12012 ft-lb	0.997 (100%)	D+S	L
	Shear	2347 lb	17'5 1/4"	12021 lb	0.195 (20%)	D+S	L
	LL Defl inch	0.181 (L/1218)	9'6 3/16"	0.459 (L/480)	0.390 (39%)	S	L
	TL Defl inch	0.483 (L/457)	9'5 13/16"	0.612 (L/360)	0.790 (79%)	D+S	L

, mining one	, 101001			o apaonty	00	0000
Moment	11981 ft-lb	9'8 7/8"	31049 ft-lb	0.386 (39%)	D+S	L
Unbraced	11981 ft-lb	9'8 7/8"	12012 ft-lb	0.997 (100%)	D+S	L
Shear	2347 lb	17'5 1/4"	12021 lb	0.195 (20%)	D+S	L
LL Defl inch	0.181 (L/1218)	9'6 3/16"	0.459 (L/480)	0.390 (39%)	S	L
TL Defl inch	0.483 (L/457)	9'5 13/16"	0.612 (L/360)	0.790 (79%)	D+S	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 8'9" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

Bearings													
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.							
1 - SPF End Grain	3.500"	24%	1619 / 951	2570	L	D+S							
2 - SPF End Grain	3.500"	26%	1720 / 1053	2773	L	D+S							

		and by man									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall	
2	Part. Uniform	0-0-0 to 9-6-0		Тор	96 PLF	0 PLF	96 PLF	0 PLF	0 PLF	M2	
3	Part. Uniform	9-6-0 to 18-10-0		Тор	117 PLF	0 PLF	117 PLF	0 PLF	0 PLF	M3	
	Self Weight				11 PLF						

Notes

NOLES
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LANGLING & Installation . LVL beams must not be cut or drilled . Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals . Damaged Beams must not be used . Design assumes top edge is laterally restrained . Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850

www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





Client:

Weaver Development

Project:

Address:

Gaston II (181035B)

Date:

6/24/2020

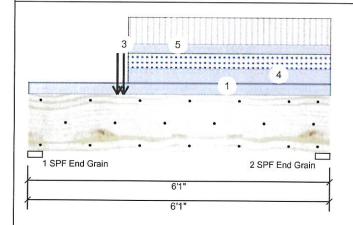
Input by: Christine Shivy Job Name: Window Hdr.

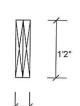
Level: Level

Project #:

Window Hdr. **Kerto-S LVL** 1.750" X 14.000"

2-Ply - PASSED





Const

0

0

0

0

Page 1 of 1

Member Information	n
--------------------	---

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application: Floor Design Method: ASD

Building Code: IBC/IRC 2015 Load Sharing: No

Deck: Not Checked

Reaction	ons UNPAT	TERNED I	(Uplift)		
Brg	Live	Dead	Snow	Wind	-

3110

2098

Bearings

1

2

2560

1638

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	60%	3110 / 3302	6412	L	D+0.75(L+S)
2 - SPF End Grain	3.500"	39%	2098 / 2082	4179	L	D+0.75(L+S)

1843

1137

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10167 ft-lb	1'11"	31049 ft-lb	CHICAGO CONTRACTOR	D+0.75(L+S)	
Unbraced	10167 ft-lb	1'11"	17799 ft-lb	0.571 (57%)	D+0.75(L+S)	L
Shear	5480 lb	1'4 3/4"	10453 lb	0.524 (52%)	D+L	L
LL Defl inch	0.027 (L/2482)	2'5 1/4"	0.141 (L/480)	0.190 (19%)	0.75(L+S)	L
TL Defl inch	0.053 (L/1280)	2'5 11/16"	0.188 (L/360)	0.280 (28%)	D+0.75(L+S)	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

1900	85 V32-007	sangra pay arream								
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
2	Point	1-9-8		Тор	1005 lb	3014 lb	0 lb	0 lb	0 lb	F8
3	Point	1-11-0		Тор	2335 lb	0 lb	2335 lb	0 lb	0 lb	C3
4	Part. Uniform	2-0-0 to 6-1-0		Тор	158 PLF	0 PLF	158 PLF	0 PLF	0 PLF	C2
5	Part. Uniform	2-0-0 to 6-1-0		Тор	97 PLF	290 PLF	0 PLF	0 PLF	0 PLF	F7
	Self Weight				11 PLF					A 2 2 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVL beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC





RE: J0620-2872 Weaver **Trenco**

818 Soundside Rd Edenton, NC 27932

Site Information:

Customer:

Project Name: J0620-2872

Lot/Block:

Model:

Address:

Subdivision:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2009/TPI2007

Design Program: MiTek 20/20 8.3

Wind Code: ASCE 7-05 Roof Load: 40.0 psf

Wind Speed: 110 mph Floor Load: N/A psf

This package includes 29 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	E14484697	A1	6/24/2020	21	E14484717	P3	6/24/2020
2	E14484698	A2	6/24/2020	22	E14484718	VB-1	6/24/2020
3	E14484699	A3	6/24/2020	23	E14484719	VB-2	6/24/2020
4	E14484700	A4	6/24/2020	24	E14484720	VB-3	6/24/2020
5	E14484701	A5	6/24/2020	25	E14484721	VB-4	6/24/2020
6	E14484702	B1	6/24/2020	26	E14484722	VB-5	6/24/2020
7	E14484703	B2	6/24/2020	27	E14484723	VC-1	6/24/2020
8	E14484704	C1	6/24/2020	28	E14484724	VC-2	6/24/2020
9	E14484705	C2	6/24/2020	29	E14484725	VC-3	6/24/2020
10	E14484706	C3	6/24/2020				
11	E14484707	D1	6/24/2020				
12	E14484708	D2	6/24/2020				
13	E14484709	M1	6/24/2020				
14	E14484710	M2	6/24/2020				
15	E14484711	M3	6/24/2020				
16	E14484712	M4	6/24/2020				
17	E14484713	M5	6/24/2020				
18	E14484714	M6	6/24/2020				
19	E14484715	P1	6/24/2020				
20	E14484716	P2	6/24/2020				

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

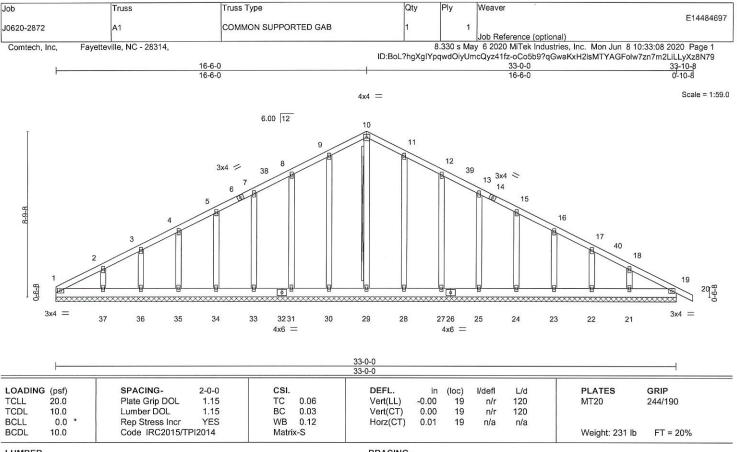
My license renewal date for the state of North Carolina is December 31, 2020

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



June 24, 2020



LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 OTHERS 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD WEBS Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

T-Brace: 2x4 SPF No.2 - 10-29

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c.,with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 33-0-0.

(lb) - Max Horz 1=-181(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except

37=-106(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 1, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22, 21, 19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 8-9=-89/266, 9-10=-109/319, 10-11=-109/319, 11-12=-89/267

NOTES-

1) Unbalanced roof live loads have been considered for this design.

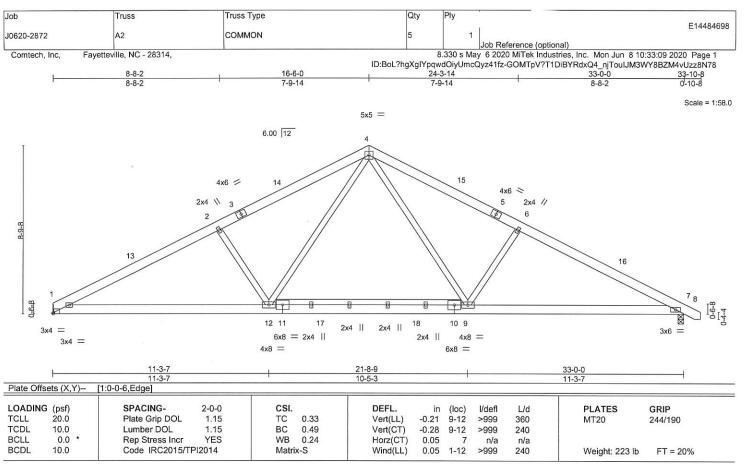
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-0-0 to 4-6-0, Exterior(2) 4-6-0 to 16-6-0, Corner(3) 16-6-0 to 20-10-13, Exterior(2) 20-10-13 to 33-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- Gable requires continuous botto
 Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=106.
 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



TRENCO

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucallapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/PH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1

2x4 SP No.2 WEBS

REACTIONS. (size) 1=Mechanical, 7=0-3-8

Max Horz 1=-113(LC 8)

Max Uplift 1=-79(LC 12), 7=-91(LC 13) Max Grav 1=1311(LC 1), 7=1364(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-2297/531, 2-4=-2064/545, 4-6=-2055/529, 6-7=-2283/512 TOP CHORD

1-12=-347/2015, 9-12=-110/1324, 7-9=-344/1965 **BOT CHORD**

WEBS 4-9=-142/850, 6-9=-495/297, 4-12=-145/865, 2-12=-509/305

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 33-8-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-8-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

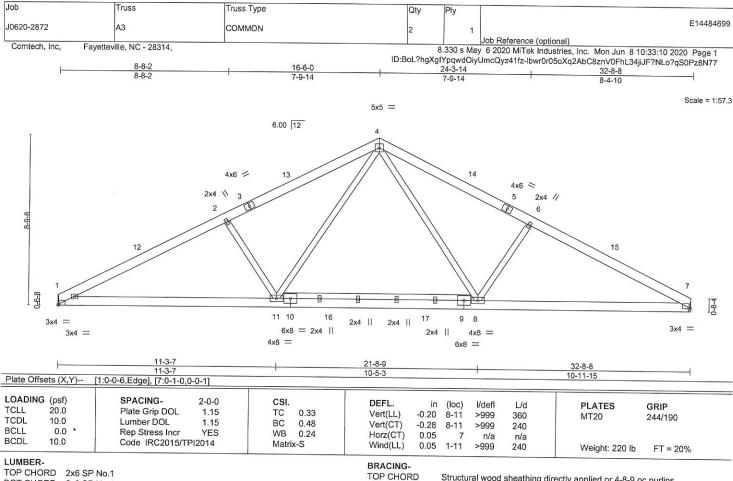


႔ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design, Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BOT CHORD

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1

WEBS 2x4 SP No.2

REACTIONS. (size) 1=Mechanical, 7=Mechanical Max Horz 1=-108(LC 8)

Max Uplift 1=-79(LC 12), 7=-78(LC 13)

Max Grav 1=1303(LC 1), 7=1303(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-2280/528, 2-4=-2048/543, 4-6=-2021/538, 6-7=-2242/521 TOP CHORD

BOT CHORD 1-11=-354/1997, 8-11=-116/1306, 7-8=-347/1920

WEBS 4-8=-139/822, 6-8=-473/295, 4-11=-145/866, 2-11=-509/305

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 32-7-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-8-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

June 8,2020

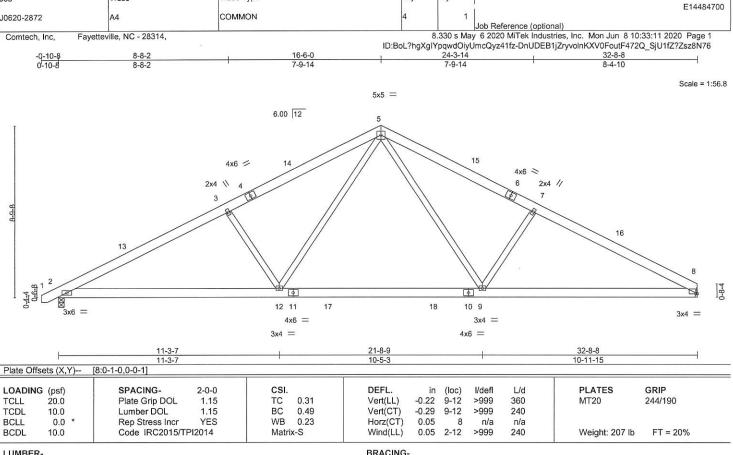
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

BOT CHORD

Qty

LUMBER-

Job

TOP CHORD 2x6 SP No.1 2x6 SP No.1

BOT CHORD 2x4 SP No.2 WEBS

REACTIONS.

(size) 2=0-3-8, 8=Mechanical

Max Horz 2=113(LC 9)

Truss

Truss Type

Max Uplift 2=-91(LC 12), 8=-78(LC 13) Max Grav 2=1352(LC 1), 8=1299(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2261/507, 3-5=-2051/524, 5-7=-2034/536, 7-8=-2238/519 TOP CHORD

BOT CHORD 2-12=-347/1992, 9-12=-113/1312, 8-9=-341/1925

5-9=-137/833, 7-9=-473/294, 5-12=-141/861, 3-12=-495/297

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 32-7-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



Structural wood sheathing directly applied or 4-9-10 oc purlins.

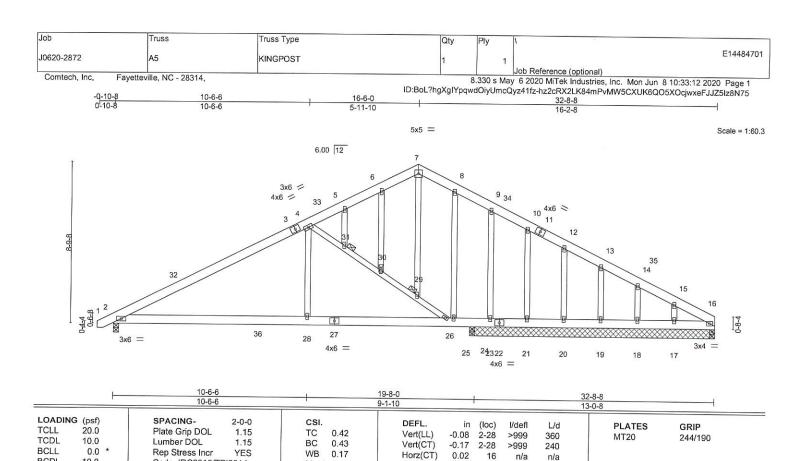
Rigid ceiling directly applied or 10-0-0 oc bracing.



🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





LUMBER-

BCDL

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1 **WEBS** 2x4 SP No.2

10 0

Wind(LL) BRACING- 0.07 2-28

TOP CHORD **BOT CHORD** JOINTS

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Weight: 246 lb

FT = 20%

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Brace at Jt(s): 29, 30, 31

>999

REACTIONS.

All bearings 13-4-0 except (jt=length) 2=0-3-8, 16=0-3-8, 16=0-3-8, 24=0-3-8,

(lb) -Max Horz 2=180(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 21, 20, 19, 18 except 2=-192(LC 12), 23=-491(LC 19),

17=-114(LC 13), 24=-297(LC 12)

Code IRC2015/TPI2014

Max Grav All reactions 250 lb or less at joint(s) 23, 21, 20, 19, 18, 17, 16, 16 except 2=848(LC 1), 24=1302(LC

Matrix-S

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-4=-1128/201, 4-5=-292/128, 14-15=-251/0, 15-16=-294/21

BOT CHORD

2-28=-203/943, 26-28=-202/948, 25-26=-17/266, 24-25=-17/266, 23-24=-17/266, 21-23=-17/266, 20-21=-17/266, 19-20=-17/266, 18-19=-17/266, 17-18=-17/266,

WEBS

4-31=-927/384, 30-31=-905/365, 29-30=-923/382, 26-29=-932/381, 4-28=0/455,

9-23=-263/134

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 32-6-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 21, 20, 19, 18 except (jt=lb) 2=192, 23=491, 17=114, 24=297.



June 8,2020

👠 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Ro

Job Truss Truss Type Qty E14484702 COMMON SUPPORTED GAB J0620-2872 B1 Job Reference (optional) Fayetteville, NC - 28314, 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:13 2020 Page 1 Comtech, Inc, $ID: BoL? hgXgIYpqwdOiyUmcQyz41fz-99c_es3z5SCd13xifv2jtJzf1xqFSPxnUz26dkz8N74\\$ 5-7-0 Scale = 1:40.1 5x5 =

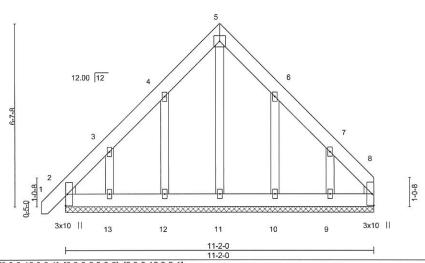


Plate Offsets (X,Y)--[2:0-0-6,0-0-6], [2:0-0-12,0-3-1], [8:0-0-6,0-0-6], [8:0-0-12,0-3-1] SPACING-CSL DEFL. L/d PLATES GRIP LOADING (psf) 2-0-0 (loc) I/defl Plate Grip DOL 0.03 -0.00 244/190 TCLL 20.0 1.15 TC Vert(LL) n/r 120 MT20 BC -0.00 TCDL 10.0 Lumber DOL 1 15 0.02 Vert(CT) n/r 120 BCLL 0.0 Rep Stress Incr YES WB 0.06 Horz(CT) 0.00 8 n/a n/a Code IRC2015/TPI2014 Weight: 93 lb FT = 20%**BCDI** 10.0 Matrix-S

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD 2x4 SP No.2 **OTHERS**

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. All bearings 11-2-0.

(lb) - Max Horz 2=187(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8 except 12=-130(LC 12), 13=-189(LC 12), 10=-126(LC 13),

9=-194(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 11, 12, 13, 10, 9, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-9-2 to 3-6-13, Exterior(2) 3-6-13 to 5-7-0, Corner(3) 5-7-0 to 9-11-13, Exterior(2) 9-11-13 to 11-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8 except (jt=lb) 12=130, 13=189, 10=126, 9=194
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

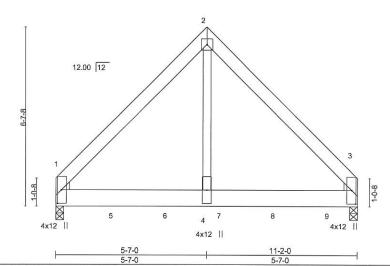
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty E14484703 B2 COMMON GIRDER J0620-2872 2 Job Reference (optional) Comtech, Inc. Fayetteville, NC - 28314, 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:14 2020 Page 1 ID: BoL? hgXgIYpqwdOiyUmcQyz41 fz-dM9MsC3bsmKUfCWvCdayQXVkxK1PBk0wjdog9Bz8N735-7-0 5-7-0 5x5 || Scale = 1:41.0



LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.40	Vert(LL)	-0.03	3-4	>999	360	MT20	244/190
CDL	10.0	Lumber DOL	1.15	BC	0.60	Vert(CT)	-0.06	3-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.52	Horz(CT)	0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-S	Wind(LL)	0.02	3-4	>999	240	Weight: 165 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x8 SP No.1 WEBS 2x4 SP No.2

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 3=0-3-8

Max Horz 1=-144(LC 23)

Max Uplift 1=-301(LC 9), 3=-265(LC 8)

Max Grav 1=4586(LC 1), 3=3979(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-3212/267, 2-3=-3207/266

BOT CHORD 1-4=-137/2118, 3-4=-137/2118 **WEBS** 2-4=-239/4237

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-6-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (|t=|b| 1=301, 3=265.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1293 lb down and 88 lb up at 0-1-12, 1283 lb down and 98 lb up at 2-2-0, 1279 lb down and 98 lb up at 4-2-0, 1279 lb down and 98 lb up at 6-2-0, and 1279 lb down and 98 lb up at 8-2-0, and 1281 lb down and 96 lb up at 10-2-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

M WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property amage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Plv		
J0620-2872	B2	COMMON GIRDER	1	2	Job Reference (optional)	E14484703

Comtech, Inc,

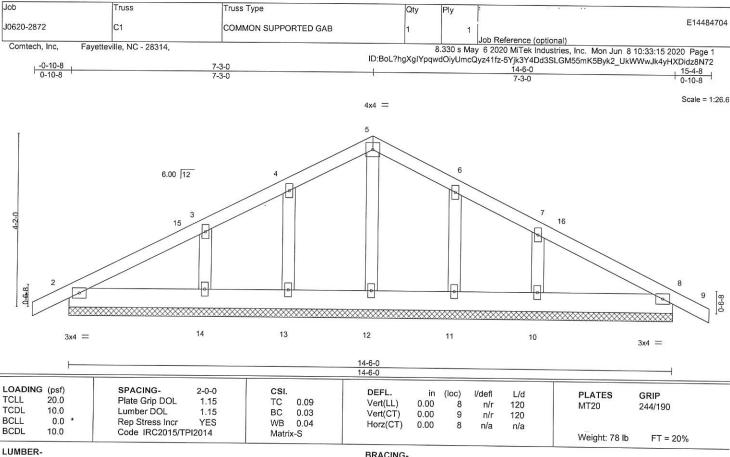
Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTesk Industries, Inc. Mon Jun 8 10:33:14 2020 Page 2 ID:BoL?hgXglYpqwdOiyUmcQyz41fz-dM9MsC3bsmKUfCWvCdayQXVkxK1PBk0wjdog9Bz8N73

LOAD CASE(S) Standard

Uniform Loads (plf)
Vert: 1-2-60, 2-3=-60, 1-3=-20
Concentrated Loads (lb)
Vert: 1=-1293(B) 5=-1283(B) 6=-1279(B) 7=-1279(B) 8=-1279(B) 9=-1281(B)





TOP CHORD

2x4 SP No.1 **BOT CHORD** 2x6 SP No.1

OTHERS 2x4 SP No.2 BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 14-6-0

(lb) - Max Horz 2=-82(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 11 except 14=-112(LC 12), 10=-112(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 11 except 14=250(LC 1), 10=250(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-3-0, Exterior(2) 3-3-0 to 7-3-0, Corner(3) 7-3-0 to 11-7-13, Exterior(2) 11-7-13 to 15-4-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 11 except (it=lb) 14=112, 10=112,



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracking indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty Ply F14484705 COMMON C2 J0620-2872 Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:16 2020 Page 1 ID: BoL? hgXgIYpqwdOiyUmcQyz41 fz-ZkH6 Hu5sONaCuWgHK2cQVyb4o8pWflLDAxHmE3z8N71-0-10-8 0-10-8 14-6-0 0-10-8 Scale = 1:26.6 4x6 =3 6.00 12 10 6 2x4 || 3x6 = 3x6 > 14-6-0 Plate Offsets (X,Y)--[2:0-0-12,0-1-8], [4:0-0-12,0-1-8] LOADING (psf) SPACING-2-0-0 CSI. DEFL L/d (loc) I/defl **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.44 Vert(LL) -0.02 2-6 244/190 >999 360 MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.24 Vert(CT) -0.05 2-6 >999 240 BCLL 0.0 Rep Stress Incr YES WB 0.08 Horz(CT) 0.01 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Wind(LL) 0.02 2-6 >999 240 Weight: 66 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 **BOT CHORD** 2x6 SP No.1

WEBS 2x4 SP No.2

REACTIONS.

(size) 2=0-3-8, 4=0-3-8

Max Horz 2=-53(LC 10)

Max Uplift 2=-48(LC 12), 4=-48(LC 13) Max Grav 2=630(LC 1), 4=630(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-803/237, 3-4=-803/237 **BOT CHORD** 2-6=-75/617, 4-6=-75/617

WEBS 3-6=0/366

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 7-3-0, Exterior(2) 7-3-0 to 11-7-13, Interior(1) 11-7-13 to 15-4-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

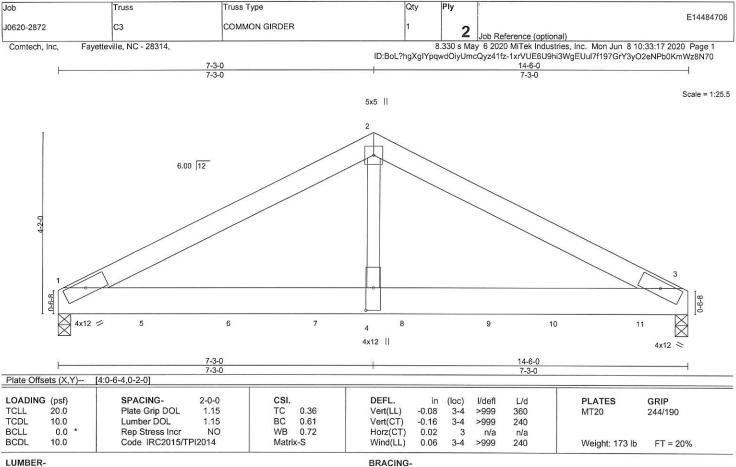
🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property amanae. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

BOT CHORD

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x8 SP 2400F 2.0E

WEBS 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 3=0-3-8 Max Horz 1=-47(LC 25)

Max Uplift 1=-324(LC 8), 3=-382(LC 9)

Max Grav 1=4670(LC 1), 3=5490(LC 1)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-7127/503, 2-3=-7121/502

BOT CHORD 1-4=-396/6308, 3-4=-396/6308

WEBS 2-4=-316/5855

NOTES-

FORCES.

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-7-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=324, 3=382.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1291 lb down and 99 lb up at 2-0-12, 1291 lb down and 99 lb up at 4-0-12, 1291 lb down and 99 lb up at 6-0-12, 1291 lb down and 99 lb up at 8-0-12, 1291 lb down and 99 lb up at 10-0-12, and 1283 lb down and 99 lb up at 11-6-0, and 1285 lb down and 98 lb up at 13-6-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 1-3=-20



Structural wood sheathing directly applied or 5-10-10 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	E14484706
J0620-2872	С3	COMMON GIRDER	1	2	Job Reference (optional)

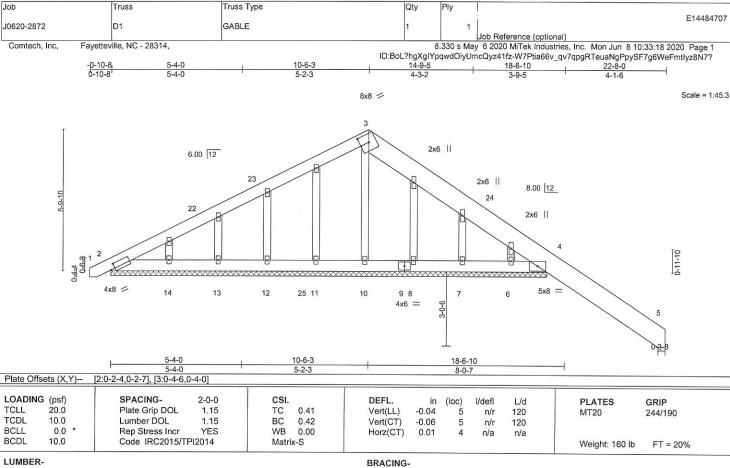
Fayetteville, NC - 28314, Comtech, Inc,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:17 2020 Page 2 ID:BoL?hgXglYpqwdOiyUmcQyz41fz-1xrVUE6U9hi3WgEUul7f197GrY3yO2eNPb0KmWz8N70

LOAD CASE(S) Standard Concentrated Loads (lb)

Vert: 5=-1291(F) 6=-1291(F) 7=-1291(F) 8=-1291(F) 9=-1291(F) 10=-1283(F) 11=-1285(F)





TOP CHORD

BOT CHORD

2x6 SP No.1 *Except* TOP CHORD

3-5: 2x10 SP No.1

BOT CHORD 2x6 SP No.1

OTHERS 2x4 SP No.2

REACTIONS. All bearings 17-9-15.

(lb) - Max Horz 2=-218(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 13, 2 except 4=-430(LC 13), 14=-259(LC 12), 6=-139(LC 1)

Max Grav All reactions 250 lb or less at joint(s) 10, 11, 12, 13, 8, 7, 6 except 4=979(LC 1), 14=423(LC 23),

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-587/244, 3-4=-627/277

BOT CHORD 2-14=-80/413, 13-14=-80/413, 12-13=-80/413, 11-12=-80/413, 10-11=-80/413,

8-10=-80/413, 7-8=-80/413, 6-7=-80/413, 4-6=-80/413

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 10-6-3, Exterior(2) 10-6-3 to 14-11-0, Interior(1) 14-11-0 to 22-6-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13, 2 except (jt=lb) 4=430, 14=259, 6=139.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

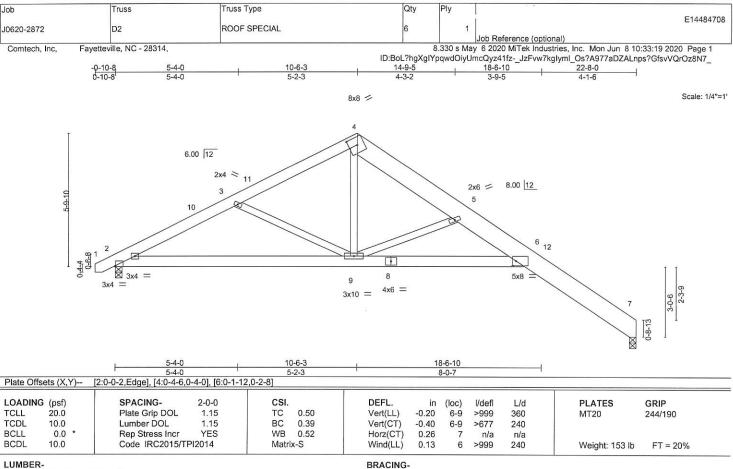
🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property anage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

BOT CHORD

LUMBER-

2x6 SP No.1 *Except* TOP CHORD

4-7: 2x10 SP 2400F 2.0E

BOT CHORD 2x6 SP No.1

2x4 SP No.2 **WEBS**

REACTIONS. (size) 7=0-3-8, 2=0-3-8

Max Horz 2=176(LC 11)

Max Uplift 7=-57(LC 13), 2=-58(LC 12)

Max Grav 7=909(LC 1), 2=949(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1528/386, 3-4=-1206/274, 4-5=-1286/301, 5-6=-2001/430, 6-7=-452/169 **BOT CHORD** 2-9=-189/1315, 6-9=-273/2152

WEBS 3-9=-349/230, 4-9=-109/874, 5-9=-1256/333

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 10-6-3, Exterior(2) 10-6-3 to 15-1-11, Interior(1) 15-1-11 to 22-6-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and permanent. Answer of the property designs are considered as a constant of the property designs and a second of the property designs and a second of the property designs are always required for stability and to prevent collapse with possible personal injury and permanent property designs and a second of the property designs are always represented by the property designs and a second of the property designs and a second of the property designs and a second of the property designs are always and a second of the property designs and a second of the property designs are always and a second of the property designs are always and a second of the property designs are always and a second of the property designs are always and a second of the property designs and a second of the property designs are always and a second of the property designs are always and a second of the property designs are always and a second of the property designs are always and a second of the property designs are always and a second of the property designs are always and a second of the property designs are always and a second of the property designs are always and a second of the property designs and a second of the property designs are always and a second of the property designs are always and a second of the property designs are always and a second of the property designs are always and a second of the property designs are always and a second of the property designs are always and a second of the property designs are always and a second of t



				lou	Di			
lob		Truss	Truss Type	Qty	Ply			E14484709
10620-2872		м1	GABLE	1	1	Job Reference (options	al)	
					8 330 s Mar	6 2020 MiTek Industri	ies, Inc. Mon Jun 8 10:33:20	2020 Page 1
Comtech, Inc,	Fayettev	rille, NC - 28314,		ID:Bol 2haYa	VnawdΩiyl Im	COVZ41fz-SWXd6G8MF	Rc4dN7z3ZtgMfolp8lDUba0p52	ZF Maz8N6z
			4-6-8	ID.Boc:rigAg	. pqwaoiyom	8-4-0	10 (0. 1) <u>20 2</u> 1g	_
	-0-10-8		4-6-8			8-4-0 3-9-8		
	0-10-8		4-0-0					
								Scale = 1:17.0
							2x4	
							5	
T						22 2 W		
						2x4		
				3.00 12		4		
				2x4				
li i				3				
				10				
4								
2-4-10							1	2-1-0
2			9					2
		3x4 =						
		2		-		10	0	_
1-4-0								
ļď			***************************************	***************************************	**********	***************************************	***************************************	
			***************************************	***************************************	****	·····	****	
						7	6	
				8				
				2x4		2x4	2x4	
LOADING (psf	1	SPACING- 2-0-	o csi.	DEFL.	in (loc)	I/defl L/d	PLATES GRIP	
	(Plate Grip DOL 1.1:		Vert(LL) -	0.00 1	n/r 120	MT20 244/1	90
TCLL 20.0 TCDL 10.0		Lumber DOL 1.1			0.00 1	n/r 120		
	o *	Rep Stress Incr YES			0.00	n/a n/a		
		Code IRC2015/TPI2014		(5.1)		covered to tempo.	Weight: 40 lb FT	= 20%
BCDL 10.0	,	Code IRC2013/11/12014	Widdix				9	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD

2x6 SP No.1 2x6 SP No.1

WEBS 2x4 SP No.2 **OTHERS** REACTIONS.

ONS. All bearings 8-2-8.
(lb) - Max Horz 2=109(LC 8) Max Uplift All uplift 100 lb or less at joint(s) 6, 2, 7 except 8=-107(LC 12) Max Grav All reactions 250 lb or less at joint(s) 6, 2, 7 except 8=344(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

3-8=-255/218 **WEBS**

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 8-1-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2, 7 except (jt=lb) 8=107.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

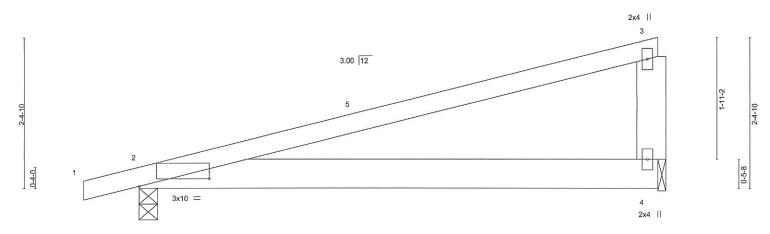
ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	E14484710
J0620-2872	M2	MONOPITCH	5	1	E14464710
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		TO ACCURACY OF BOOK TO CONTROL SUPER TO		1.0	Job Reference (optional)
Comtech, Inc, Faye	tteville, NC - 28314,				y 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:20 2020 Page 1
		j	D:BoL?hgXgIY	pqwdOiyUr	mcQyz41fz-SWXd6G8MRc4dN7z3ZtgMfoldtl98bacp5ZF_Mqz8N6z
-0-1	0-8	4-6-8		ALL CONTROL OF THE PROPERTY OF THE PARTY OF	8-4-0
0-1	0-8	4-6-8	1		3-9-8

Scale = 1:17.5



		1				8-4-0							
Plate Off	sets (X,Y)	[2:1-1-4,0-1-7]											_
LOADIN	G (psf)	SPACING- 2-	-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL 1	1.15	TC	0.95	Vert(LL)	-0.05	2-4	>999	360	MT20	244/190	
TCDL	10.0	Lumber DOL 1	1.15	BC	0.26	Vert(CT)	-0.11	2-4	>880	240			
BCLL	0.0 *	Rep Stress Incr	'ES	WB	0.00	Horz(CT)	0.00		n/a	n/a			
BCDL	10.0	Code IRC2015/TPI20	14	Matri	x-P	Wind(LL)	0.00	2	****	240	Weight: 37 lb	FT = 20%	

8-4-0

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1

BOT CHORD 2x6 SP No.1

2x6 SP No.1 **WEBS**

REACTIONS. (size) 2=0-3-8, 4=0-1-8

Max Horz 2=77(LC 8)

Max Uplift 2=-66(LC 8), 4=-41(LC 12)

Max Grav 2=384(LC 1), 4=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 8-1-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Structural wood sheathing directly applied or 2-2-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

June 8,2020





Job	Truss	Truss Type	Qty	Ply	E14484711
J0620-2872	мз	ROOF SPECIAL	6	1	Job Reference (optional)
Comtech, Inc,	Fayetteville, NC - 28314,		ID:BoL?hgXgIY	8.330 s Ma pqwdOiyUm	ay 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:21 2020 Page 1 cQyz41fz-wi4?Kc9_CvCU_HYF7bCbC?IrG9M?K1syKD_XuHz8N6y
	-0-10-8		7-0-0		
	0-10-8		7-0-0		,

Scale = 1:17.1

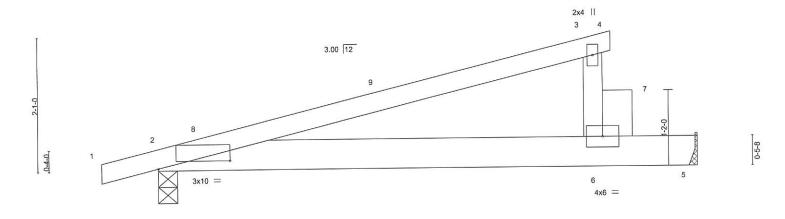


Plate Offsets (X,Y)	2:1-1-4,0-1-7]		T						
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15	CSI. TC 0.7 BC 0.8	in -0.11 -0.26	(loc) 2-6 2-6	I/defl >884 >373	L/d 360 240	PLATES MT20	GRIP 244/190
3CLL 0.0 *	Rep Stress Incr Code IRC2015/TR	NO	WB 0.0 Matrix-P	0.00 0.17	5 2-6	n/a >569	n/a 240	Weight: 35 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 2x6 SP No.1

BOT CHORD 2x4 SP No.2 *Except* **WEBS**

6-7: 2x6 SP No.1

BRACING-

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, TOP CHORD

except end verticals

Rigid ceiling directly applied or 7-8-6 oc bracing.

REACTIONS. (size) 2=0-3-8, 5=Mechanical

Max Horz 2=68(LC 8)

Max Uplift 2=-75(LC 8), 5=-58(LC 12) Max Grav 2=468(LC 1), 5=656(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 7-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 500 lb down and 265 lb up at 7-1-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-20, 2-5=-20

Concentrated Loads (lb) Vert: 6=-500

SEAL 036322 A. GILBERT

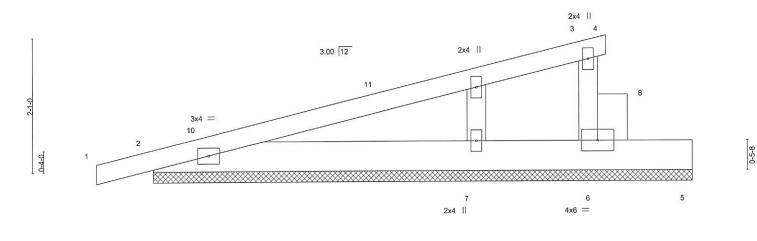
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type		Qty	Ply	E14484712
J0620-2872	M4	GABLE		1	1	Job Reference (optional)
Comtech, Inc,	Fayetteville, NC - 28314,		ID:B	8 L?hgXgI\	3.330 s Ma /pqwdOiyU	y 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:22 2020 Page 1 JmcQyz41fz-OueOXxAczDKLcR7RgljqkDr1sZug3U66Ztk5Qjz8N6x
20	-0-10-8		7-0-0		3800 00 000 000 000	
-	0-10-8		7-0-0			1

Scale = 1:17.1



8-4-0												
LOADING TCLL TCDL	20.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.66 0.06 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 0.01 0.00	(loc) 4 4	I/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0 * 10.0	Code IRC2015/TI		Matri		11012(01)	0.00	J	11/4	1110	Weight: 37 lb	FT = 20%

8-4-0

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 **BOT CHORD** 2x6 SP No.1

WEBS 2x4 SP No.2 *Except* 6-8: 2x6 SP No.1

OTHERS 2x4 SP No.2

REACTIONS. (size) 6=8-4-0, 2=8-4-0, 7=8-4-0

Max Horz 2=96(LC 8) Max Uplift 6=-109(LC 12), 2=-133(LC 8)

Max Grav 6=235(LC 1), 2=303(LC 1), 7=181(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 7-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=109, 2=133.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Truss Type Qty Job Truss E14484713 M5 GABLE J0620-2872 Job Reference (optional)

Comtech, Inc,

Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:22 2020 Page 1 ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-OueOXxAczDKLcR7RgIjqkDr5_ZvK3UC6Ztk5Qjz8N6x

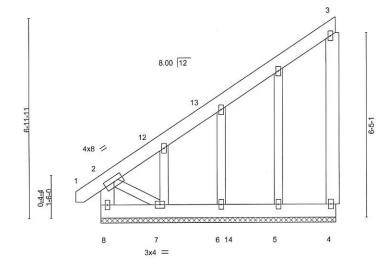
Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

0-10-8 0-10-8

Scale = 1:38.6



LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.40 BC 0.02 WB 0.06 Matrix-P	DEFL. in Vert(LL) -0.02 Vert(CT) 0.01 Horz(CT) -0.00	(loc) 1 1 4	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 79 lb	GRIP 244/190 FT = 20%
--	---	---	---	----------------------	-----------------------------	--------------------------	---------------------------------	-----------------------------

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1

WEBS

2x6 SP No.1 *Except* 2-7: 2x4 SP No.2

OTHERS 2x4 SP No.2

REACTIONS.

All bearings 8-2-8. (lb) - Max Horz 8=271(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) except 4=-206(LC 12), 7=-142(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 5, 6, 7 except 4=311(LC 19), 8=311(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

3-4=-301/226, 2-8=-298/0 TOP CHORD

BOT CHORD 7-8=-280/225 **WEBS** 2-7=-265/330

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-12 to 3-8-1, Interior(1) 3-8-1 to 8-1-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 206 lb uplift at joint 4 and 142 lb uplift at joint 7.



June 8,2020



🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucollapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Qty Truss Type Job Truss E14484714 MONOPITCH M6 J0620-2872 Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:23 2020 Page 1 Fayetteville, NC - 28314, Comtech, Inc, ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-s5CmlHAFkXTCEbidE0E3HQNLXzD5ovxFnXTez9z8N6w 3-10-0 0-10-8 Scale = 1:39.3 2x4 || 8.00 12 2x4 N 3 6-6-3 2x6 || 1-6-0 -5-8 \bigotimes 5 4x6 = 2x6 || GRIP DEFL. I/defl L/d **PLATES** LOADING (psf) SPACING-2-0-0 CSI. in (loc) 244/190 Vert(LL) -0.03 5-6 >999 360 MT20 TC BC 0.09 20.0 Plate Grip DOL 1.15 TCLL 0.18 Vert(CT) -0.06 5-6 >999 240 Lumber DOL 1.15 TCDL 10.0 Horz(CT) -0.00 n/a n/a WB 0.16 Rep Stress Incr YES **BCLL** 0.0

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

0.07

5-6 >999

except end verticals.

240

Rigid ceiling directly applied or 10-0-0 oc bracing

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Weight: 69 lb

FT = 20%

LUMBER-

BCDL

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1 2x6 SP No.1 *Except* **WEBS**

3-5: 2x4 SP No.2

(size) 6=0-3-0, 5=0-1-8 REACTIONS.

Max Horz 6=178(LC 12)

Max Uplift 6=-27(LC 9), 5=-143(LC 9) Max Grav 6=376(LC 1), 5=310(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code IRC2015/TPI2014

2-3=-268/39, 2-6=-290/86 TOP CHORD

5-6=-250/259 **BOT CHORD** 3-5=-313/285 **WEBS**

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-12 to 3-7-11, Interior(1) 3-7-11 to 8-1-0 zone; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-S

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 6 and 143 lb uplift at joint 5.





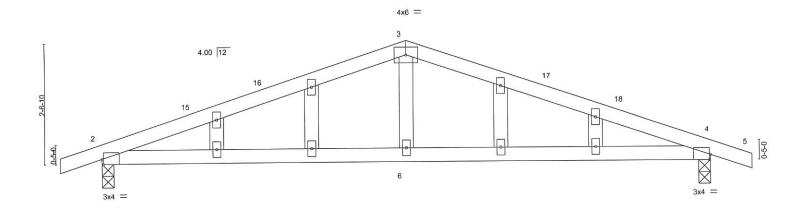
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTeke onnectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucollapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Qty Truss Truss Type Job E14484715 GABLE J0620-2872 Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:24 2020 Page 1 Fayetteville, NC - 28314, Comtech, Inc. ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-KHm8ydBtVqb3rlHqojIIpewRjMWmXNXP0BDBVcz8N6v 12-10-0 -0-10-8 0-10-8

Scale = 1:23.4



<u> </u>	6-5-0 6-5-0						6-5-0		
Plate Offsets (X,Y)	[2:0-0-5,Edge], [4:0-0-5,Edge]							I	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.44 BC 0.34 WB 0.07 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.11 -0.09 0.01	(loc) 4-6 2-6 4	l/defl >999 >999 n/a	L/d 240 240 n/a	PLATES MT20 Weight: 51 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1

BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2 **OTHERS**

REACTIONS.

(size) 2=0-3-0, 4=0-3-0

Max Horz 2=-49(LC 17)

Max Uplift 2=-309(LC 8), 4=-309(LC 9) Max Grav 2=563(LC 1), 4=563(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-910/965, 3-4=-910/965 **BOT CHORD** 2-6=-828/799, 4-6=-828/799

WEBS 3-6=-383/303

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 1) Uniodantice from the loads have been considered for time design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 6-5-0, Exterior(2) 6-5-0 to 10-9-13, Interior(1) 10-9-13 to 13-8-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 309 lb uplift at joint 2 and 309 lb uplift at joint 4.



Structural wood sheathing directly applied or 5-9-9 oc purlins.

Rigid ceiling directly applied or 6-6-10 oc bracing.

June 8,2020

👠 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chromothers only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and pramage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

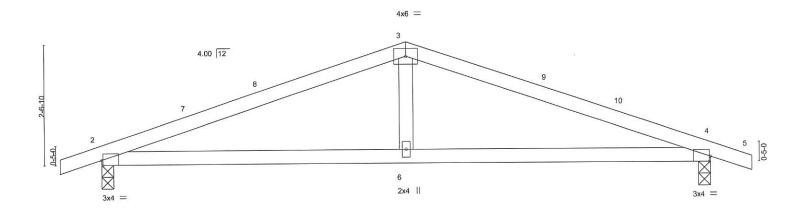
ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply		E14484716
J0620-2872	P2	COMMON	2	1	Job Reference (optional)	2000 200000100.0075
Comtech, Inc,	Fayetteville, NC - 28314,		ID:BoL?hgXgIYpo	8.330 s May	y 6 2020 MiTek Industries, Inc. Mon Jun 8 Qyz41fz-oTKWAzCVG8jwTvs0MRGXMrSc	Tmr?GqnYFryl12z8N6u
0-10-8		6-5-0	- A	\$53	12-10-0	13-8-8
0-10-8		6-5-0			6-5-0	0-10-8

Scale = 1:23.4



6-5-0 6-5-0					12-10-0 6-5-0						
Plate Offsets (X,Y)	[2:0-0-5,Edge], [4:0-0-5,Ed										
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI2	2-0-0 1.15 1.15 YES 2014	CSI. TC BC WB Matrix	0.44 0.34 0.07	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.11 -0.09 0.01	(loc) 4-6 2-6 4	l/defl >999 >999 n/a	L/d 240 240 n/a	PLATES MT20 Weight: 45 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 **BOT CHORD** 2x4 SP No.1

2x4 SP No.2 **WEBS**

REACTIONS.

(size) 2=0-3-0, 4=0-3-0

Max Horz 2=-29(LC 13) Max Uplift 2=-217(LC 8), 4=-217(LC 9) Max Grav 2=563(LC 1), 4=563(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-910/965, 3-4=-910/965 TOP CHORD

BOT CHORD 2-6=-828/799, 4-6=-828/799

3-6=-383/303 **WEBS**

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 6-5-0, Exterior(2) 6-5-0 to 10-9-13, Interior(1) 10-9-13 to 13-8-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 2 and 217 lb uplift at joint 4.



Structural wood sheathing directly applied or 5-9-9 oc purlins.

Rigid ceiling directly applied or 6-6-10 oc bracing.

June 8,2020



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

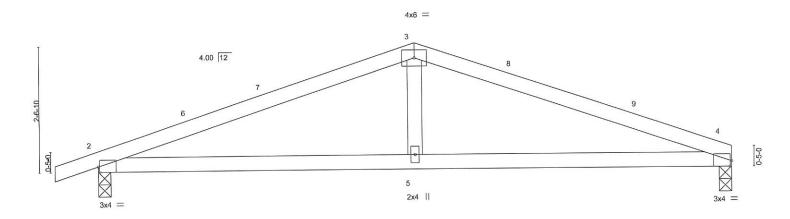
ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty Ply E1448471					
J0620-2872	P3	соммон	3 1 Job Reference (optional)					
Comtech, Inc, Fa	ayetteville, NC - 28314,		8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:26 2020 Page ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-HguuNJD71Srn52QCv8nmv3?mfABB?G1hTViIZUz8N6					
-0-10-8 0-10-8		6-5-0 6-5-0	12-10-0 6-5-0					

Scale = 1:22.5



6-5-0 6-5-0					12-10-0 6-5-0					
Plate Offsets (X,Y)	[2:0-0-5,Edge], [4:0-0-5,Edge]							1		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	TC 0.48 BC 0.35	/ert(CT) -	in (0.11 0.09 0.01	(loc) 4-5 4-5 4	I/defl >999 >999 n/a	L/d 240 240 n/a	PLATES MT20 Weight: 44 lb	GRIP 244/190 FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1

BOT CHORD 2x4 SP No.1

2x4 SP No.2

REACTIONS. (size) 4=0-3-0, 2=0-3-0

Max Horz 2=32(LC 12) Max Uplift 4=-179(LC 9), 2=-217(LC 8) Max Grav 4=501(LC 1), 2=566(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-918/984, 3-4=-916/993 2-5=-866/807, 4-5=-866/807 TOP CHORD BOT CHORD

3-5=-386/305 **WEBS**

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 6-5-0, Exterior(2) 6-5-0 to 10-9-13, Interior(1) 10-9-13 to 12-8-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint 4 and 217 lb uplift at joint 2.



Structural wood sheathing directly applied or 5-7-10 oc purlins.

Rigid ceiling directly applied or 6-4-14 oc bracing.

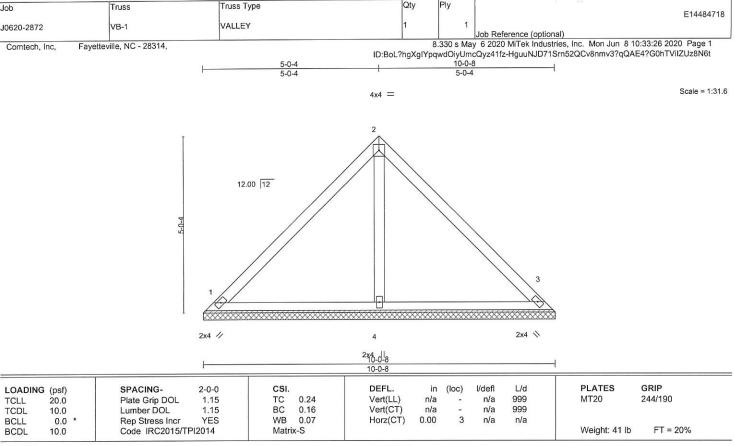
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Qty

LUMBER-

.lob

TOP CHORD BOT CHORD 2x4 SP No.1 2x4 SP No.1 **OTHERS** 2x4 SP No.2

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=10-0-8, 3=10-0-8, 4=10-0-8

Max Horz 1=112(LC 9) Max Uplift 1=-28(LC 13), 3=-28(LC 13)

Max Grav 1=212(LC 1), 3=212(LC 1), 4=324(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 1 and 28 lb uplift at ioint 3.



June 8,2020



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Truss Type Qty Job Truss F14484719 VB-2 VALLEY J0620-2872 Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:27 2020 Page 1 Fayetteville, NC - 28314, Comtech, Inc. ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-lsSHafDlolzejC?PTsI?RGY?MabHkkqri9Rs6wz8N6s 4-0-4 Scale = 1:26.2 4x4 = 2 12.00 12 2x4 || 4 2x4 \ 2x4 // SPACING-CSI. DEFL. I/defl L/d **PLATES** GRIP LOADING (psf) 2-0-0 in (loc) 20.0 Plate Grip DOL 1.15 TC 0.23 Vert(LL) n/a n/a 999 MT20 244/190 TCLL 1.15 BC 0.10 Vert(CT) n/a n/a 999 TCDL 10.0 Lumber DOL Rep Stress Incr YES WB 0.03 Horz(CT) 0.00 3 n/a n/a **BCLL** 0.0 Code IRC2015/TPI2014 Matrix-P Weight: 33 lb FT = 20% BCDL 10.0 **BRACING-**LUMBER-

2x4 SP No.1 2x4 SP No.1 TOP CHORD **BOT CHORD OTHERS** 2x4 SP No.2 TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=8-0-8, 3=8-0-8, 4=8-0-8

Max Horz 1=-88(LC 10)

Max Uplift 1=-32(LC 13), 3=-32(LC 13)

Max Grav 1=179(LC 1), 3=179(LC 1), 4=230(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 32 lb uplift at joint 1 and 32 lb uplift at joint 3.





🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/PH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Qty Ply Truss Truss Type Job E14484720 VALLEY J0620-2872 VB-3 Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:28 2020 Page 1 Fayetteville, NC - 28314, Comtech, Inc. ID:BoL?hgXglYpqwdOiyUmcQyz41fz-D20fo?ENZ35VKMab1ZqE_U4Cq_yFTBM_xpBPeNz8N6r 3-0-4 Scale = 1:21.0 4x4 = 12.00 12 2x4 \ 2x4 // 2x4 || (loc) I/defl L/d **PLATES** GRIP CSI. DEFL. in LOADING (psf) SPACING-2-0-0 244/190 999 MT20 TC 0.12 Vert(LL) n/a n/a Plate Grip DOL 1.15 20.0 TCLL BC Vert(CT) n/a n/a 999 Lumber DOL 1.15 0.05 TCDL 10.0 WB Horz(CT) 0.00 3 0.02 n/a YES BCLL 0.0 Rep Stress Incr Weight: 24 lb FT = 20%Code IRC2015/TPI2014 BCDL 10.0

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 **OTHERS**

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=6-0-8, 3=6-0-8, 4=6-0-8

Max Horz 1=64(LC 9)

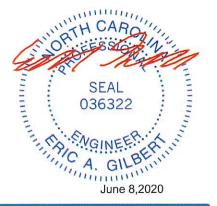
Max Uplift 1=-23(LC 13), 3=-23(LC 13)

Max Grav 1=130(LC 1), 3=130(LC 1), 4=167(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

- 7) Onio and the first of the grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 1 and 23 lb uplift at joint 3.

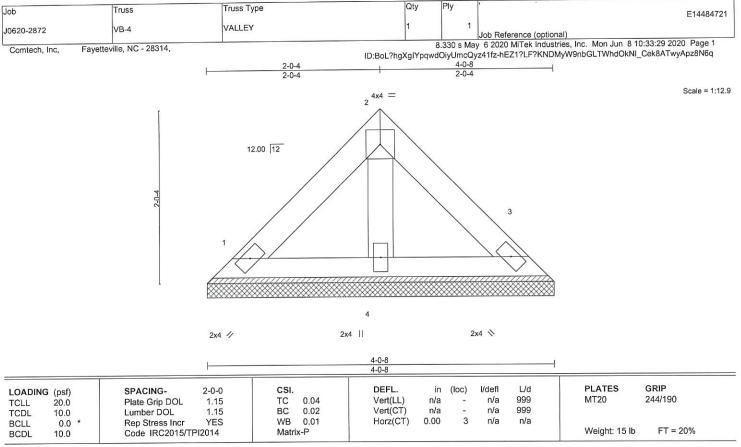




🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters have received the second of the seco





LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 **BOT CHORD** 2x4 SP No.2 **OTHERS**

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 4-0-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=4-0-8, 3=4-0-8, 4=4-0-8

Max Horz 1=-40(LC 10)

Max Uplift 1=-14(LC 13), 3=-14(LC 13) Max Grav 1=81(LC 1), 3=81(LC 1), 4=104(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 1 and 14 lb uplift at joint 3.





Design valid for use only with MITe® connectors. This design is based only upon parameters have represented by the second of the second parameters and individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/THI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932

Qty Truss Truss Type Job E14484722 VALLEY J0620-2872 VB-5 Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:30 2020 Page 1 Fayetteville, NC - 28314, Comtech, Inc. ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-9R7PDhGe5gLDagk_8_si3vAZ1neKx54HO7gWiFz8N6p 1-0-4 1-0-4 Scale = 1:7.9 3x4 = 12.00 12 3 2x4 // 2x4 \ Plate Offsets (X,Y)--[2:0-2-0,Edge] **PLATES** DEFL. I/defl L/d LOADING (psf) CSI. in (loc) SPACING-2-0-0 MT20 244/190 Plate Grip DOL TC 0.01 Vert(LL) n/a n/a 999 TCLL 20.0 1.15 BC 0.01 Vert(CT) n/a n/a 999 1.15 TCDL 10.0 Lumber DOL WB 0.00 Horz(CT) 0.00 3 n/a n/a BCLL 0.0 Rep Stress Incr YES Code IRC2015/TPI2014 Matrix-P Weight: 6 lb FT = 20% BCDL 10.0

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

(size) 1=2-0-8, 3=2-0-8

Max Horz 1=-16(LC 8) Max Uplift 1=-2(LC 12), 3=-2(LC 12)

Max Uplift 1=-2(LC 12), 3=-2(LC 12) Max Grav 1=54(LC 1), 3=54(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

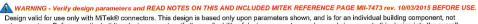
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2 lb uplift at joint 1 and 2 lb uplift at joint 3.



Structural wood sheathing directly applied or 2-0-8 oc purlins.

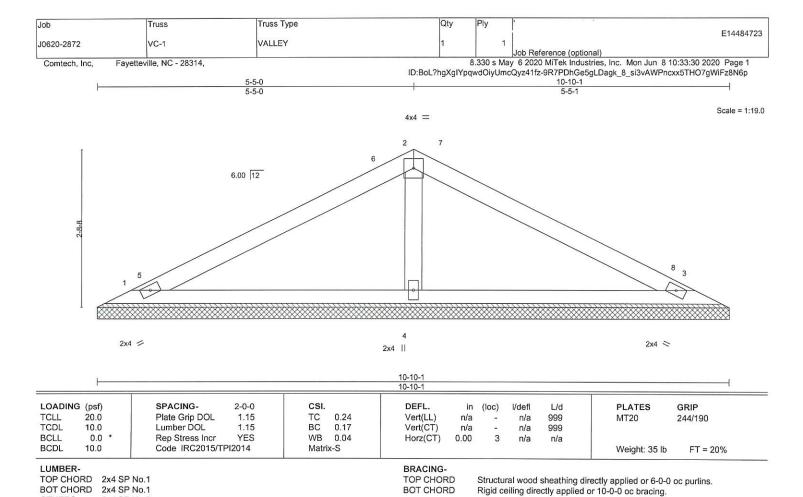
Rigid ceiling directly applied or 10-0-0 oc bracing.



Design valid for use only with Mil ek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
ANISTIPHI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932



OTHERS REACTIONS.

(size) 1=10-10-1, 3=10-10-1, 4=10-10-1

Max Horz 1=31(LC 11)

2x4 SP No.2

Max Uplift 1=-23(LC 12), 3=-29(LC 13)

Max Grav 1=178(LC 23), 3=178(LC 24), 4=417(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-4=-276/183

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-7-7 to 5-0-3, Interior(1) 5-0-3 to 5-5-0, Exterior(2) 5-5-0 to 9-9-13, Interior(1) 9-9-13 to 10-2-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 1 and 29 lb uplift at
- 6) Non Standard bearing condition. Review required.





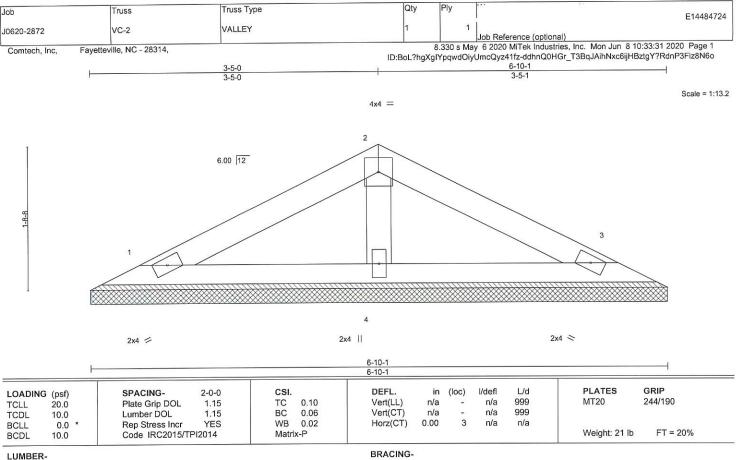
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





2x4 SP No.1 TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=6-10-1, 3=6-10-1, 4=6-10-1

Max Horz 1=-18(LC 8)

Max Uplift 1=-18(LC 12), 3=-21(LC 13)

Max Grav 1=114(LC 1), 3=114(LC 1), 4=220(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1 and 21 lb uplift at joint 3.
- 6) Non Standard bearing condition. Review required.

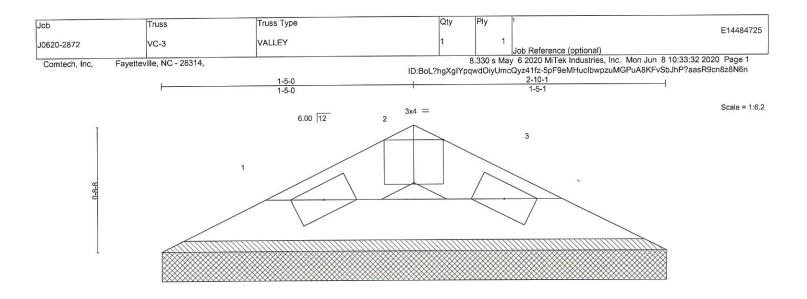




MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTFeW connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





2x4 /

2x4 >

late Offse	ets (X,Y) [2:0-2-0,Edge]				_		_				
LOADING TCLL TCDL BCLL	(psf) 20.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.01 0.02 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-P						Weight: 7 lb	FT = 20%

2-10-1

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1

BOT CHORD 2x4 SP No 1

(size) 1=2-10-1, 3=2-10-1

Max Horz 1=-5(LC 8)

Max Uplift 1=-4(LC 12), 3=-4(LC 13) Max Grav 1=64(LC 1), 3=64(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 4 lb uplift at joint 1 and 4 lb uplift at joint
- 6) Non Standard bearing condition. Review required.
- 7) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



Structural wood sheathing directly applied or 2-10-1 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

June 8,2020

M WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTreWe connectors. This design is based only upon parameters had recovered to the connectors. This design is based only upon parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

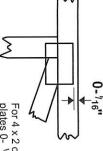


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- v_{te} from outside edge of truss.

œ

6

5

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE



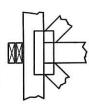
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction. DSB-89: Design Standard for Bracing.

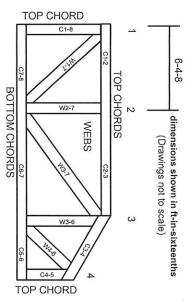
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information.

Guide to Good Practice for Handling,

Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

© 2012 MiTek® All Rights Reserved





MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- . Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- . Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.