FRONT ELEVATION WITH SIDE LOAD GARAGE

SCALE 1/8" = 1'-0"

PLANS DESIGNED TO THE 2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE

MEAN ROOF HEIGHT 25'-8"					
ZONE 3A	ZONE 4A	ZONE 5A			
0.35	0.35	0.35			
0.55	0.55	0.55			
0.30	0.30	0.30			
38 or 30cl	38 or 30cl	38 or 30d			
15	15	19			
19	19	30			
5/13	10/15	10/15			
0	10	10			
5/13	10/15	10/19			
	ZONE 3A 0,35 0,55 0,30 38 or 30cl 15 19 5/13	ZONE 3A ZONE 4A 0.35 0.35 0.55 0.55 0.30 0.30 38 or 30d 38 or 30d 15 15 19 19 5/13 10/15 0 10			

"10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION ** INSULATION DEPTH WITH MONOLITHIC SLAB 24° OR FROM INSPECTION GAP TO BOTTOM OF FOOTING; INSULATION DEPTH WITH STEM WALL SLAB 24° OR TO BOTTOM OF FOUNDATION WALL

COMPONENT								
MEAN ROOF		O 30'		TO 35'				
ZONE 1	14.2	-15.0	14.9	-15.8	15.5	-16.4	15.9	-16.8
ZONE 2	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	-21.8	17.4	-22.4
DESIGNED FOR WIN	O SPEED	OF 130 MF	H, 3 SEC	OND CLIST	(101 FAS	TEST MULE) EXPOS	RE "8"
COMPONENT	& CL/	DDING	DESIG	NED FO	R THE	FOLLO	WING	LOADS
MEAN ROOF	UP 1	O 30°	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45
ZONE 1	16.7	-18.0	17.5	-18.9	18.2	-19.6	18.7	-20.2
ZONE 2	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 3	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 4	18.2	-19.0	19.1	-20.0	19.8	-20.7	20.4	-21.3
ZONE 5				-25.2	19.8		20.4	

AIR LEAKAGE

Section N1102.4

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Development Company,

N1102 A 1 Building thermal envelope. The building thermal envelope shall be durably sealed with an air barrier system to limit Infiltration. The sealing methods between dissimilar materials shall allow for differential expansion and contraction. For all homes, where present, the following shall be caulked, gasketed, weathe stripped or otherwise sealed with an air barrier material or solid

material consistent with Appendix E-2.4 of this code:

1. Blocking and sealing floor/ceiling systems and under knee walls open to unconditioned or exterior space. Capping and sealing shafts or chases, including flue shafts.

3. Capping and sealing soffit or dropped ceiling areas

ROOF VENTILATION

SECTION RADA

SECTION ROBUS

R805.1 Ventilation required. Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings. protected against the entrance of rain or snow. Ventilation openings shall have a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Ventilation openings having a least dimension larger than Intily inaction. Vertilation operating a least dimension larger than 1/4 linch (6.4 mm) shall be provided with corrosion-resistant wire cloth screening, herdware cloth, or similar material with openings having a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Openings in roof framing members shall conform to the requirements of Section R802.7.

R808.2 Minimum area. The total net free ventilating area shall not be less

than 1/150 of the area of the space ventilated except that reduction of the total area to 1/300 is permitted provided that at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above the eave or comice vents with the balance of the required ventilation provided by eave or cornice vents. As an alternative, the net free cross-ventilation area may be reduced to 1/300 when a Class I or II vapor retarder is installed on the warm-in-winter side of the ceiling.

Exceptions:
1. Enclosed attic/rafter spaces requiring less than 1 square foot (0.0929 m2) of ventilation may be vented with continuous soffit ventilation only.

2. Enclosed attic/rafter spaces over unconditioned space may be vented with continuous soffit vent only.

SQUARE FOOTAGE OF ROOF TO BE VENTED = 1558 SQ.FT. NET FREE CROSS VENTILATION NEEDED:

WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 10.39 SQ.FT. WITH 50% TO 80% OF VENTING 3'-0" AROVE FAVE: OR WITH O ASS 1 OR 11 VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 5.16 SQ.FT

GUARD RAIL NOTES

SECTION R312

R312.1 Where required. Guards shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or grade below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect

R312.2 Height. Required guards at open-sided walking surfaces, including stalls, porches, balconies or landings, shall be not less than 36 inches (914). mm) high measured vertically above the adjacent welking surface, adjacent fixed seating or the line connecting the leading edges of the treads. Exceptions:

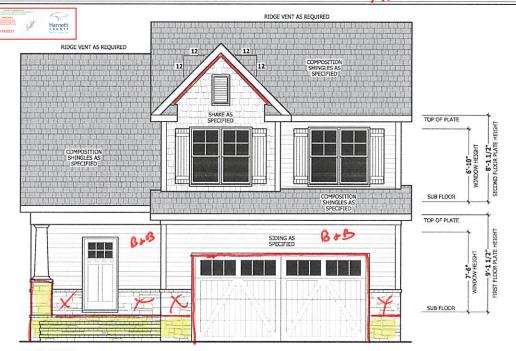
1. Guards on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the

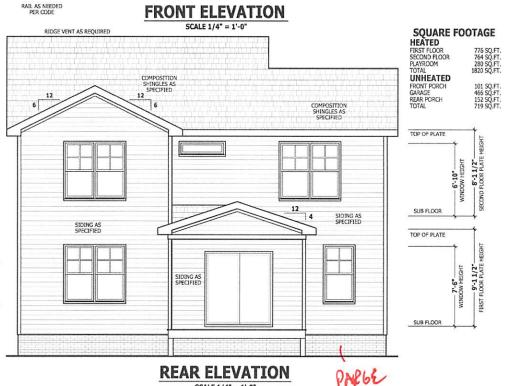
2. Where the top of the guard also serves as a handrail on the open sides of stairs, the top of the guard shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

R312.3 Opening limitations, Required guards shall not have openings from the walking surface to the required guard height which allow passage of a sphere 4 inches (102 mm)in diameter

1. The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153 and oction rail of a guard, stail not allow passage or a sphere of incres (mm) in diameter.

2. Guards on the open sides of stairs shall not have openings which allow passage of a sphere 4 3/8 inches (111 mm) in diameter.





HAYNES HOME PLANS, INC. ASSUMES NO LEABELITY FOR ENTRACTORS PRACTICES AN PROCEDURES.

CODES AND CONDITIONS MAY MAY WITH LOCATION. A LOCAL DESIGNER, ARCHITECT OR GINEER SHOULD BE CONSULTE BEFORE CONSTRUCTION.

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& REAR ELEVATIONS GASTON H FRONT (

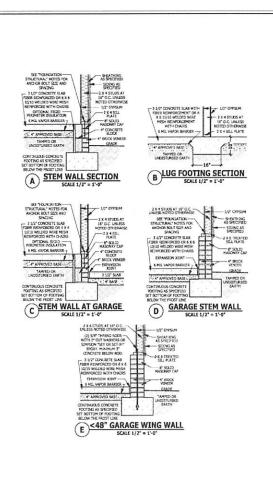


SQUARE FOOTAGE HEATED FIRST FLOOR 776 SQ.FT. 776 SQ FT 784 SQ FT 280 SQ FT 1820 SQ FT TOTAL UNHEATED FRONT PORCH GAPAGE REAR POPCH 101 SQF 466 SQF 157 SQF 719 SQF

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SCALE 1/4" = 1'-0"



FOUNDATION STRUCTURAL

115 to 130 mph wind zone (1 1/2 to 2 1/2 story)
CONTINUOUS FOOTING: 16" wide and 8" thick minimum. 20" wide minimum at brick veneer. Must extended 2" to either side of supported wall

GIRDERS: (3) 2 x 10 girder unless noted otherwise.

PIERS: 16" X 16" plers with 8" solid masonry cap on 30" X 30" X 10" concrete footing with maximum pier height of 64* with hollow masonry and 160" with solid masonry.

POINT LOADS: designates significant point load and should have solid

blocking to pier, girder or foundation wall.

115 and 120 MPH ANCHORS BOLTS: 1/2" diameter anchor bolts embedded minimum 7", maximum 6"-0" on center, within 12" of plate ends, and minimum two anchor bolts per plate.

minimum two anchor boxs per plate.

130 MPH ANCHORS BOLTS: 1/2" diameter anchor bolts embedded minimum

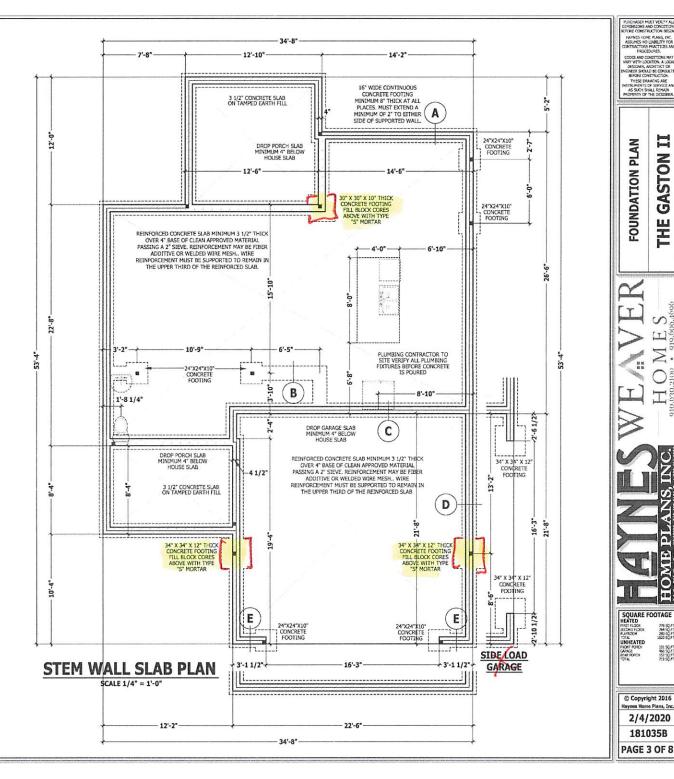
15", maximum 4"-0" on center, within 12" of plate ends, and minimum two

anchor botts per plate.

CONCRETE: Concrete shall have a minimum 28 day strength of 3000 psi and a maximum 5' slump. Air entrained per table 402.2. All concrete shall be in accordance with ACI standards. All samples for pumping shall be taken

from the exit end of the pump.

SOILS: Allowable soil bearing pressure assumed to be 2000 PSF. The contractor must contact a geotechnical engineer and a structural engineer if unsatisfactory subsurface conditions are encountered. The surface area adjacent to the foundation wall shall be provided with adequate drainage, and shall be graded so as to drain surface water away from foundation walls.



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776 SQ.FT. 764 SQ.FT. 280 SQ.FT. 1820 SQ.FT.

101 SQ.FT 466 SQ.FT 157 SQ.FT 719 SQ.FT

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DESIGNER, ARCHITECT OR INEER SHOULD BE CONSULT BEFORE CONSTRUCTION. THESE DRAWING ARE ITRUMENTS OF SERVICE AN

AS SUCH SHALL REMAIN ROPERTY OF THE DESIGNER

FIRST FLOOR PLAN

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SQUARE FOOTAGE HEATED 775 SQ F 784 SQ F 280 SQ F 1820 SQ F

UNHEATED FRONT PORCH GAPAGE REAR PORCH 101 SQ FT 466 SQ FT 157 SQ FT 719 SQ FT

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STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code.

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconles and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		
Guardrail in-fill components	50		
Passenger vehide garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40	-	L/360

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x10⁴ PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x10⁶ PSI Laminated strand lumber (LSL) Fib=2300 PSI, Fy=400 PSI, E=1.55x106 PSI
Laminated strand lumber (LSL) Fib=2350 PSI, Fy=400 PSI, E=1.55x106 PSI
Linstall all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be

prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" strel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center toist spacing, minimum 5/8" thick for 19.2" on center toist spacing, and minimum 3/4" thick for 24" on center loist spacing. ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick. CONCRETE AND SOILS: See foundation notes.

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Havnes Home Plan. Inc. attention before construction begins. KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Havnes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to upilft or bearing shall meet the requirements as specified on the truss schematics. BEARING. All trusses shalf be designed for bearing on SPF #2 plates or ledgers unless noted atherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

EXTERIOR HEADERS

(2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16' KING STUD(S) 1 2 3 5 6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE NON LOAD BEARING HEADERS TO BE LADDER FRAMED

BRACE WALL PANEL NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless

GYPSUM: All interior sides of exterior walls and both sides Interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method

GB to be fastened per table R602.10.1.
REQUIRED LENGTH OF BRACING: Required brace wall length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length.

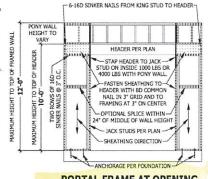
HD: 800 lbs hold down hold down device fastened to the edge of the brace wall panel closets to the corner.

Methods Per Table R602.10.1

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d/2 1/2" iong x 0.113" diameter). CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing

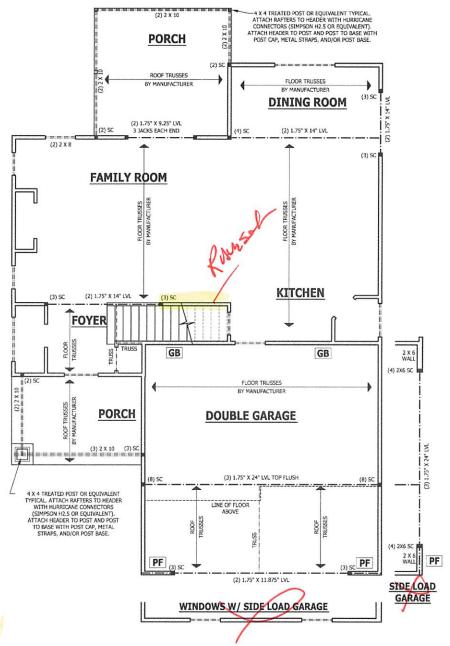
GB: Interior walls show as GB are to have minimum 1/2' gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with

PF: Portal fame per figure R602.10.1



PORTAL FRAME AT OPENING (METHOD PF PER FIGURE AND SECTION R602.10.1)

SCALE 1/4" = 1'-0"



FIRST FLOOR STRUCTURAL

SCALE 1/4" = 1'-0"

PURCHASER MUST VERIFY ALL DIMENSIONS AND CONDITION SEFORE CONSTRUCTION SEGIN HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOI ONTRACTORS PRACTICES A

CODES AND CONDITIONS MAY

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FLOOR STRUCTURAL GASTON FIRST

SQUARE FOOTAGE HEATED UNHEATED FRONT PORCH

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be construed to supersede the code. TOR CITE DEACTICES AND SAFETY: Hauman Homa Diane Inc. assumes no liability for contractors practices and procedures or safety program. Havnes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(U)
Attics without storage	10	10	L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardralis and handralis	200		
Guardrall in-fill components	50	**	
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20	**	**

FRAMING LUMBER: All non treated framing lumber shall be SPE #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise

ENGINEERED WOOD REAMS

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nt Company,

Laminated veneer lumber (LVL) = Fb=2500 PSI, Fy=285 PSI, E=1.9x106 PSI Parallel strand imber (PSL) = Fh= 2900 PSL Fv= 290 PSL E= 2.0x106 PSL Perais stand under (PSL) = PS-RM PSL PSL PSL PSL ESLEVIBLES.

Laminated strand lumber (LSL) PS-2230 PSL, FV=400 PSL, E=1.55x106 PSL

Install all connections per manufactures instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-JoIst

layouts shall be prepared in accordance with this document.

Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-loist layout shall be coordinated with Havnes Homes Plans. Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" stee angle for up to 6'-0" span, 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9"-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing.

ROOF SHEATHING: OSB or CDX roof sheathing minimum

CONCRETE AND SOILS: See foundation notes

ATTIC ACCESS

SECTION R807

RB07.1 Attic access. An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have a vertical height of 60 inches (1524 mm) or greater. The net dear opening shall not be less than 20 inches by 30 inches (508 mm by 762 mm) and shall be located in a hallway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided a some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

Exceptions:

1. Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc.

are not required to have access.

2. Pull down stair treads, stringers, handrails, and hardware may protrude into the net clear opening

EXTERIOR HEADERS

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE . KING STIIDS FACH END DED TARI E RELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16'

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END LINI ESS NOTED OTHERWISE NON LOAD BEARING HEADERS TO BE

LADDER FRAMED

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. KNEE WALL AND CEILING HEIGHTS. All finished knee wall belobts and celling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished celling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics.

BEARING, All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted atherwise

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses

WALL THICKNESSES

are drawn as 4" or as noted 2 X 6 are drawn as 6" to include 1/2" sheathing or gypsum. Subtract 1/2" for stud face.

Exterior walls and walls adjacent to a parage area

Interior walls are drawn as 3 1/2" or as noted 2 X 6 are drawn as 5 1/2*, and do not include gypsum.

EXTERIOR WINDOWS AND DOORS

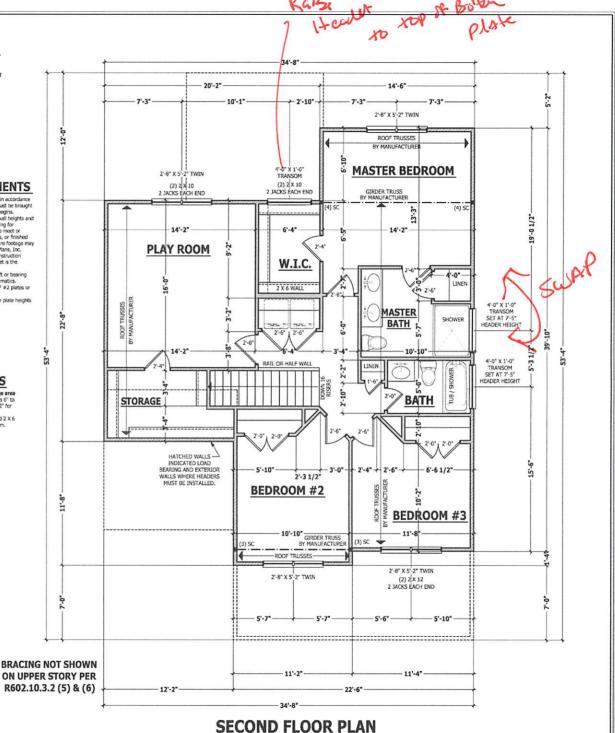
SECTION R617

R612.1 General. This section prescribes performance and construction requirements for exterior windows and doors installed in walls. Windows and doors shall be installed and flashed in accordance with the fenestration manufacturer's written installation instructions. Window and door openings shall be flashed in accordance with Section R703.8. Written installation instructions shall be provided by the fenestration manufacturer for each window

R612.2 Window sills. In dwelling units, where the opening of an operable window is located more than 72 inches (1829 mm) above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches (610 mm) above the finished floor of the room in which the window is located. Operable sections of windows shall not permit openings that allow passage of a 4 inch (102 mm) diameter sphere where such openings are located within 24 inches (610 mm) of the finished floor.

- 1. Windows whose openings will not allow a 4-inch diameter (102 mm) sphere to pass through the opening when the opening is in its largest opened position.

 2. Openings that are provided with window fall prevention devices that comply with Section
- 3. Openings that are provided with fall prevention devices that comply with ASTM E 2090.
- Windows that are provided with opening limiting devices that comply with Sarting R612.4.
 R612.3 Window fall prevention devices. Window fall prevention devices and window guards, where provided, shall comply with the requirements of ASTM F 2090.



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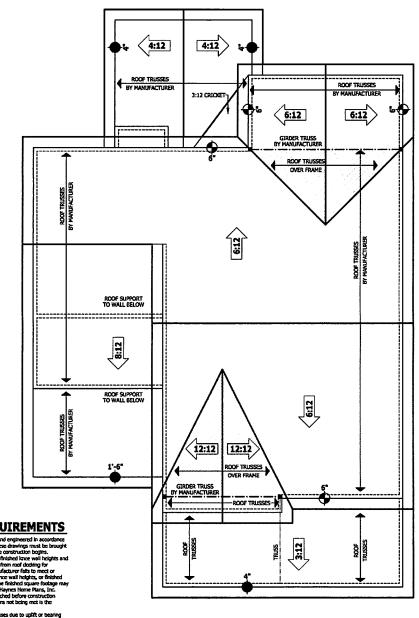
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PLAN GASTON SECOND FLOOR 罪

SQUARE FOOTAGE 7% SQ F1 764 SQ F1 280 SQ F1 1820 SQ F1 TOTAL UNHEATED FRONT PORCH GAPACE REAR PORCH TOTAL 101 SQ F1 466 SQ F1 157 SQ F1 719 SQ F1

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PAGE 6 OF 8



ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineeral in accordance with these drawings. Any versistion with these drawings must be brought to layers know pain, and established before construction bogins.

INSEE WALL ARD CELLING NEIGHTS. All finished ince wash heights and calling heights are shown furred down 10° from road dodding for insulation. If for any reason the truss manufacture rifes to meet or exceed designated hach heights, finished ince wash heights and calling heights also when on these deswings the finished square floatage may vary. Any discrepancy must be brought to Haymas Home Plans, Inc. sateroine, so a studies exhation can be reached before construction logists. Any version due to these conditions not being main to the reasonability of the busin smartfacturer.

ANCHORAGE. All required anchors for usees due to upfilt or bearing shall most the engineenist as specified on the truss surremarks.

BEARITION. All trusses shall be designed for bearing on SPF #2 pistes or ledgers unless noted otherwise.

educations for discuss sale to design or observing on SPF #2 places or leaders unless noted otherwise.

Plata Heights & Roor Systems. See elevation page(s) for plate heights and floor system thicknesses.

HEEL HEIGHT ABOVE FIRST PLOOR PLATE

HEEL HEIGHT ABOVE SECOND FLOOR PLATE

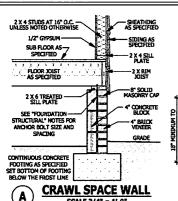
ROOF PLAN SCALE 1/4" = 1'-0"

I GASTON ROOF PLAN THE

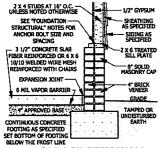
SQUARE FOOTAGE
MEATED
POST FORCE
MEATED
POST FORCE
MEATED
POST FORCE
MEATED
POST FORCE
MARKET
MEATED
POST FORCE
MARKET
MA MASSET.

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SCALE 3/4" = 1'-0"



GARAGE STEM WALL D SCALE 3/4" = 1'-0"

DECK STAIR NOTES

SECTION AM110

AM110.1 Stairs shall be constructed per Figure AM110. Stringer spens shall be no greater than 7 foot span between supports. Specing between stringers shall be based upon deciding material used per AM107.1. Each Stringer shall have minimum 3 1/2 inches between step out and back of stringer d headers shall shall be attached with 3/8 Inch galvanized bots with nuts and washers to securely

DECK BRACING

AM109.1 Deck bracing, Decks shall be braced to provide lateral stability. The following are acceptable meens to provide lateral stability.

AM109.3.1. When the deck floor height is less than 4'-0" ove finished grade per Figure AM109 and the deck is attached to the structure in accordance with Section AM104, lateral bracing is not required.

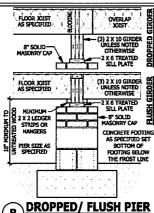
AM109.1.2. 4 x 4 wood knee braces may be provided or

each column in both directions. The knee braces shall attach to each post at a point not less than 1/3 of the post length from the top of the post, and the braces shall be angled between 45 degrees and 60 degrees from the horizontal. Knee braces shall be botted to the post and the girder/double band with one 5/8 inch hot dipped galvanized bolt with nut and washer at both ends of the brace per Floure AM109.1

AM109.1.3. For forestanding decks without knee braces or diagonal bracing, lateral stability may be provided by embedding the post in accordance with Figure AM109.2

POST	TRIBUTARY	HAX POST HEIGHT	EHBEDKENT DEFTH	CONCRETE CLANETER
4X4	48 SF	4'-0"	2'-6"	1'-0"
6 X 6	120 SF	60.	3-6*	1'-8"

AM109.1.4. 2 x 6 diagonal vertical cross bracing may be provided in two perpondicular directions for freestanding decks or parallel to the structure at the exterior column line for strached decks. The 2 x 6's shall be attached to the posts with one 5/8 inch hot dipped zed bolt with nut and washer at each end of ch brading member per Figure AMI AM109.1.5. For embedment of piles in Coastal Regions



В SCALE 3/4" = 1'-0" 2 X 4 STUDS AT 16° O.C. — UNLESS NOTED OTHERWISE 1/2" GYPSUN (2) 5/8" THREAD RODS WITH 2" CUT WASHERS OF EPOXY, MINIMUM 3* CONCRETE BELOW ROD. 2 X 6 TREATED SILL PLATE 3 1/2" CONCRETE SLAR BER REINFORCED OR 6 X 6 ---- 8" SOLID MASONRY CAP 10/10 WELDED WIRE MESH REINFORCED WITH CHAIRS EXPANSION JOINT 4" BRICK VENEER 6 MIL VAPOR BARRIER CRADE 4" APPROVED BASE" TAMPED OR CONTINUOUS CONCRETT EARTH

<48" GARAGE WING WALL E SCALE 3/4" = 1'-0'

SET BOTTOM OF POOTING

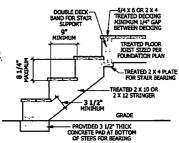


FIGURE AM110 TYPICAL DECK STAIR DETAIL

SCALE 3/4" = 1'-0"

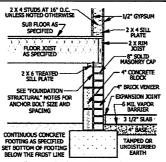
SHEATHING-AS SPECIFIED STONE VEENER AS SPECIFIED LATE VAPOR BARRIER WEED SCREEN MINIMUM 4" TO GROUND OR 2 TO PAVEMENT SEE FOUNDATION FOR FOUNDATION GRADE

> **WEEP SCREED** SCALE 3/4" = 1'-0"

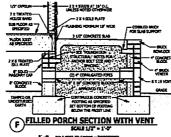
WEEP SCREEDS

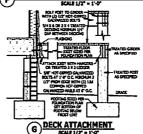
All weep screeds and stone veneer to be per the 2012 North Carolina Residential

R703.6.2.1 - A minimum 0.019-inch (0.5 mm) (No. 26 galvanized sheet gage), -resistant ween screed or plastic extraorresistant weep screet or passe weep screed, with a minimum vertical attachment flange of 31/2 inches (89 mm) shall be provided at or below the foundation plate line on exterior stud walls in accordance with ASTM C 926. The weep screed shall be placed a minimum of 4 Inches (102 mm) above the earth or 2 inches (51 mm) above paved areas and shall be of a type that will allow trapped water to drain to the exterior of the building. The weather-resistant barrier shall izp the attachment flange. The exterior lath shall cover and terminate on the attachment flange of the weep screed



CRAWL SPACE AT GARAGE SCALE 3/4" = 1'-0"





SMOKE ALARMS

SECTION 8314

listed in accordance with UL 217 and installed in accordance with the provisions of this code and the household fire warning

or provisors of NFPA 72.

R314.2 Smoln detaction systems. Household fire alarm systems installed in accordance with NFPA 72 that include smoke starms, a combination of smoke detactor and suddle notification device. installed as recurred by this section for smoke alarms, shall be recursor as required by this section for smooth alarms, shall be permitted. The household fire alarm system shall provide the same level of smoke detection and alarm as required by this section for smoke alarms. Where a household fire warning system is installed using a combination of smoke detector and suctive notification devica(s), it will become a permanent finite of the occupancy arowned by the homeowner. The system shall be monitored by an ed supervising station and be maintained in accordance with NEPA 77.

lion: Where smoke alarms are provided meeting the

requirements of Section R314.4.
R324-3 Location. Smoke elements that be installed in the following

locations:

1. In each slooping room.

2. Outside each separate slooping area in the Immediate vicinity of

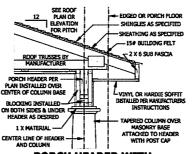
the bodizational.

3. On each additional story of the dwellings including besoments and helistable situs (filestreal) but not including carely speace, unstrabelables (includinated) assistant uninhabitables (un'inhabitable satisfications. In dwellings or dwelling untils with spicil levels and without an intermediate but without and intermedia global between the adjusters levels, a minute additional intermediated on the stopped level and suffered for the adjusters between the control of the stopped levels and suffered for the adjusters and the stopped levels and suffered for the adjusters and the stopped levels and suffered for the adjusters and the stopped levels and suffered for the adjusters and the suffered levels and the stopped levels and suffered levels.

below the upper level. When more than one smoke alarm is received to be installed within when more than one smoke alarm is required to be installed whom an individual dwalling unit the alarm devices shall be interconnected in such a manner that the admittion of one alarm will activate all of the alarms in the individual unit.

the same in the enrollment text.

REJAA Power source. Snoke alarms shall recove their primary power from the budding wiring when such withing is served from a commercial source, and when primary power is interrupted, shall recove power from a buttery. Wiring shall be permanent and without a disconnecting within other than those required for ent protection. Smoke starms shall be inte



PORCH HEADER WITH TAPERED COLUMN

SCALE 3/4" = 1'-0"

CARBON MONOXIDE ALARMS

SECTION R315

R315.1 Carbon de alarms. In new construction, dwelling units shall be provided with an approved carbon monoxide alarm installed outside of each

separate aborting area in the immediate vicinity of the bodroots) as directed by the atem menufacturer. RS15.2 Where required in existing dwellings. In existing dwellings, where interior alterations, repairs, fuel-fired appliance replacements, or additions requiring a permit occurs, or where one or more sleeping mores are added a created, carbon monoxide alarms shall be provided in accordance with Section

R315.3 Alasm requirements. The required carbon monoxide alarms shall be was as a seem requirements. The required carbon monotone amins shall be audible in all bedrooms over background notes levels with all intervening doors closed. Single station carbon monotode alarms shall be listed as complying with UI, 2014 and shall be installed in accordance with this code and the

STAIRWAY NOTES

R31L7.2 Had room. The minimum headroom in all parts of the stairway shall not be less than 6 feet 8 inches (2022 mm) measured vertically from the sloped line adjusting the tread nosing or from the floor surface of the landing or platform on their portion of the stainway. 8311.7.4 Stair breads and risons. Stair treads and risons shall meet the

ents of this section. For the purposes of this section all dimensioned surfaces shall be exclusive of carpets, rugs or runners. RS11.7A.1 Riser height. The maximum riser height shall be 8 1/4 inches (210 mm). The riser shall be measured vertically between leading edges of the adjacent treads.

the adjoinst treads. The dispits. The minimum tread depth shall be 9 inches (229 mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjoinst treads and at a right angle to the tread's leading edge. Whiteir treads shall have a minimum tread depth of the tread's leading edge. Whiteir treads shall have a minimum tread depth. of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a

minimum tread depth of 4 inches (102 mm) at any point. 8311.7.4.3 Profile. The radius of curvature at the rosing shall be no greater than 9/15 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stainways with solid

R311.7.7 Handralis, Handralis shall be provided on at least one side of each IRSILIZ/ Interdets Hendrets shall be provided on at less one side of each continuous run of breads or flight with four or more reser-IRSILIZ/A. Height. Hendrall height, measured vertically from the ploped plane adjoining the tread nosing, or firsth surface of ramp slope, shall be not less than 34 Inches (645 mm) and not more than 38 Inches (

1. The use of a volute, turnout or starting easing shall be allowed over the

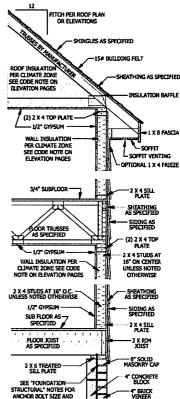
lowest trad.

2. When hanning fittings or bendings are used to provide continuous transition between flights, the transition from handrals to quadrals, or used at the sizer of a flight, the handral heigh at the fittings or bendings shall be permitted to second the maximum height.

2. State of the flight, from a poten directly above the top rises of the flight to the flight, from a poten directly above the top rises of the flight to extinute or of the flight continuous flight to extinute or of the flight continuous flight to extinute or of the flight continuous flight to extinute or of the between the wall and the handralis.

andrails shall be permitted to be interrupted by a newel post.

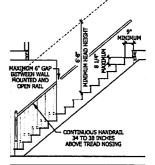
 The use of a volute, turnout, starting easing or starting newel shall be allowed over the lowest troad.
 Two or more separate rails shall be considered continuous if the termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the



CONTINUOUS CONCRETE FOOTING AS SPECIFIED SET BOTTOM OF FOOTING BELOW THE FROST LINE TYPICAL WALL DETAIL SCALE 3/4" = 1'-0"

GRADE

SPACING



TYPICAL STAIR DETAIL

DETAILS GASTON **IYPICAL** ш

HAVRES HOME PLANS, INC ISSUMES NO LIABILITY FO INTRACTORS PRACTICES A PROCEDURES.

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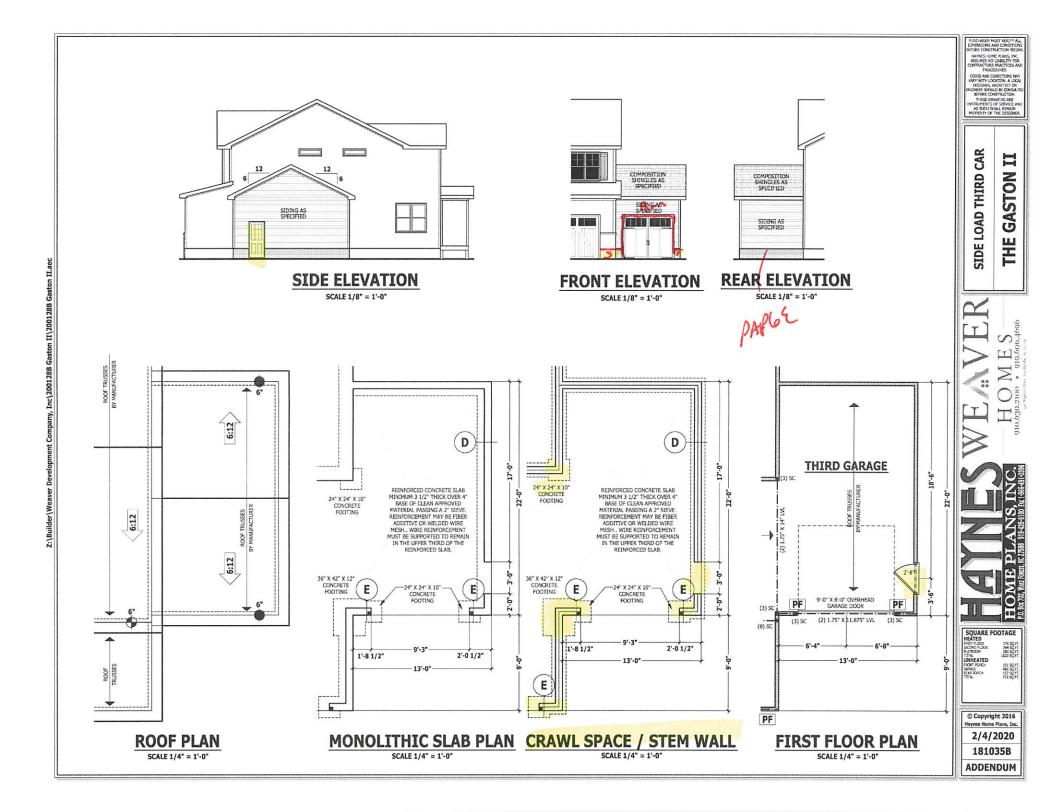
THESE DRAWING ARE

Ξ

SCHARF FOOTAGE 78 ST TOTAL UNCHEATED FRORT FORCH GARACE BLAR FORCH 111 S.F. 46 S.F. 12 S.F.

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PAGE 8 OF 8





RE: J0620-2873

Trenco

818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Lot/Block: Project Name: J0620-2873

Model:

Address:

Subdivision:

City:

State

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2009/TPI2007

Design Program: MiTek 20/20 8.3

Wind Code: N/A

Wind Speed: N/A mph

Roof Load: N/A psf

Floor Load: 55.0 psf

This package includes 15 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E14484726	F1	6/24/2020
2	E14484727	F3	6/24/2020
3	E14484728	F4	6/24/2020
4	E14484729	F5	6/24/2020
5	E14484730	F6	6/24/2020
6	E14484731	F7	6/24/2020
7	E14484732	F8	6/24/2020
8	E14484733	F9	6/24/2020
9	E14484734	F10	6/24/2020
10	E14484735	F11	6/24/2020
11	E14484736	F12	6/24/2020
12	E14484737	KW	6/24/2020
13	E14484738	KW1	6/24/2020
14	E14484739	KW2	6/24/2020
15	E14484740	KW3	6/24/2020

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



June 24, 2020

Job	Truss	Truss Type	Qty	Ply		2 Marine 1922 1920 1920 1920 1920 1920 1920 1920
J0620-2873	F1	FLOOR	8	1		E14484726
		(100 to	1850		Job Reference (optional)	

Comtech, Inc, Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:24 2020 Page 1 ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-MeuOvwf1TRjrPwe_ay0FyewWIMXNXFoP0BDBVcz8N6v

Structural wood sheathing directly applied or 6-0-0 oc purlins,

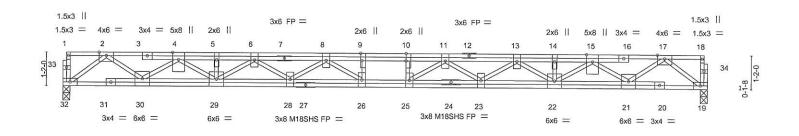
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

0-1-8 H 1-3-0

1-5-0

0-1-8 Scale = 1:38.6



2-9 2-9 Plate Offsets (X,Y)	0 5-1-8	14-6-8 6-8-0	19-8-0 5-1-8	22-5-0 2-9-0
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 1-7-3 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014		in (loc) I/defl L/d -0.30 25-26 >889 480 -0.41 25-26 >646 360 0.06 19 n/a n/a	PLATES GRIP MT20 244/190 M18SHS 244/190 Weight: 164 lb FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP 2400F 2.0E(flat) **BOT CHORD** 2x4 SP 2400F 2.0E(flat)

WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 32=0-3-0, 19=0-3-0

Max Grav 32=970(LC 1), 19=970(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-4=-2267/0, 4-5=-4172/0, 5-6=-4172/0, 6-8=-5247/0, 8-9=-5785/0, 9-10=-5785/0, 10-11=-5785/0, 11-13=-5247/0, 13-14=-4172/0, 14-15=-4172/0, 15-17=-2267/0 30-32=0/1227, 29-30=0/3336, 28-29=0/4843, 26-28=0/5628, 25-26=0/5785, 23-25=0/5628, **BOT CHORD**

22-23=0/4843, 21-22=0/3336, 19-21=0/1227

WEBS 17-19=-1536/0, 2-32=-1536/0, 17-21=0/1317, 2-30=0/1317, 15-21=-1329/0,

4-30=-1329/0, 15-22=0/1021, 4-29=0/1021, 13-22=-819/0, 6-29=-819/0, 13-23=0/501,

6-28=0/501, 11-23=-483/0, 8-28=-483/0, 11-25=-216/559, 8-26=-216/559

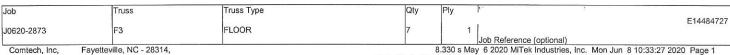
NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 3x6 MT20 unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.
- 5) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



June 8,2020





Fayetteville, NC - 28314, Comtech, Inc,

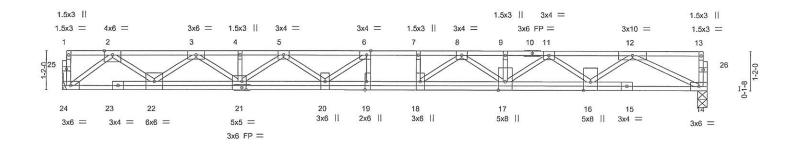
ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-nDaXXyhvmM5QHNMZF5ZyaGYypaXrkbori9Rs6wz8N6s

0-1-8

H | 1-3-0

1-4-8

2-0-0 0-1-8 Scale = 1:33.3



-	10-3-12 10-3-12				11-4-8			19-4 8-0-			
Plate Offsets (X,Y)	[6:0-1-8,Edge], [19:0-3-0,		I-8,Edge]		1-0-12	11/2			8-0-	-0	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.00 1.00 YES PI2014	CSI. TC BC WB Matrix	0.39 0.32 0.61 ×-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.28 -0.38 0.05	(loc) 19 19 14	I/defl >826 >602 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 120 lb	GRIP 244/190 FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP 2400F 2.0E(flat) 2x4 SP 2400F 2.0E(flat) **BOT CHORD**

2x4 SP No.3(flat) WEBS

BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD

REACTIONS. (size) 24=Mechanical, 14=0-3-8

Max Grav 24=1046(LC 1), 14=1046(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2347/0, 3-4=-4043/0, 4-5=-4040/0, 5-6=-4852/0, 6-7=-5063/0, 7-8=-5063/0, TOP CHORD

8-9=-4353/0, 9-11=-4353/0, 11-12=-2866/0

BOT CHORD 22-24=0/1335, 21-22=0/3312, 20-21=0/4601, 19-20=0/5063, 18-19=0/5063, 17-18=0/4812,

16-17=0/3736, 14-16=0/1964

2-24=-1673/0, 2-22=0/1285, 3-22=-1226/0, 3-21=0/897, 5-21=-713/0, 5-20=0/445,

6-20=-593/162, 6-19=-277/217, 12-14=-2182/0, 12-16=0/1147, 11-16=-1105/0,

11-17=0/769, 8-17=-605/0, 8-18=-83/605

NOTES-

WEBS

- Unbalanced floor live loads have been considered for this design.
 Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Design valid for use only with MTick® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	
J0620-2873	F4	FLOOR	3	1	E14484728
					Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:28 2020 Page 1 ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-FP8vIliXXgDHuXxlpo5B7U423_I5T5n_xpBPeNz8N6r

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

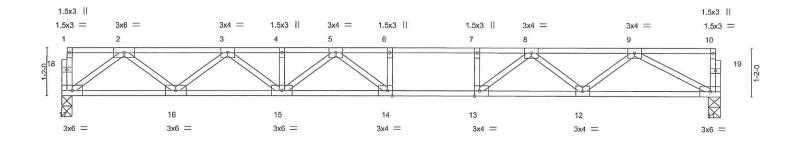
except end verticals.

0 - 1 - 8

H | 1-3-0







i L	9-3-8					10-7-0					
		9-3-8					1-3-8			5-4-8	
Plate Offse	ets (X,Y)	[13:0-1-8,Edge], [14:0-1-	8,Edge]								
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.74	Vert(LL)	-0.24 14-15	>785	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.90	Vert(CT)	-0.33 14-15	>570	360	21750U (2.1047250)	
BCLL	0.0	Rep Stress Incr	YES	WB	0.44	Horz(CT)	0.05 11	n/a	n/a		
BCDL	5.0	Code IRC2015/TF	PI2014	Matrix	-S					Weight: 80 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 17=0-3-0, 11=0-3-8

Max Grav 17=858(LC 1), 11=858(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1774/0, 3-4=-2887/0, 4-5=-2887/0, 5-6=-3157/0, 6-7=-3157/0, 7-8=-3157/0,

8-9=-2067/0

16-17=0/1070, 15-16=0/2453, 14-15=0/3153, 13-14=0/3157, 12-13=0/2674, 11-12=0/1453 2-17=-1340/0, 2-16=0/916, 3-16=-884/0, 3-15=0/554, 5-15=-340/0, 5-14=-241/390, 9-11=-1641/0, 9-12=0/799, 8-12=-790/0, 8-13=0/814, 7-13=-365/0 **BOT CHORD**

WEBS

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see __ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply		
J0620-2873	F5	FLOOR	6	1		E14484729
		Control and the State of State			Job Reference (optional)

Comtech Inc. Favetteville, NC - 28314.

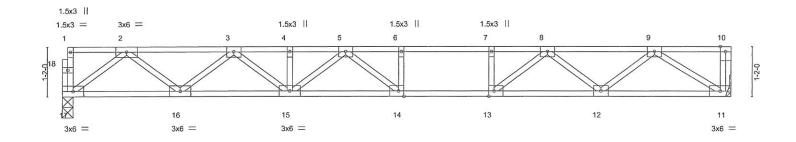
8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:29 2020 Page 1 $ID: BoL? hgXgIYpqwdOiyUmcQyz41fz-jbhHyej9I_L8WhWxNVcQfhdDVN4BCYD8ATwyApz8N6q\\$

0-1-8

H | 1-3-0

2-0-0

Scale = 1:26.0



T.			9-1-12				10-3-8	_1		15-8-0	
Г			9-1-12				1-1-12			5-4-8	
Plate Off	sets (X,Y)	[13:0-1-8,Edge], [14:0-1-	8,Edge]								
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.76	Vert(LL)	-0.24 14-15	>776	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.90	Vert(CT)	-0.33 14-15	>566	360		
BCLL	0.0	Rep Stress Incr	YES	WB	0.42	Horz(CT)	0.05 11	n/a	n/a		
BCDL	5.0	Code IRC2015/Ti	PI2014	Matri	x-S					Weight: 79 lb	FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

WEBS

2x4 SP No.3(flat)

BRACING-TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

(size) 17=0-3-0, 11=Mechanical Max Grav 17=842(LC 1), 11=848(LC 1) REACTIONS.

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1733/0, 3-4=-2808/0, 4-5=-2808/0, 5-6=-3022/0, 6-7=-3022/0, 7-8=-3022/0, TOP CHORD

BOT CHORD

16-17=0/1049, 15-16=0/2394, 14-15=0/3053, 13-14=0/3022, 12-13=0/2502, 11-12=0/1241

2-17=-1313/0, 2-16=0/891, 3-16=-861/0, 3-15=0/528, 5-15=-314/0, 5-14=-267/352,

9-11=-1463/0, 9-12=0/821, 8-12=-822/0, 8-13=0/841, 7-13=-375/0

NOTES-

WEBS

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.





Job	Truss	Truss Type	Qty	Ply	5444	10.4700
J0620-2873	F6	FLOOR	1	1	E144	184730
		1 (1) (1) (1) (1) (1) (1) (1) (1) (1) (1			Job Reference (optional)	

Comtech, Inc, Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:30 2020 Page 1 $ID: BoL? hgXgIYpqwdOiyUmcQyz41 fz-BoFfA_ko3HT? 8r58wD7fCvANAnQ_x?GHO7gWiFz8N6parter for the control of the co$

0-1-8

HI-3-0

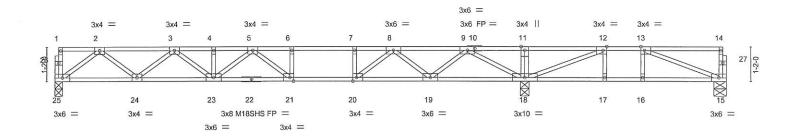
2-0-0

1-9-12 2-6-0 1-1-12 2-6-0

Structural wood sheathing directly applied or 5-8-12 oc purlins,

Rigid ceiling directly applied or 2-2-0 oc bracing.

except end verticals.



1	9-3-6 9-3-6	10-6-12 1-3-6					
Plate Offsets (X,Y)	- [12:0-1-8,Edge], [13:0-1-8,Edge], [20:0	-1-8,Edge], [21:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.83 BC 0.93 WB 0.44	DEFL. in (loc) Vert(LL) -0.24 21-23 Vert(CT) -0.33 21-23 Horz(CT) 0.04 15	l/defl >783 >568 n/a	L/d 480 360 n/a	PLATES MT20 M18SHS	GRIP 244/190 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S				Weight: 113 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

WEBS

TOP CHORD 2x4 SP No.1(flat) 2x4 SP No.1(flat) **BOT CHORD**

2x4 SP No.3(flat)

(size) 25=0-3-0, 18=0-3-8, 15=0-5-0

Max Uplift 15=-52(LC 3)

Max Grav 25=802(LC 10), 18=1452(LC 1), 15=308(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1634/0, 3-4=-2617/0, 4-5=-2617/0, 5-6=-2689/0, 6-7=-2689/0, 7-8=-2689/0,

8-9=-1399/0, 9-11=0/1071, 11-12=0/1075, 12-13=-444/311

BOT CHORD 24-25=0/996, 23-24=0/2251, 21-23=0/2810, 20-21=0/2689, 19-20=0/2082, 18-19=0/726,

17-18=-311/444, 16-17=-311/444, 15-16=-311/444 2-25=-1248/0, 2-24=0/830, 3-24=-803/0, 3-23=0/467, 5-21=-347/212, 9-18=-1758/0,

9-19=0/906, 8-19=-935/0, 8-20=0/916, 7-20=-404/0, 12-18=-1130/0, 13-15=-468/336

NOTES-

WEBS

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 1.5x3 MT20 unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.



June 8,2020

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters and properly design parameters and properly design parameters and properly design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucallapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/PH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply		
J0620-2873	F7	FLOOR	3	1		E14484731
					Job Reference (optional)	

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:31 2020 Page 1 ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-f_p1NJIQpbbrl?gKUweul6igDBqcgTNRdnP3Fiz8N6o

Structural wood sheathing directly applied or 6-0-0 oc purlins,

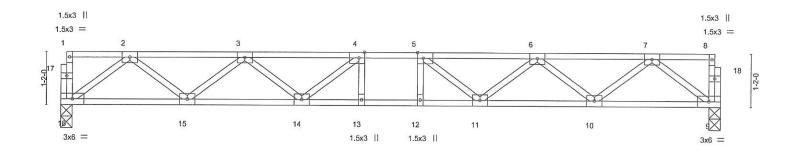
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

0-1-8

H | 1-3-0 | 1-2-0

0-1-8 Scale = 1:24.3



			14-5-0					1
			14-5-0					
Plate Offsets (X,Y)	[4:0-1-8,Edge], [5:0-1-8,Edge]							
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	BC 0.	0.59 Vert(CT) -	-0.12 12-13 >	/defl L -999 48 -999 36	30 50	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S	-S				Weight: 73 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

WEBS 2x4 SP No.3(flat)

(size) 16=0-3-0, 9=0-3-0

Max Grav 16=773(LC 1), 9=773(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1570/0, 3-4=-2405/0, 4-5=-2647/0, 5-6=-2405/0, 6-7=-1570/0

BOT CHORD 15-16=0/955, 14-15=0/2151, 13-14=0/2647, 12-13=0/2647, 11-12=0/2647, 10-11=0/2151,

9-10=0/955 WEBS 7-9=-1195/0,

7-9=-1195/0, 7-10=0/801, 6-10=-756/0, 6-11=0/386, 5-11=-454/0, 2-16=-1195/0,

2-15=0/801, 3-15=-756/0, 3-14=0/386, 4-14=-454/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





818 Soundside Road Edenton, NC 27932 Job Truss Truss Type Qty E14484732 J0620-2873 F8 FLOOR GIRDER 2 | **Z** | Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:32 2020 Page 1

Comtech, Inc. Fayetteville, NC - 28314,

ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-7ANPbfl2avjiN8FW2e97HKFr?bCVPpuasR9cn8z8N6n

0-1-8 H - 1-3-0

0-11-0

0-1-8 Scale = 1:24.7

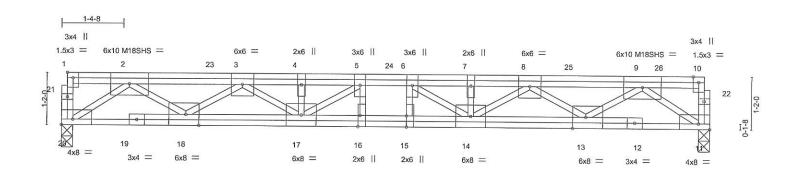


Plate Offsets (X,Y)	9-0-8 9-0-8 [1:Edge,0-1-8], [11:Edge,0-1-8], [13:0-3		, [16:0-3-0,Edge], [18:0-3	-8,Edge], [20:E		14-5-0 5-4-8	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.30 BC 0.48 WB 0.81 Matrix-S	DEFL. in Vert(LL) -0.19 Vert(CT) -0.26 Horz(CT) 0.06	(loc) I/defl 15 >903	L/d 480 360 n/a	PLATES MT20 M18SHS Weight: 221 lb	GRIP 244/190 244/190 FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP 2400F 2.0E(flat) **BOT CHORD** 2x4 SP 2400F 2.0E(flat) WEBS

2x4 SP No.3(flat) *Except* 2-18,9-13: 2x4 SP No.2(flat) BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 20=0-3-0, 11=0-3-0

Max Grav 20=4019(LC 1), 11=4153(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

10-11=-255/0, 2-3=-9312/0, 3-4=-14910/0, 4-5=-14910/0, 5-6=-15908/0, 6-7=-14837/0, 7-8=-14837/0, 8-9=-9126/0

BOT CHORD 18-20=0/5647, 17-18=0/12930, 16-17=0/15908, 15-16=0/15908, 14-15=0/15908, 13-14=0/12661, 11-13=0/5549

2-20=-6930/0, 2-18=0/4544, 3-18=-4487/0, 3-17=0/2416, 4-17=-649/0, 5-17=-1297/0,

9-11=-6793/0, 9-13=0/4437, 8-13=-4384/0, 8-14=0/2654, 7-14=-690/0, 6-14=-1406/0

NOTES-

WEBS

- 1) Fasten trusses together to act as a single unit as per standard industry detail, or loads are to be evenly applied to all plies.
- 2) Unbalanced floor live loads have been considered for this design.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1026 lb down at 1-4-8, 1026 lb down at 3-4-8, 1026 lb down at 5-4-8, 971 lb down at 7-4-8, 1026 lb down at 9-4-8, and 1026 lb down at 11-4-8, and 1027 lb down at 13-4-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 11-20=-10, 1-10=-100

Concentrated Loads (lb)

Vert: 2=-946(F) 4=-946(F) 7=-946(F) 23=-946(F) 24=-946(F) 25=-946(F) 26=-953(F)



June 8,2020

<u> A WARNING</u> - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Road

ob		Truss	Truss Type		Qty	Ply	F1448	84733
0620-2873		F9	FLOOR		3	1	Job Reference (optional)	
Comtech, Inc,	Fayettev	ville, NC - 28314,		ID:BoL	.8 hgXgIYp?۔	330 s May qwdOiyUn	6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:33 2020 Page ncQyz41fz-bNxoo?mgLCrZ?lqicLgMqXo?_?c88QBj44uAJaz8N6	im
	0-1-8	2-6-0		0-10-8	-		Scale =	= 1:12.4
			3x4 =	=			50 m W	
	11.5x3	1	2		3 3x4 =		4 3x4	mat
1-2-0		3x4 =		•				1-2-0
	8		7 1.5	ix3 II	1.5x3 6		5	
	3x6 =	=					3x6 =	

Ply

Qty

Plate Offsets (X,Y)	[2:0-1-8,Edge], [3:0-1-8,Edge], [9:0-1-8,	0-1-8]		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.35 BC 0.20 WB 0.17 Matrix-S	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) -0.03 5-6 >999 480 MT20 244/190 Vert(CT) -0.03 5-6 >999 360 Horz(CT) 0.01 5 n/a n/a Weight: 35 lb FT = 20	%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1(flat) TOP CHORD BOT CHORD 2x4 SP No.1(flat)

2x4 SP No.3(flat) WEBS

REACTIONS. (size) 8=0-5-0, 5=Mechanical

Max Grav 8=344(LC 1), 5=351(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-578/0

BOT CHORD 2-8=-613/0, 3-5=-620/0 **WEBS**

NOTES-

7-8=0/578, 6-7=0/578, 5-6=0/578

- 1) Unbalanced floor live loads have been considered for this design.
- Plates checked for a plus or minus 1 degree rotation about its center.
 Refer to girder(s) for truss to truss connections.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss well and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see MSI/TPI fuality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932

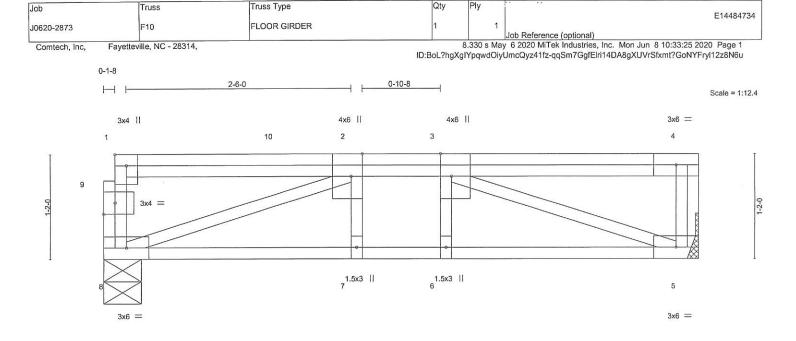


Plate Off	sets (X,Y)	[1:Edge,0-1-8], [2:0-3-0,E	dge], [3:0-3-0	,Edge], [9:0-	1-8,0-1-8]	6-7-8						
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.22	Vert(LL)	-0.02	7-8	>999	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	ВС	0.22	Vert(CT)	-0.03	7-8	>999	360		
BCLL	0.0	Rep Stress Incr	NO	WB	0.22	Horz(CT)	0.01	5	n/a	n/a		
BCDL	5.0	Code IRC2015/TF	PI2014	Matri	x-S						Weight: 44 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

6-7-8

LUMBER-

REACTIONS.

2x4 SP No.1(flat)

TOP CHORD **BOT CHORD** 2x4 SP No.1(flat)

WEBS 2x4 SP No.3(flat)

(size) 8=0-5-0, 5=Mechanical

Max Grav 8=440(LC 1), 5=403(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-773/0

7-8=0/773, 6-7=0/773, 5-6=0/773 **BOT CHORD**

2-8=-813/0, 3-5=-822/0 **WEBS**

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 149 lb down at 1-11-8, and 101 Ib down at 3-1-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 5-8=-10, 1-4=-100

Concentrated Loads (lb)

Vert: 2=-73(B) 10=-75(B)



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

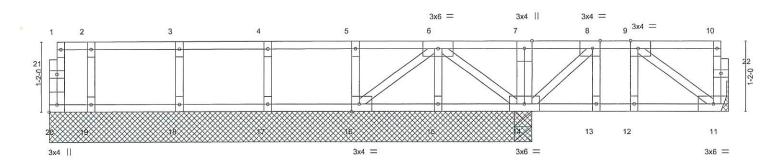
<u>k</u> WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTel® connectors. This case only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer. Bracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Flate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	G	lty	Ply		E14484735
J0620-2873	F11	FLOOR	1	fi S	1	Job Reference (optional)	£14404760
Comtech, Inc, Fayet	teville, NC - 28314,		ID:Bol		330 s May	y 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:2 UmcQyz41fz-J108KchH?3zZfDnMhN2j23?stAGi?H	
0-1-8							
H 1 ⁰⁻⁴⁻⁸ 1	1-4-0 1-4	-0 1-4-0	-l 1-3-0	4			0 <u>1-8</u> Scale = 1:18.4



—				7-10-8 7-10-8						-0-0 -1-8	11-3-0 3-3-0	
Plate Offse	ets (X,Y)	[8:0-1-8,Edge], [9:0-1-8,E	Edge], [16:0-1-		Edge,0-1-8]							
LOADING TCLL TCDL BCLL	(psf) 40.0 10.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.00 1.00 YES	CSI. TC BC WB	0.08 0.06 0.04	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 0.00	(loc) 12 12	l/defl >999 >999 n/a	L/d 480 360 n/a	PLATES MT20	GRIP 244/190
BCDL	5.0	Code IRC2015/TR		Matri		11012(01)	0.00		IIIa	11/4	Weight: 59 lb	FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.1(flat)

2x4 SP No.1(flat) BOT CHORD

2x4 SP No.3(flat) **WEBS**

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 8-0-0 except (jt=length) 11=Mechanical.

(lb) - Max Uplift All uplift 100 lb or less at joint(s) 20

Max Grav All reactions 250 lb or less at joint(s) 11, 15, 16, 17, 18, 19 except 14=278(LC 15), 14=265(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

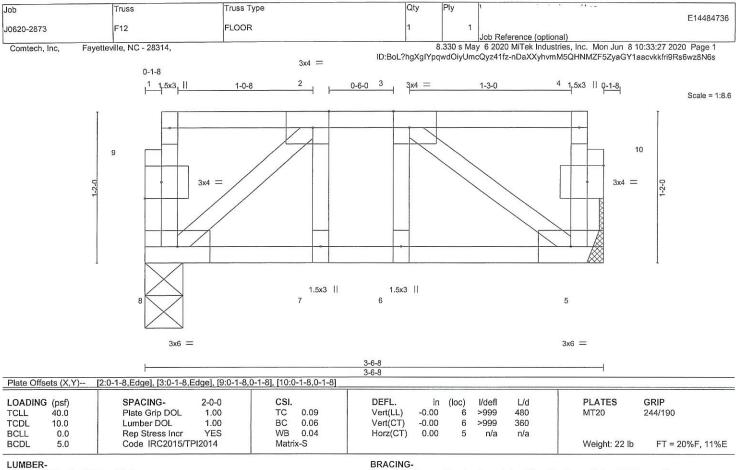
NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 1.5x3 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.





818 Soundside Road Edenton, NC 27932



TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1(flat) **BOT CHORD** 2x4 SP No.1(flat)

WEBS 2x4 SP No.3(flat)

> (size) 8=0-3-8, 5=Mechanical Max Grav 8=175(LC 1), 5=175(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





Structural wood sheathing directly applied or 3-6-8 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

Job	Truss	Truss Type	Qty	Ply		E14484737
J0620-2873	кw	FLOOR SUPPORTED GABL	1	1	Job Reference (optional)	
					COCCATTAL Laboration Laboration Col	

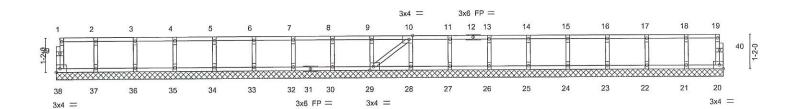
Comtech, Inc, Fayetteville, NC - 28314,

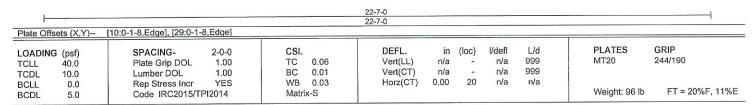
0-118

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:34 2020 Page 1 ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-4ZVA?LnI6WzQcSPv93CbMIKEEO?OtvbtJkejs0z8N6I

0-1-8

Scale = 1:37.6





LUMBER-

TOP CHORD 2x4 SP No.1(flat)

BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)

WEBS 2x4 SP No.3(flat)
OTHERS 2x4 SP No.3(flat)

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 22-7-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 38, 20, 37, 36, 35, 34, 33, 32, 30, 29, 28, 27, 26, 25, 24, 23, 22, 21

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, crection and bracing of trusses and truss systems, see ANSITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	E14484738
J0620-2873	KW1	FLOOR SUPPORTED GABL	1	1	Job Reference (optional)

Comtech, Inc,

Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:35 2020 Page 1 ID: BoL? hgXgIYpqwdOiyUmcQyz41fz-YI3YDhowtq5HEcz5jmjqvytPxoLdcMq0YONHOTz8N6k

Scale: 3/8"=1"

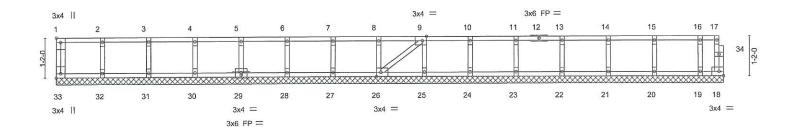


Plate Offsets (X,Y)	[1:Edge,0-1-8], [9:0-1-8,Edge], [26:0-1-	6,Eugej, [55.Euge,0-1-6]							
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.01	Vert(CT)	n/a	2	n/a	999		
BCLL 0.0	Rep Stress Incr YES	WB 0.03	Horz(CT)	0.00	18	n/a	n/a	1	
3CDL 5.0	Code IRC2015/TPI2014	Matrix-S						Weight: 84 lb	FT = 20%F, 11%E

2x4 SP No.1(flat)

BOT CHORD **WEBS** 2x4 SP No.3(flat)

2x4 SP No.3(flat) OTHERS

except end verticals. **BOT CHORD**

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 19-4-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 33, 18, 32, 31, 30, 29, 28, 27, 26, 25, 24, 23, 22, 21, 20, 19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see _ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	E14484739
J0620-2873	KW2	FLOOR SUPPORTED GABL	1	1	L14404703
20020 20.0		TOURNA COSCIONA CROSS DE CONTROLLE MONTA CONTROLLA CONTROLA CONTRO			Job Reference (optional)

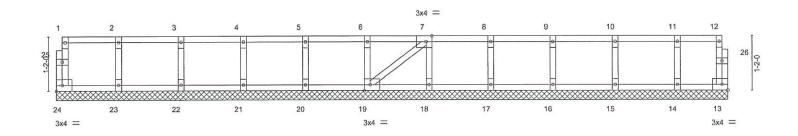
Comtech, Inc,

Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:37 2020 Page 1 ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-U8AleNpBPRL?Tw7UrBII_NylUc154GKJ?isNSLz8N6i

0-1-8

0-1-8 Scale: 1/2"=1"



	5		14-5-0						
Plate Offsets (X,Y)	[7:0-1-8,Edge], [19:0-1-8,Edge]		14-5-0						
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.06 BC 0.01 WB 0.03	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 13	I/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S	J. 202					Weight: 63 lb	FT = 20%F, 11%l
LUMBER- TOP CHORD 2x4 SF	P No.1(flat)		BRACING- TOP CHORD			al wood		rectly applied or 6-0-0	oc purlins,

WEBS 2x4 SP No.3(flat)

2x4 SP No.3(flat) **OTHERS**

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 14-5-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 24, 13, 23, 22, 21, 20, 19, 18, 17, 16, 15, 14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

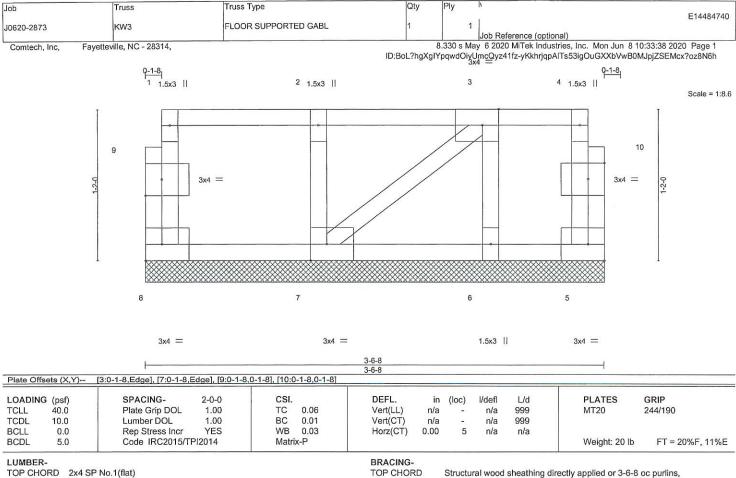
- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTeke connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/dor chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.1(flat) **BOT CHORD** 2x4 SP No.1(flat) **WEBS** 2x4 SP No.3(flat)

OTHERS 2x4 SP No.3(flat)

REACTIONS. All bearings 3-6-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Plates checked for a plus or minus 1 degree rotation about its center.
- 2) Gable requires continuous bottom chord bearing.
- 3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 4) Gable studs spaced at 1-4-0 oc.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



🔼 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

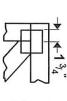
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly anage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



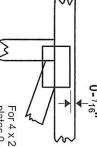
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.

Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

œ

BOTTOM CHORDS

6

This symbol indicates the required direction of slots in connector plates.

Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

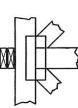
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only

Industry Standards:

ANSI/TP11: National Design Specification for Metal
Plate Connected Wood Truss Construction.
Design Standard for Bracing

DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information,

Guide to Good Practice for Handling,

Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

dimensions shown in ft-in-sixteenths
(Drawings not to scale)

1 2 3

TOP CHORDS

Ci-2 C2-3

WEBS

OP CHORDS

Ci-2 C2-3

TOP CHORDS

JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

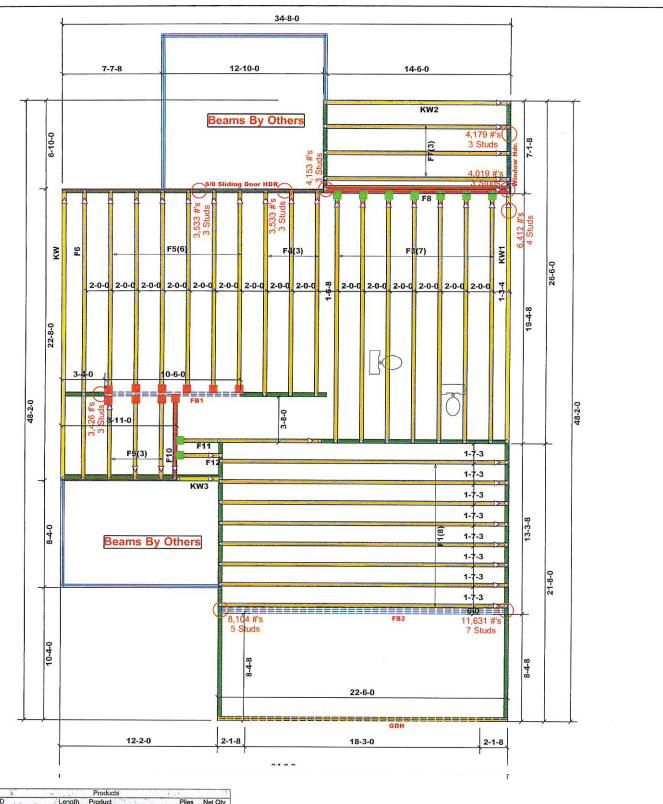
General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.

4

- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



		Products		Sand and
PlotID	Length	Product	Plies	Net Qty
6/0 Sliding Door HDR	7-0-0	1-3/4"x 9-1/4" LVL Kerto-S	2	2
GDH	23-0-0	1-3/4"x 14" LVL Kerto-S	2	2
FB1	12-0-0	1-3/4"x 14" LVL Kerto-S	2	2
Window Hdr.	7-0-0	1-3/4"x 14" LVL Kerto-S	2	2
FB2	23-0-0	1-3/4"x 23-7/8" LVL Kerto-S	3	3

■= USP HUS410 2x Hanger

■ = USP MSH422 2x Strap Hanger

Truss Placement Plan SCALE: NTS

Marshall Naylor

Lenny Norris

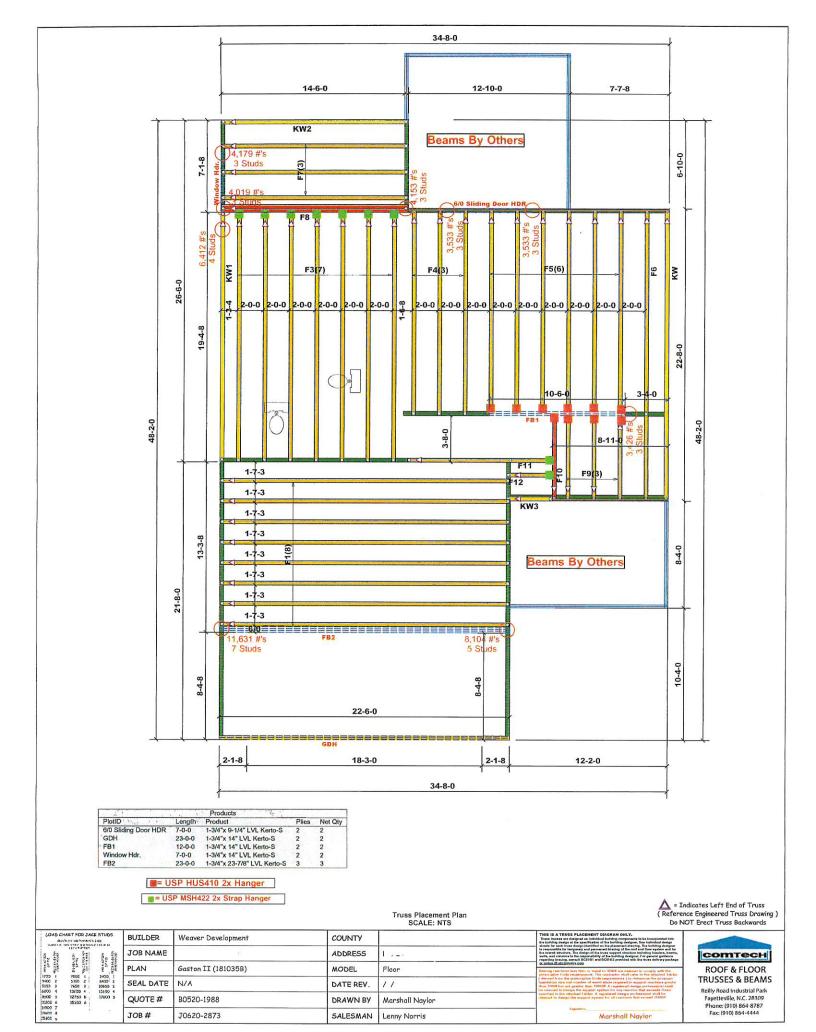
A = Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

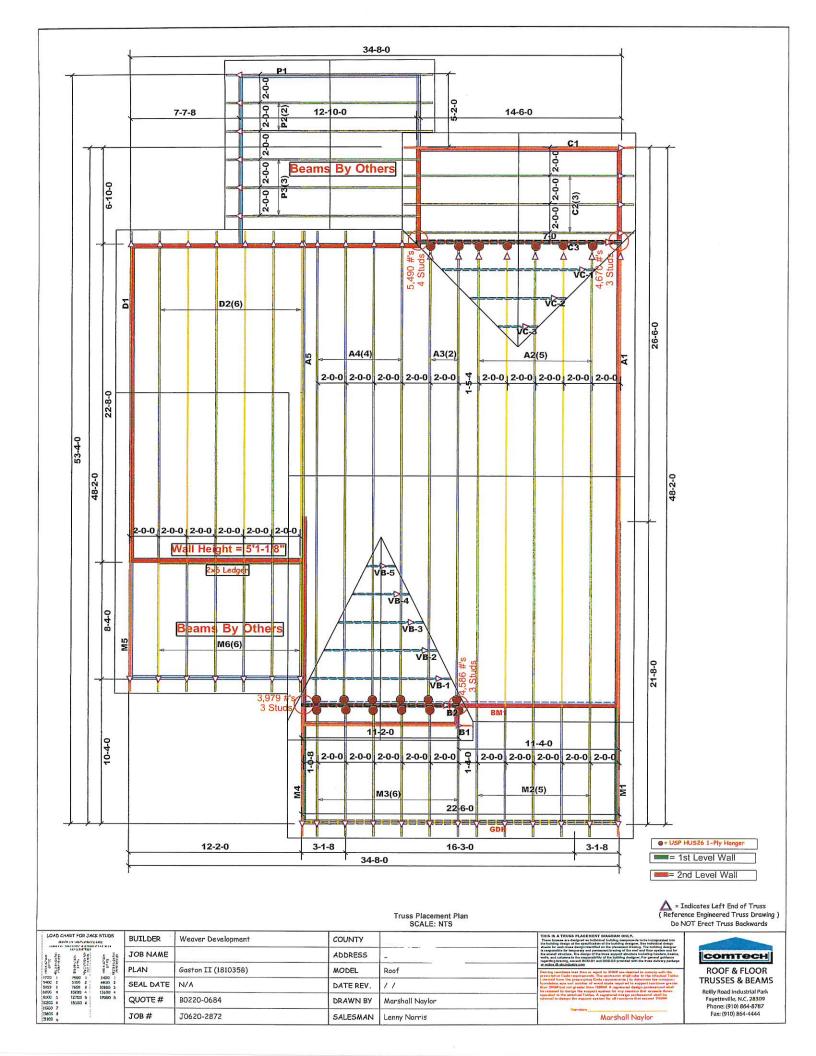
LOAD CHART FOR JACK STUDS	BUILDER	Weaver Development	COUNTY
ECHAPPETRA	JOB NAME	Lot 2 St. Andrews	ADDRESS
MANAGED OF THE PARTY OF THE PAR	PLAN		MODEL
1700 1 2000 1 3400 1 3400 2 5300 2 5800 2 5150 3 7650 3 10200 5	SEAL DATE	N/A	DATE REV.
6800 4 10700 4 13600 4 8500 5 12753 5 17000 5 80200 6 15500 6	QUOTE#	B0520-1988	DRAWN BY
11900 7 13600 # 15100 y	JOB#	J0620-2873	SALESMAN

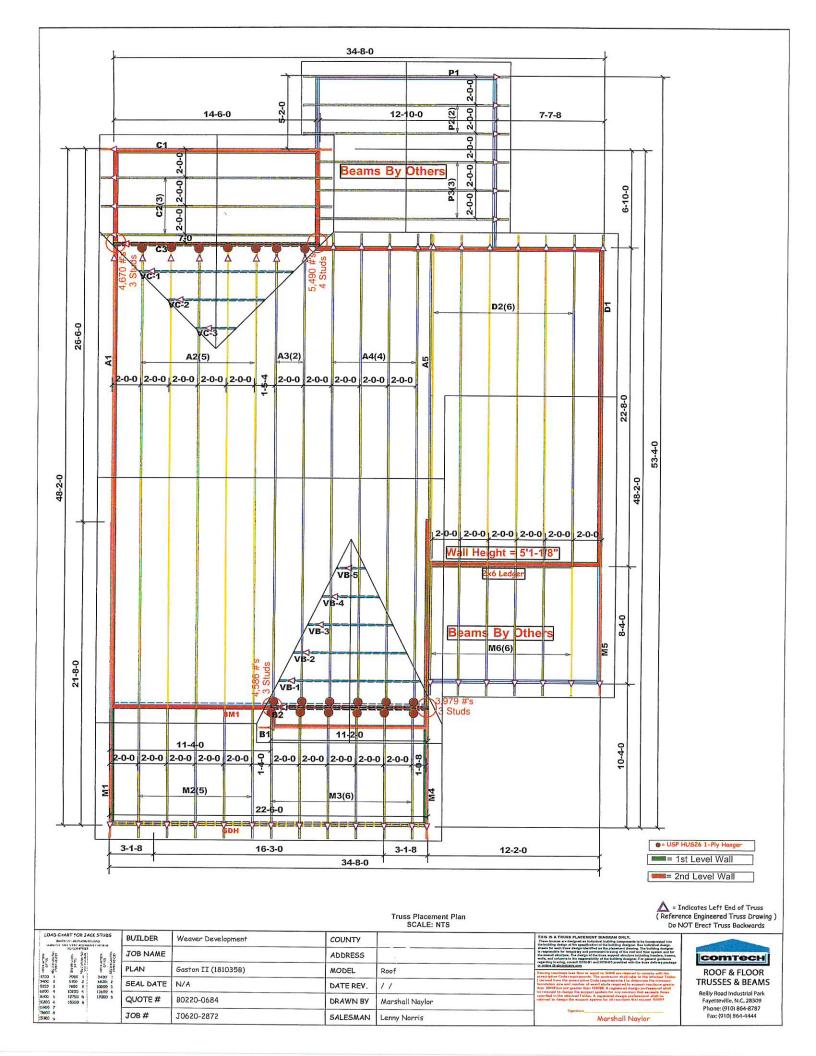
-11176	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These between its relatings in inclinical bothings semperated to be incorporated into the control of
	Occasion a constitute is less than our support to 20000 in so demonst in conseque out to the present epither for the support configuration of the configurat
	Signature
	Marshall Naylor

ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444







isDesign

Client: Project:

Address:

Weaver Development

Gaston II (181035B)

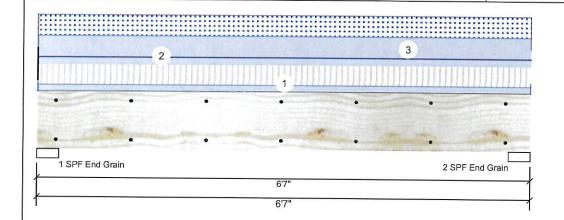
Date: Input by: 6/24/2020

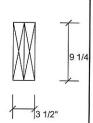
Christine Shivy

Job Name: 6/0 Sliding Door HDR Project #:

6/0 Sliding Door HDR Kerto-S LVL 1.750" X 9.250"

2-Ply - PASSED





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0

Page 1 of 1

Member	nformat	ion
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Type: Girder Plies: 2

Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360

Importance: Temperature:

Normal Temp <= 100°F

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Brg Live Dead Wind Const 1060 1 1903 1113 0 2 1060 1903 1113 0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5033 ft-lb	3'3 1/2"	14423 ft-lb	0.349 (35%)	D+0.75(L+S)	L
Unbraced	5033 ft-lb	3'3 1/2"	10451 ft-lb	0.482 (48%)	D+0.75(L+S)	L
Shear	2459 lb	1'	7943 lb	0.310 (31%)	D+0.75(L+S)	L
LL Defl inch	0.042 (L/1741)	3'3 1/2"	0.153 (L/480)	0.280 (28%)	0.75(L+S)	L
TL Defl inch	0.092 (L/803)	3'3 1/2"	0.204 (L/360)	0.450 (45%)	D+0.75(L+S)	L

Bearings

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	33%	1903 / 1629	3533	L	D+0.75(L+S)
2 - SPF End Grain	3.500"	33%	1903 / 1629	3533	L	D+0.75(L+S)

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	1 17	1								
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	108 PLF	322 PLF	0 PLF	0 PLF	0 PLF	F4
2	Uniform			Тор	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Load
3	Uniform			Тор	338 PLF	0 PLF	338 PLF	0 PLF	0 PLF	A4
	Self Weight				7 PLF					0.50.505

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be out or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633





Client:

Weaver Homes

Project:

Address:

Gaston II (181035B)

Date:

6/24/2020

Input by: Christine Shivy

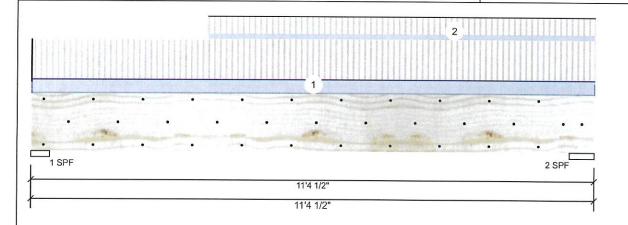
Job Name:

Kerto-S LVL FB₁

1.750" X 14.000"

Project #:

2-Ply - PASSED Level: Level



Floor

ASD

No

IBC 2012

Not Checked



Page 1 of 1

Member Inforn	nation
---------------	--------

Girder Type: Plies: 2

Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360

Importance: Temperature: Normal

Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const	
1	2129	771	0	0	0	
2	2523	904	0	0	0	

Bearings

Bearing	Length	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.	
1-SPF	4.500"	43%	771 / 2129	2899	L	D+L	
2-SPF	6.000"	38%	904 / 2523	3426	L	D+L	

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8168 ft-lb	5'9 3/16"	26999 ft-lb	0.303 (30%)	D+L	L
Unbraced	8168 ft-lb	5'9 3/16"	10258 ft-lb	0.796 (80%)	D+L	L
Shear	2941 lb	9'9 1/4"	10453 lb	0.281 (28%)	D+L	L
LL Defl inch	0.090 (L/1419)	5'8 3/16"	0.266 (L/480)	0.340 (34%)	L	L
TL Defl inch	0.122 (L/1044)	5'8 3/16"	0.354 (L/360)	0.340 (34%)	D+L	L

Application:

Design Method:

Building Code:

Load Sharing:

Deck:

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	•
1	Uniform			Far Face	106 PLF	318 PLF	0 PLF	0 PLF	0 PLF	F5	
2	Part. Uniform	3-6-8 to 11-4-8		Тор	44 PLF	132 PLF	0 PLF	0 PLF	0 PLF	F9	
	Self Weight				11 PLF						

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loade.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation
 LVL beams must not be out or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info





Client:

Weaver Homes

Project:

Address: Gaston II (181035B) Date:

6/24/2020

Page 1 of 1

Marshall Naylor Input by:

Job Name: BM1 Project #:

Kerto-S LVL FB2

1.750" X 24.000"

3-Ply - PASSED

Level: Level

Reactions UNPATTERNED Ib (Uplift)

Cap. React D/L lb

73% 6536 / 5095

51% 4429 / 3676

Total Ld. Case

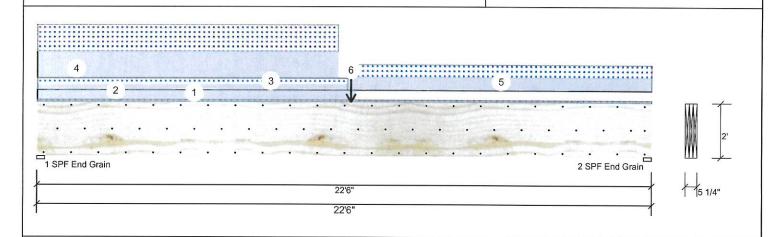
11631 L

8104 L

Ld. Comb.

D+S

D+S



							(-)		
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const
Plies:	3	Design Method:	ASD	1	225	6536	5095	0	0
Moisture Condition:	Dry	Building Code:	IBC 2012	2	225	4429	3676	0	0
Deflection LL:	480	Load Sharing:	Yes					ä	Ü
Deflection TL:	360	Deck:	Not Checked	1					
Importance:	Normal								
Temperature:	Temp <= 100°F								

Bearings Bearing Length

End Grain 2-SPF 3.500"

End Grain

1-SPF 3.500"

Analysis Re	sults					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	65477 ft-lb	11'5 3/4"	131295 ft-lb	0.499 (50%)	D+S	L
Unbraced	65477 ft-lb	11'5 3/4"	65903 ft-lb	0.994 (99%)	D+S	L
Shear	10093 lb	2'2 5/8"	30912 lb	0.327 (33%)	D+S	L
II Define	0.206 (1.44.74)	4414 44 /4 (1)	0.550 (1.1400)	0 440 (440()	_	

LL Defl inch 0.226 (L/1171) 11'1 11/16" 0.552 (L/480) 0.410 (41%) S TL Defl inch 0.501 (L/528) 11' 7/8" 0.735 (L/360) 0.680 (68%) D+S

Design Notes

Member Information

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 3'11 5/8" o.c.
- 6 Bottom braced at bearings.

	/ Lateral siende	rness ratio based on s	ingle ply width.									
	ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	_
	1	Tie-In	0-0-0 to 22-6-0	0-6-0	Far Face	15 PSF	40 PSF	0 PSF	0 PSF	0 PSF	1' Floor	
:	2	Part. Uniform	0-0-0 to 11-7-8		Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall	
,	3	Part. Uniform	0-0-0 to 11-4-0		Near Face	79 PLF	0 PLF	79 PLF	0 PLF	0 PLF	M2	
4	4	Part. Uniform	0-0-0 to 11-0-0		Тор	341 PLF	0 PLF	341 PLF	0 PLF	0 PLF	A2	
!	5	Part. Uniform	11-4-0 to 22-6-0		Near Face	164 PLF	0 PLF	164 PLF	0 PLF	0 PLF	МЗ	
(6	Point	11-5-12		Тор	2293 lb	0 lb	2293 lb	0 lb	0 lb	B2	
		Self Weight				28 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Identifing & Installation

 LVI beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info





Front GDH

Client:

Weaver Homes

Project:

Address:

Gaston II (181035B)

Date:

6/24/2020

Page 1 of 1

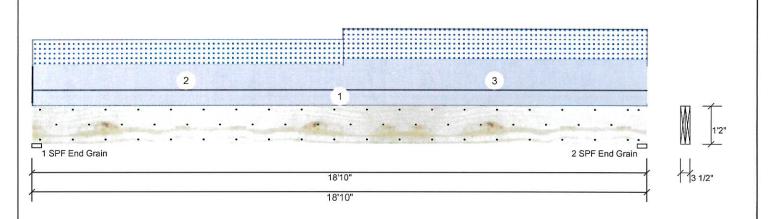
Input by: Christine Shivy Job Name: Front GDH

Project #:

1.750" X 14.000" Kerto-S LVL

2-Ply - PASSED

Level: Level



Member	Information
--------	-------------

Type: Girder Plies: 2

Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360

Importance: Temperature: Normal

Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const	
1	0	1619	951	0	0	
2	0	1720	1053	0	0	
1						

Floor

ASD

No

IBC 2012

Not Checked

Application:

Design Method:

Building Code:

Load Sharing:

Deck:

Analysis Results

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	11981 ft-lb	9'8 7/8"	31049 ft-lb	0.386 (39%)		L
	Unbraced	11981 ft-lb	9'8 7/8"	12012 ft-lb	0.997 (100%)	D+S	L
	Shear	2347 lb	17'5 1/4"	12021 lb	0.195 (20%)	D+S	L
	LL Defl inch	0.181 (L/1218)	9'6 3/16"	0.459 (L/480)	0.390 (39%)	S	L
	TL Defl inch	0.483 (L/457)	9'5 13/16"	0.612 (L/360)	0.790 (79%)	D+S	L

_							
	TL Defl inch	0.483 (L/457)	9'5 13/16"	0.612 (L/360)	0.790 (79%)	D+S	L
	LL Defl inch	0.181 (L/1218)	9'6 3/16"	0.459 (L/480)	0.390 (39%)	S	L
	Shear	2347 lb	17'5 1/4"	12021 lb	0.195 (20%)	D+S	L
	Unbraced	11981 ft-lb	9'8 7/8"	12012 ft-lb	0.997 (100%)	D+S	L
	Moment	11981 ft-lb	9'8 7/8"	31049 ft-lb	0.386 (39%)	D+S	L
	7 ti laiyolo	/ lotudi	Location	/ tilovvca	Capacity	COITID.	Casc

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 8'9" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width

Bearing	s					
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	24%	1619 / 951	2570	L	D+S
2 - SPF End Grain	3.500"	26%	1720 / 1053	2773	L	D+S

/ Luterur Sici	naciness ratio basea (on onigic pry width.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall	
2	Part. Uniform	0-0-0 to 9-6-0		Тор	96 PLF	0 PLF	96 PLF	0 PLF	0 PLF	M2	
3	Part. Uniform	9-6-0 to 18-10-0		Тор	117 PLF	0 PLF	117 PLF	0 PLF	0 PLF	M3	
	Self Weight				11 PLF						

Notes

NOLES
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LANGLING & Installation . LVL beams must not be cut or drilled . Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals . Damaged Beams must not be used . Design assumes top edge is laterally restrained . Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is valid until 2/26/2023

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850

Manufacturer Info

www.metsawood.com/us ICC-ES: ESR-3633





Client:

Weaver Development

Project:

Address:

Gaston II (181035B)

Date:

6/24/2020

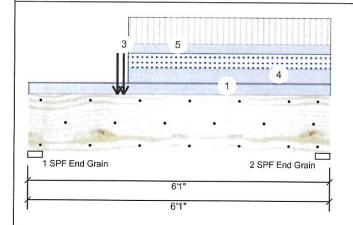
Input by: Christine Shivy Job Name: Window Hdr.

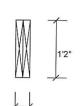
Level: Level

Project #:

Window Hdr. **Kerto-S LVL** 1.750" X 14.000"

2-Ply - PASSED





Const

0

0

0

0

Page 1 of 1

Member Information	n
--------------------	---

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application: Floor Design Method: ASD

Building Code: IBC/IRC 2015 Load Sharing: No

Deck: Not Checked

Reaction	ons UNPAT	TERNED II	(Uplift)		
Brg	Live	Dead	Snow	Wind	-

3110

2098

Bearings

1

2

2560

1638

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	60%	3110 / 3302	6412	L	D+0.75(L+S)
2 - SPF End Grain	3.500"	39%	2098 / 2082	4179	L	D+0.75(L+S)

1843

1137

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10167 ft-lb	1'11"	31049 ft-lb	CHICAGO CONTRACTOR	D+0.75(L+S)	
Unbraced	10167 ft-lb	1'11"	17799 ft-lb	0.571 (57%)	D+0.75(L+S)	L
Shear	5480 lb	1'4 3/4"	10453 lb	0.524 (52%)	D+L	L
LL Defl inch	0.027 (L/2482)	2'5 1/4"	0.141 (L/480)	0.190 (19%)	0.75(L+S)	L
TL Defl inch	0.053 (L/1280)	2'5 11/16"	0.188 (L/360)	0.280 (28%)	D+0.75(L+S)	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width							
			THE WIGHT	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
2	Point	1-9-8		Тор	1005 lb	3014 lb	0 lb	0 lb	0 lb	F8
3	Point	1-11-0		Тор	2335 lb	0 lb	2335 lb	0 lb	0 lb	C3
4	Part. Uniform	2-0-0 to 6-1-0		Тор	158 PLF	0 PLF	158 PLF	0 PLF	0 PLF	C2
5	Part. Uniform	2-0-0 to 6-1-0		Тор	97 PLF	290 PLF	0 PLF	0 PLF	0 PLF	F7
	Self Weight				11 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVL beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC





RE: J0620-2872 Weaver Trenco

818 Soundside Rd Edenton, NC 27932

Site Information:

Customer:

Project Name: J0620-2872

Lot/Block:

Model:

Address:

Subdivision:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2009/TPI2007

Design Program: MiTek 20/20 8.3

Wind Code: ASCE 7-05

Wind Speed: 110 mph

Roof Load: 40.0 psf

Floor Load: N/A psf

This package includes 29 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	E14484697	A1	6/24/2020	21	E14484717	P3	6/24/2020
2	E14484698	A2	6/24/2020	22	E14484718	VB-1	6/24/2020
3	E14484699	A3	6/24/2020	23	E14484719	VB-2	6/24/2020
4	E14484700	A4	6/24/2020	24	E14484720	VB-3	6/24/2020
5	E14484701	A5	6/24/2020	25	E14484721	VB-4	6/24/2020
6	E14484702	B1	6/24/2020	26	E14484722	VB-5	6/24/2020
7	E14484703	B2	6/24/2020	27	E14484723	VC-1	6/24/2020
8	E14484704	C1	6/24/2020	28	E14484724	VC-2	6/24/2020
9	E14484705	C2	6/24/2020	29	E14484725	VC-3	6/24/2020
10	E14484706	C3	6/24/2020				
11	E14484707	D1	6/24/2020				
12	E14484708	D2	6/24/2020				
13	E14484709	M1	6/24/2020				
14	E14484710	M2	6/24/2020				
15	E14484711	M3	6/24/2020				
16	E14484712	M4	6/24/2020				
17	E14484713	M5	6/24/2020				
18	E14484714	M6	6/24/2020				
19	E14484715	P1	6/24/2020				
20	E14484716	P2	6/24/2020				

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

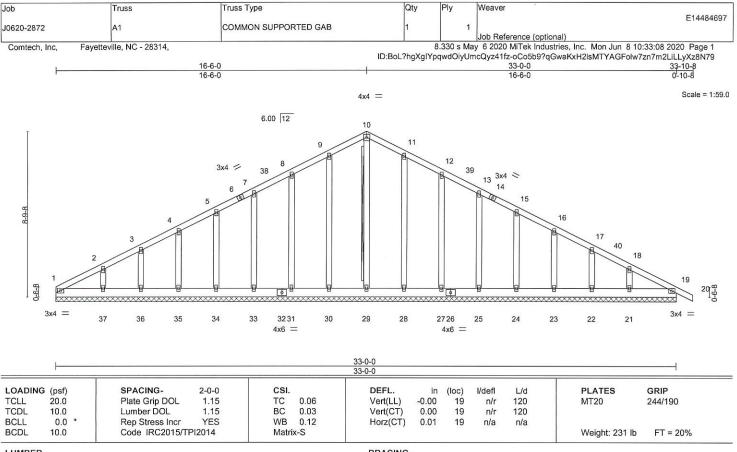
My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



June 24, 2020



LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 OTHERS 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD WEBS Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

T-Brace: 2x4 SPF No.2 - 10-29

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c.,with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 33-0-0.

(lb) - Max Horz 1=-181(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except

37=-106(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 1, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22, 21, 19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 8-9=-89/266, 9-10=-109/319, 10-11=-109/319, 11-12=-89/267

NOTES-

1) Unbalanced roof live loads have been considered for this design.

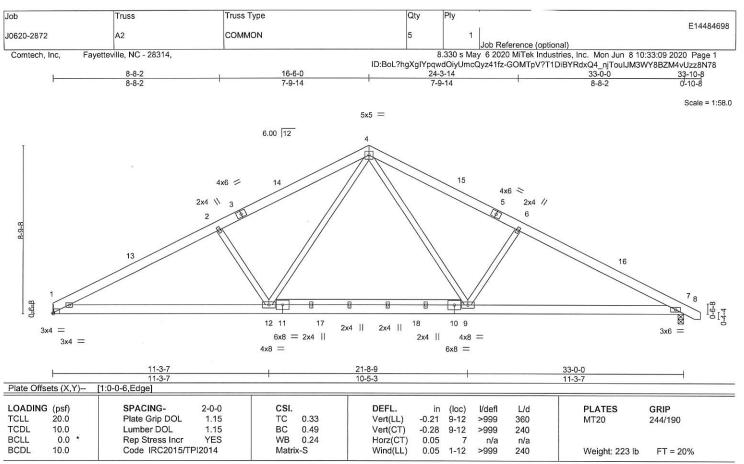
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-0-0 to 4-6-0, Exterior(2) 4-6-0 to 16-6-0, Corner(3) 16-6-0 to 20-10-13, Exterior(2) 20-10-13 to 33-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=106.
 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucallapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/PH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1

2x4 SP No.2 WEBS

REACTIONS. (size) 1=Mechanical, 7=0-3-8

Max Horz 1=-113(LC 8)

Max Uplift 1=-79(LC 12), 7=-91(LC 13) Max Grav 1=1311(LC 1), 7=1364(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-2297/531, 2-4=-2064/545, 4-6=-2055/529, 6-7=-2283/512 TOP CHORD

1-12=-347/2015, 9-12=-110/1324, 7-9=-344/1965 **BOT CHORD**

WEBS 4-9=-142/850, 6-9=-495/297, 4-12=-145/865, 2-12=-509/305

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 33-8-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-8-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

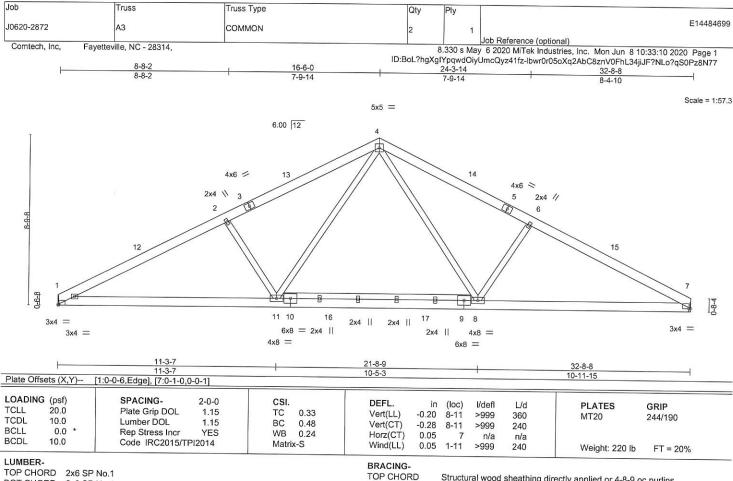


🛝 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design, Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BOT CHORD

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1

WEBS 2x4 SP No.2

REACTIONS. (size) 1=Mechanical, 7=Mechanical Max Horz 1=-108(LC 8)

Max Uplift 1=-79(LC 12), 7=-78(LC 13)

Max Grav 1=1303(LC 1), 7=1303(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-2280/528, 2-4=-2048/543, 4-6=-2021/538, 6-7=-2242/521 TOP CHORD

BOT CHORD 1-11=-354/1997, 8-11=-116/1306, 7-8=-347/1920

WEBS 4-8=-139/822, 6-8=-473/295, 4-11=-145/866, 2-11=-509/305

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 32-7-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-8-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

June 8,2020

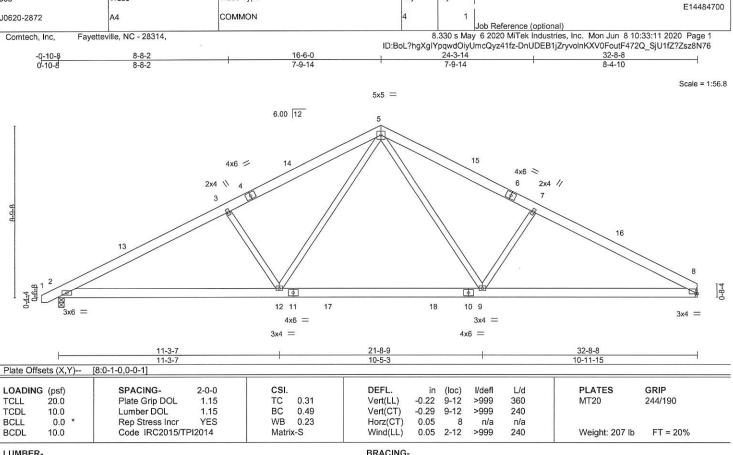
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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TOP CHORD

BOT CHORD

Qty

LUMBER-

Job

TOP CHORD 2x6 SP No.1 2x6 SP No.1

BOT CHORD 2x4 SP No.2 WEBS

REACTIONS.

(size) 2=0-3-8, 8=Mechanical

Max Horz 2=113(LC 9)

Truss

Truss Type

Max Uplift 2=-91(LC 12), 8=-78(LC 13) Max Grav 2=1352(LC 1), 8=1299(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2261/507, 3-5=-2051/524, 5-7=-2034/536, 7-8=-2238/519 TOP CHORD

BOT CHORD 2-12=-347/1992, 9-12=-113/1312, 8-9=-341/1925

5-9=-137/833, 7-9=-473/294, 5-12=-141/861, 3-12=-495/297

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 32-7-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



Structural wood sheathing directly applied or 4-9-10 oc purlins.

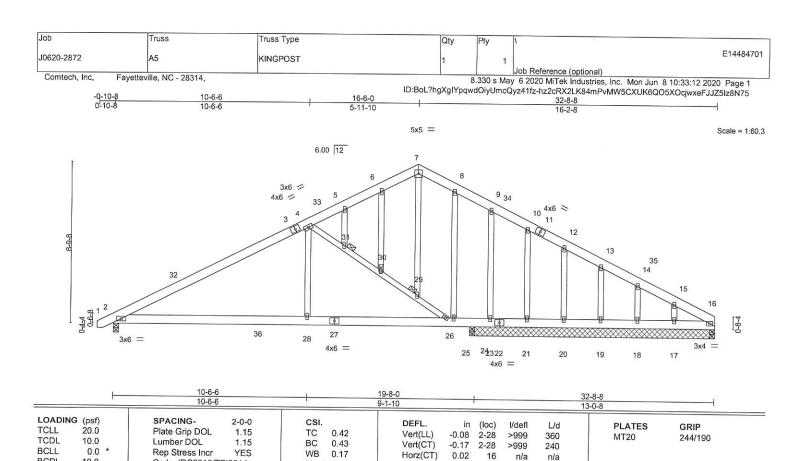
Rigid ceiling directly applied or 10-0-0 oc bracing.



🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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LUMBER-

BCDL

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1 **WEBS** 2x4 SP No.2

10 0

Wind(LL) BRACING- 0.07 2-28

TOP CHORD **BOT CHORD** JOINTS

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Weight: 246 lb

FT = 20%

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Brace at Jt(s): 29, 30, 31

>999

REACTIONS.

All bearings 13-4-0 except (jt=length) 2=0-3-8, 16=0-3-8, 16=0-3-8, 24=0-3-8,

(lb) -Max Horz 2=180(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 21, 20, 19, 18 except 2=-192(LC 12), 23=-491(LC 19),

17=-114(LC 13), 24=-297(LC 12)

Code IRC2015/TPI2014

Max Grav All reactions 250 lb or less at joint(s) 23, 21, 20, 19, 18, 17, 16, 16 except 2=848(LC 1), 24=1302(LC

Matrix-S

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-4=-1128/201, 4-5=-292/128, 14-15=-251/0, 15-16=-294/21

BOT CHORD

2-28=-203/943, 26-28=-202/948, 25-26=-17/266, 24-25=-17/266, 23-24=-17/266, 21-23=-17/266, 20-21=-17/266, 19-20=-17/266, 18-19=-17/266, 17-18=-17/266,

WEBS

4-31=-927/384, 30-31=-905/365, 29-30=-923/382, 26-29=-932/381, 4-28=0/455,

9-23=-263/134

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 32-6-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 21, 20, 19, 18 except (jt=lb) 2=192, 23=491, 17=114, 24=297.



June 8,2020

👠 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE

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ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Ro

Job Truss Truss Type Qty E14484702 COMMON SUPPORTED GAB J0620-2872 B1 Job Reference (optional) Fayetteville, NC - 28314, 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:13 2020 Page 1 Comtech, Inc, $ID: BoL? hgXgIYpqwdOiyUmcQyz41fz-99c_es3z5SCd13xifv2jtJzf1xqFSPxnUz26dkz8N74\\$ 5-7-0 Scale = 1:40.1 5x5 =

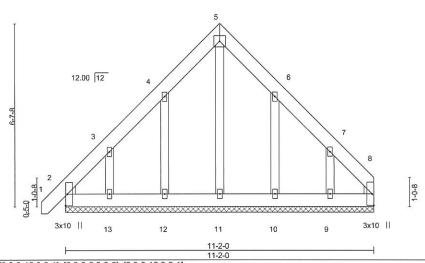


Plate Offsets (X,Y)--[2:0-0-6,0-0-6], [2:0-0-12,0-3-1], [8:0-0-6,0-0-6], [8:0-0-12,0-3-1] SPACING-CSL DEFL. L/d PLATES GRIP LOADING (psf) 2-0-0 (loc) I/defl Plate Grip DOL 0.03 -0.00 244/190 TCLL 20.0 1.15 TC Vert(LL) n/r 120 MT20 BC -0.00 TCDL 10.0 Lumber DOL 1 15 0.02 Vert(CT) n/r 120 BCLL 0.0 Rep Stress Incr YES WB 0.06 Horz(CT) 0.00 8 n/a n/a Code IRC2015/TPI2014 Weight: 93 lb FT = 20%**BCDI** 10.0 Matrix-S

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD 2x4 SP No.2 **OTHERS**

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. All bearings 11-2-0.

(lb) - Max Horz 2=187(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8 except 12=-130(LC 12), 13=-189(LC 12), 10=-126(LC 13),

9=-194(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 11, 12, 13, 10, 9, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-9-2 to 3-6-13, Exterior(2) 3-6-13 to 5-7-0, Corner(3) 5-7-0 to 9-11-13, Exterior(2) 9-11-13 to 11-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8 except (jt=lb) 12=130, 13=189, 10=126, 9=194
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

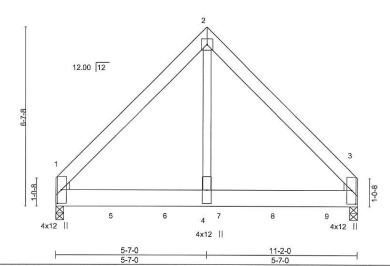
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty E14484703 B2 COMMON GIRDER J0620-2872 2 Job Reference (optional) Comtech, Inc. Fayetteville, NC - 28314, 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:14 2020 Page 1 ID: BoL? hgXgIYpqwdOiyUmcQyz41fz-dM9MsC3bsmKUfCWvCdayQXVkxK1PBk0wjdog9Bz8N735-7-0 5-7-0 5x5 || Scale = 1:41.0



LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.40	Vert(LL)	-0.03	3-4	>999	360	MT20	244/190
CDL	10.0	Lumber DOL	1.15	BC	0.60	Vert(CT)	-0.06	3-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.52	Horz(CT)	0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-S	Wind(LL)	0.02	3-4	>999	240	Weight: 165 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x8 SP No.1 WEBS 2x4 SP No.2

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 3=0-3-8

Max Horz 1=-144(LC 23)

Max Uplift 1=-301(LC 9), 3=-265(LC 8)

Max Grav 1=4586(LC 1), 3=3979(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-3212/267, 2-3=-3207/266

BOT CHORD 1-4=-137/2118, 3-4=-137/2118 WEBS 2-4=-239/4237

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-6-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (|t=|b| 1=301, 3=265.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1293 lb down and 88 lb up at 0-1-12, 1283 lb down and 98 lb up at 2-2-0, 1279 lb down and 98 lb up at 4-2-0, 1279 lb down and 98 lb up at 6-2-0, and 1279 lb down and 98 lb up at 8-2-0, and 1281 lb down and 96 lb up at 10-2-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

M WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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Job	Truss	Truss Type	Qty	Plv		
J0620-2872	B2	COMMON GIRDER	1	2	Job Reference (optional)	E14484703

Comtech, Inc,

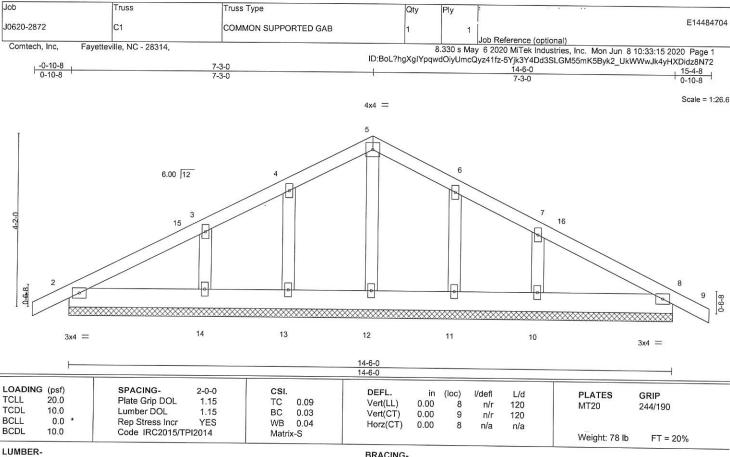
Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTesk Industries, Inc. Mon Jun 8 10:33:14 2020 Page 2 ID:BoL?hgXglYpqwdOiyUmcQyz41fz-dM9MsC3bsmKUfCWvCdayQXVkxK1PBk0wjdog9Bz8N73

LOAD CASE(S) Standard

Uniform Loads (plf)
Vert: 1-2-60, 2-3=-60, 1-3=-20
Concentrated Loads (lb)
Vert: 1=-1293(B) 5=-1283(B) 6=-1279(B) 7=-1279(B) 8=-1279(B) 9=-1281(B)





TOP CHORD

2x4 SP No.1 **BOT CHORD** 2x6 SP No.1

OTHERS 2x4 SP No.2 BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 14-6-0

(lb) - Max Horz 2=-82(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 11 except 14=-112(LC 12), 10=-112(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 11 except 14=250(LC 1), 10=250(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-3-0, Exterior(2) 3-3-0 to 7-3-0, Corner(3) 7-3-0 to 11-7-13, Exterior(2) 11-7-13 to 15-4-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 11 except (it=lb) 14=112, 10=112,



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Job Truss Truss Type Qty Ply F14484705 COMMON C2 J0620-2872 Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:16 2020 Page 1 ID: BoL? hgXgIYpqwdOiyUmcQyz41 fz-ZkH6 Hu5sONaCuWgHK2cQVyb4o8pWflLDAxHmE3z8N71-0-10-8 0-10-8 14-6-0 0-10-8 Scale = 1:26.6 4x6 =3 6.00 12 10 6 2x4 || 3x6 = 3x6 > 14-6-0 Plate Offsets (X,Y)--[2:0-0-12,0-1-8], [4:0-0-12,0-1-8] LOADING (psf) SPACING-2-0-0 CSI. DEFL L/d (loc) I/defl **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.44 Vert(LL) -0.02 2-6 244/190 >999 360 MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.24 Vert(CT) -0.05 2-6 >999 240 BCLL 0.0 Rep Stress Incr YES WB 0.08 Horz(CT) 0.01 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Wind(LL) 0.02 2-6 >999 240 Weight: 66 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 **BOT CHORD** 2x6 SP No.1

WEBS 2x4 SP No.2

REACTIONS.

(size) 2=0-3-8, 4=0-3-8

Max Horz 2=-53(LC 10)

Max Uplift 2=-48(LC 12), 4=-48(LC 13) Max Grav 2=630(LC 1), 4=630(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-803/237, 3-4=-803/237 **BOT CHORD** 2-6=-75/617, 4-6=-75/617

WEBS 3-6=0/366

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 7-3-0, Exterior(2) 7-3-0 to 11-7-13, Interior(1) 11-7-13 to 15-4-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

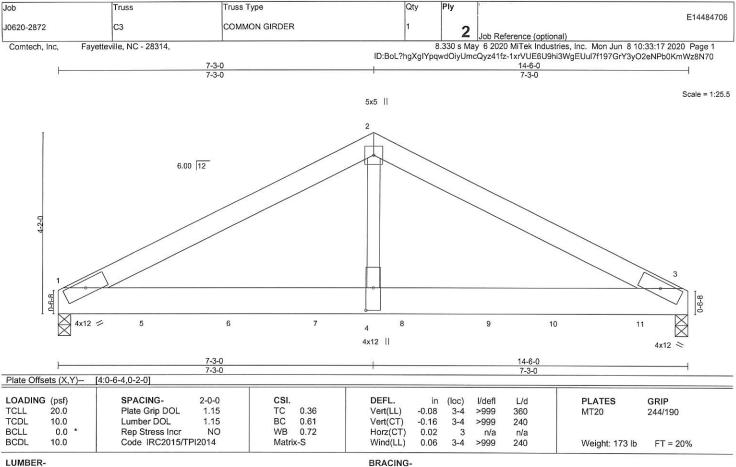
🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property amanae. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

BOT CHORD

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x8 SP 2400F 2.0E

WEBS 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 3=0-3-8 Max Horz 1=-47(LC 25)

Max Uplift 1=-324(LC 8), 3=-382(LC 9)

Max Grav 1=4670(LC 1), 3=5490(LC 1)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-7127/503, 2-3=-7121/502

BOT CHORD 1-4=-396/6308, 3-4=-396/6308

WEBS 2-4=-316/5855

NOTES-

FORCES.

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-7-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=324, 3=382.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1291 lb down and 99 lb up at 2-0-12, 1291 lb down and 99 lb up at 4-0-12, 1291 lb down and 99 lb up at 6-0-12, 1291 lb down and 99 lb up at 8-0-12, 1291 lb down and 99 lb up at 10-0-12, and 1283 lb down and 99 lb up at 11-6-0, and 1285 lb down and 98 lb up at 13-6-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 1-3=-20



Structural wood sheathing directly applied or 5-10-10 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	E14484706
J0620-2872	С3	COMMON GIRDER	1	2	Job Reference (optional)

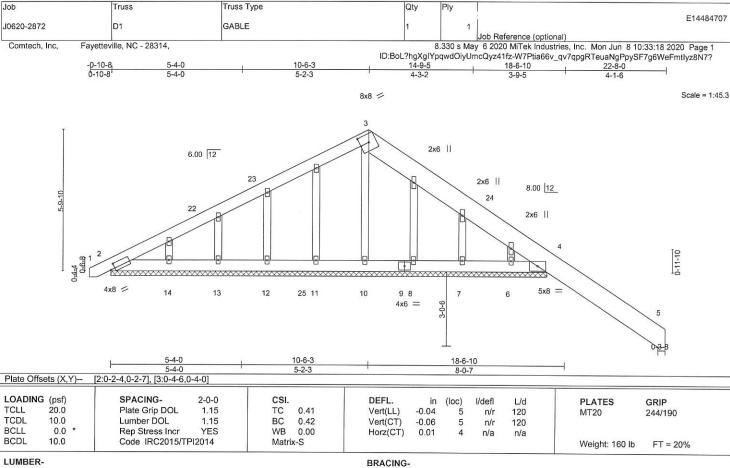
Fayetteville, NC - 28314, Comtech, Inc,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:17 2020 Page 2 ID:BoL?hgXglYpqwdOiyUmcQyz41fz-1xrVUE6U9hi3WgEUul7f197GrY3yO2eNPb0KmWz8N70

LOAD CASE(S) Standard Concentrated Loads (lb)

Vert: 5=-1291(F) 6=-1291(F) 7=-1291(F) 8=-1291(F) 9=-1291(F) 10=-1283(F) 11=-1285(F)





TOP CHORD

BOT CHORD

2x6 SP No.1 *Except* TOP CHORD

3-5: 2x10 SP No.1

BOT CHORD 2x6 SP No.1

OTHERS 2x4 SP No.2

REACTIONS. All bearings 17-9-15.

(lb) - Max Horz 2=-218(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 13, 2 except 4=-430(LC 13), 14=-259(LC 12), 6=-139(LC 1)

Max Grav All reactions 250 lb or less at joint(s) 10, 11, 12, 13, 8, 7, 6 except 4=979(LC 1), 14=423(LC 23),

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-587/244, 3-4=-627/277

BOT CHORD 2-14=-80/413, 13-14=-80/413, 12-13=-80/413, 11-12=-80/413, 10-11=-80/413,

8-10=-80/413, 7-8=-80/413, 6-7=-80/413, 4-6=-80/413

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 10-6-3, Exterior(2) 10-6-3 to 14-11-0, Interior(1) 14-11-0 to 22-6-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13, 2 except (jt=lb) 4=430, 14=259, 6=139.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

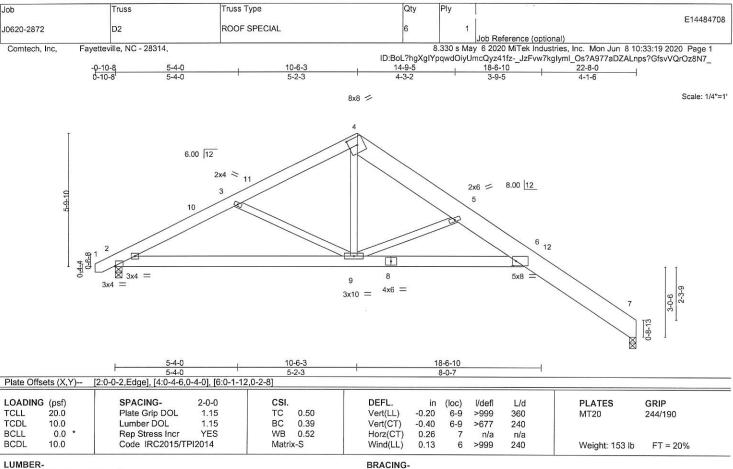
🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property anage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

BOT CHORD

LUMBER-

2x6 SP No.1 *Except* TOP CHORD

4-7: 2x10 SP 2400F 2.0E

BOT CHORD 2x6 SP No.1

2x4 SP No.2 **WEBS**

REACTIONS. (size) 7=0-3-8, 2=0-3-8

Max Horz 2=176(LC 11)

Max Uplift 7=-57(LC 13), 2=-58(LC 12)

Max Grav 7=909(LC 1), 2=949(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1528/386, 3-4=-1206/274, 4-5=-1286/301, 5-6=-2001/430, 6-7=-452/169 **BOT CHORD** 2-9=-189/1315, 6-9=-273/2152

WEBS 3-9=-349/230, 4-9=-109/874, 5-9=-1256/333

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 10-6-3, Exterior(2) 10-6-3 to 15-1-11, Interior(1) 15-1-11 to 22-6-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

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ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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lob		Truss	Truss Type	Qty	Ply			E14484709
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			4-6-8	ID.Boc:rigAg	. pqwaoiyom	8-4-0	10 (0. 1) <u>20 2</u> 1g	_
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	0-10-8		4-0-0					
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LOADING (psf	1	SPACING- 2-0-	o csi.	DEFL.	in (loc)	I/defl L/d	PLATES GRIP	
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TCLL 20.0 TCDL 10.0		Lumber DOL 1.1			0.00 1	n/r 120		
	o *	Rep Stress Incr YES			0.00	n/a n/a		
		Code IRC2015/TPI2014		(5.1)		covered to tempo.	Weight: 40 lb FT	= 20%
BCDL 10.0	,	Code IRC2013/11/12014	Widdix				9	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD

2x6 SP No.1 2x6 SP No.1

WEBS 2x4 SP No.2 **OTHERS** REACTIONS.

ONS. All bearings 8-2-8.
(lb) - Max Horz 2=109(LC 8) Max Uplift All uplift 100 lb or less at joint(s) 6, 2, 7 except 8=-107(LC 12) Max Grav All reactions 250 lb or less at joint(s) 6, 2, 7 except 8=344(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

3-8=-255/218 **WEBS**

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 8-1-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2, 7 except (jt=lb) 8=107.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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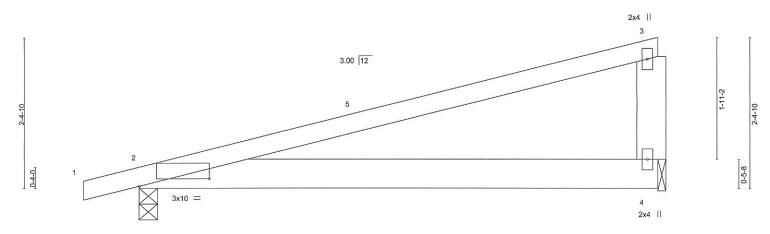
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Job	Truss	Truss Type	Qty	Ply	E14484710
J0620-2872	M2	MONOPITCH	5	1	E14464710
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		TO ACCURACY OF BOOK TO CONTROL SUPER TO		1.0	Job Reference (optional)
Comtech, Inc, Faye	tteville, NC - 28314,				y 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:20 2020 Page 1
		j	D:BoL?hgXgIY	pqwdOiyUr	mcQyz41fz-SWXd6G8MRc4dN7z3ZtgMfoldtl98bacp5ZF_Mqz8N6z
-0-1	0-8	4-6-8		ALL CONTROL OF THE PROPERTY OF THE PARTY OF	8-4-0
0-1	0-8	4-6-8	1		3-9-8

Scale = 1:17.5



		1				8-4-0							
Plate Off	sets (X,Y)	[2:1-1-4,0-1-7]											_
LOADIN	G (psf)	SPACING- 2-	-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL 1	1.15	TC	0.95	Vert(LL)	-0.05	2-4	>999	360	MT20	244/190	
TCDL	10.0	Lumber DOL 1	1.15	BC	0.26	Vert(CT)	-0.11	2-4	>880	240			
BCLL	0.0 *	Rep Stress Incr	'ES	WB	0.00	Horz(CT)	0.00		n/a	n/a			
BCDL	10.0	Code IRC2015/TPI20	14	Matri	x-P	Wind(LL)	0.00	2	****	240	Weight: 37 lb	FT = 20%	

8-4-0

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1

BOT CHORD 2x6 SP No.1

2x6 SP No.1 **WEBS**

REACTIONS. (size) 2=0-3-8, 4=0-1-8

Max Horz 2=77(LC 8)

Max Uplift 2=-66(LC 8), 4=-41(LC 12)

Max Grav 2=384(LC 1), 4=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 8-1-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Structural wood sheathing directly applied or 2-2-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

June 8,2020



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 10/03/2015 BEFORE USE.

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Job	Truss	Truss Type	Qty	Ply	E14484711
J0620-2872	мз	ROOF SPECIAL	6	1	Job Reference (optional)
Comtech, Inc,	Fayetteville, NC - 28314,		ID:BoL?hgXgIY	8.330 s Ma pqwdOiyUm	ay 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:21 2020 Page 1 cQyz41fz-wi4?Kc9_CvCU_HYF7bCbC?IrG9M?K1syKD_XuHz8N6y
	-0-10-8		7-0-0		
	0-10-8		7-0-0		,

Scale = 1:17.1

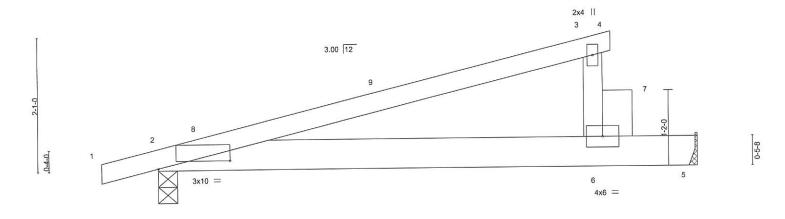


Plate Offsets (X,Y)	2:1-1-4,0-1-7]		T						
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15	CSI. TC 0.7 BC 0.8	in -0.11 -0.26	(loc) 2-6 2-6	I/defl >884 >373	L/d 360 240	PLATES MT20	GRIP 244/190
3CLL 0.0 *	Rep Stress Incr Code IRC2015/TR	NO	WB 0.0 Matrix-P	0.00 0.17	5 2-6	n/a >569	n/a 240	Weight: 35 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 2x6 SP No.1

BOT CHORD 2x4 SP No.2 *Except* **WEBS**

6-7: 2x6 SP No.1

BRACING-

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, TOP CHORD

except end verticals

Rigid ceiling directly applied or 7-8-6 oc bracing.

REACTIONS. (size) 2=0-3-8, 5=Mechanical

Max Horz 2=68(LC 8)

Max Uplift 2=-75(LC 8), 5=-58(LC 12) Max Grav 2=468(LC 1), 5=656(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 7-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 500 lb down and 265 lb up at 7-1-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-20, 2-5=-20

Concentrated Loads (lb) Vert: 6=-500

SEAL 036322 A. GILBERT

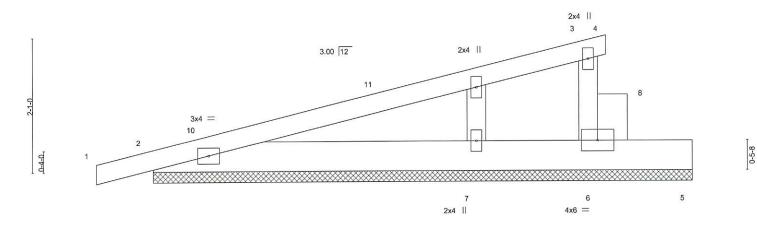
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type		Qty	Ply	E14484712
J0620-2872	M4	GABLE		1	1	Job Reference (optional)
Comtech, Inc,	Fayetteville, NC - 28314,		ID:B	8 L?hgXgI\	3.330 s Ma /pqwdOiyU	y 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:22 2020 Page 1 JmcQyz41fz-OueOXxAczDKLcR7RgljqkDr1sZug3U66Ztk5Qjz8N6x
20	-0-10-8		7-0-0		3800 00 000 000 000	
-	0-10-8		7-0-0			1

Scale = 1:17.1



8-4-0												
LOADING TCLL TCDL	20.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.66 0.06 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 0.01 0.00	(loc) 4 4	I/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0 * 10.0	Code IRC2015/TI		Matri		11012(01)	0.00	J	11/4	1110	Weight: 37 lb	FT = 20%

8-4-0

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 **BOT CHORD** 2x6 SP No.1

WEBS 2x4 SP No.2 *Except* 6-8: 2x6 SP No.1

OTHERS 2x4 SP No.2

REACTIONS. (size) 6=8-4-0, 2=8-4-0, 7=8-4-0

Max Horz 2=96(LC 8) Max Uplift 6=-109(LC 12), 2=-133(LC 8)

Max Grav 6=235(LC 1), 2=303(LC 1), 7=181(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 7-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=109, 2=133.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



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Truss Type Qty Job Truss E14484713 M5 GABLE J0620-2872 Job Reference (optional)

Comtech, Inc,

Fayetteville, NC - 28314,

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:22 2020 Page 1 ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-OueOXxAczDKLcR7RgIjqkDr5_ZvK3UC6Ztk5Qjz8N6x

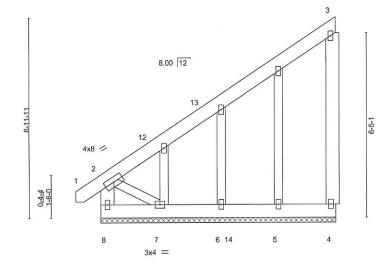
Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

0-10-8 0-10-8

Scale = 1:38.6



LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.40 BC 0.02 WB 0.06 Matrix-P	DEFL. in Vert(LL) -0.02 Vert(CT) 0.01 Horz(CT) -0.00	(loc) 1 1 4	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 79 lb	GRIP 244/190 FT = 20%
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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1

WEBS

2x6 SP No.1 *Except* 2-7: 2x4 SP No.2

OTHERS 2x4 SP No.2

REACTIONS.

All bearings 8-2-8. (lb) - Max Horz 8=271(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) except 4=-206(LC 12), 7=-142(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 5, 6, 7 except 4=311(LC 19), 8=311(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

3-4=-301/226, 2-8=-298/0 TOP CHORD

BOT CHORD 7-8=-280/225 **WEBS** 2-7=-265/330

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-12 to 3-8-1, Interior(1) 3-8-1 to 8-1-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 206 lb uplift at joint 4 and 142 lb uplift at joint 7.



June 8,2020



🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucollapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Qty Truss Type Job Truss E14484714 MONOPITCH M6 J0620-2872 Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:23 2020 Page 1 Fayetteville, NC - 28314, Comtech, Inc, ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-s5CmlHAFkXTCEbidE0E3HQNLXzD5ovxFnXTez9z8N6w 3-10-0 0-10-8 Scale = 1:39.3 2x4 || 8.00 12 2x4 N 3 6-6-3 2x6 || 1-6-0 -5-8 \bigotimes 5 4x6 = 2x6 || GRIP DEFL. I/defl L/d **PLATES** LOADING (psf) SPACING-2-0-0 CSI. in (loc) 244/190 Vert(LL) -0.03 5-6 >999 360 MT20 TC BC 0.09 20.0 Plate Grip DOL 1.15 TCLL 0.18 Vert(CT) -0.06 5-6 >999 240 Lumber DOL 1.15 TCDL 10.0 Horz(CT) -0.00 n/a n/a WB 0.16 Rep Stress Incr YES BCLL 0.0

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

0.07

5-6 >999

except end verticals.

240

Rigid ceiling directly applied or 10-0-0 oc bracing

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Weight: 69 lb

FT = 20%

LUMBER-

BCDL

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1 2x6 SP No.1 *Except* **WEBS**

3-5: 2x4 SP No.2

(size) 6=0-3-0, 5=0-1-8 REACTIONS.

Max Horz 6=178(LC 12)

Max Uplift 6=-27(LC 9), 5=-143(LC 9) Max Grav 6=376(LC 1), 5=310(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code IRC2015/TPI2014

2-3=-268/39, 2-6=-290/86 TOP CHORD

5-6=-250/259 **BOT CHORD** 3-5=-313/285 **WEBS**

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-12 to 3-7-11, Interior(1) 3-7-11 to 8-1-0 zone; end vertical left exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-S

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 6 and 143 lb uplift at joint 5.





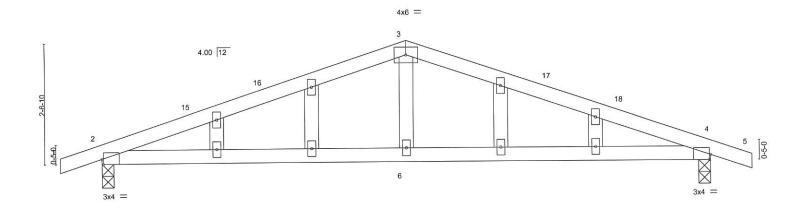
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTeke onnectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucollapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Qty Truss Truss Type Job E14484715 GABLE J0620-2872 Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:24 2020 Page 1 Fayetteville, NC - 28314, Comtech, Inc. ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-KHm8ydBtVqb3rlHqojIIpewRjMWmXNXP0BDBVcz8N6v 12-10-0 -0-10-8 0-10-8

Scale = 1:23.4



<u> </u>	6-5-0 6-5-0						6-5-0		
Plate Offsets (X,Y)	[2:0-0-5,Edge], [4:0-0-5,Edge]							I	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.44 BC 0.34 WB 0.07 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.11 -0.09 0.01	(loc) 4-6 2-6 4	l/defl >999 >999 n/a	L/d 240 240 n/a	PLATES MT20 Weight: 51 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1

BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2 **OTHERS**

REACTIONS.

(size) 2=0-3-0, 4=0-3-0

Max Horz 2=-49(LC 17)

Max Uplift 2=-309(LC 8), 4=-309(LC 9) Max Grav 2=563(LC 1), 4=563(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-910/965, 3-4=-910/965 **BOT CHORD** 2-6=-828/799, 4-6=-828/799

WEBS 3-6=-383/303

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 1) Uniodantice from the loads have been considered for time design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 6-5-0, Exterior(2) 6-5-0 to 10-9-13, Interior(1) 10-9-13 to 13-8-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 309 lb uplift at joint 2 and 309 lb uplift at joint 4.



Structural wood sheathing directly applied or 5-9-9 oc purlins.

Rigid ceiling directly applied or 6-6-10 oc bracing.

June 8,2020

👠 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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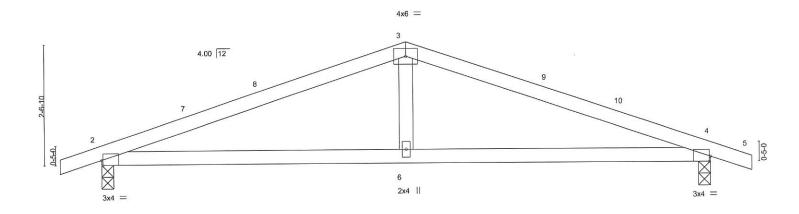
ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply		E14484716
J0620-2872	P2	COMMON	2	1	Job Reference (optional)	2000 200000100.0075
Comtech, Inc,	Fayetteville, NC - 28314,		ID:BoL?hgXgIYpo	8.330 s May	y 6 2020 MiTek Industries, Inc. Mon Jun 8 Qyz41fz-oTKWAzCVG8jwTvs0MRGXMrSc	Tmr?GqnYFryl12z8N6u
0-10-8		6-5-0	- A	\$53	12-10-0	13-8-8
0-10-8		6-5-0			6-5-0	0-10-8

Scale = 1:23.4



6-5-0 6-5-0					12-10-0 6-5-0						
Plate Offsets (X,Y)	[2:0-0-5,Edge], [4:0-0-5,Ed										
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI2	2-0-0 1.15 1.15 YES 2014	CSI. TC BC WB Matrix	0.44 0.34 0.07	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.11 -0.09 0.01	(loc) 4-6 2-6 4	l/defl >999 >999 n/a	L/d 240 240 n/a	PLATES MT20 Weight: 45 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 **BOT CHORD** 2x4 SP No.1

2x4 SP No.2 **WEBS**

REACTIONS.

(size) 2=0-3-0, 4=0-3-0

Max Horz 2=-29(LC 13) Max Uplift 2=-217(LC 8), 4=-217(LC 9) Max Grav 2=563(LC 1), 4=563(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-910/965, 3-4=-910/965 TOP CHORD

BOT CHORD 2-6=-828/799, 4-6=-828/799

3-6=-383/303 **WEBS**

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 6-5-0, Exterior(2) 6-5-0 to 10-9-13, Interior(1) 10-9-13 to 13-8-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 2 and 217 lb uplift at joint 4.



Structural wood sheathing directly applied or 5-9-9 oc purlins.

Rigid ceiling directly applied or 6-6-10 oc bracing.

June 8,2020



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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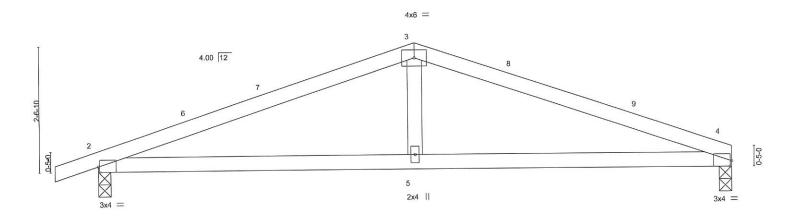
ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty Ply E1448471					
J0620-2872	P3	соммон	3 1 Job Reference (optional)					
Comtech, Inc, Fa	ayetteville, NC - 28314,		8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:26 2020 Page ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-HguuNJD71Srn52QCv8nmv3?mfABB?G1hTViIZUz8N6					
-0-10-8 0-10-8		6-5-0 6-5-0	12-10-0 6-5-0					

Scale = 1:22.5



6-5-0 6-5-0					12-10-0 6-5-0					
Plate Offsets (X,Y)	[2:0-0-5,Edge], [4:0-0-5,Edge]							1		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	TC 0.48 BC 0.35	/ert(CT) -	in (0.11 0.09 0.01	(loc) 4-5 4-5 4	I/defl >999 >999 n/a	L/d 240 240 n/a	PLATES MT20 Weight: 44 lb	GRIP 244/190 FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1

BOT CHORD 2x4 SP No.1

2x4 SP No.2

REACTIONS. (size) 4=0-3-0, 2=0-3-0

Max Horz 2=32(LC 12) Max Uplift 4=-179(LC 9), 2=-217(LC 8) Max Grav 4=501(LC 1), 2=566(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-918/984, 3-4=-916/993 2-5=-866/807, 4-5=-866/807 TOP CHORD BOT CHORD

3-5=-386/305 **WEBS**

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 6-5-0, Exterior(2) 6-5-0 to 10-9-13, Interior(1) 10-9-13 to 12-8-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint 4 and 217 lb uplift at joint 2.



Structural wood sheathing directly applied or 5-7-10 oc purlins.

Rigid ceiling directly applied or 6-4-14 oc bracing.

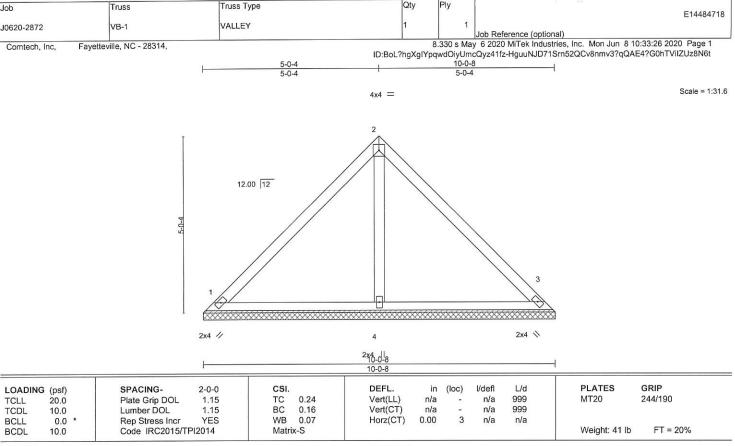
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Qty

LUMBER-

.lob

TOP CHORD BOT CHORD 2x4 SP No.1 2x4 SP No.1 **OTHERS** 2x4 SP No.2

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=10-0-8, 3=10-0-8, 4=10-0-8

Max Horz 1=112(LC 9) Max Uplift 1=-28(LC 13), 3=-28(LC 13)

Max Grav 1=212(LC 1), 3=212(LC 1), 4=324(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 1 and 28 lb uplift at ioint 3.



June 8,2020



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Truss Type Qty Job Truss F14484719 VB-2 VALLEY J0620-2872 Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:27 2020 Page 1 Fayetteville, NC - 28314, Comtech, Inc. ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-lsSHafDlolzejC?PTsI?RGY?MabHkkqri9Rs6wz8N6s 4-0-4 Scale = 1:26.2 4x4 = 2 12.00 12 2x4 || 4 2x4 \ 2x4 // SPACING-CSI. DEFL. I/defl L/d **PLATES** GRIP LOADING (psf) 2-0-0 in (loc) 20.0 Plate Grip DOL 1.15 TC 0.23 Vert(LL) n/a n/a 999 MT20 244/190 TCLL 1.15 BC 0.10 Vert(CT) n/a n/a 999 TCDL 10.0 Lumber DOL Rep Stress Incr YES WB 0.03 Horz(CT) 0.00 3 n/a n/a **BCLL** 0.0 Code IRC2015/TPI2014 Matrix-P Weight: 33 lb FT = 20% BCDL 10.0 **BRACING-**LUMBER-

2x4 SP No.1 2x4 SP No.1 TOP CHORD **BOT CHORD OTHERS** 2x4 SP No.2 TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=8-0-8, 3=8-0-8, 4=8-0-8

Max Horz 1=-88(LC 10)

Max Uplift 1=-32(LC 13), 3=-32(LC 13)

Max Grav 1=179(LC 1), 3=179(LC 1), 4=230(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 32 lb uplift at joint 1 and 32 lb uplift at joint 3.





🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/PH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Qty Ply Truss Truss Type Job E14484720 VALLEY J0620-2872 VB-3 Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:28 2020 Page 1 Fayetteville, NC - 28314, Comtech, Inc. ID:BoL?hgXglYpqwdOiyUmcQyz41fz-D20fo?ENZ35VKMab1ZqE_U4Cq_yFTBM_xpBPeNz8N6r 3-0-4 Scale = 1:21.0 4x4 = 12.00 12 2x4 \ 2x4 // 2x4 || (loc) I/defl L/d **PLATES** GRIP CSI. DEFL. in LOADING (psf) SPACING-2-0-0 244/190 999 MT20 TC 0.12 Vert(LL) n/a n/a Plate Grip DOL 1.15 20.0 TCLL BC Vert(CT) n/a n/a 999 Lumber DOL 1.15 0.05 TCDL 10.0 WB Horz(CT) 0.00 3 0.02 n/a YES BCLL 0.0 Rep Stress Incr Weight: 24 lb FT = 20%Code IRC2015/TPI2014 BCDL 10.0

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 **OTHERS**

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=6-0-8, 3=6-0-8, 4=6-0-8

Max Horz 1=64(LC 9)

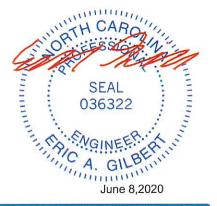
Max Uplift 1=-23(LC 13), 3=-23(LC 13)

Max Grav 1=130(LC 1), 3=130(LC 1), 4=167(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

- 7) Onio and the first of the grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 1 and 23 lb uplift at joint 3.

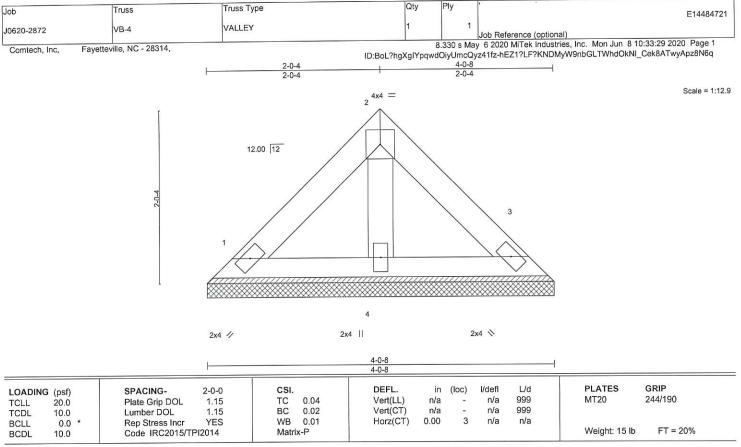




🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters have received the second of the seco





LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 **BOT CHORD** 2x4 SP No.2 **OTHERS**

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 4-0-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=4-0-8, 3=4-0-8, 4=4-0-8

Max Horz 1=-40(LC 10)

Max Uplift 1=-14(LC 13), 3=-14(LC 13) Max Grav 1=81(LC 1), 3=81(LC 1), 4=104(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 1 and 14 lb uplift at joint 3.





Design valid for use only with MITe® connectors. This design is based only upon parameters have represented by the second of the second parameters and individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/THI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932

Qty Truss Truss Type Job E14484722 VALLEY J0620-2872 VB-5 Job Reference (optional) 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 10:33:30 2020 Page 1 Fayetteville, NC - 28314, Comtech, Inc. ID:BoL?hgXgIYpqwdOiyUmcQyz41fz-9R7PDhGe5gLDagk_8_si3vAZ1neKx54HO7gWiFz8N6p 1-0-4 1-0-4 Scale = 1:7.9 3x4 = 12.00 12 3 2x4 // 2x4 \ Plate Offsets (X,Y)--[2:0-2-0,Edge] **PLATES** DEFL. I/defl L/d LOADING (psf) CSI. in (loc) SPACING-2-0-0 MT20 244/190 Plate Grip DOL TC 0.01 Vert(LL) n/a n/a 999 TCLL 20.0 1.15 BC 0.01 Vert(CT) n/a n/a 999 1.15 TCDL 10.0 Lumber DOL WB 0.00 Horz(CT) 0.00 3 n/a n/a BCLL 0.0 Rep Stress Incr YES Code IRC2015/TPI2014 Matrix-P Weight: 6 lb FT = 20% BCDL 10.0

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

(size) 1=2-0-8, 3=2-0-8

Max Horz 1=-16(LC 8) Max Uplift 1=-2(LC 12), 3=-2(LC 12)

Max Uplift 1=-2(LC 12), 3=-2(LC 12) Max Grav 1=54(LC 1), 3=54(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

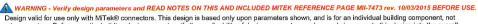
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2 lb uplift at joint 1 and 2 lb uplift at joint 3.



Structural wood sheathing directly applied or 2-0-8 oc purlins.

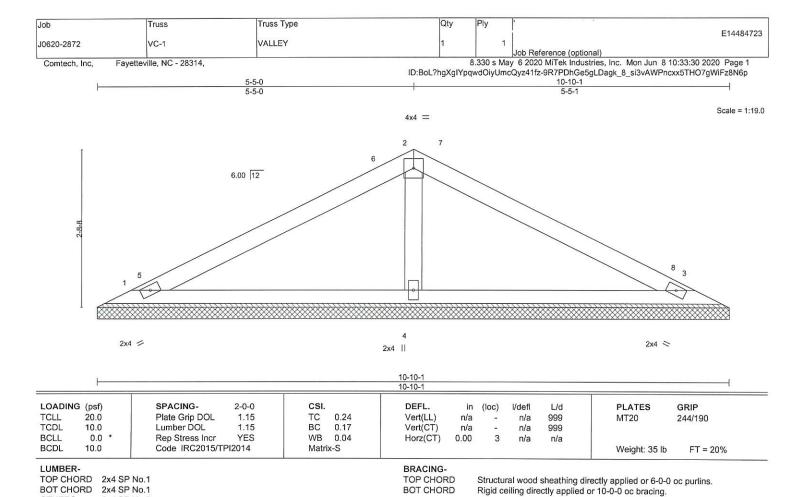
Rigid ceiling directly applied or 10-0-0 oc bracing.



Design valid for use only with Mil ek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANISTIP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932



OTHERS REACTIONS.

(size) 1=10-10-1, 3=10-10-1, 4=10-10-1

Max Horz 1=31(LC 11)

2x4 SP No.2

Max Uplift 1=-23(LC 12), 3=-29(LC 13)

Max Grav 1=178(LC 23), 3=178(LC 24), 4=417(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-4=-276/183

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-7-7 to 5-0-3, Interior(1) 5-0-3 to 5-5-0, Exterior(2) 5-5-0 to 9-9-13, Interior(1) 9-9-13 to 10-2-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 1 and 29 lb uplift at
- 6) Non Standard bearing condition. Review required.





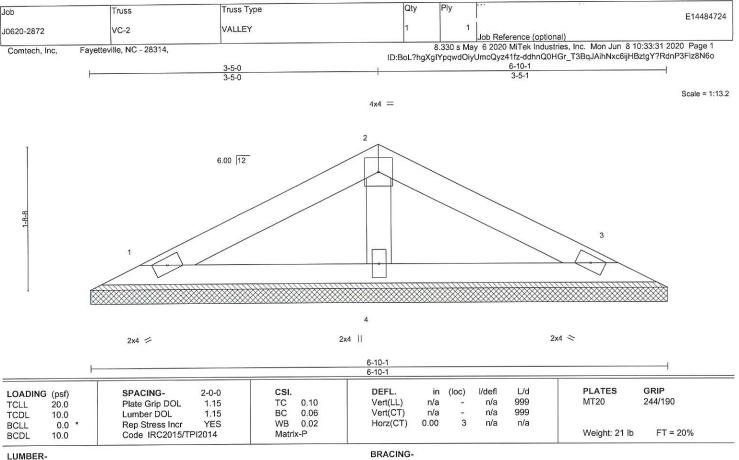
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





2x4 SP No.1 TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=6-10-1, 3=6-10-1, 4=6-10-1

Max Horz 1=-18(LC 8)

Max Uplift 1=-18(LC 12), 3=-21(LC 13)

Max Grav 1=114(LC 1), 3=114(LC 1), 4=220(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1 and 21 lb uplift at joint 3.
- 6) Non Standard bearing condition. Review required.

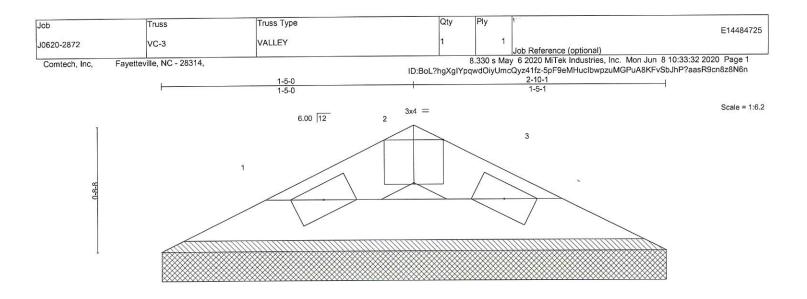




MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTFeW connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





2x4 /

2x4 >

late Offse	ets (X,Y) [2:0-2-0,Edge]				_		_				
LOADING TCLL TCDL BCLL	(psf) 20.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.01 0.02 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-P						Weight: 7 lb	FT = 20%

2-10-1

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1

BOT CHORD 2x4 SP No 1

(size) 1=2-10-1, 3=2-10-1

Max Horz 1=-5(LC 8)

Max Uplift 1=-4(LC 12), 3=-4(LC 13) Max Grav 1=64(LC 1), 3=64(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 4 lb uplift at joint 1 and 4 lb uplift at joint
- 6) Non Standard bearing condition. Review required.
- 7) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



Structural wood sheathing directly applied or 2-10-1 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

June 8,2020

M WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MTreWe connectors. This design is based only upon parameters had recovered to the connectors. This design is based only upon parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

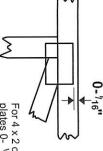


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- v_{te} from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE



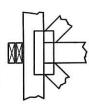
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction. DSB-89: Design Standard for Bracing.

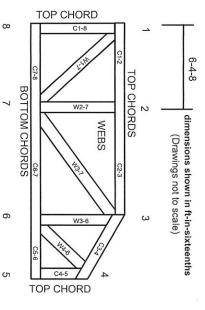
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information.

Guide to Good Practice for Handling,

Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.