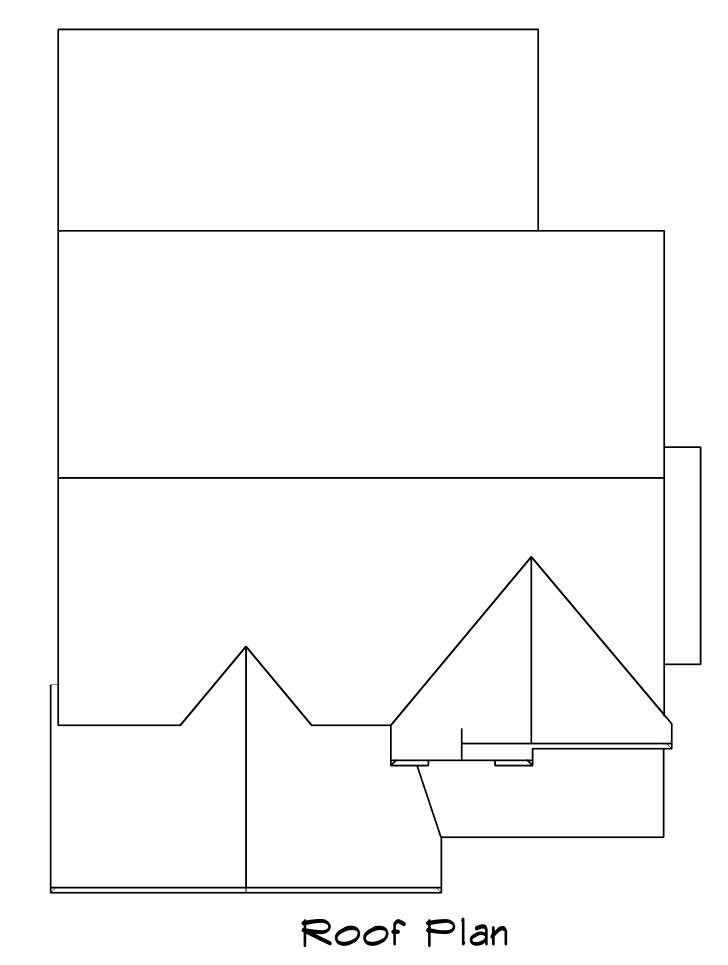
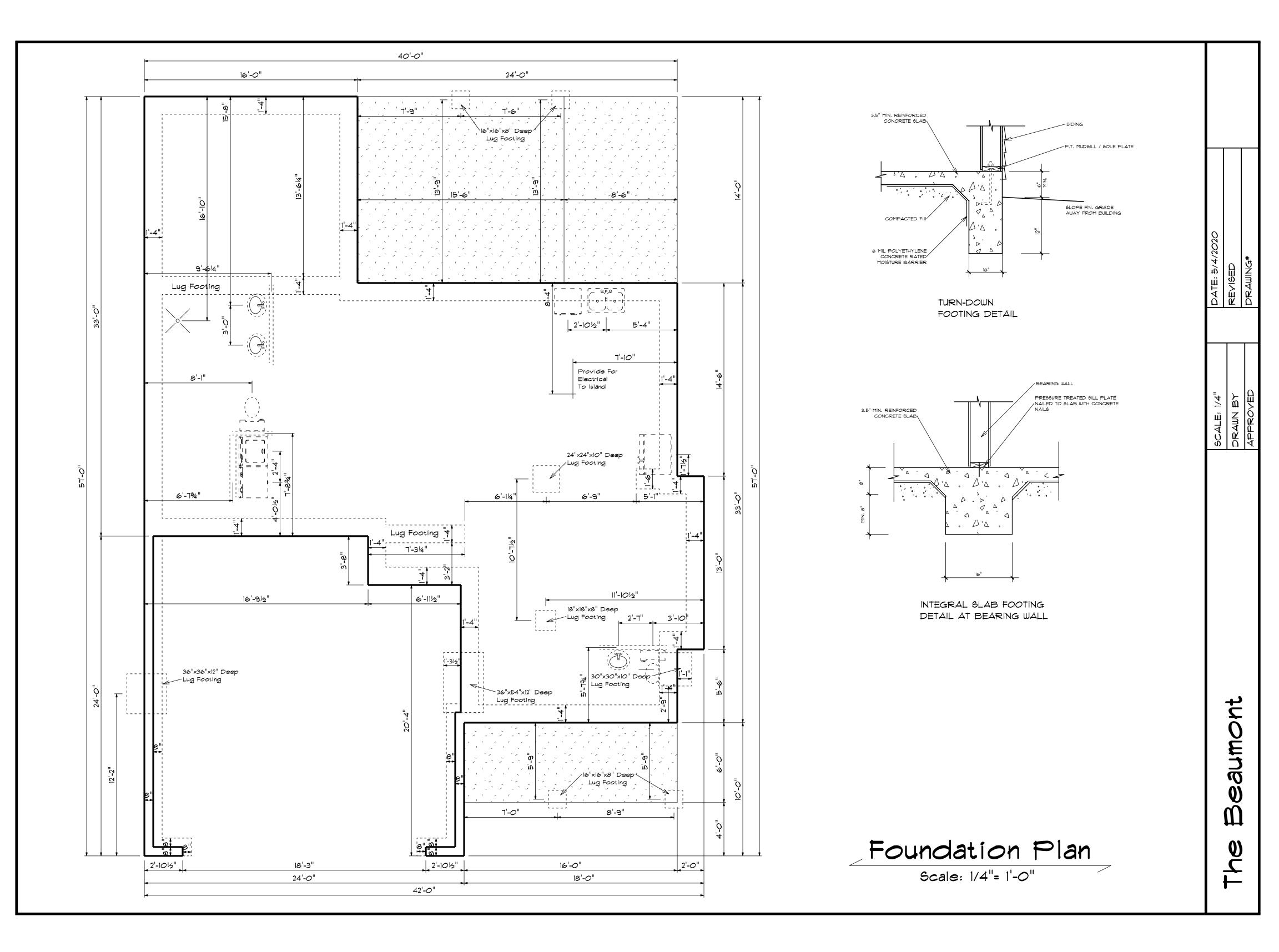


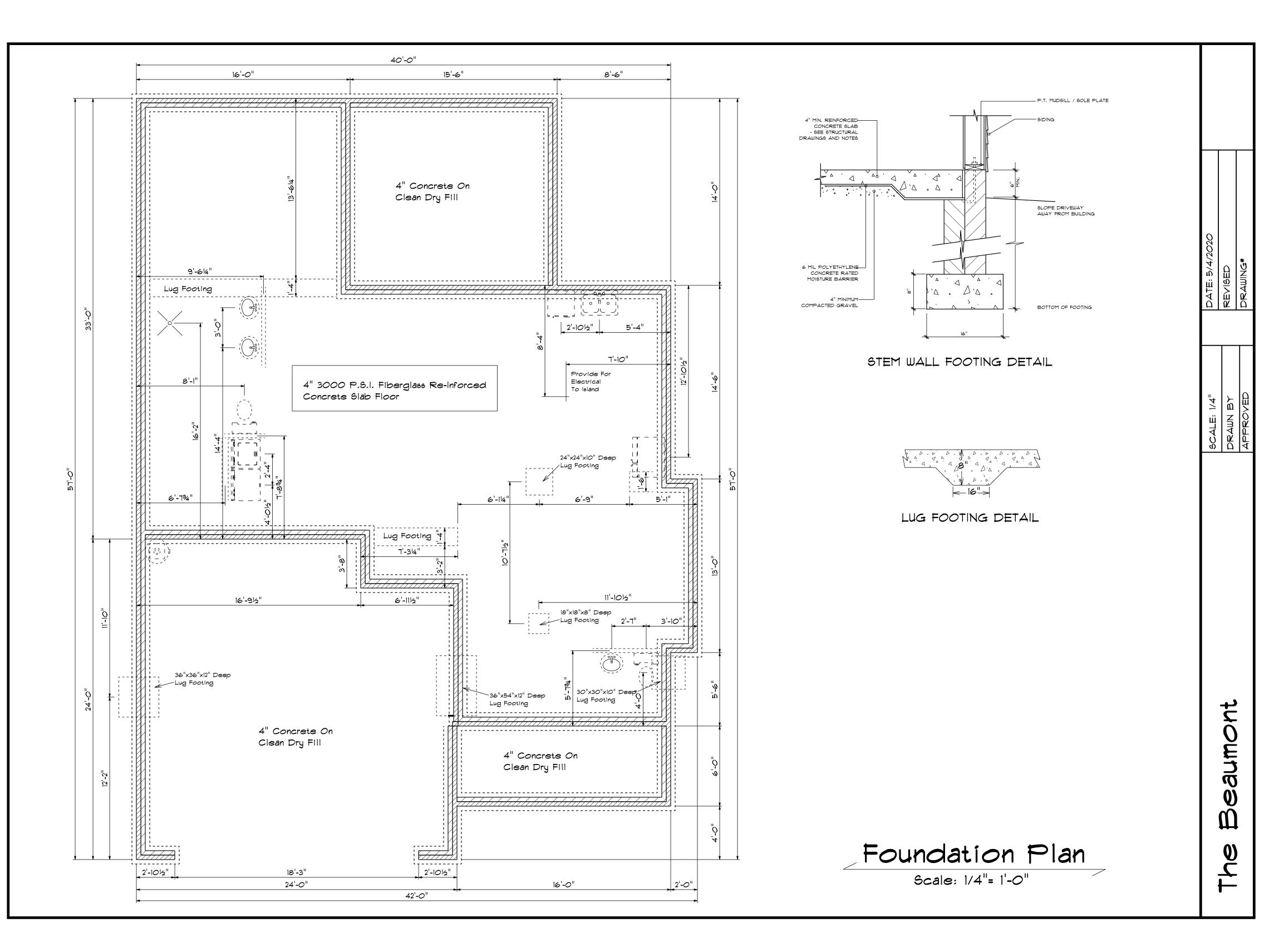
PRODUCT CODE	SIZE	HINGE	COUNT
1-6 Door Unit	1'-6"	L	2
2-0 Door Unit	2'-0"	L	1
2-4 Door Unit	2'-4"	R	3
2-4 Door Unit	2'-4"	L	3
2-6 Door Unit	2'-6"	R	1
2-6 Door Unit	2'-6"	L	3
3-0 Doublehung Door Unit	3'-0"	LR	2
24X32 Single	2'-4" x 3'-2"	N	1
28x52 single	2'-8" x 5'-2"	N	8
28x52 twin	5'-4" x 5'-2"	NN	1
36X12 TRANSOM	3'-0" x 1'-0"	N	1

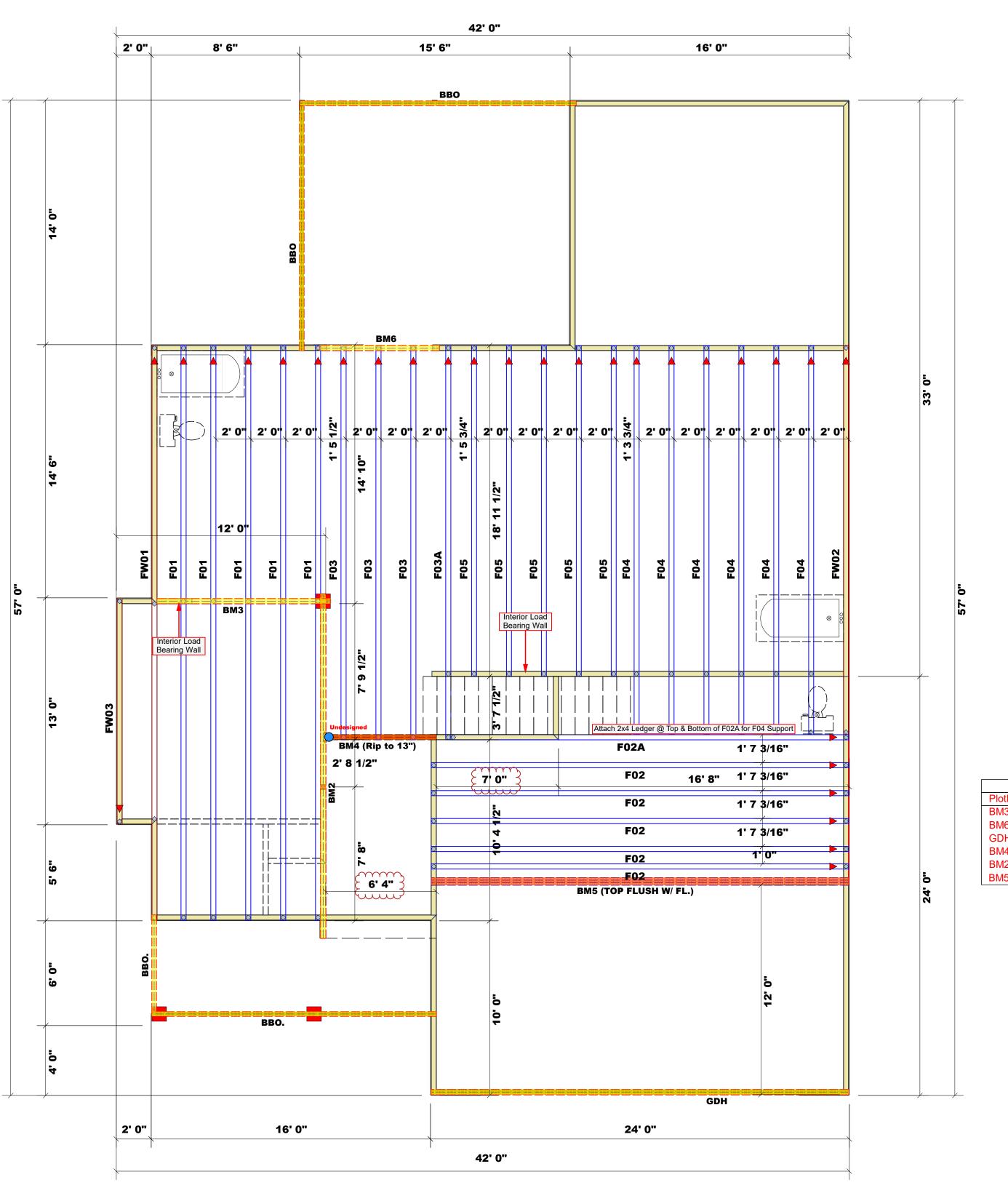


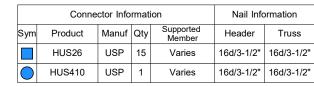
Beaumont

Scale: 1/4"= 1'-0"





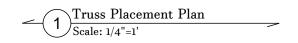






All Walls Shown Are Considered Load Bearing

▲ = Indicates Left End of Truss (Reference Engineered Truss Drawing) Do Not Erect Trusses Backwards



Products						
PlotID	Length	Product	Plies	Net Qty		
BM3	10' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2		
BM6	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2		
GDH	24' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2		
BM4 (Rip to 13")	7' 0"	1-3/4"x 14" LVL Kerto-S	2	2		
BM2	20' 0"	1-3/4"x 16" LVL Kerto-S	2	2		
BM5 (TOP FLUSH W/ FL.)	24' 0"	1-3/4"x 23-7/8" LVL Kerto-S	3	3		



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature___

Neil Baggett

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF
HEADER/GIRDER

END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (4) PLY HEADER
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				
	_	 		 	

<u>≻</u> N	Cumberland
ORESS	Lot 34 Forest Ridge
DEL	Floor
re rev.	1/72021
AWN BY	Neil Baggett
ESMAN	Marshall Naylor

Ben Stout Real Estate	COUNIY
Lot 34 Forest Ridge	ADDRESS
Beaumont/GL (180706B)	MODEL
8/15/2018	DATE REV.
N/A	DRAWN BY
J1220-5676	SALESMAN

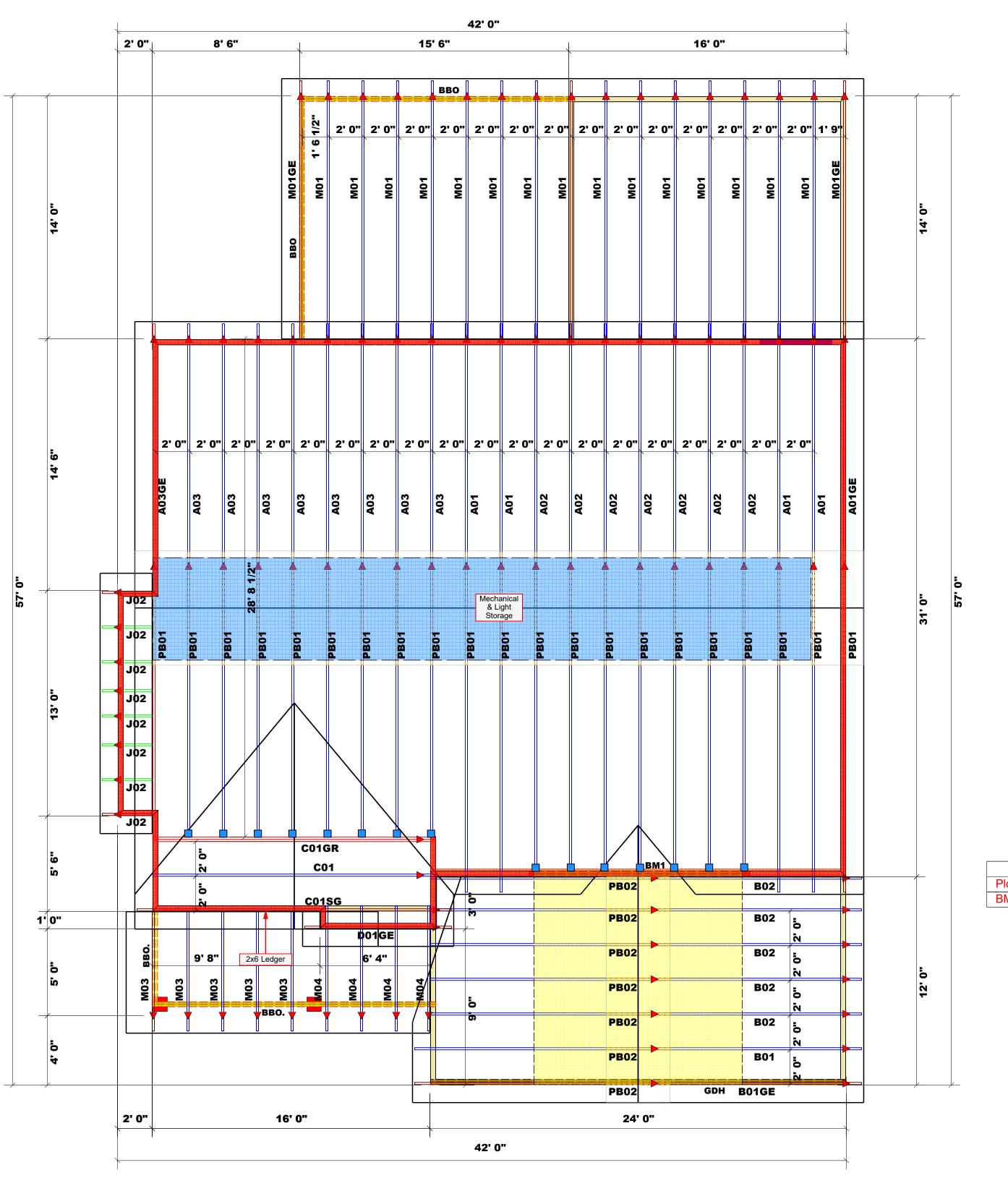
SEAL DATE

PLAN

QUOTE # JOB #

JOB NAME

BUILDER





Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

g reactions less than or equal to 3000# are d to comply with the prescriptive Code ements. The contractor shall refer to the ed Tables (derived from the prescriptive Co

Neil Baggett

Neil Baggett

13600 4

17000 5

Marshall Naylor

J1220-5675

Quote #

LOAD CHART FOR JACK STUDS (BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER 3400 1 1700 1 2550 1 3400 2 5100 2 6800 2 5100 3 10200 3

7650 3 6800 4 10200 4 12750 5 15300 6

Cumberland / Cumberland

Neil Baggett

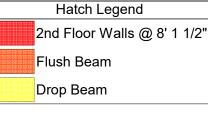
1/7/2021 DRAWN BY SALES REP. DATE REV.

CITY / CO.

Beaumont/GL (180706B) Ben Stout Real Estate 8/15/18 BUILDER

JOB NAME SEAL DATE QUOTE ; THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

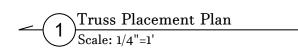


Roof Area = 3035.75 sq.ft. Ridge Line = 73 ft. Hip Line = 0 ft. Horiz. OH = 167.33 ft.

Raked OH = 221.93 ft. Decking = 104 sheets

All Walls Shown Are Considered Load Bearing

▲ = Indicates Left End of Truss (Reference Engineered Truss Drawing) Do Not Erect Trusses Backwards



	Conne	Nail Info	ormation			
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	15	Varies	16d/3-1/2"	16d/3-1/2"
	HUS410	USP	1	Varies	16d/3-1/2"	16d/3-1/2"

		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	13' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2

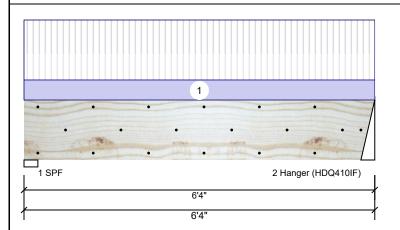


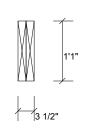
Date: 3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

Project #:

1.750" X 13.000" 2-Ply - PASSED **Kerto-S LVL** BM4

Level: Level





Const

D+L

D+L

0

0

1954 L

1954 L

0

0

Page 1 of 16

Member	Information
Tyne:	Girder

Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temp <= 100°F Temperature:

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Dead Snow Wind Brg Live 513 0

513

44%

21%

1441

1441

1-SPF 3.000"

3.000"

1

2

2 -

Hanger

Bearings			
Bearing Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.

513 / 1441

513 / 1441

0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2739 ft-lb	3'2"	23540 ft-lb	0.116 (12%)	D+L	L
Unbraced	2739 ft-lb	3'2"	15248 ft-lb	0.180 (18%)	D+L	L
Shear	1170 lb	1'3 1/4"	9707 lb	0.121 (12%)	D+L	L
LL Defl inch	0.015 (L/4710)	3'2"	0.149 (L/480)	0.100 (10%)	L	L
TL Defl inch	0.021 (L/3473)	3'2"	0.199 (L/360)	0.100 (10%)	D+L	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	152 PLF	455 PLF	0 PLF	0 PLF	0 PLF	F03 FL. TRUSSES
	Self Weight				10 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/27/2023

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Version 20.80.210 Powered by iStruct™

CSD DESIGN

Client: Project: Address:

Date: 3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

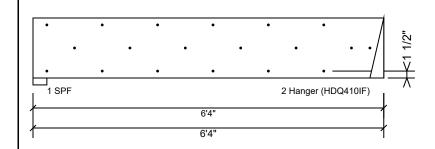
Project #:

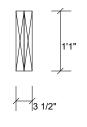
Kerto-S LVL BM4

1.750" X 13.000"

2-Ply - PASSED

Level: Level





Page 2 of 16

Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

rasteri ali piles asirig s rows	or roa box rians (. 120x3) at
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



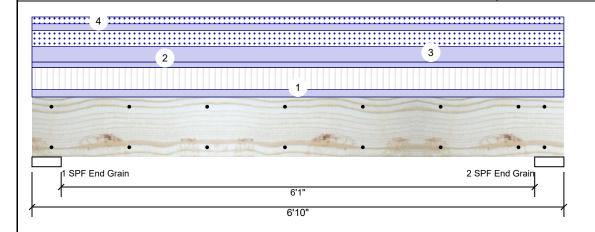


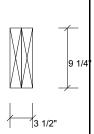
Date: 3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** BM6

Level: Level





Page 3 of 16

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F

Application: Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reaction	Reactions UNPATTERNED lb (Uplift)										
Brg	Live	Dead	Snow	Wind	Const						
1	1548	2508	1579	0	0						
2	1548	2508	1579	0	0						

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6844 ft-lb	3'5"	14423 ft-lb	0.474 (47%)	D+0.75(L+S)	L
Unbraced	6844 ft-lb	3'5"	10370 ft-lb	0.660 (66%)	D+0.75(L+S)	L
Shear	3503 lb	1'1"	7943 lb	0.441 (44%)	D+0.75(L+S)	L
LL Defl inch	0.061 (L/1212)	3'5"	0.155 (L/480)	0.400 (40%)	0.75(L+S)	L
TL Defl inch	0.127 (L/586)	3'5"	0.207 (L/360)	0.610 (61%)	D+0.75(L+S)	L

Bearings

Grain

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 4.500" 2508 / 2345 4853 L D+0.75(L+S) End Grain 2 - SPF 4.500" 2508 / 2345 4853 L D+0.75(L+S) End

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

	g	F-7								
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	151 PLF	453 PLF	0 PLF	0 PLF	0 PLF	F03 FL. TRUSSES
2	Uniform			Тор	114 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL ABOVE
3	Uniform			Тор	322 PLF	0 PLF	322 PLF	0 PLF	0 PLF	A03 RF. TRUSSES
4	Uniform			Far Face	140 PLF	0 PLF	140 PLF	0 PLF	0 PLF	M01 RF. TRUSSES
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





Client: Project: Address: Date: 3/22/2021 Input by: Neal Baggett

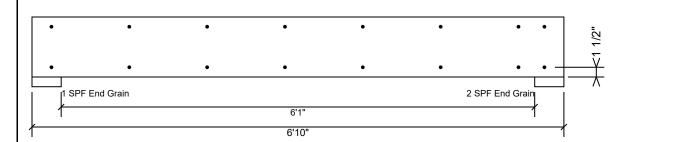
Job Name: BEAUMONT Project #:

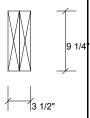
Kerto-S LVL BM6

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 4 of 16

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	74.4 %
Load	140.0 PLF
Yield Limit per Foot	188.3 PLF
Yield Limit per Fastener	94.1 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	D+S
Duration Factor	1.15

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



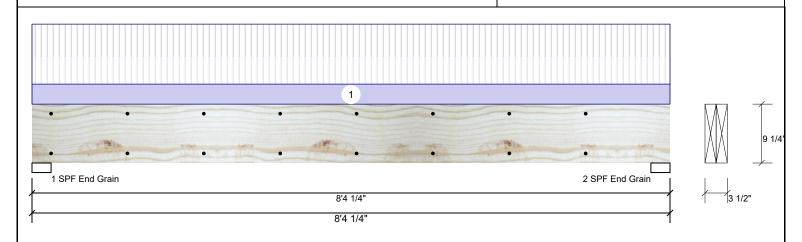


3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

Project #:

1.750" X 9.250" 2-Ply - PASSED **BM3** Kerto-S LVL

Level: Level



Member Info	rmation			Reactio	Reactions UNPATTERNED lb (Uplift)					
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const	
Plies:	2	Design Method:	ASD	1	3254	1116	0	0	0	
Moisture Condition	on: Dry	Building Code:	IBC/IRC 2015	2	3254	1116	0	0	0	
Deflection LL:	480	Load Sharing:	No							
Deflection TL:	360	Deck:	Not Checked							
Importance:	Normal - II									
Temperature:	Temp <= 100°F									
				Bearing	gs					
				Bearing	g Length	Cap. Reac	t D/L lb	Total Ld. Case	Ld. Comb.	
				1 - SPF	3.000"	48% 1116	6 / 3254	4370 L	D+L	
				End						
Analysis Resu	ılts			Grain						

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8326 ft-lb	4'2 1/8"	12542 ft-lb	0.664 (66%)	D+L	L
Unbraced	8326 ft-lb	4'2 1/8"	8569 ft-lb	0.972 (97%)	D+L	L
Shear	3367 lb	7'4 3/4"	6907 lb	0.488 (49%)	D+L	L
LL Defl inch	0.176 (L/544)	4'2 3/16"	0.199 (L/480)	0.880 (88%)	L	L
TL Defl inch	0.236 (L/405)	4'2 3/16"	0.266 (L/360)	0.890 (89%)	D+L	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

Bearings	5						
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF End Grain	3.000"	48%	1116 / 3254	4370	L	D+L	
2 - SPF End Grain	3.000"	48%	1116 / 3254	4370	L	D+L	

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	260 PLF	779 PLF	0 PLF	0 PLF	0 PLF	F01 FL. TRUSSES
	Self Weight				7 PLF					

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 5 of 16

Client: Project: Address: Date: 3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

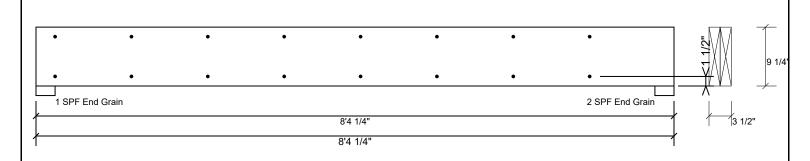
Project #:

Kerto-S LVL BM₃

1.750" X 9.250"

2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	163.7 PLF	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 6 of 16

This design is valid until 11/27/2023 CSD DESIGN

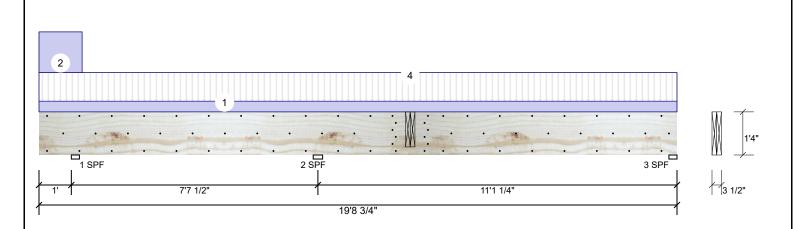


3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

Project #:

Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED BM₂

Level: Level



Member Information Type:

Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance:

Normal - II Temperature: Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift) Application: Floor

1	Brg	Live	Dead	Snow	Wind	Const	
1	1	94	243	0	0	0	
1	2	2305	973	0	0	0	
	3	621	287	0	0	0	

Bearings

Bearing Length	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF 3.000"	13%	230 / 366 5	96 (-63)	LL_	D+L(D+L)	
2 - SPF 3.500"	65%	994 / 2364	3358	_LL	D+L	
3 - SPF 3.000"	20%	278 / 620	899	LL	D+L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-3543 ft-lb	8'7 1/2"	34565 ft-lb	0.103 (10%)	D+L	_LL
Unbraced	-3543 ft-lb	8'7 1/2"	11260 ft-lb	0.315 (31%)	D+L	_LL
Pos Moment	3140 ft-lb	12'4 11/16"	34565 ft-lb	0.091 (9%)	D+L	L_L
Unbraced	3140 ft-lb	12'4 11/16"	11260 ft-lb	0.279 (28%)	D+L	L_L
Shear	2274 lb	9'11 1/2"	11947 lb	0.190 (19%)	D+L	_LL
LL Defl inch	0.027 (L/4886)	13'5 15/16"	0.273 (L/480)	0.100 (10%)	L	L_L
TL Defl inch	0.037 (L/3498)	13'6 1/2"	0.364 (L/360)	0.100 (10%)	D+L	L_L
LL Cant	0.001 (2L/16363)	Lt Cant	0.200 (2L/480)	0.007 (1%)	L	L_L
TL Cant	0.002 (2L/12927)	Lt Cant	0.300 (2L/360)	0.006 (1%)	D+L	L_L

Design Method:

Building Code:

Load Sharing:

Deck:

ASD

No

IBC/IRC 2015

Not Checked

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Tie-down connection required at bearing 1 for uplift 63 lb (Combination D+L, Load Case L).
- 7 Top braced at bearings.
- 8 Bottom braced at bearings.
- 9 Lateral slenderness ratio based on single ply width.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

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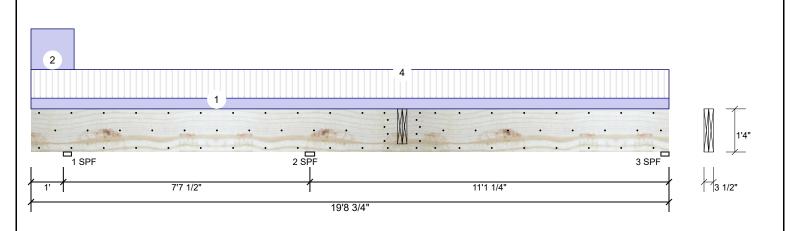


Date: 3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

Project #:

1.750" X 16.000" 2-Ply - PASSED **Kerto-S LVL BM2**

Level: Level



ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	30 PLF	80 PLF	0 PLF	0 PLF	0 PLF	FL. LOADING	
2	Part. Uniform	0-0-0 to 1-4-0		Тор	114 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL ABOVE	
4	Point	11-5-12		Far Face	513 lb	1441 lb	0 lb	0 lb	0 lb	7'-FB. @ FOYER Brg 2	
	Self Weight				12 PLF						

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. IVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

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BM₂

Client: Project: Address:

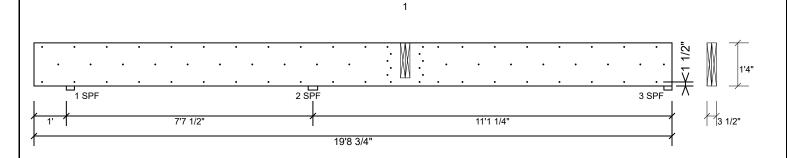
3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

Project #:

1.750" X 16.000" **Kerto-S LVL**

2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

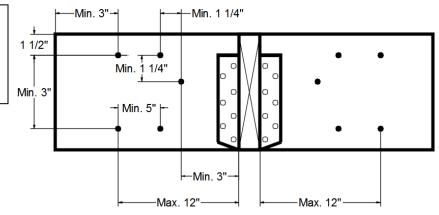
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 11-5-12 with a minimum of (12) - 10d Box nails (.128x3") in the pattern shown.

P 4		
Capacity Load	99.5 %	
Load	977.1lb.	
Total Yield Limit	982.0 lb.	
Cg	0.9998	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	IV	
Load Combination	D+L	
Duration Factor	1 00	

Min/Max fastener distances for Concentrated Side Loads



Notes

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/27/2023

ICC-ES: ESR-3633

Manufacturer Info

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Date: 3/22/2021 Input by: Neal Baggett

Job Name: BEAUMONT

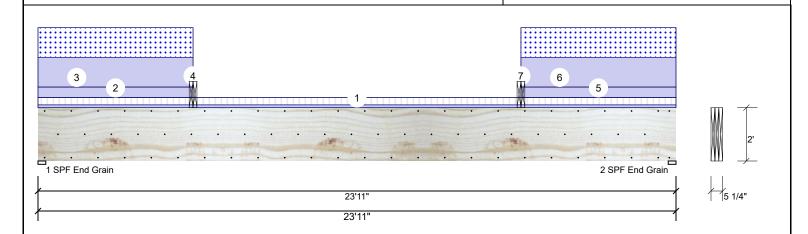
Project #:

2 - SPF 3.500"

End Grain

1.750" X 24.000" 3-Ply - PASSED **Kerto-S LVL** BM₅

Level: Level



Reactions UNPATTERNED Ib (Uplift) Application: Brg Wind Type: Floor Live Dead Snow Plies: 3 Design Method: ASD 957 5205 3791 0 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 957 5205 3791 0 Deflection LL: 480 Load Sharing: Yes Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temp <= 100°F Temperature: **Bearings** Bearing Length Cap. React D/L lb Total Ld. Case 1-SPF 3.500" 5205 / 3791 8996 L End Grain

Analysis Results

Member Information

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	37841 ft-lb	11'11 1/2"	131295 ft-lb	0.288 (29%)	D+0.75(L+S)	L
Unbraced	37841 ft-lb	11'11 1/2"	37957 ft-lb	0.997 (100%)	D+0.75(L+S)	L
Shear	7185 lb	21'8 3/8"	30912 lb	0.232 (23%)	D+S	L
LL Defl inch	0.160 (L/1763)	11'11 9/16"	0.587 (L/480)	0.270 (27%)	S	L
TL Defl inch	0.378 (L/746)	11'11 9/16"	0.783 (L/360)	0.480 (48%)	D+S	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 7'1 1/8" o.c.
- 6 Bottom braced at bearings.

7 Lateral slend	derness ratio based	on single ply width.								
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	30 PLF	80 PLF	0 PLF	0 PLF	0 PLF	FL. LOADING
2	Part. Uniform	0-0-0 to 5-9-12		Тор	114 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL ABOVE
3	Part. Uniform	0-0-0 to 5-9-12		Тор	322 PLF	0 PLF	322 PLF	0 PLF	0 PLF	A01 RF. TRUSSES
4	Point	5-9-12		Тор	1977 lb	0 lb	1919 lb	0 lb	0 lb	13'-FB. @ PLAY ROOM Brg 1
5	Part. Uniform	18-1-4 to 23-11-0		Тор	114 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL ABOVE

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

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5205 / 3791

8996 L

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Const

0

0

Ld. Comb.

D+S

D+S

This design is valid until 11/27/2023

Manufacturer Info

BM5

Kerto-S LVL

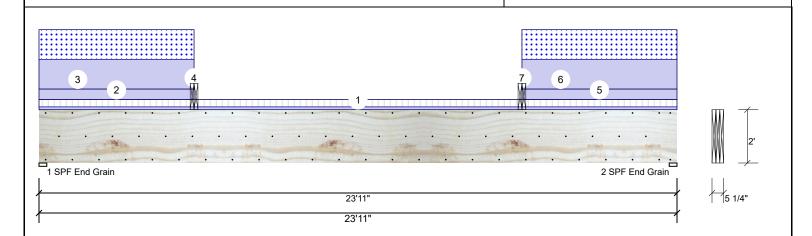
Client: Project: Address:

Date: 3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

Project #:

1.750" X 24.000" 3-Ply - PASSED

Level: Level



Continued	from	page	1

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
6	Part. Uniform	18-1-4 to 23-11-0		Тор	322 PLF	0 PLF	322 PLF	0 PLF	0 PLF	A01 RF. TRUSSES
7	Point	18-1-4		Тор	1977 lb	0 lb	1919 lb	0 lb	0 lb	13'-FB. @ PLAY ROOM Brg 2

Self Weight 28 PLF

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. IVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

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Client: Project: Address:

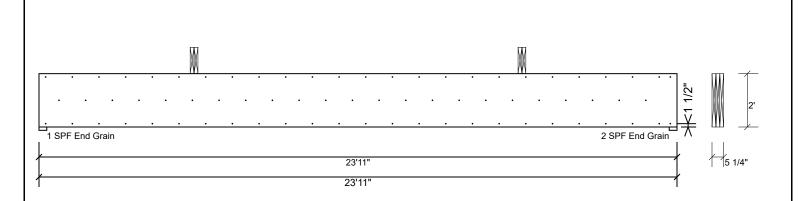
3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

Project #:

1.750" X 24.000" **Kerto-S LVL** BM5

3-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Informing & Installation

 I. VIL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

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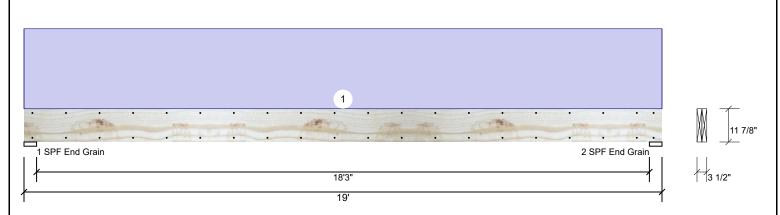


3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

Project #:

1.750" X 11.875" 2-Ply - PASSED **Kerto-S LVL GDH**

Level: Level



Member Infor	mation						Reaction	ns UNPAT	TERNED	lb (Uplift))		
Type:	Girder		Applicati	on: F	loor		Brg	Live	Dead	Snow	,	Wind	Const
Plies:	2		Design N	Method: A	SD		1	0	2016	0		0	0
Moisture Condition	n: Dry		Building	Code: IE	3C/IRC 2015		2	0	2016	0		0	0
Deflection LL:	480		Load Sh	aring: N	О								
Deflection TL:	360		Deck:	N	ot Checked								
Importance:	Normal - II												
Temperature:	Temp <= 10	0°F											
							Bearing	s					
							Bearing	Length	Cap. R	leact D/L lb	Total	Ld. Case	Ld. Comb
							1 - SPF End	4.500"	15%	2016 / 0	2016	Uniform	D
Analysis Resul	ts						Grain						
Analysis A	ctual	Location	Allowed	Capacity	Comb.	Case	2 - SPF End	4.500"	15%	2016 / 0	2016	Uniform	D
Moment 89	957 ft-lb	9'6"	17919 ft-lb	0.500 (50%) D	Uniform	Grain						
Unbraced 89	57 ft-lb	9'6"	8966 ft-lb	0.999	D	Uniform							

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8957 ft-lb	9'6"	17919 ft-lb	0.500 (50%)	D	Uniform
Unbraced	8957 ft-lb	9'6"	8966 ft-lb	0.999 (100%)	D	Uniform
Shear	1740 lb	17'8 3/8"	7980 lb	0.218 (22%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.582 (L/379)	9'6 1/16"	0.612 (L/360)	0.950 (95%)	D	Uniform

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 10'4 1/8" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

. Lateral eleriae	minese rade bassa sir sirigis	p.,a									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	203 PLF	0 PLF	0 PLF	0 PLF	0 PLF	END WALL / GABLE	
	Self Weight				9 PLF						

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

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Client: Project: Address:

3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

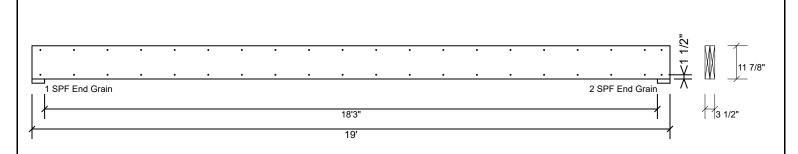
Level: Level

Project #:

Kerto-S LVL GDH

1.750" X 11.875"

2-Ply - PASSED



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

rasterrain pries asing E	TOWS OF TOO BOX Halls (.TEOXS) at
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

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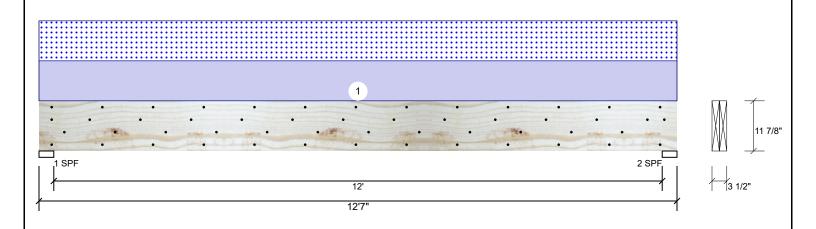


3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

Project #:

Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED BM1

_evel: Level



Member Inforr	nation			Reactio	ns UNPAT	TERNED II	b (Uplift)		
Type:	Girder	Application:	Roof	Brg	Live	Dead	Snow	Wind	Const
Plies:	2	Slope:	0/12	1	0	1977	1919	0	0
Moisture Condition	: Dry	Design Method:	ASD	2	0	1977	1919	0	0
Deflection LL:	360	Building Code:	IBC/IRC 2015						
Deflection TL:	240	Load Sharing:	No						
Importance:	Normal - II	Deck:	Not Checked						
Temperature:	Temp <= 100°F								
				Bearing	S				
				Bearing	Length	Cap. Rea	ct D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	3.500"	75% 19	77 / 1919	3896 L	D+S
				2 - SPF	3.500"	75% 19	77 / 1919	3896 L	D+S

Analysis Results

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	11380 ft-lb	6'3 1/2"	22897 ft-lb	0.497 (50%)	D+S	L
Unbraced	11380 ft-lb	6'3 1/2"	11401 ft-lb	0.998 (100%)	D+S	L
Shear	3745 lb	1'2 5/8"	10197 lb	0.367 (37%)	D+S	L
LL Defl inch	0.167 (L/869)	6'3 1/2"	0.404 (L/360)	0.410 (41%)	S	L
TL Defl inch	0.340 (L/428)	6'3 1/2"	0.606 (L/240)	0.560 (56%)	D+S	L

Design Notes

- 1 Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top must be laterally braced at a maximum of 7'9" o.c.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

		<u> </u>									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Far Face	305 PLF	0 PLF	305 PLF	0 PLF	0 PLF	A02 RF. TRUSSES	
	Self Weight				9 PLF						

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Informing & Installation

 I. VIL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation

Handling & Installation

For flat roofs provide proper drainage to prevent ponding

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Client: Project: Address:

3/22/2021 Input by: Neal Baggett Job Name: BEAUMONT

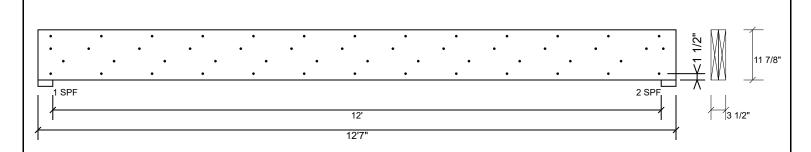
Project #:

Kerto-S LVL BM1

1.750" X 11.875"

2-Ply - PASSED

_evel: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

1 3		`	,
Capacity	81.0 %		
Load	305.0 PLF		
Yield Limit per Foot	376.5 PLF		
Yield Limit per Fastener	94.1 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination	D+S		
Duration Factor	1.15		

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

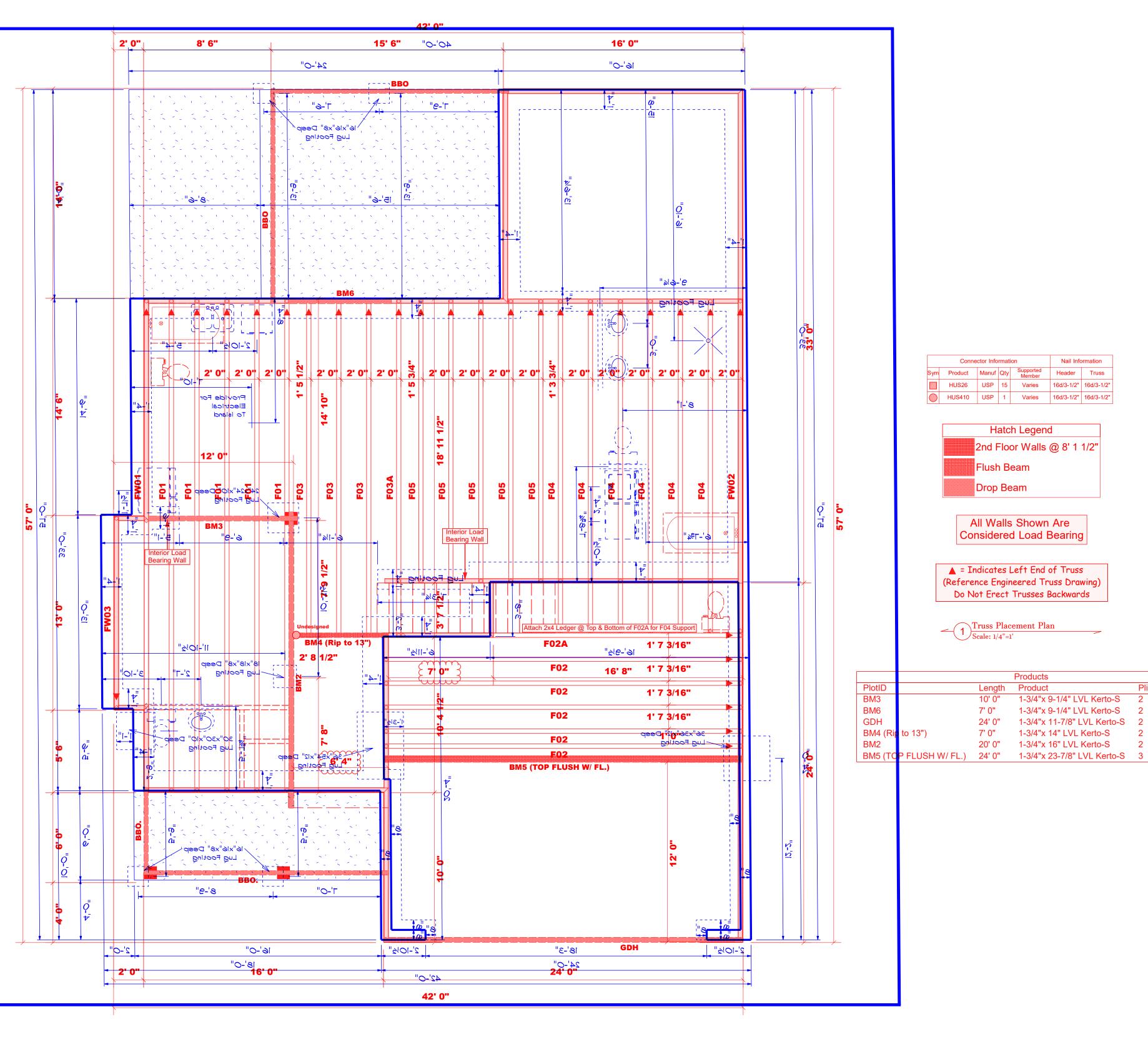
Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



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COMTECH **ROOF & FLOOR TRUSSES & BEAMS**

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. at the specification of the building designer. See individual design sheets for each truss design identifie on the placement drawing. The building designer is responsible for temporary and permanent bracing of throof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached requirements. The contractor shall refer to the attache Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Neil Baggett

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF

		HEADER/	GIRDE	₹		
END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (4) PLY HEADER
1700	1	2550	1		3400	1
3400	2	5100	2		6800	2
5100	3	7650	3		10200	3
6800	4	10200	4		13600	4
8500	5	12750	5		17000	5
10200	6	15300	6			
11900	7					
13600	8					
15300	٥					

			Products		
PlotID		Length	Product	Plies	Net Qty
3M3		10' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
3M6		7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
3DH .		24' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
3M4 (Ri <mark>p</mark> t	to 13")	7' 0"	1-3/4"x 14" LVL Kerto-S	2	2
3M2		20' 0"	1-3/4"x 16" LVL Kerto-S	2	2
3M5 (TOP	FLUSH W/ FL)	24' 0"	1-3/4"x 23-7/8" LVL Kerto-S	3	3

Stout Real Estate	>	Cumberland	8500 8500 10200 11900 13600 15300
34 Forest Ridge	S	S Lot 34 Forest Ridge	5 6 7 8 9
umont/GL (180706B)		Floor	1275 1530
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	>	Y Neil Baggett	1700
20-5676	Z	N Marshall Naylor	

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