

NOTICE TO CONTRACTOR
 All construction must comply with current NC Building Codes and is subject to field inspection and verification.

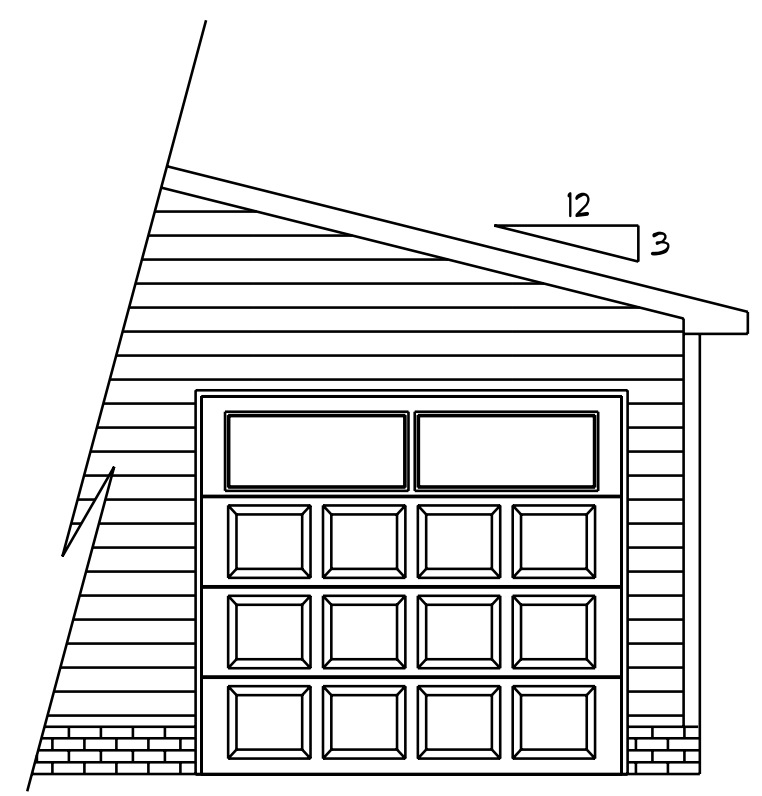
APPROVED
 Limited building only review
 Permit holder responsible for full compliance with the code

03/08/2021

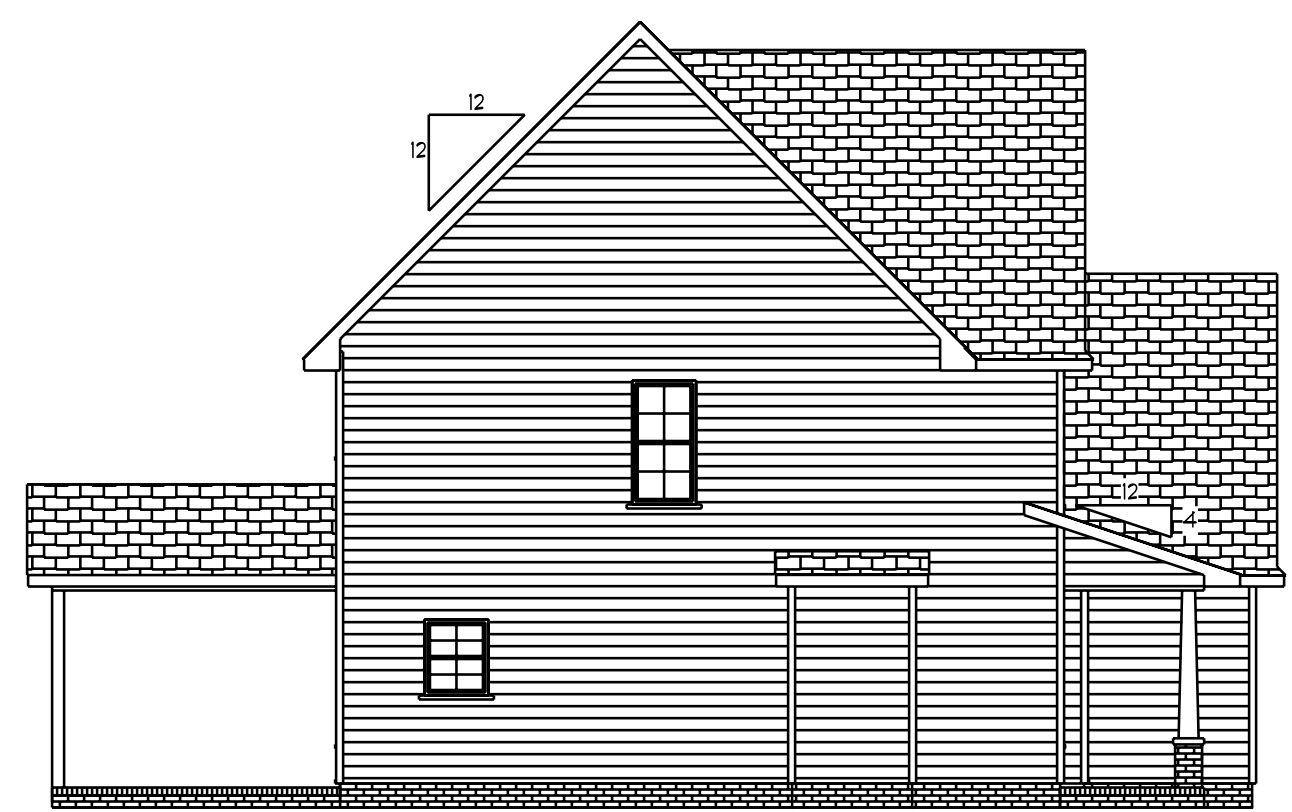




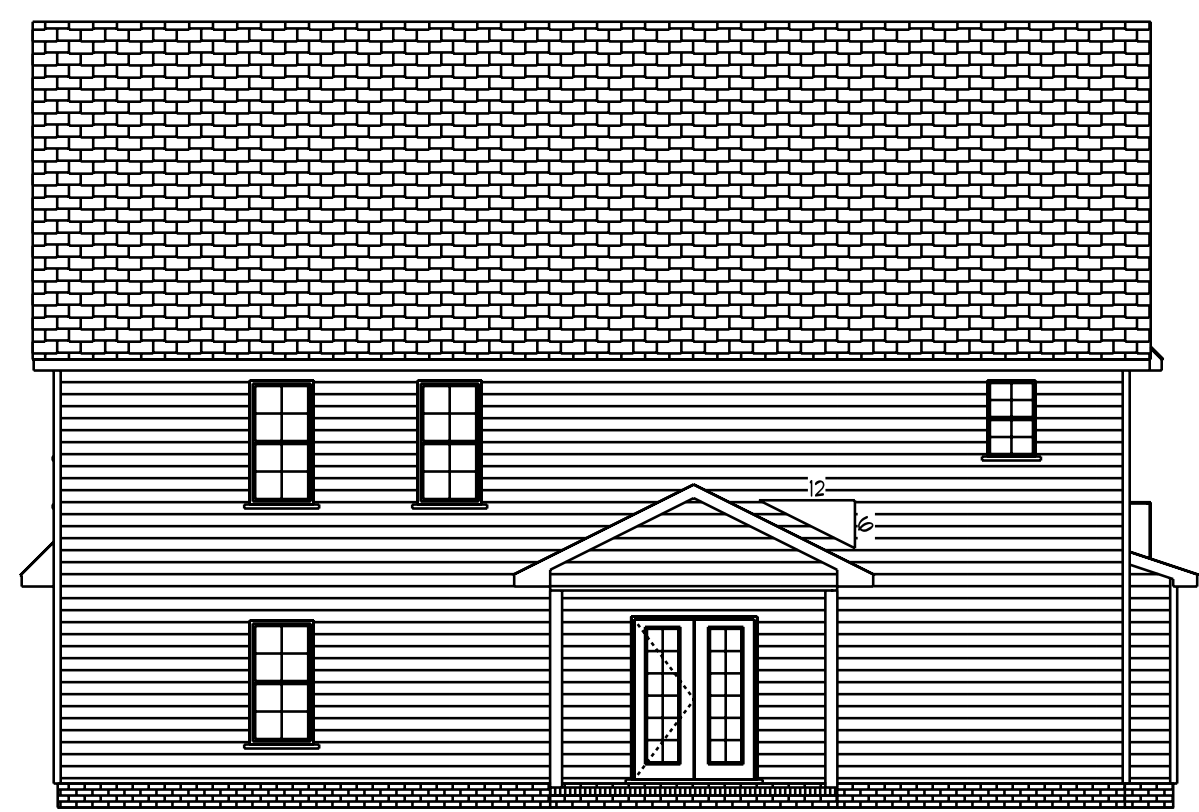
Front Elevation
 Scale: 1/4" = 1'0"



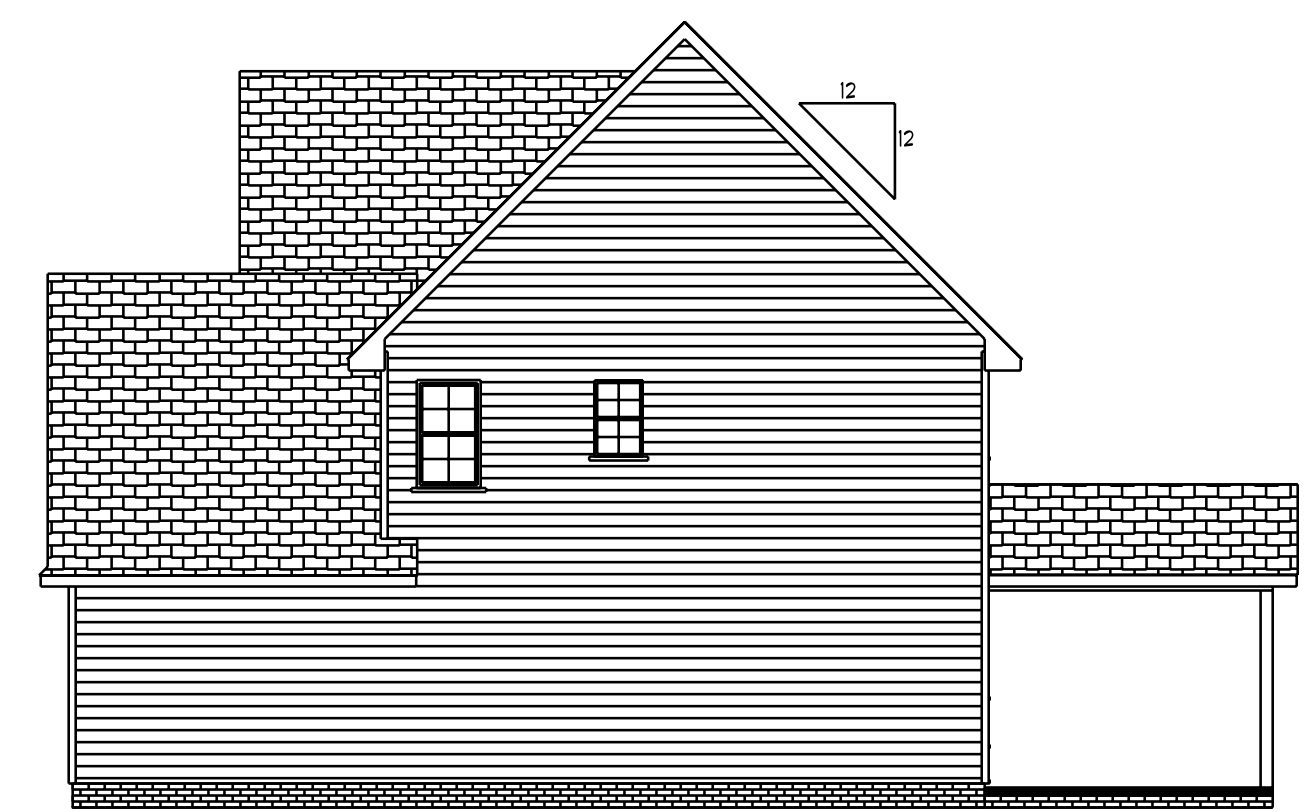
Optional
 3rd Garage



Left Elevation
 Scale: 1/8" = 1'0"



Rear Elevation
 Scale: 1/8" = 1'0"

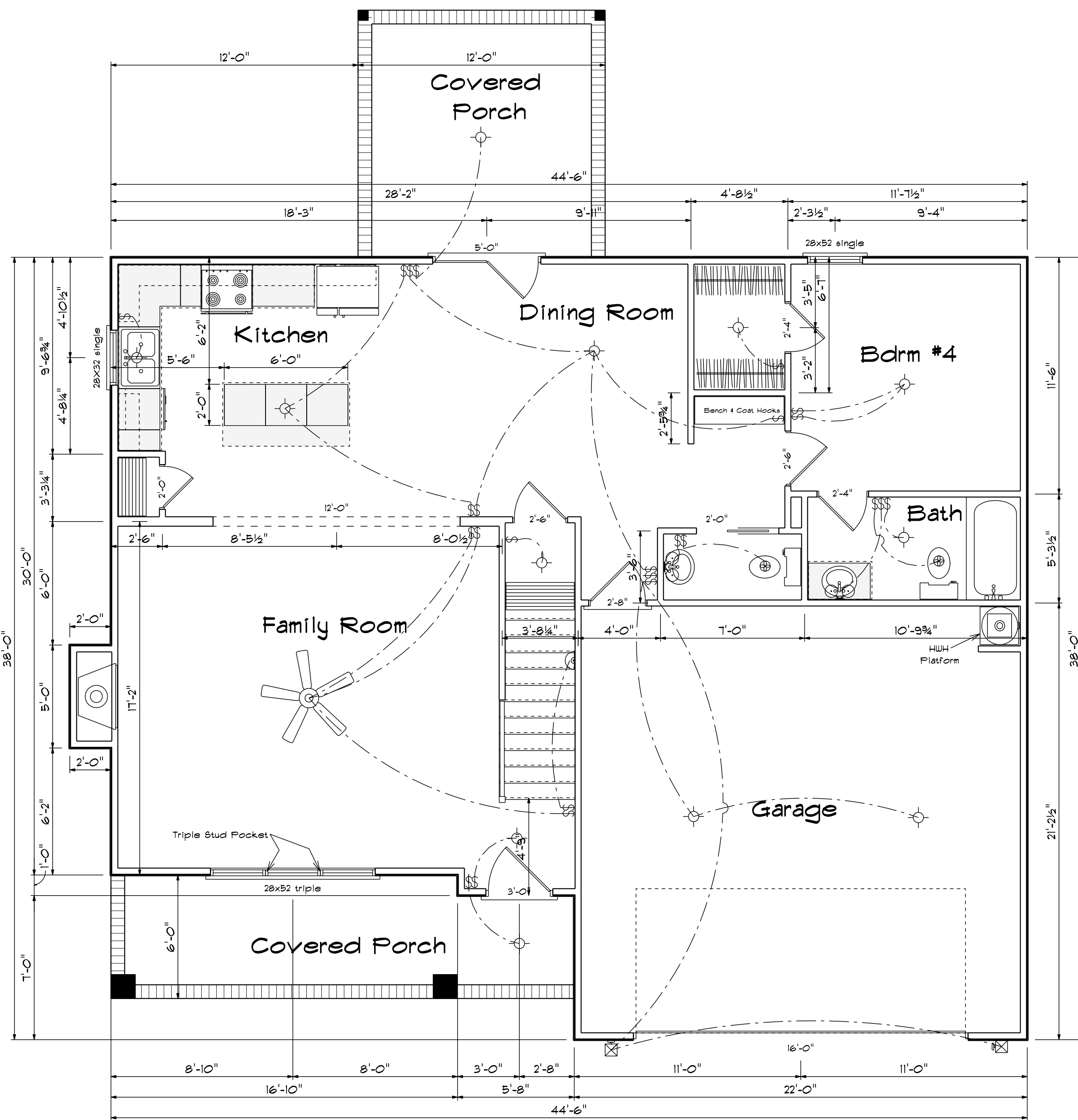


Right Elevation
 Scale: 1/8" = 1'0"

DATE: 4/16/2020
 REVISED
 DRAWING#

SCALE: 1/4"
 DRAWN BY
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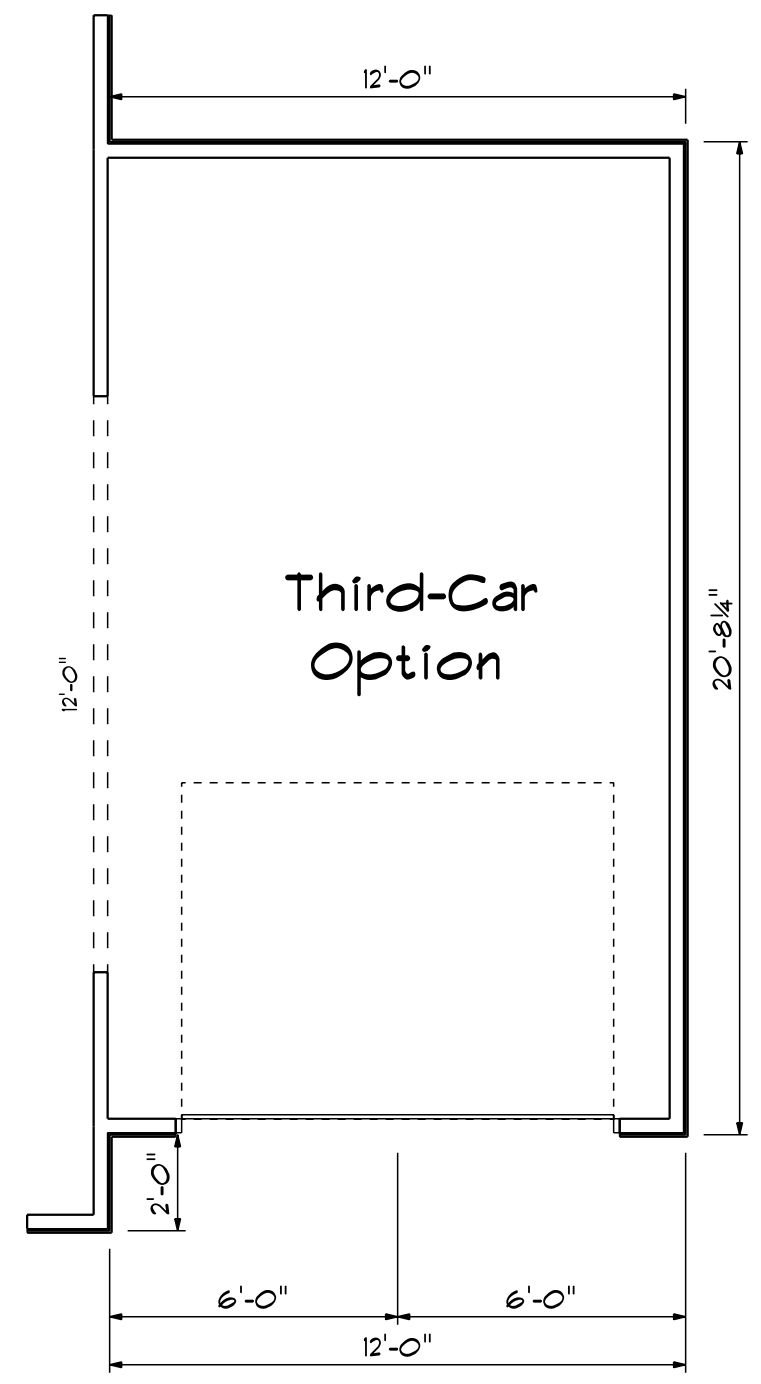
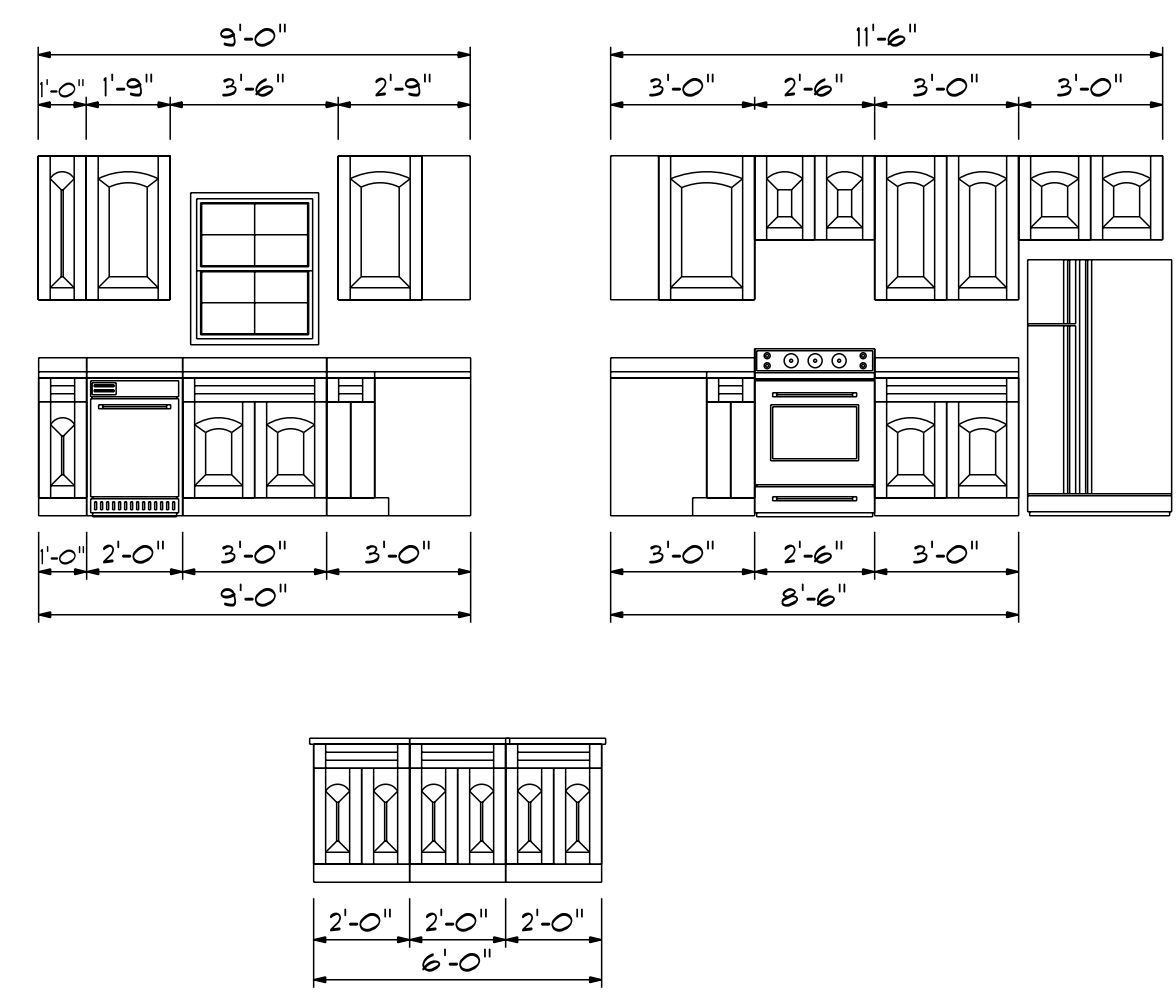
The Ashville



First Floor Plan

Scale: 1/4" = 1'-0"

Kitchen Cabinets



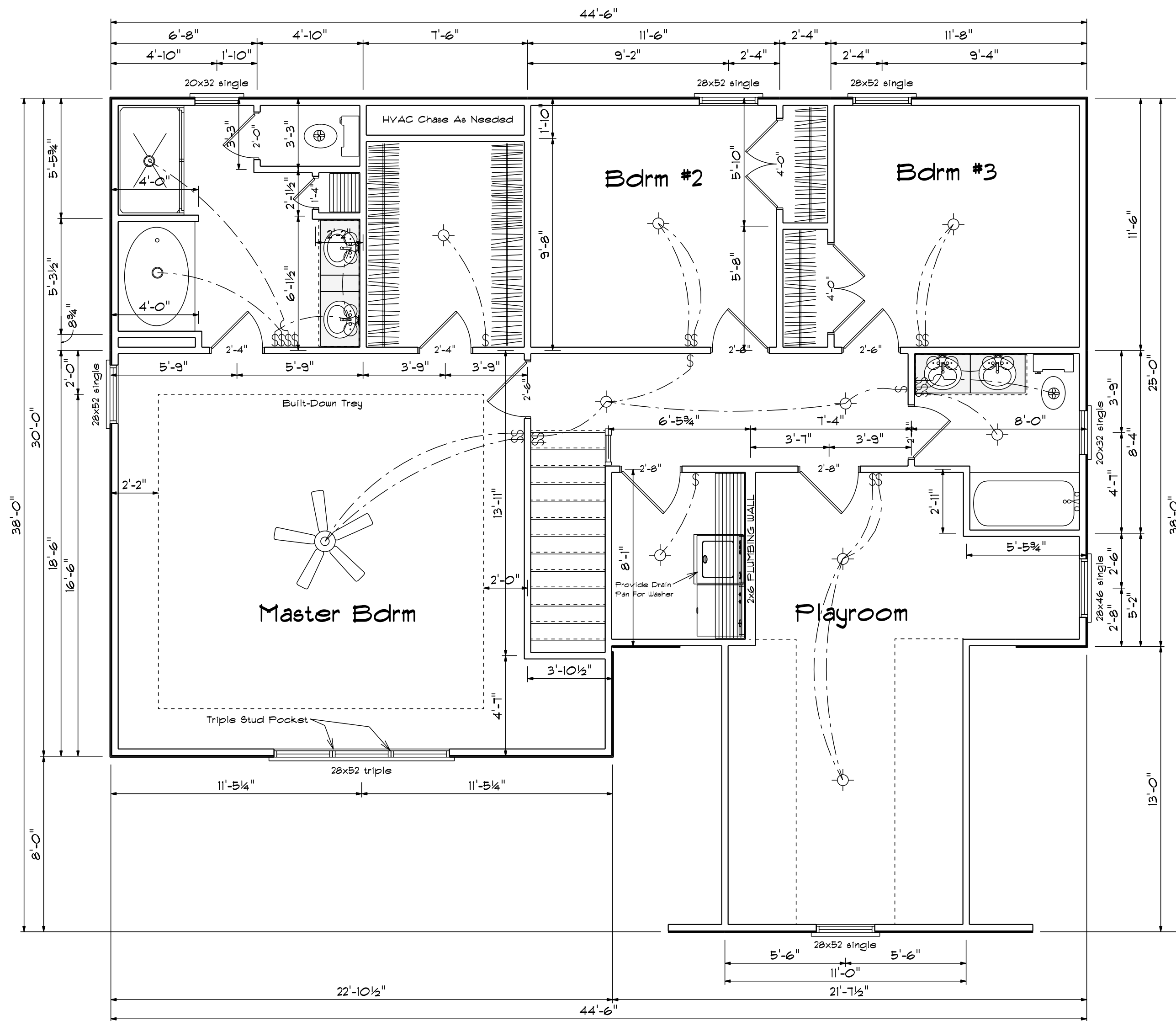
FIRST FLOOR OPENING SCHEDULE			
PRODUCT CODE	SIZE	HINGE	COUNT
36X80 COLONIAL A 1	3'-0"	R	1
60X80 FRENCH A 2	5'-0"	RN	1
192X84 - 8 PANEL - 4 WINDOW	16'-0"	U	1
2-4 Door Unit	2'-4"	R	1
2-4 Door Unit	2'-4"	L	1
2-6 Door Unit	2'-6"	R	2
2-8 Door Unit	2'-8"	L	1
3-0 Doublehung Door Unit	3'-0"	LR	1
20 pocket	2'-0"	N	1
28X32 single	2'-8" x 3'-2"	N	1
28x52 single	2'-8" x 5'-2"	N	1
28x52 triple	8'-0" x 5'-2"	NA	1

Areas

First Floor	1063
Second Floor	1344
=====	
Total Heated	2407
Garage	461
Front Porch	128
Covered Porch	144
Optionag Garage	257

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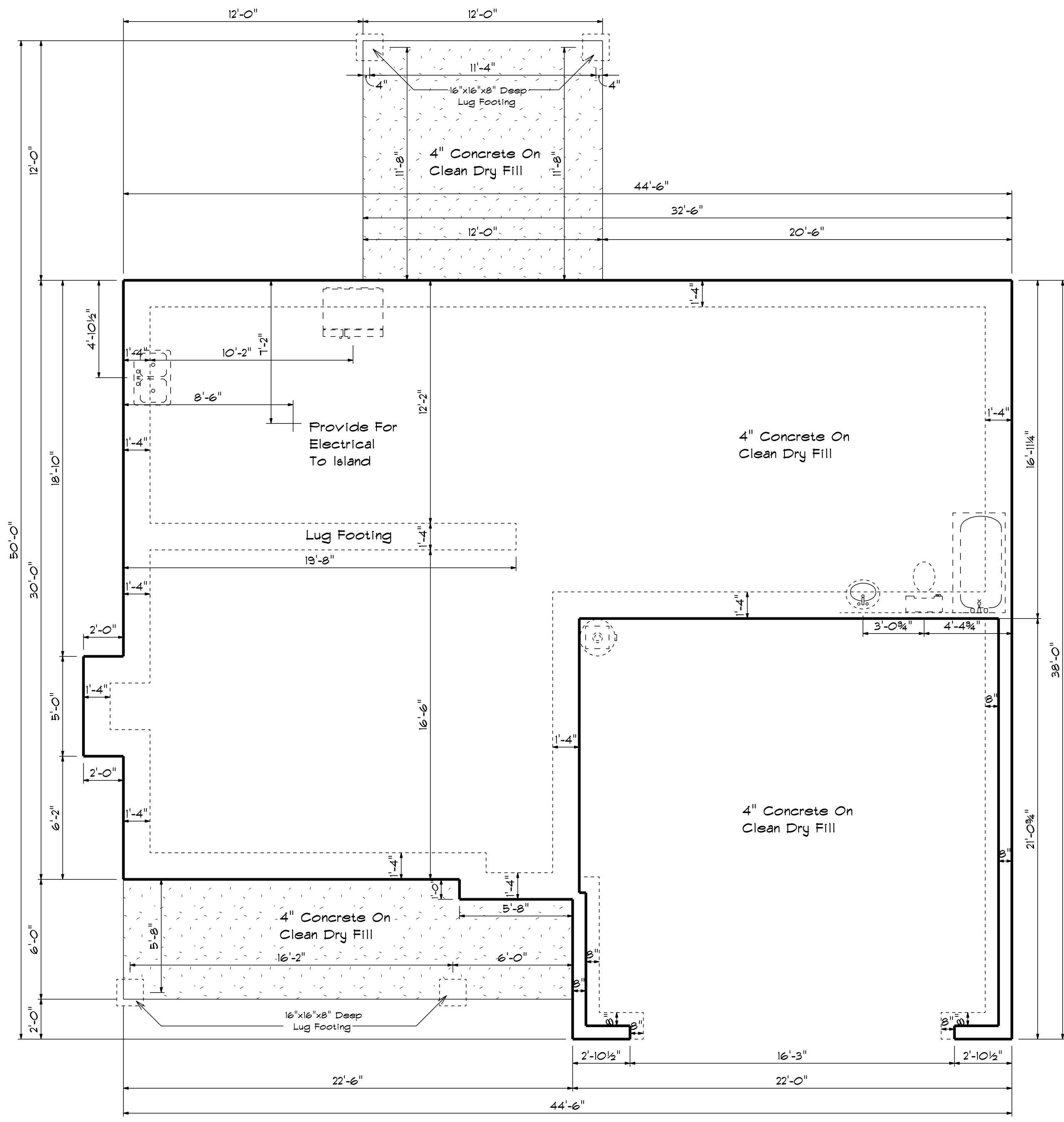
Ashville



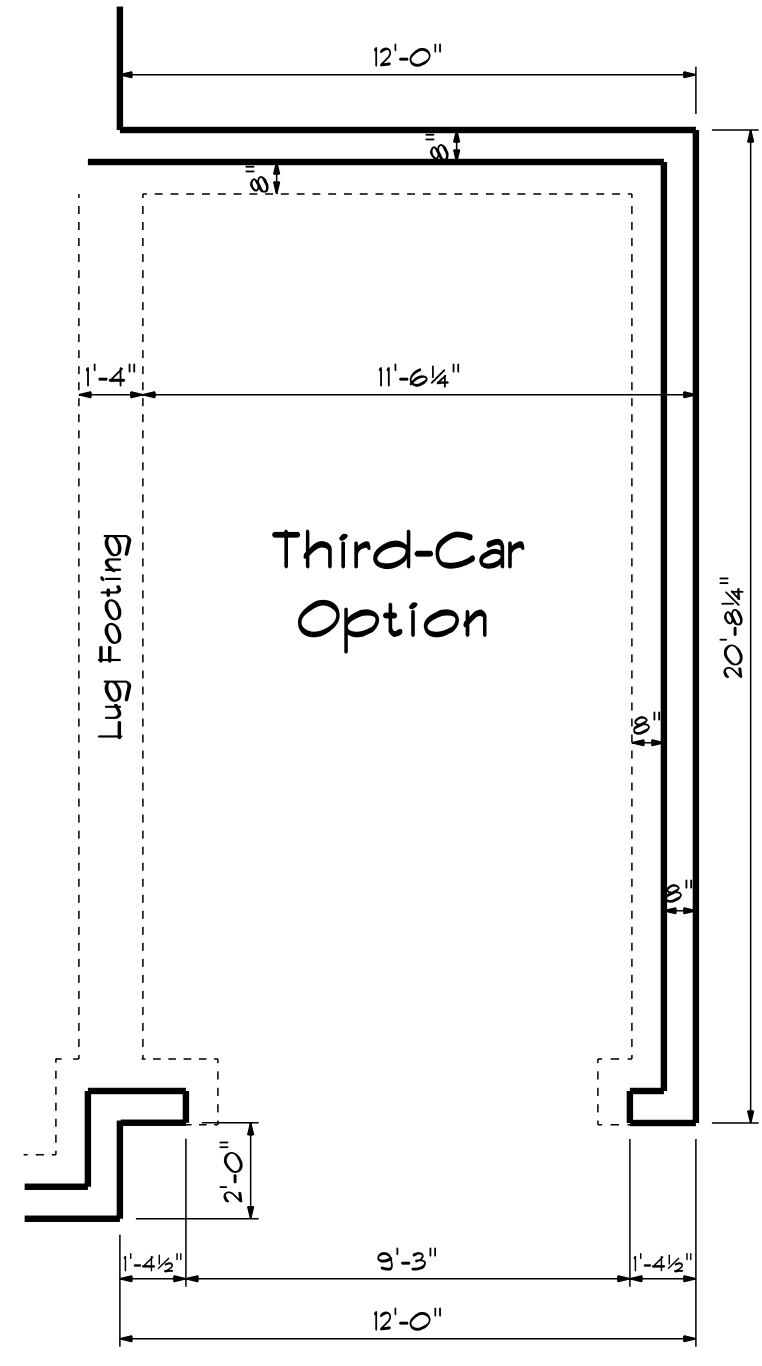
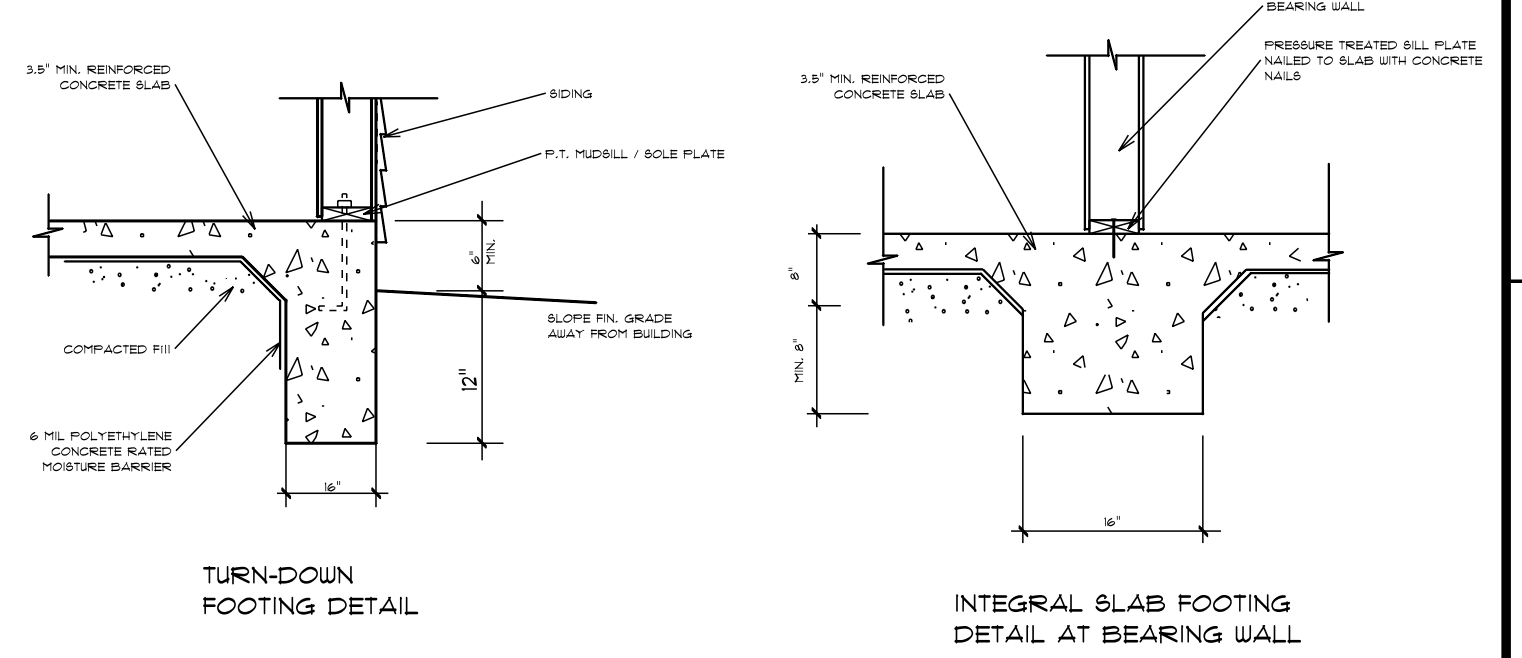
SECOND FLOOR OPENING SCHEDULE			
PRODUCT CODE	SIZE	HINGE	COUNT
1-6 Door Unit	1'-4"	R	1
2-0 Door Unit	2'-0"	R	1
2-4 Door Unit	2'-4"	R	1
2-4 Door Unit	2'-4"	L	2
2-6 Door Unit	2'-6"	R	2
2-6 Door Unit	2'-6"	L	1
2-8 Door Unit	2'-8"	R	2
4-0 Doublehung Door Unit	4'-0"	LR	2
20x32 single	2'-0" x 3'-2"	N	2
28x52 single	2'-8" x 5'-2"	N	5
28x52 triple	8'-0" x 5'-2"	NA	1

Second Floor Plan
 Scale: 1/4" = 1'-0"

<p>Ashville</p>	<p>DATE: 4/16/2020 REVISIONS DRAWING#</p>
<p>SCALE: 1/4" DRAWN BY APPROVED</p>	



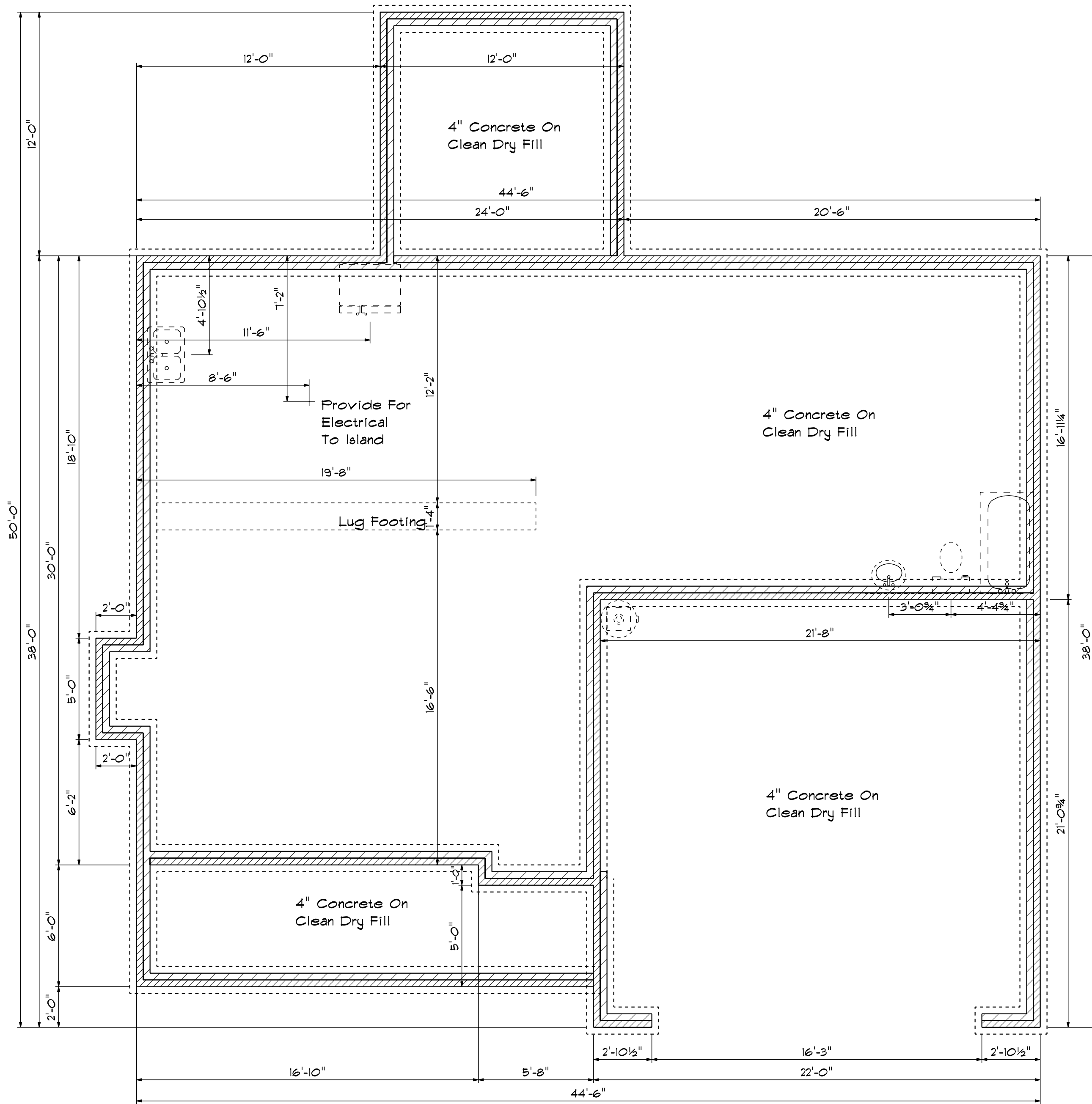
Foundation Plan
 Scale: 1/4" = 1'-0"



DATE: 4/16/2020
 REVISED
 DRAWING#

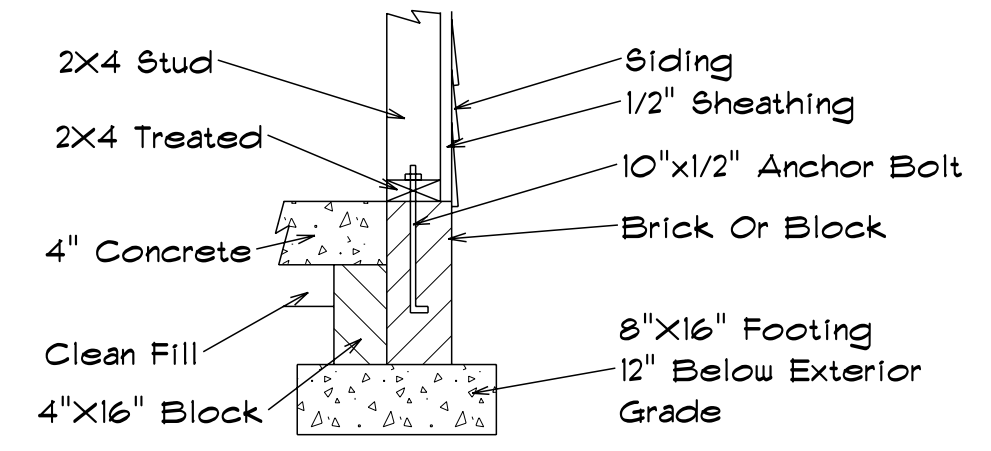
SCALE: 1/4"
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The Ashville

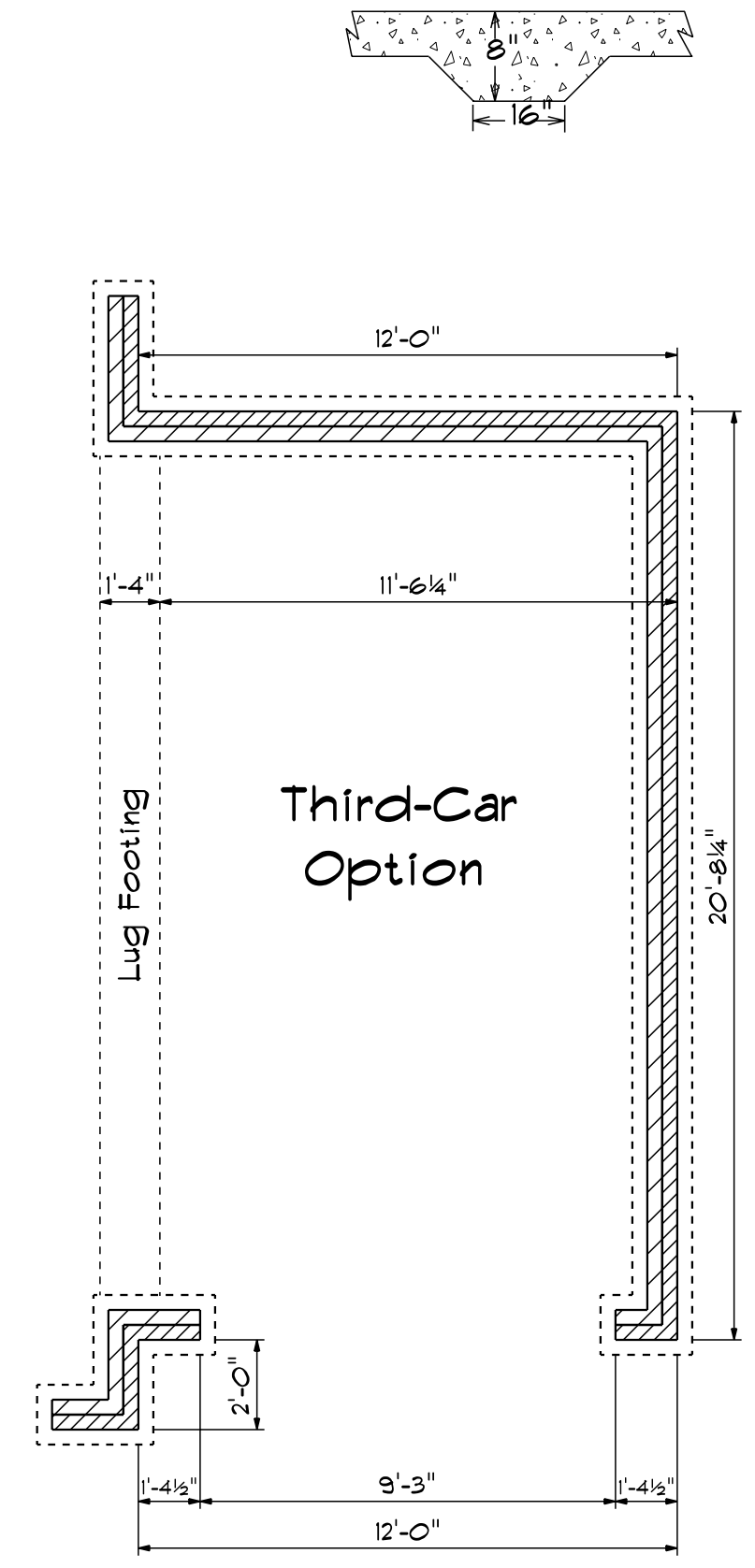


Foundation Plan
Scale: 1/4" = 1'-0"

Foundation Detail Siding



Lug Footing Detail



DATE: 4/16/2020
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DRAWING#

SCALE: 1/4"
DRAWN BY
APPROVED

The Ashville

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

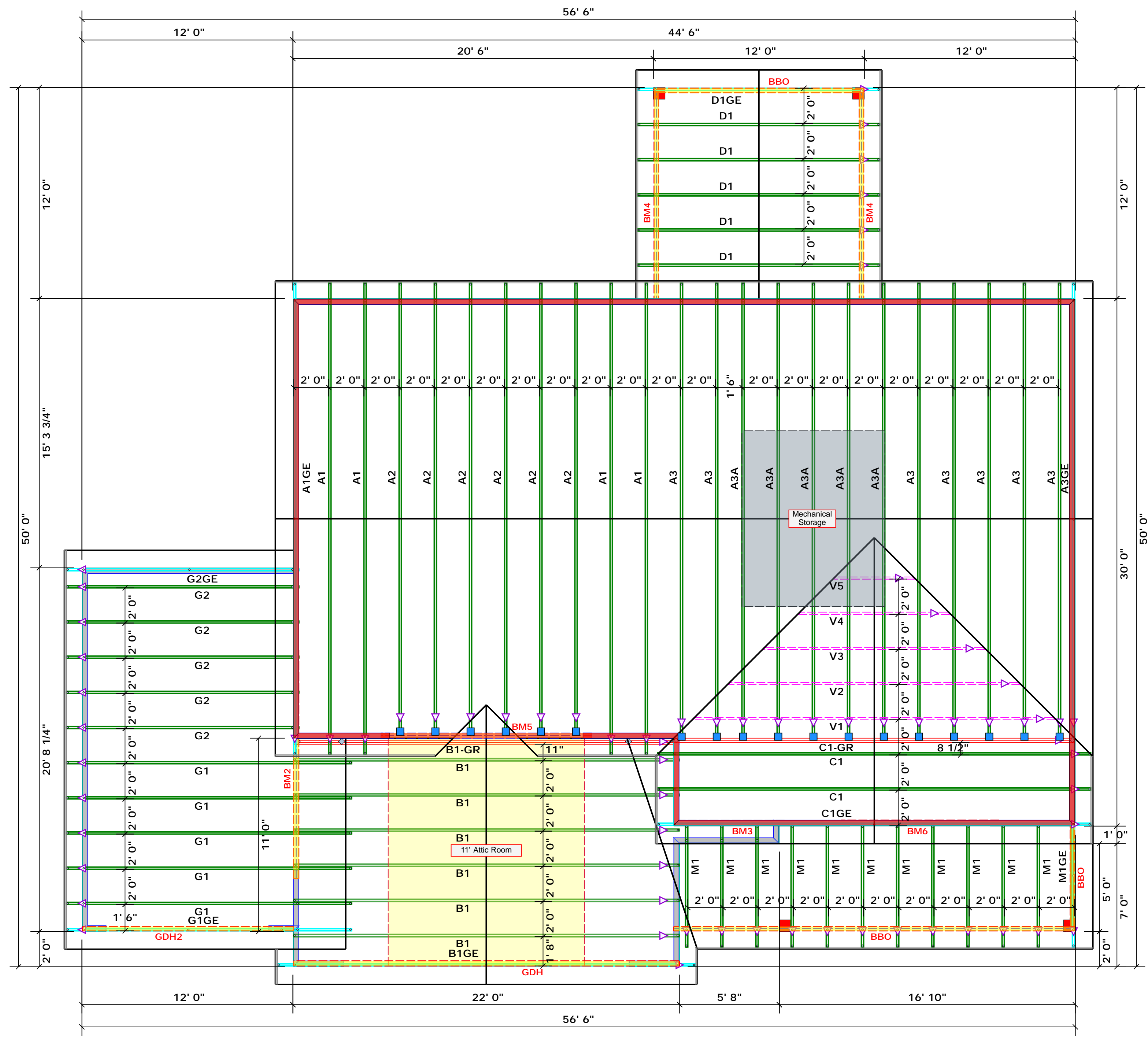
Signature _____
David Landry

LOAD CHART FOR JACK STUDS

(BASED ON TABLES ROEHLIC 6/13)

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADERS/STROPS

END REACTION (IP/T)	REQ'D STUDS FOR 12" BY 12" BEAM	END REACTION (IP/T)	REQ'D STUDS FOR 12" BY 12" BEAM
1700	1	2550	1
3400	2	5100	2
5100	3	7650	3
6800	4	10200	4
8500	5	12750	5
10200	6	15300	6
11900	7		
13600	8		
15300	9		



PlotID	Length	Product	Plies	Net Qty
BM5	12' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2

Hatch Legend

	Padded HVAC
	Second Floor Walls

Roof Area = 3050.02 sq.ft.
Ridge Line = 92.83 ft.
Hip Line = 0 ft.
Horiz. OH = 148.49 ft.
Raked OH = 216.1 ft.
Decking = 105 sheets

1 Truss Placement Plan
Scale: 1/4"=1'

- Dimension Notes**
- All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 - All interior wall dimensions are to face of frame wall unless noted otherwise
 - All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

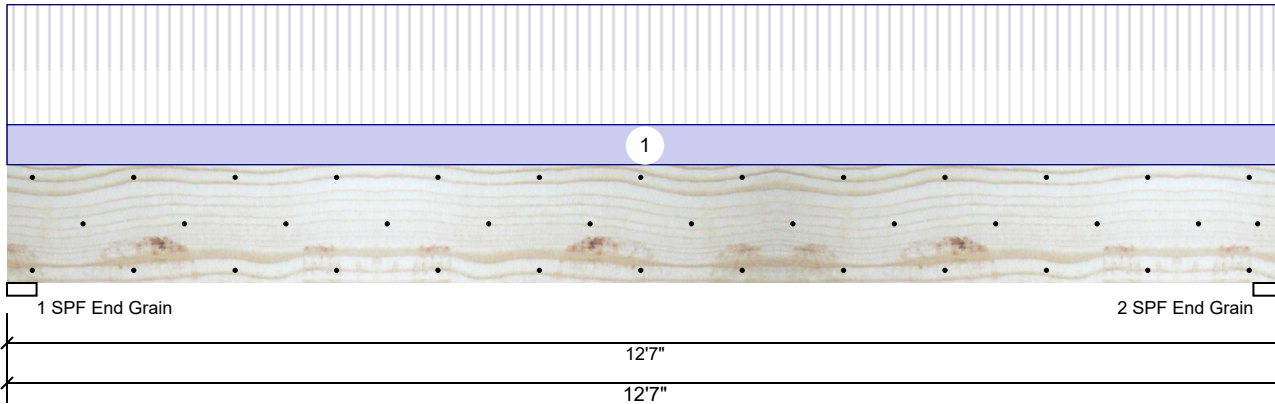
Connector Information				Nail Information		
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	18	Varies	16d/3-1/2"	16d/3-1/2"

BUILDER	Ben Stout Real Estate	COUNTY	Harnett
JOB NAME	Lot 13 Forest Ridge	ADDRESS	Tanna Place
PLAN	The Ashville	MODEL	Roof
SEAL DATE	8/15/18	DATE REV.	01/07/21
QUOTE #	Quote #	DRAWN BY	David Landry
JOB #	J1220-5668	SALESMAN	Marshall Naylor

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

BM1 Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	4568	1591	0	0	0
2	4568	1591	0	0	0

Bearings

Bearing	Length	Cap.	React D/L	Ib	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	58%	1591 / 4568	6159	L	D+L	
2 - SPF End Grain	3.500"	58%	1591 / 4568	6159	L	D+L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	17989 ft-lb	6'3 1/2"	26999 ft-lb	0.666 (67%)	D+L	L
Unbraced	17989 ft-lb	6'3 1/2"	18055 ft-lb	0.996 (100%)	D+L	L
Shear	4792 lb	11'2 1/4"	10453 lb	0.458 (46%)	D+L	L
LL Defl inch	0.252 (L/578)	6'3 1/2"	0.303 (L/480)	0.830 (83%)	L	L
TL Defl inch	0.340 (L/428)	6'3 1/2"	0.404 (L/360)	0.840 (84%)	D+L	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 5'4 1/2" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	242 PLF	726 PLF	0 PLF	0 PLF	0 PLF	F1
	Self Weight				11 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Manufacturer Info

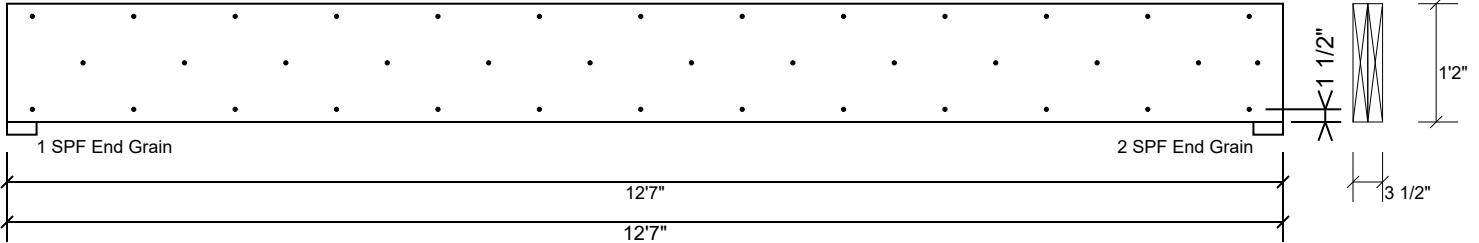
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 28314
 910-864-TRUS



BM1 Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

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Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Manufacturer Info

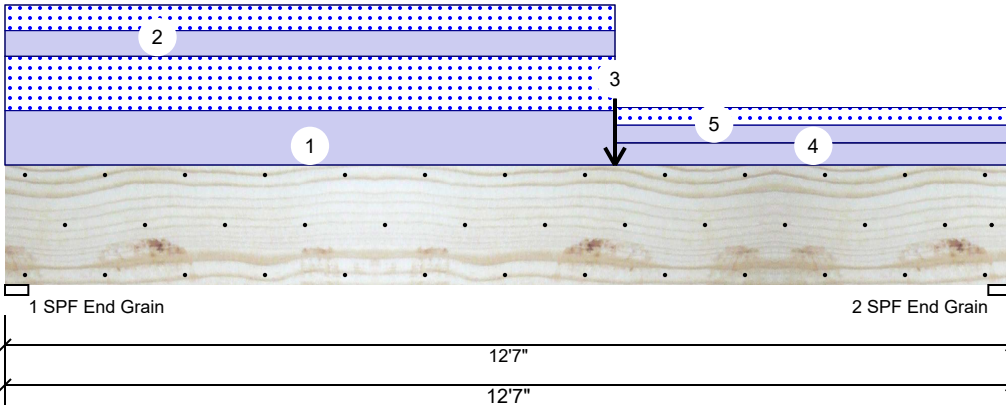
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BM2 Kerto-S LVL 1.750" X 18.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	5114	4887	0	0
2	0	5329	4635	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	94%	5114 / 4887	10001	L	D+S
2 - SPF End Grain	3.500"	93%	5329 / 4635	9964	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	42293 ft-lb	7'7 1/2"	49428 ft-lb	0.856 (86%)	D+S	L
Unbraced	42293 ft-lb	7'7 1/2"	42329 ft-lb	0.999 (100%)	D+S	L
Shear	9273 lb	10'10 3/8"	15456 lb	0.600 (60%)	D+S	L
LL Defl inch	0.179 (L/816)	6'8 1/8"	0.304 (L/480)	0.590 (59%)	S	L
TL Defl inch	0.368 (L/396)	6'8 5/16"	0.405 (L/360)	0.910 (91%)	D+S	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 2'6" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Part. Uniform	0-0-0 to 7-7-8		Top	367 PLF	0 PLF	367 PLF	0 PLF	0 PLF	B1
2	Part. Uniform	0-0-0 to 7-7-8		Top	173 PLF	0 PLF	173 PLF	0 PLF	0 PLF	G1
3	Point	7-7-8		Top	4815 lb	0 lb	4815 lb	0 lb	0 lb	B1-GR
4	Part. Uniform	7-7-8 to 12-7-0		Top	150 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Above
5	Part. Uniform	7-7-8 to 12-7-0		Top	119 PLF	0 PLF	119 PLF	0 PLF	0 PLF	G2
	Self Weight				14 PLF					

Notes

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Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Manufacturer Info

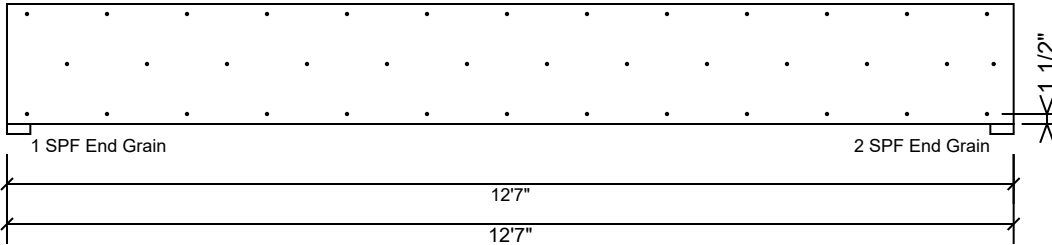
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BM2 Kerto-S LVL 1.750" X 18.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

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chemicals

Handling & Installation

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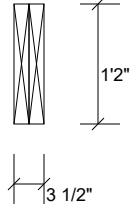
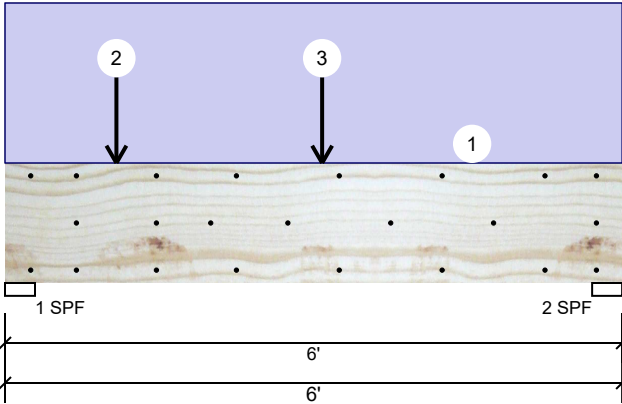
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BM3 Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	678	709	0	0	0
2	189	546	0	0	0

Bearings

Bearing	Length	Cap.	React D/L	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	27%	709 / 678	1387	L	D+L
2 - SPF	3.500"	14%	546 / 189	734	L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1305 ft-lb	2'8 7/16"	26999 ft-lb	0.048 (5%)	D+L	L
Unbraced	1305 ft-lb	2'8 7/16"	17623 ft-lb	0.074 (7%)	D+L	L
Shear	1162 lb	1'4 3/4"	10453 lb	0.111 (11%)	D+L	L
LL Defl inch (L/21799)	0.003	2'7 3/8"	0.139 (L/480)	0.020 (2%)	L	L
TL Defl inch (L/8727)	0.008	2'10 1/16"	0.185 (L/360)	0.040 (4%)	D+L	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	150 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Above
2	Point	1-1-0		Far Face	238 lb	714 lb	0 lb	0 lb	0 lb	F2A
3	Point	3-1-0		Far Face	51 lb	153 lb	0 lb	0 lb	0 lb	F7
	Self Weight				11 PLF					

Notes

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Lumber

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chemicals

Handling & Installation

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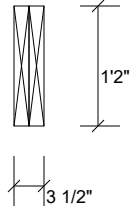
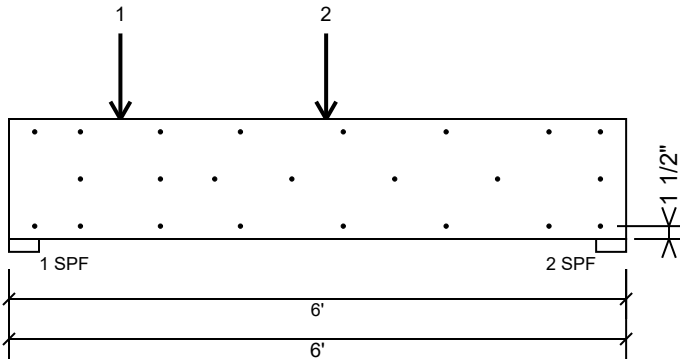
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Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

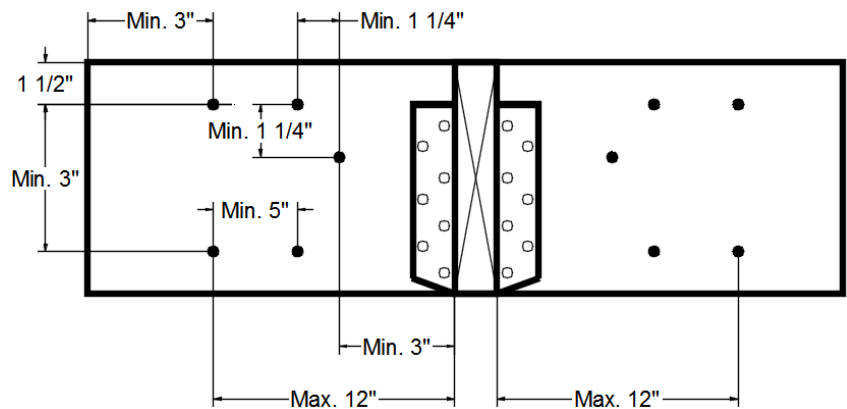
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 1-1-0 with a minimum of (6) – 10d Box nails (.128x3") in the pattern shown.

Capacity	96.9 %
Load	476.0lb.
Total Yield Limit	491.0 lb.
Cg	0.9998
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Load Combination	D+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads



Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
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 www.metsawood.com/us
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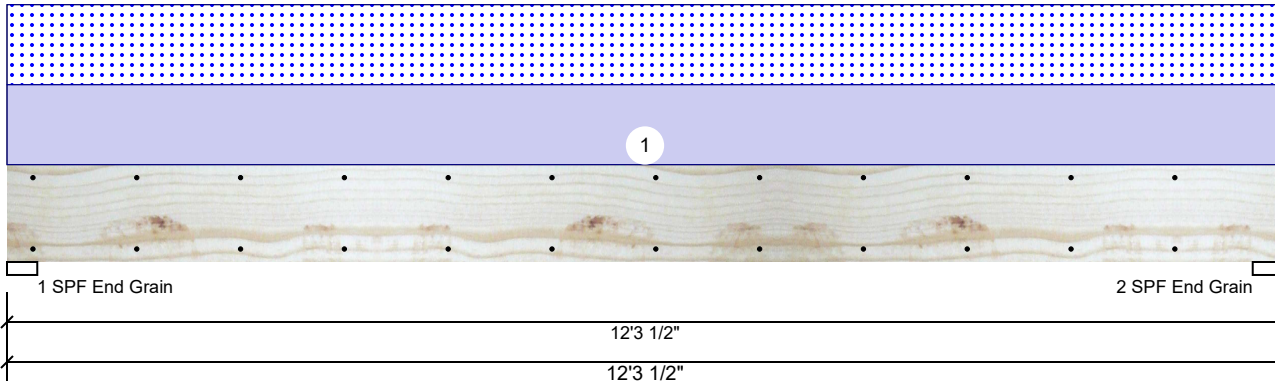
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This design is valid until 2/26/2023

BM4 S-P-F #2 2.000" X 12.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	799	799	0	0
2	0	799	799	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	36%	799 / 799	1598	L	D+S
2 - SPF End Grain	3.500"	36%	799 / 799	1598	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4551 ft-lb	6'1 3/4"	5306 ft-lb	0.858 (86%)	D+S	L
Unbraced	4551 ft-lb	6'1 3/4"	4558 ft-lb	0.998 (100%)	D+S	L
Shear	1295 lb	1'2"	3493 lb	0.371 (37%)	D+S	L
LL Defl inch	0.115 (L/1234)	6'1 3/4"	0.296 (L/480)	0.390 (39%)	S	L
TL Defl inch	0.230 (L/617)	6'1 3/4"	0.394 (L/360)	0.580 (58%)	D+S	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 6'1 1/2" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	130 PLF	0 PLF	130 PLF	0 PLF	0 PLF	D1

Manufacturer Info

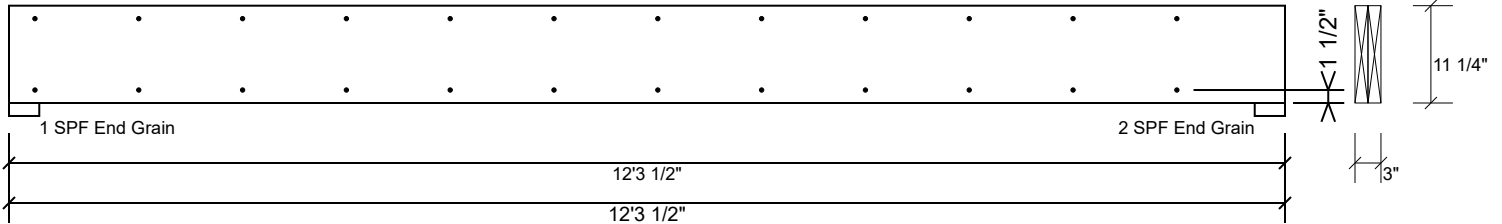
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This design is valid until 2/26/2023

BM4 S-P-F #2 2.000" X 12.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	157.4 PLF
Yield Limit per Fastener	78.7 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

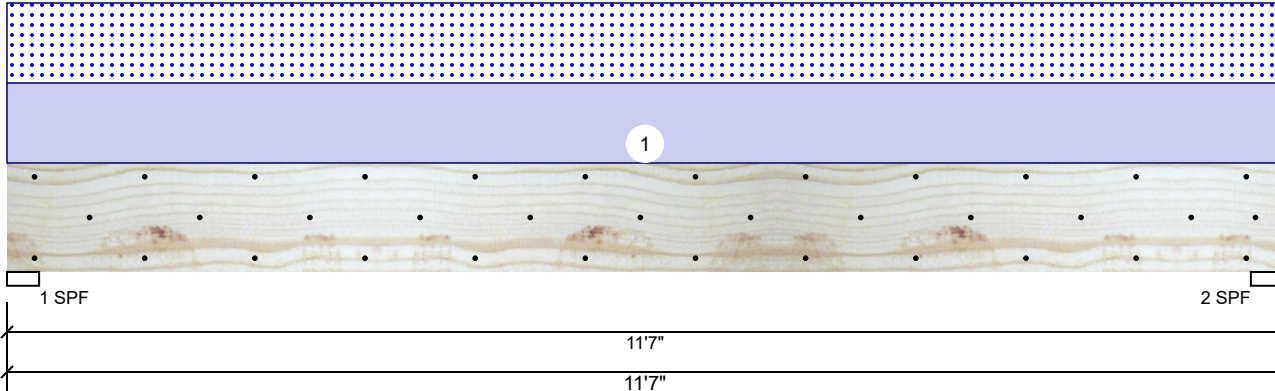
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This design is valid until 2/26/2023

BM5 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	1559	1506	0	0
2	0	1559	1506	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	59%	1559 / 1506	3065	L	D+S
2 - SPF	3.500"	59%	1559 / 1506	3065	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8188 ft-lb	5'9 1/2"	22897 ft-lb	0.358 (36%)	D+S	L
Unbraced	8188 ft-lb	5'9 1/2"	8589 ft-lb	0.953 (95%)	D+S	L
Shear	2935 lb	1'2 5/8"	10197 lb	0.288 (29%)	D+S	L
LL Defl inch	0.103 (L/1298)	5'9 1/2"	0.278 (L/480)	0.370 (37%)	S	L
TL Defl inch	0.209 (L/637)	5'9 1/2"	0.371 (L/360)	0.560 (56%)	D+S	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	260 PLF	0 PLF	260 PLF	0 PLF	0 PLF	A2
	Self Weight				9 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Manufacturer Info

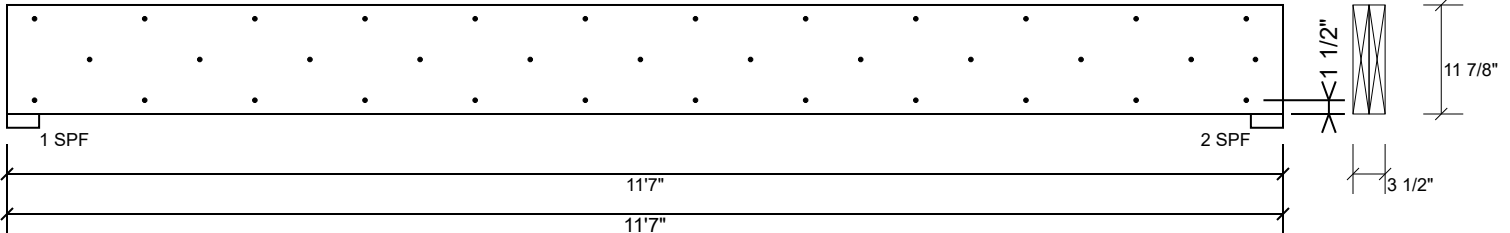
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BM5 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	92.1 %
Load	260.0 PLF
Yield Limit per Foot	282.4 PLF
Yield Limit per Fastener	94.1 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	D+S
Duration Factor	1.15

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Manufacturer Info

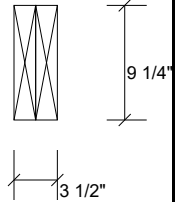
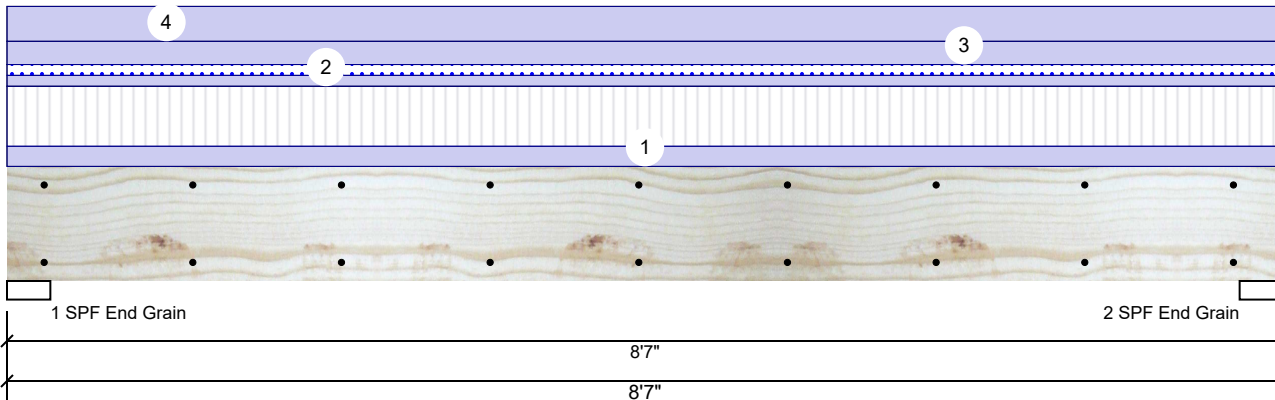
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BM6 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	1330	2005	240	0	0
2	1330	2005	240	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	31%	2005 / 1330	3335	L	D+L
2 - SPF End Grain	3.500"	31%	2005 / 1330	3335	L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6413 ft-lb	4'3 1/2"	12542 ft-lb	0.511 (51%)	D+L	L
Unbraced	6413 ft-lb	4'3 1/2"	8468 ft-lb	0.757 (76%)	D+L	L
Shear	2558 lb	1'	6907 lb	0.370 (37%)	D+L	L
LL Defl inch	0.075 (L/1301)	4'3 9/16"	0.203 (L/480)	0.370 (37%)	L	L
TL Defl inch	0.188 (L/519)	4'3 9/16"	0.271 (L/360)	0.690 (69%)	D+L	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	104 PLF	310 PLF	0 PLF	0 PLF	0 PLF	F1
2	Uniform			Top	56 PLF	0 PLF	56 PLF	0 PLF	0 PLF	M1
3	Uniform			Top	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Above
4	Uniform			Top	180 PLF	0 PLF	0 PLF	0 PLF	0 PLF	C1GE
	Self Weight				7 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Manufacturer Info

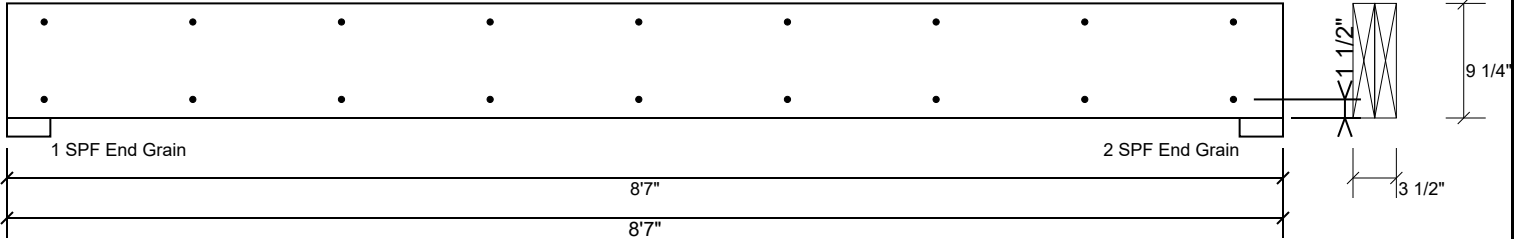
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BM6 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Manufacturer Info

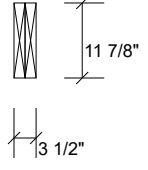
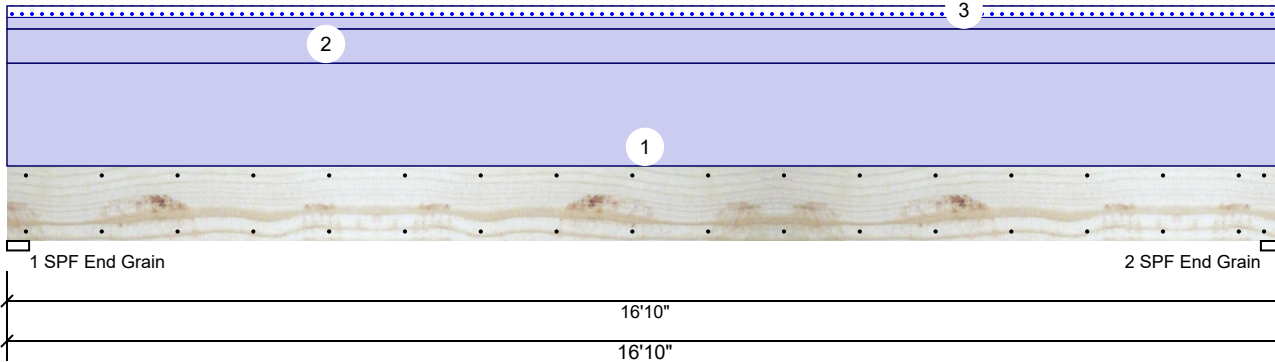
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GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	2266	168	0	0
2	0	2266	168	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	23%	2266 / 168	2434	L	D+S
2 - SPF End Grain	3.500"	23%	2266 / 168	2434	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9024 ft-lb	8'5"	17919 ft-lb	0.504 (50%)	D	Uniform
Unbraced	9694 ft-lb	8'5"	9704 ft-lb	0.999 (100%)	D+S	L
Shear	1938 lb	15'7 3/8"	7980 lb	0.243 (24%)	D	Uniform
LL Defl inch	0.035 (L/5617)	8'5 1/16"	0.409 (L/480)	0.090 (9%)	S	L
TL Defl inch	0.506 (L/388)	8'5 1/16"	0.546 (L/360)	0.930 (93%)	D+S	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 9'6 3/4" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	180 PLF	0 PLF	0 PLF	0 PLF	0 PLF	B1GE
2	Uniform			Top	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Above
3	Tie-In	0-0-0 to 16-10-0	1-0-0	Top	20 PSF	0 PSF	20 PSF	0 PSF	0 PSF	Roof Load
	Self Weight				9 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Manufacturer Info

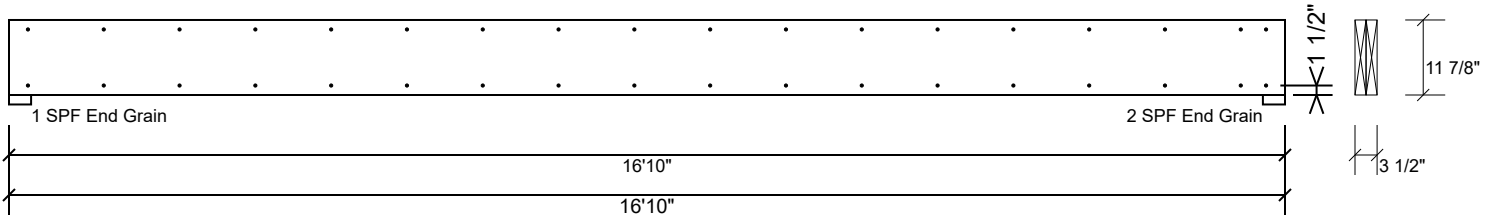
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GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Manufacturer Info

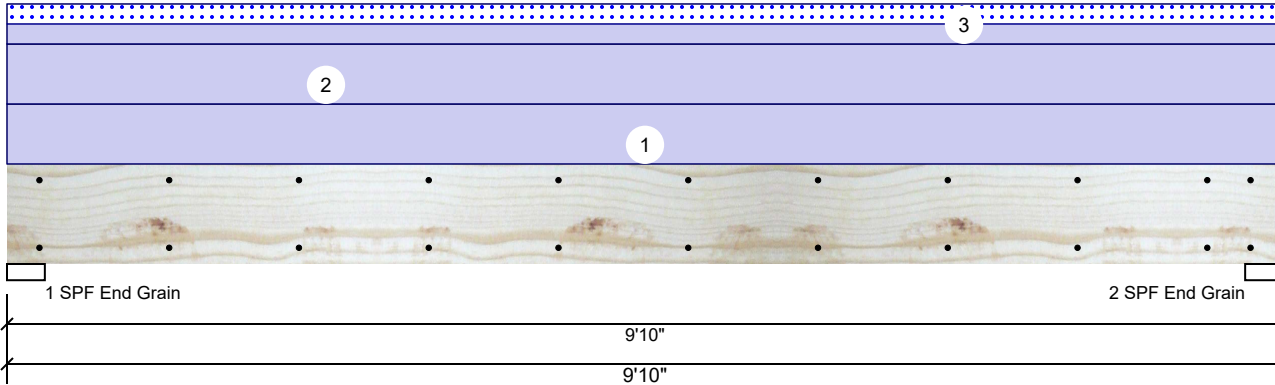
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GDH2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	688	98	0	0
2	0	688	98	0	0

Bearings

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	18%	688 / 98	787	L	D+S
2 - SPF End Grain	3.500"	18%	688 / 98	787	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1538 ft-lb	4'11"	3088 ft-lb	0.498 (50%)	D	Uniform
Unbraced	1758 ft-lb	4'11"	3017 ft-lb	0.583 (58%)	D+S	L
Shear	548 lb	8'10"	2248 lb	0.244 (24%)	D	Uniform
LL Defl inch	0.013 (L/8965)	4'11"	0.234 (L/480)	0.050 (5%)	S	L
TL Defl inch	0.100 (L/1121)	4'11"	0.312 (L/360)	0.320 (32%)	D+S	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Above
2	Uniform			Top	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	G1GE
3	Tie-In	0-0-0 to 9-10-0	1-0-0	Top	20 PSF	0 PSF	20 PSF	0 PSF	0 PSF	Roof Load

Manufacturer Info

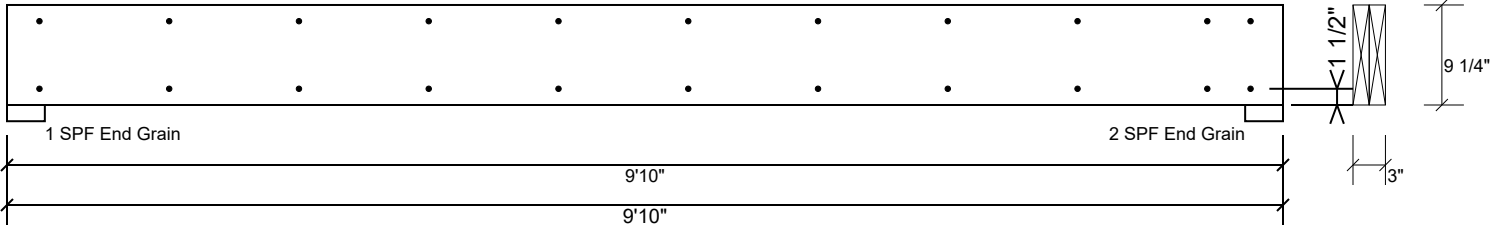
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 Fayetteville, NC
 USA
 28314
 910-864-TRUS



This design is valid until 2/26/2023

GDH2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	157.4 PLF
Yield Limit per Fastener	78.7 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS

This design is valid until 2/26/2023

Reaction Summary of Order



ROOF & FLOOR
TRUSSES & BEAMS

Reilly Road Industrial Park P.O. Box 40408
Fayetteville, N.C. 28309 (910) 864-TRUS

REQ. QUOTE DATE	04/12/19	ORDER #	J1220-5668
ORDER DATE	12/04/20	QUOTE #	
DELIVERY DATE	/ /	CUSTOMER ACCT #	0000007060
DATE OF INVOICE	/ /	CUSTOMER PO #	
ORDERED BY	Ben Stout	INVOICE #	
COUNTY	Harnett	TERMS	
SUPERINTENDANT	Ben Stout	SALES REP	Marshall Naylor
JOBSITE PHONE #	(910) 476-4502	SALES AREA	David Landry

SOLD TO	Benjamin Stout Real Estate PO Box 53798 Fayetteville, NC 28305 (910) 476-4502	JOB NAME: Lot 13 Forest Ridge	LOT # 13	SUBDIV: Forest Ridge
	Ben Stout Real Estate Tanna Place Cameron, NC	MODEL: Roof	TAG: The Ashville 3Car	JOB CATEGORY: Residential - Roof
DELIVERY INSTRUCTIONS:				
SPECIAL INSTRUCTIONS:				
PLAN SEAL DATE: 8/15/18				

BUILDING DEPARTMENT	OVERHANG INFO	HEEL HEIGHT	00-04-05	REQ. LAYOUTS	REQ. ENGINEERING	QUOTE	BY	DATE
Roof Order	END CUT	RETURN				LAYOUT		/ /
	PLUMB	GABLE STUDS	16 IN. OC	JOBSITE	1	CUTTING		/ /

ROOF TRUSSES

LOADING INFORMATION

TCLL-TCDL-BCLL-BCDL	STRESS INCR.
20.0,10.0,0.0,10.0	1.15

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	PITCH		TYPE ID	BASE O/A	LUMBER		OVERHANG		REACTIONS					
		PLY	TOP			BOT	TOP	BOT	LEFT	RIGHT					
	4		12.00	0.00	COMMON A1	24-11-08 24-11-08	2 X 6	2 X 6	00-11-00	00-11-00	Joint 2 1076.6 lbs. -41.5 lbs.	Joint 8 1076.6 lbs. -41.5 lbs.			
	1		12.00	0.00	COMMON A1GE	24-11-08 24-11-08	2 X 6	2 X 6	00-11-00	00-11-00	Joint 2 372.7 lbs. -153.7 lbs.	Joint 16 327.5 lbs. -86.7 lbs.	Joint 18 254.2 lbs. -241.6 lbs.	Joint 19 177.2 lbs. -127.7 lbs.	Joint 20 192.4 lbs. -140.7 lbs.
	6		12.00	0.00	COMMON A2	24-08-00 24-08-00	2 X 6	2 X 6		00-11-00	Joint 1 1031.4 lbs. -36.1 lbs.	Joint 8 1071.2 lbs. -41.3 lbs.			
	7		12.00	0.00	COMMON A3	24-11-08 24-11-08	2 X 6	2 X 6		00-11-00	Joint 1 1036.2 lbs. -35.0 lbs.	Joint 7 1080.7 lbs. -41.6 lbs.			
	5		12.00	0.00	COMMON A3A	24-11-08 24-11-08	2 X 6	2 X 6		00-11-00	Joint 1 1292.2 lbs. -35.0 lbs.	Joint 9 1338.5 lbs. -41.6 lbs.			
	1		12.00	0.00	COMMON A3GE	24-11-08 24-11-08	2 X 6	2 X 6		00-11-00	Joint 1 399.7 lbs. -182.0 lbs.	Joint 15 327.3 lbs. -86.8 lbs.	Joint 17 254.1 lbs. -241.5 lbs.	Joint 18 177.2 lbs. -127.7 lbs.	Joint 19 192.4 lbs. -140.7 lbs.
	6		12.00	0.00	ATTIC B1	21-11-00 21-11-00	2 X 8	2 X 10			Joint 1 1422.9 lbs. 163.2 lbs.	Joint 2 537.9 lbs. -65.7 lbs.	Joint 7 1314.8 lbs. -65.7 lbs.		
	1	3 Ply	12.00	0.00	ATTIC B1-GR	21-11-00 21-11-00	1.5 X 2	2 X 10			Joint 1 9588.9 lbs. 162.3 lbs.	Joint 7 9573.9 lbs. 162.3 lbs.			
	1		12.00	0.00	GABLE B1GE	21-11-00 21-11-00	2 X 8	2 X 10	00-11-00	00-11-00	Joint 2 1442.6 lbs. 40.1 lbs.	Joint 14 1442.6 lbs. 40.1 lbs.			
	2		12.00	0.00	COMMON C1	22-09-08 22-09-08	2 X 6	2 X 6	00-11-00	00-11-00	Joint 2 1066.1 lbs. -38.7 lbs.	Joint 8 1066.1 lbs. -38.7 lbs.			

Reaction Summary of Order



ROOF & FLOOR
TRUSSES & BEAMS

Reilly Road Industrial Park P.O. Box 40408
Fayetteville, N.C. 28309 (910) 864-TRUS

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ORDERED BY	Ben Stout	INVOICE #	
COUNTY	Harnett	TERMS	
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JOBSITE PHONE #	(910) 476-4502	SALES AREA	David Landry

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	Ben Stout Real Estate Tanna Place Cameron, NC	MODEL: Roof	TAG: The Ashville 3Car	JOB CATEGORY: Residential - Roof
DELIVERY INSTRUCTIONS:				
SPECIAL INSTRUCTIONS:				
PLAN SEAL DATE: 8/15/18				

BUILDING DEPARTMENT	OVERHANG INFO	HEEL HEIGHT	00-04-05	REQ. LAYOUTS	REQ. ENGINEERING	QUOTE	BY	DATE
Roof Order	END CUT	RETURN				LAYOUT		/ /
	PLUMB	GABLE STUDS	16 IN. OC	JOBSITE	1	CUTTING		/ /

ROOF TRUSSES

LOADING INFORMATION

TCLL-TCDL-BCLL-BCDL	STRESS INCR.
20.0,10.0,0.0,10.0	1.15

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	PITCH		TYPE ID	BASE O/A	LUMBER		OVERHANG		REACTIONS				
		PLY	TOP			BOT	TOP	BOT	LEFT		RIGHT			
	1 2 Ply	12.00	0.00	COMMON C1-GR	22-09-08 22-09-08	2 X 6	2 X 8			Joint 1 6947.2 lbs. -302.5 lbs.	Joint 7 7765.4 lbs. -319.9 lbs.			
	1	12.00	0.00	COMMON C1GE	22-09-08 22-09-08	2 X 6	2 X 6	00-11-00	00-11-00	Joint 2 407.2 lbs. -190.6 lbs.	Joint 16 361.6 lbs. -123.0 lbs.	Joint 18 188.1 lbs. -220.6 lbs.	Joint 19 195.9 lbs. -144.1 lbs.	Joint 20 188.9 lbs. -137.8 lbs.
	5	6.00	0.00	COMMON D1	11-11-00 11-11-00	2 X 6	2 X 6	00-11-00	00-11-00	Joint 2 517.2 lbs. -105.9 lbs.	Joint 4 517.2 lbs. -105.9 lbs.			
	1	6.00	0.00	GABLE D1GE	11-11-00 11-11-00	2 X 6	2 X 6	00-11-00	00-11-00	Joint 2 517.2 lbs. -137.1 lbs.	Joint 8 517.2 lbs. -137.1 lbs.			
	5	3.00	0.00	MONOPITCH G1	12-03-08 12-03-08	2 X 6	2 X 6	00-11-00	03-00-00	Joint 2 498.5 lbs. -53.2 lbs.	Joint 7 692.3 lbs. -119.2 lbs.			
	1	3.00	0.00	MONOPITCH G1GE	12-03-08 12-03-08	2 X 6	2 X 6	00-11-00	03-00-00	Joint 2 169.3 lbs. -28.2 lbs.	Joint 10 412.7 lbs. -219.1 lbs.	Joint 11 93.5 lbs. -6.4 lbs.	Joint 12 200.0 lbs. -69.3 lbs.	Joint 13 85.5 lbs. -29.6 lbs.
	5	3.00	0.00	MONOPITCH G2	12-00-00 12-00-00	2 X 6	2 X 6	00-11-00	00-03-08	Joint 2 514.1 lbs. -64.0 lbs.	Joint 6 474.6 lbs. -62.5 lbs.			
	1	3.00	0.00	GABLE G2GE	12-00-00 12-00-00	2 X 6	2 X 6	00-11-00	00-03-08	Joint 2 163.1 lbs. -35.7 lbs.	Joint 9 79.9 lbs. -28.5 lbs.	Joint 10 170.5 lbs. -49.7 lbs.	Joint 11 170.2 lbs. -55.8 lbs.	Joint 12 102.4 lbs. -35.0 lbs.
	11	4.00	0.00	MONOPITCH M1	06-00-00 06-00-00	2 X 6	2 X 6	00-11-00		Joint 2 274.1 lbs. -104.2 lbs.	Joint 4 222.8 lbs. -97.4 lbs.			
	1	4.00	0.00	GABLE M1GE	06-00-00 06-00-00	2 X 6	2 X 6	00-11-00		Joint 2 274.1 lbs. -150.6 lbs.	Joint 6 222.8 lbs. -142.1 lbs.			

Reaction Summary of Order



ROOF & FLOOR
TRUSSES & BEAMS

Reilly Road Industrial Park P.O. Box 40408
Fayetteville, N.C. 28309 (910) 864-TRUS

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	Ben Stout Real Estate Tanna Place Cameron, NC	SPECIAL INSTRUCTIONS:	PLAN SEAL DATE: 8/15/18

BUILDING DEPARTMENT Roof Order	OVERHANG INFO	HEEL HEIGHT	00-04-05	REQ. LAYOUTS	REQ. ENGINEERING	QUOTE	BY	DATE
	END CUT	RETURN				LAYOUT		/ /
	PLUMB		GABLE STUDS	16 IN. OC	JOBSITE	1	CUTTING	/ /

ROOF TRUSSES

LOADING INFORMATION

TCLL-TCDL-BCLL-BCDL	STRESS INCR.
20.0,10.0,0.0,10.0	1.15

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	PITCH		TYPE ID	BASE O/A	LUMBER		OVERHANG		REACTIONS				
		TOP	BOT			TOP	BOT	LEFT	RIGHT	Joint 1	Joint 7	Joint 8	Joint 9	Joint 11
	1	12.00	0.00	VALLEY V1	20-07-04 20-07-04	2 X 4	2 X 4			Joint 1 186.2 lbs. -104.2 lbs.	Joint 7 161.5 lbs. -65.2 lbs.	Joint 8 291.5 lbs. -136.0 lbs.	Joint 9 488.8 lbs. -184.1 lbs.	Joint 11 440.4 lbs. 71.9 lbs.
	1	12.00	0.00	VALLEY V2	16-07-04 16-07-04	2 X 4	2 X 4			Joint 1 188.9 lbs. -26.0 lbs.	Joint 5 165.3 lbs. 5.4 lbs.	Joint 6 518.7 lbs. -199.1 lbs.	Joint 8 416.6 lbs. 61.7 lbs.	Joint 9 518.9 lbs. -199.3 lbs.
	1	12.00	0.00	VALLEY V3	12-07-04 12-07-04	2 X 4	2 X 4			Joint 1 119.2 lbs. -50.5 lbs.	Joint 5 101.5 lbs. -27.0 lbs.	Joint 6 340.9 lbs. -160.7 lbs.	Joint 7 223.5 lbs. 56.2 lbs.	Joint 8 341.2 lbs. -160.8 lbs.
	1	12.00	0.00	VALLEY V4	08-07-04 08-07-04	2 X 4	2 X 4			Joint 1 192.4 lbs. -34.3 lbs.	Joint 3 192.4 lbs. -34.3 lbs.	Joint 4 247.2 lbs. 32.6 lbs.		
	1	12.00	0.00	VALLEY V5	04-07-04 04-07-04	2 X 4	2 X 4			Joint 1 95.0 lbs. -16.9 lbs.	Joint 3 95.0 lbs. -16.9 lbs.	Joint 4 122.0 lbs. 16.1 lbs.		

ITEMS

QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES
18	Hangers, USP	HUS 26			SIMPSON (HUS26)
2	LVL Beams (Sized)	LVL, 1-3/4" x 11-7/8" (S)	12-00-00		BM5



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

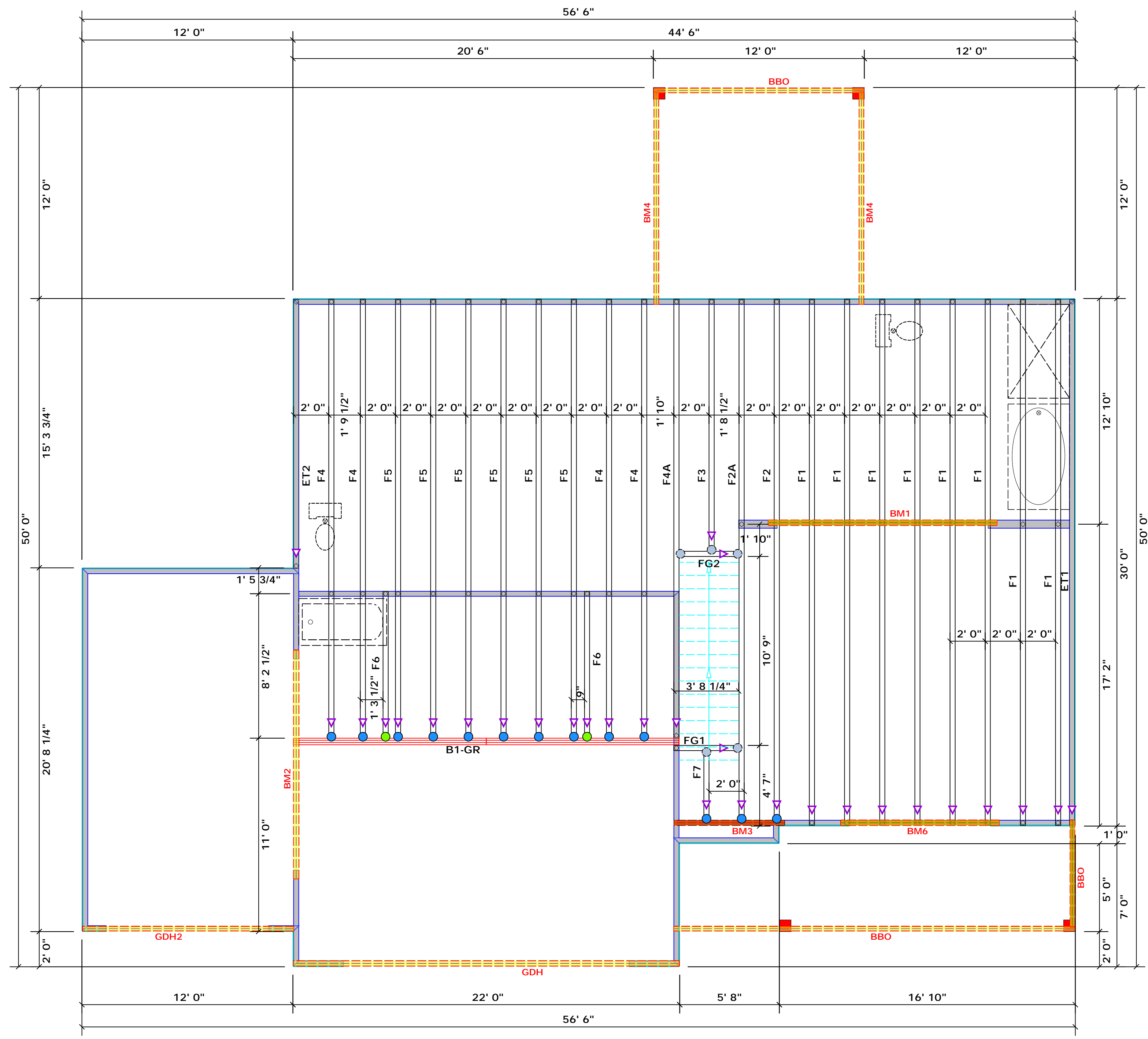
Signature _____
David Landry

LOAD CHART FOR JACK STUDS

(BASED ON TABLES ROEHLIC 6 (3))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADERS/STROBES

END REACTION (IP TO)	REQ'D STUDS FOR EACH END OF HEADERS/STROBES	END REACTION (IP TO)	REQ'D STUDS FOR EACH END OF HEADERS/STROBES
1700	1	2550	1
3400	2	5100	2
5100	3	7650	3
6800	4	10200	4
8500	5	12750	5
10200	6	15300	6
11900	7		
13600	8		
15300	9		



Products				
PlotID	Length	Product	Plies	Net Qty
BM1	13' 0"	1-3/4"x 14" LVL Kerto-S	2	2
BM2	13' 0"	1-3/4"x 18" LVL Kerto-S	2	2
BM3	7' 0"	1-3/4"x 14" LVL Kerto-S	2	2
BM4	14' 0"	2x12 SPF No.2	2	4
BM6	9' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
GDH	22' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
GDH2	12' 0"	2x10 SPF No.2	2	2

1 Truss Placement Plan
Scale: 1/4"=1'

Dimension Notes

- All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
- All interior wall dimensions are to face of frame wall unless noted otherwise
- All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Connector Information				Nail Information	
Sym	Product	Manuf	Qty	Supported Member	Header / Truss
●	HUS410	USP	13	Varies	16d/3-1/2" / 16d/3-1/2"
○	MSH422	USP	5	Varies	10d/3" / 10d/3"
●	THD410	USP	2	Varies	16d/3-1/2" / 10d/3"

Plumbing Drop Notes

- Plumbing drop locations shown are NOT exact.
- Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
- Adjust spacing as needed not to exceed 24"oc.

COUNTY	Harnett
ADDRESS	Tanna Place
MODEL	Roof
DATE REV.	01/07/21
DRAWN BY	David Landry
SALESMAN	Marshall Naylor
BUILDER	Ben Stout Real Estate
JOB NAME	Lot 13 Forest Ridge
PLAN	The Ashville
SEAL DATE	8/15/18
QUOTE #	Quote #
JOB #	J1220-5668

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com