





SCALE: 1/4" = 1'-Ø"



CLARK POINTE

PLANS DESIGNED TO THE 2018 NORTH CAROLINA STATE **RESIDENTIAL BUILDING CODE.**



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REVIEWED BY:

A-1

A - ELEVATIONS

RIGHT ELEVATION SCALE: 1/8" = 1'-0"



SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE

A-4





SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE





DATE: AUGUST 25, 2020
REV.:
SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:
FIRST FLOOR ELECTRICAL PLAN
E-1

SCALE NOTE: 18x24 PRINTS ARE
TO SCALE AS NOTED.
11x17 PRINTS ARE NOT TO SCALE

ELECTRICAL LAYOUT NOTES:

1.) BLOCK AND WIRE FOR ALL

2.) VANITY LIGHTS TO BE SET @ 90" A.F.F. (TYP.)

3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE

4.) PLACE SWITCHES 8" (MIN.) FROM

LOCATED BY ELECTRICIAN.

ROUGH OPENINGS.

ELECTRICAL LEGEND

+ 4-PLEX

IV V GFI OUTLET

BB 🕂 110 V BASEBOARD OUTLET

COUNTER OR FLOOR MOUNTED

Ø 110 V DEDICATED CIRCUIT

120 V DEDICATED CIRCUIT

- WALL MOUNT LIGHT

- PENDANT LIGHT

MINI CAN LIGHT

EYEBALL LIGHT

UNDERCABINET LIGHT

SWITCH

\$_D DIMMER SWITCH

TELEPHONE

TV- TV CONNECTION

SPEAKER

EXHAUST FAN

ALARM ALARM PANEL

TELEPHONE AND DATA

CD- CONDUIT FOR COMPONENT WIRING

110 V SMOKE/ CM DETECTOR

110 V SMOKE DETECTOR

LOW VOLTAGE PANEL

CEILING FAN

\$

6P

56

SD

- CEILING MOUNT LIGHT

DH SPECIAL PURPOSE (240 V, ETC.)

COUNTER OR FLOOR MOUNTED 110V GFI

CELING FANS PER PLAN.



ELECTRICAL LAYOUT NOTES:

1.) BLOCK AND WIRE FOR ALL CELING FANG PER PLAN.

2.) VANITY LIGHTS TO BE SET @ 90" A.F. (TYP.)

3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.

4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

ELECTRICAL LEGEND

- ÷ 110 y GFI OUTLET
- IND V SWITCHED OUTLET
- BB 110 V BASEBOARD OUTLET
- + ↓ 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110Y GFI

- Ø 110 V DEDICATED CIRCUIT
- 120 V DEDICATED CIRCUIT
- ●H SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- CEILING MOUNT LIGHT
- PENDANT LIGHT
- MINI CAN LIGHT
- EYEBALL LIGHT
- FLUORESCENT LIGHT
- \$ SWITCH
- \$ DIMMER SWITCH

- TELEPHONE AND DATA
- TV- TV CONNECTION
- CD- CONDUIT FOR COMPONENT WIRING
- SP SPEAKER
- MOV SMOKE/ CO DETECTOR
- 5D IIO V SMOKE DETECTOR
- EXHAUST FAN
- LOW VOLTAGE PANEL
- - CEILING FAN W/ LIGHT
 - LING FAN W/ LIGHT



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ne art of transforming your vision into re

WEAVER HOME	HICKORY DRIVE
DATE: AUGUS	T 25, 2020
REV.:	
SCALE: 1/4" = 1	'-0"
DRAWN BY: W	G
ENGINEERED	BY:
REVIEWED BY:	:
SECOND H ELCTRICA PLAN F.?	FLOOR

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE



ULTIMATE DESIGN WIND SPEED NOTES FOR LESS THAN 30' MEAN ROOF HEIGHT:

- 1. STRUCTURAL DESIGN PER NORTH CAROLINA
- RESIDENTIAL CODE, 2018 EDITION.
 2. FOR 120 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 6'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
- 3. FOR 130 MPH WIND ZONES INSTALL 1/2" ANCHOR BOLTS 4'-0" O.C. AND WITHIN 1'-0" FROM END OF EACH CORNER. ANCHOR BOLTS MUST EXTEND A MINIMUM OF 7" INTO CONCRETE OR 15" INTO MASONRY. LOCATE BOLT WITHIN MIDDLE THIRD OF PLATE WIDTH.
- MEAN ROOF HEIGHT IS LESS THAN 30 FEET.
 EXTERIOR WALLS DESIGNED FOR 120 OR 130 MPH WINDS.
- INSTALL 7/16" OSB SHEATHING ON ALL EXTERIOR WALLS OF ALL STORIES IN ACCORDANCE WITH SECTION R602.10.3 OF THE NCRC, 2018 EDITION.
- 7. ENERGY EFFICIENCY COMPLIANCE AND INSULATION VALUES OF THE BUILDING TO BE IN ACCORDANCE WITH CHAPTER 11 OF THE NCRC, 2018 EDITION.

ANCHOR SPACING AND EMBEDMENT										
WIND ZONE	130 MPH									
SPACING	4'-0" O.C.									
EMBEDMENT	7"	7" INTO CONCRETE 15" INTO MASONRY								

NOTE: HORIZONTAL FOOTING REBAR REQUIRED IN HIGH WIND ZONES ONLY (140-150 MPH)





STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 4 (UNO).
- INSTALL AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
 WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 5. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)
- 6. ALL 4 X 4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES (OR EQUAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4 AND 6 X 6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS AT TOP (UNO.)
- FOR FIBERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/ (2) METAL ANGLES USING 2" CONC. SCREWS. FASTEN ANGLES TO COLUMNS W/ 1/4" THROUGH BOLTS W/ NUTS AND WASHERS. LOCATE ANGLES ON OPPOSITE SIDES OF COLUMN. THROUGH BOLTS MUST BE INSTALLED PRIOR TO SETTING COLUMN.

BRACE WALL PANEL NOTES:

EXTERIOR WALLS: ALL EXTERIOR WALLS TO BE SHEALTHED WITH CS-WSP OR CS-SFB IN ACCORDANCE WITH SECTION R602.10.3 UNLESS NOTED OTHERWISE.

REQUIRED LENGTH OF BRACING: REQUIRED BRACE WALL LENGTH FOR EACH SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE INTERPOLATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB CONTRIBUTE THIER ACTUAL LENGTH. METHOD GB CONTRIBUTES 0.5 ITS ACTUAL LENGTH. METHOD PF CONTRIBUTES 1.5 TIMES ITS ACTUAL LENGTH.

GYPSUM: ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS TO HAVE 1/2" GYPSUM INSTALLED. WHEN NOT USING METHOD GB GYPSUM TO BE FASTENED PER TABLE R702.3.5. METHOD GB TO BE FASTENED PER TABLE R602.10.1.

HD: 800 LBS HOLD DOWN DEVICE FASTENED TO THE EDGE OF THE BRACE WALL PANEL NEAREST TO THE CORNER

METHODS: PER TABLE R602.10.1

—4 x 4 TRTD. POST MIN. (TYP.)

DATE: AUGUST 25, 202	20
REV.:	
SCALE: 1/4" = 1'-0"	
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ENGINEERED BY:	
REVIEWED BY:	

SECOND FLOOR FRAMING PLAN

S-2

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE

TABLE R602.7.5 MINIMUM NUMBER OF FULL HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS

	_	
HEADER SPAN (FFFT)	MAXIMUM STUD (PER TABL)	SPACING (INCHES) E R602.3(5)
()	16	24
UP TO 3'	1	1
4'	2	1
8'	3	2
12'	5	3
16'	6	4

STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)

DSP - DOUBLE STUD POCKET TSP - TRIPLE STUD POCKET RENAISSANCE RESIDENTIAL DESIGN; INC. RESERVES THE RIGHT TO MAKE MODIFICATIONS TO FLOOR PLANS, AND SPECIFICATIONS WITHOUT NOTICE. THESE DIAWINGS ARE FOR THE MICHAND SPECIFICATIONS WITHOUT NOTICE. THESE DIAWINGS ARE FOR THE PROPERTY RESERVES ITS HERENCE TO COMPETIVE REPORTS A CONCEPTION OF MATERIAL CONCEPT ONLY. REPRAYS REPORT OF DEAD AND THESE DIAMINGS ARE FOR THE PROPERTY RIGHTS IN THESE PLANS AND DIAWINGS AND DAWINGS AND DAWINGS AND DAWINGS AND THESE PLANS AND DIFERION OF REAL PROPERTY RIGHTS IN THESE PLANS AND DIAWINGS AND THESE PLANS AND CONSENT OF REAL PROPERTY RESIDENTIAL DESIGNS, INC. NOAP ARE THE YOUT FIRST OBTAINING THE EXPRINES WITTEN CONSENT OF REAMS AND CONSENT OF DEADS AND DIAWINGS AND CONSENT OF REAMS AND CONSENT OF REAMS AND CONSENT OF DEADS AND THESE PLANS AND CONSENT OF REAMS AND CONSENT OF REAMS AND CONSENT OF REAMS AND CONSENT OF DEADS AND THE DESIGN AND CONSENT OF DEADS AND THE SET PLANS AND CONSENT OF DEADS AND THE DESIGN AND CONSENT OF DEADS AND THE DEAD AND THE DESIGN AND CONSENT OF DEADS AND THE DEVINION FORMAL MANNER AND CONSENT OF DEADS AND THE DEVINION FORMAL MANNER AND CONSENT ON THE DEAD AND THE DEADS AND AND THE DEAD AND THE DEADS AND THE

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> WEAVER HOMES CAROLINA COLLECT HICKORY DRIVE LEF

DATE: AUGUST 25, 2020

REV.:	
SCALE: 1/4" = 1'-0"	
DRAWN BY: WG	
ENGINEERED BY:	
REVIEWED BY:	

ATTIC FLOOR FRAMING PLAN

S-3

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE

ATTIC VENT CALCULATION:

1040 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 6.9 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE #2 SPF (UNO). 2. HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
- 3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
- 4. FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
- 5. REFER TO SECTION R802.11 OF THE 2018 NCRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.

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SS동문 PRICE-FLOC DIMEN SQUA MAY V HOUS PLOT ATTIS

> ECTION Ē Ц \mathcal{O} r -) WEAVER HOME CAROLINA COL DRIVI CAROLINA HICKORY

DATE: AUGUST 25, 2020

REV.: SCALE: 1/4" = 1'-0" DRAWN BY: WG ENGINEERED BY: REVIEWED BY:

ROOF PLAN

S-4

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE

ATTIC VENT CALCULATION:

1116 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 7.4 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

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Dimension Notes 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise						
All Walls Shown Are Considered Load Bearing Hatch Legend Roof Area = 1468.1 sq.ft. Ridge Line = 52.07 ft. Hip Line = 0 ft. Horiz. OH = 98.57 ft. Raked OH = 159.04 ft. Decking = 50 sheets	Dunn / Harnett	Lot 1 Clark Point	Roof	01/27/21	David Landry	Lenny Norris
Drop BeamConnector InformationSymProductManufQtySupported MemberHeaderTrussHUS26USP4NA16d/3-1/2"16d/3-1/2"	сі ТҮ / со.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
Products PlotID Length Product Plies Net Qty BM1 12'0" 1-3/4"x 16" LVL Kerto-S 2 2 BM2 15'0" 1-3/4"x 16" LVL Kerto-S 2 2 BM3 12'0" 2x10 SPF No.2 2 2 GDH 12'0" 2x12 SPF No.2 2 2	Weaver Development Co. I nc.	Lot 1 Clark Point	Hickory "A"			J0121-0465
	BUI LDER	JOB NAME s a trus	PLAN	SEAL DATE	GRAM OF	

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the <u>truss delivery package or online @ sbcindustry.com</u>

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32' 6"	Dimension Notes 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise 3. All exterior wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise 3. All exterior walls to truss dimensions are to face of frame wall unless noted otherwise 3. All exterior walls Match Legend Padded HVAC Ridge Line = 52.07 ft. Hip Line = 0 ft. Horiz. OH = 98.57 ft. Raked OH = 159.04 ft. Decking = 50 sheets	Dunn / Harnett	Lot 1 Clark Point	Roof	01/27/21		David Landry
	Drop Beam Connector Information Nail Information Sym Product Manuf Qty Supported Member Header Truss HUS26 USP 4 NA 16d/3-1/2" 16d/3-1/2"	CI TY / CO.	ADDRESS	MODEL	DATE REV.		DRAWN BY
2.0"	PlotID Length Product Plies Net Qty BM1 12'0" 1-3/4*x 16" LVL Kerto-S 2 2 BM2 15'0" 1-3/4*x 16" LVL Kerto-S 2 2 BM3 12'0" 2x10 SPF No.2 2 2 GDH 12'0" 2x12 SPF No.2 2 2 GDH 12'0" 2x12 SPF No.2 2 2	3UI LDER Weaver Development Co. I nc.	JOB NAME Lot 1 Clark Point	JLAN Hickory "A"	SEAL DATE		2UOIE #

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

Trenco RE: J0121-0465 818 Soundside Rd Lot 1 Clark Point Edenton, NC 27932 Site Information: Customer: Lot/Block: Project Name: J0121-0465 Model: Address: Subdivision: City: State: General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions): Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.3 Wind Code: ASCE 7-10 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf This package includes 25 individual, dated Truss Design Drawings and 0 Additional Drawings. No. Seal# Truss Name Date No. Seal# Truss Name Date E14898224 1/27/2021 A1 1/27/2021 21 E14898244 V3 1 E14898245 2 E14898225 A1GE V4 1/27/2021 22 1/27/2021 3 E14898226 A2 1/27/2021 23 E14898246 V5 1/27/2021 4 E14898227 A3 1/27/2021 E14898247 V6 1/27/2021 24 5 E14898228 A4 1/27/2021 25 E14898248 V7 1/27/2021

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1/27/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

A5

B1

B2

C1

M1

M2

V2

A5GE

B1GE

C1GE

D1-GR

D1GE

M1GE

M2-GR

V1GE

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

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E14898242 E14898243

My license renewal date for the state of North Carolina is December 31, 2021 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Gilbert, Eric

BRACING-

TOP CHORD

BOT CHORD

11	M	R	F	R.

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

 REACTIONS.
 (size)
 2=0-3-8, 8=0-3-8

 Max Horz
 2=-110(LC 10)

Max Uplift 2=-89(LC 12), 8=-89(LC 13) Max Grav 2=1337(LC 1), 8=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2307/486, 3-5=-2125/534, 5-7=-2125/534, 7-8=-2307/486

BOT CHORD 2-13=-316/2007, 10-13=-106/1303, 8-10=-320/1964

WEBS 5-10=-147/921, 7-10=-454/288, 5-13=-147/921, 3-13=-454/288

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 30 0 nsf on the bottom chord in all areas where a rectangle 3-6

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

Structural wood sheathing directly applied or 4-11-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

	L	10-2-0			10-2-0	19-2-0	21-1-0	2 <u>-</u>	+-2-0	30-1	-0 32-3-0	
	1	10-2-8		1	6-0-0	3-0-0	1-11-0)' 3	-1-0 '	5-11	-0 2-3-8	
Plate Offsets (X,)	Y) [2:	0-4-0,0-2-14], [6:0-3-0	,Edge], [10:0-4	-0,Edge], [12	2:Edge,0-2-0], [14:Edge,0-2-0],	[15:0-3	-8,0-4-1	12]			
LOADING (psf) TCLL 20.0 TCDL 10.0		SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15	CSI. TC BC	0.75 0.67	DEFL. Vert(LL) Vert(CT)	in -0.21 -0.38	(loc) 17 17	l/defl >999 >999	L/d 360 240	PLATES MT20	GRIP 244/190
BCLL 0.0 BCDL 10.0	*	Rep Stress Incr Code IRC2015/T	YES PI2014	WB Matrix	0.70 x-S	Horz(CT) Wind(LL)	0.09 0.17	10 2-17	n/a >999	n/a 240	Weight: 247 lb	FT = 20%
LUMBER- TOP CHORD 2 BOT CHORD 2 1 WEBS 2	2x6 SP N 2x10 SP I 0-15: 2x0 2x4 SP N	o.1 No.1 *Except* 6 SP 2400F 2.0E o.2				BRACING- TOP CHOP BOT CHOP WEBS	RD RD	Structu Rigid c 1 Row	ral wood eiling dir at midpt	sheathing dir ectly applied c 5	rectly applied or 4-0-8 o or 10-0-0 oc bracing. -7	oc purlins.
REACTIONS.	(size) Max Horz Max Uplif Max Grav	2=0-3-8, 10=0-3-8 z 2=110(LC 11) ft 2=-90(LC 12), 10=-9 v 2=1393(LC 2), 10=1	00(LC 13) 353(LC 2)									

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-2217/403, 4-5=-1870/483, 7-9=-2258/519, 9-10=-2889/551

BOT CHORD 2-17=-193/1848, 15-17=-195/1860, 13-15=-371/2525, 10-13=-380/2525

WEBS 4-17=-29/402, 7-15=-114/967, 9-15=-1075/232, 9-13=0/616, 5-7=-1955/459

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to

33-1-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

TOP CHORD 2x6 SP No.1 TOP CHORD Structural wood sheathing directly applied or 3-10-0 oc purlins. BOT CHORD 2x10 SP No.1 *Except* BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 9-14: 2x6 SP 2400F 2.0E WEBS 1 Row at midpt 4-6 WEBS 2x4 SP No.2 REACTIONS. (size) 1=Mechanical, 9=0-3-8 Max Horz 1=-111(LC 8)

Max Uplift 1=-76(LC 12), 9=-90(LC 13) Max Grav 1=1345(LC 2), 9=1347(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-3=-2189/401, 3-4=-1853/491, 6-8=-2237/517, 8-9=-2874/549

BOT CHORD 1-16=-198/1827, 14-16=-200/1839, 12-14=-375/2511, 9-12=-384/2511 WEBS

6-14=-117/966, 3-16=-53/392, 4-6=-1931/474, 8-14=-1081/228, 8-12=0/620

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 7=0-3-8 Max Horz 1=-111(LC 10) Max Uplift 1=-77(LC 12), 7=-89(LC 13) Max Grav 1=1284(LC 1), 7=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2310/503, 2-4=-2129/551, 4-6=-2126/535, 6-7=-2308/487

BOT CHORD 1-12=-327/2012, 9-12=-111/1304, 7-9=-326/1966

WEBS 4-9=-147/921, 6-9=-454/288, 4-12=-149/924, 2-12=-458/292

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0ps for the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.

September 23,2020

TOP CHORD 9-10=-114/284, 10-11=-114/284

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=101.

BOT CHORD 1-7=0/307, 5-7=0/307

WEBS 3-7=0/277

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 5-11-8, Exterior(2) 5-11-8 to 10-4-5, Interior(1) 10-4-5 to 12-8-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 14, 12 except (it=lb) 15=181, 11=169.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 9.

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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 1 Clark Point
10121 0465	P2	BOOE SPECIAL	1		E14898233
50121-0405	D2		1	2	Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		. 8.	330 s Jul 1	22 2020 MiTek Industries, Inc. Wed Sep 23 09:20:33 2020 Page 2

ID:1yUksKymplk2404ufZYCrxyoKUD-YMIYrN1R7ztTvdjv2B0CgKSFDouzo9E02Dpqfxyao3C

	12.1 yokor ympiliz 404uiz
LOAD CASE(S) Standard	
2) Dead + 0.75 Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (plf)	
Vert: 1-2=-250, 3-4=-50, 4-7=-50, 6-10=-20	
3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25	
Uniform Loads (plf)	
Vert: 1-2=-220, 3-4=-20, 4-7=-20, 6-10=-40	
4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-156, 3-4=27, 4-12=35, 6-12=27, 6-7=20, 6-10=-12	
Horz: 3-4=-39, 4-12=47, 6-12=39, 6-7=32	
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (pit)	
Vert: 1-2=-170, 3-4=35, 4-11=27, 6-11=35, 6-7=58, 6-10=-12	
HO[2: $3-4=-47$, $4-11=39$, $6-11=47$, $6-7=70$	
b) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60	
Unition Loads (pii)	
Ven. 1-2=-255, 5-4=-56, 4-6=-56, 0-7=-51, 0-10=-20 Horz: 3-4-38, 4-6=-38, 6-7=-31	
7) Dead ± 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase -1.60. Plate Increase -1.60	
Liniform Loads (nif)	
Vert: 1-2=-235 3-4=-58 4-6=-58 6-7=11 6-10=-20	
Horz: 3-4=38, 4-6=-38, 6-7=31	
8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60. Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-179, 3-4=-13, 4-6=11, 6-7=4, 6-10=-12	
Horz: 3-4=1, 4-6=23, 6-7=16	
9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-191, 3-4=11, 4-6=-13, 6-7=2, 6-10=-12	
Horz: 3-4=-23, 4-6=-1, 6-7=14	
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=-201, 3-4=-35, 4-6=-11, 6-7=-4, 6-10=-20	
Horz: 3-4=15, 4-6=9, 6-7=16	
11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60	
Vert: 1-2=-213, 3-4=-11, 4-b=-35, b-7=-28, b-10=-20	
12) Dood + 0.6 MWEPS Wind (Pool Internal) 1ct Parallel: Lumber Increase-1.60. Plate Increase-	-1 60
12) Dedu + 0.0 WWFRS Willu (F0S. Internal) TSt Fataliel. Lumber increase=1.00, Flate increase=	=1.00
Vert: $1_{2} = 170_{3} = 4 = 21_{4} = 6 = 6 = 7 = 2_{6} = 10 = -12_{1}$	
Vert. $122 - 173, 3 - 4 - 21, 4 - 0 - 3, 0 - 7 - 2, 0 - 10 - 12$ Horz: $3 - 4 - 33, 4 - 6 - 7 - 14$	
13) Dead + 0.6 MWERS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60. Plate Increase	=1.60
Liniform Loads (nif)	-1.00
Vert: 1-2=-191 3-4=9 4-6=21 6-7=14 6-10=-12	
Horz: 3-4=-21, 4-6=33, 6-7=26	
14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60. Plate Increase	=1.60
Uniform Loads (plf)	
Vert: 1-2=-179, 3-4=21, 4-6=9, 6-7=2, 6-10=-12	
Horz: 3-4=-33, 4-6=21, 6-7=14	
15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=	=1.60
Uniform Loads (plf)	
Vert: 1-2=-191, 3-4=9, 4-6=21, 6-7=14, 6-10=-12	
Horz: 3-4=-21, 4-6=33, 6-7=26	
16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=	=1.60
Uniform Loads (plf)	
Vert: 1-2=-201, 3-4=-1, 4-6=-13, 6-7=-6, 6-10=-20	
Horz: 3-4=-19, 4-6=7, 6-7=14	
17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase	=1.60
Uniform Loads (plf)	
Vert: 1-2=-213, 3-4=-13, 4-6=-1, 6-7=6, 6-10=-20	
H01Z: 3-4=-7, 4-6=19, 6-7=26	
18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90	
Vigit: 1 2= 220, 2 4= 20, 4 7= 20, 6 10= 20	
voil. 1-2=-220, 3-4=-20, 4-7=-20, 0-10=-20 10) Dead + 0.75 Roof Live (bal) + 0.75(0.6 MW/EPS Wind (Neg. Int) Left): Lumber Increase-1.60	Plate Increase-1.60
Inform Loads (off)	, i ale increase=1.00
Vert 1-2=-236 3-4=-61 4-6=-43 6-7=-38 6-10=-20	
Horz: 3-4=11 4-6=7 6-7=12	
20) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1 f	60. Plate Increase=1 60
Uniform Loads (plf)	
Vert: 1-2=-245. 3-4=-43. 4-6=-61. 6-7=-56. 6-10=-20	
Horz: 3-4=-7, 4-6=-11, 6-7=-6	

21) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60

Job	Truss	Truss Type	Qty	Ply	Lot 1 Clark Point
					E14898233
J0121-0465	B2	ROOF SPECIAL	1	2	
				_	Job Reference (optional)
Comtech, Inc, Fayet	eville, NC - 28314,		. 8	330 s Jul 2	22 2020 MiTek Industries, Inc. Wed Sep 23 09:20:33 2020 Page 3

8.330 s Jul 22 2020 MiTek Industries, Inc. Wed Sep 23 09:20:33 2020 Page ID:1 y UksKymplk 2404 uf ZYC rxyo KUD-YMIYr N1 R7 zt Tvdjv 2B0 Cg KSFD ou zo 9E02 Dpq fxya o 3C Variable Structure Structure

LOAD CASE(S) Standard Uniform Loads (plf)

Vert: 1-2=-236, 3-4=-36, 4-6=-45, 6-7=-40, 6-10=-20 Horz: 3-4=-14, 4-6=5, 6-7=10

22) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-245, 3-4=-45, 4-6=-36, 6-7=-31, 6-10=-20 Horz: 3-4=-5, 4-6=14, 6-7=19

23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-260, 3-4=-60, 4-7=-20, 6-10=-20

- 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-2=-220, 3-4=-20, 4-7=-60, 6-10=-20
- 25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-250, 3-4=-50, 4-7=-20, 6-10=-20

26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-220, 3-4=-20, 4-7=-50, 6-10=-20

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			0-3-0				
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [6:0-2-0,0-1-8]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.84 BC 0.24 WB 0.00	DEFL. ir Vert(LL) -0.05 Vert(CT) -0.10 Horz(CT) 0.00	(loc) 2-5 2-5 5	l/defl L/d >999 360 >969 240 n/a n/a	PLATES GRI MT20 244	P /190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.10	2-5	>886 240	Weight: 37 lb F	-T = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x6 S	P No.1 P No.1	11	BRACING- TOP CHORD	Structu except	Iral wood sheathing end verticals.	directly applied or 5-3-4 oc p	urlins,

2x4 SP No.2 WEBS 2x6 SP No.1 OTHERS

REACTIONS. (size) 2=0-3-0, 5=0-3-8 Max Horz 2=74(LC 8) Max Uplift 2=-150(LC 8), 5=-127(LC 8) Max Grav 2=375(LC 1), 5=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- NOTES 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 8-0-0 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 5=127.

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BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

			<u>8-3-8</u> 8-3-8		
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [12:0-2-0,0-1-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.35 BC 0.26 WB 0.01 Matrix-S	DEFL. ir Vert(LL) 0.09 Vert(CT) -0.08 Horz(CT) -0.00	n (loc) l/defl L/d 9 10-11 >999 240 3 10-11 >999 240 0 8 n/a n/a	PLATES GRIP MT20 244/190 M18SHS 244/190 Weight: 41 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF OTHERS 2x4 SF 8-12: 2	P No.1 P No.1 P No.2 *Except* 2x6 SP No.1		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	ectly applied or 6-0-0 oc purlins, r 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 8=0-3-8 Max Horz 2=105(LC 8) Max Uplift 2=-216(LC 8), 8=-188(LC 8)

Max Grav 2=375(LC 1), 8=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

BOT CHORD 2-11=-284/207, 10-11=-284/207, 9-10=-284/207, 8-9=-284/207

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are MT20 plates unless otherwise indicated.

4) Gable studs spaced at 2-0-0 oc.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216, 8=188.

4x4 ||

Scale = 1:28.1

Plate Offsets (X,Y)	[1:0-0-4,0-0-5], [1:0-0-8,0-3-6], [3:0-0-4,	0-0-5], [3:0-0-8,0-3-6]		-11-0			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.38 BC 0.57 WB 0.39 Matrix-P	DEFL. in Vert(LL) -0.02 Vert(CT) -0.04 Horz(CT) 0.01 Wind(LL) 0.01	(loc) l/defl 3-4 >999 3-4 >999 3 n/a 3-4 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 100 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2 WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 3=0-3-8

Max Horz 1=91(LC 24) Max Uplift 1=-191(LC 8), 3=-180(LC 9) Max Grav 1=2919(LC 1), 3=2779(LC 2)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 1-2=-2418/177, 2-3=-2418/177

 BOT CHORD
 1-4=-100/1678, 3-4=-100/1678

 WEBS
 2-4=-154/3142

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=191, 3=180.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1261 lb down and 93 lb up at 0-9-12, 1258 lb down and 96 lb up at 2-9-12, and 1325 lb down and 96 lb up at 4-9-12, and 1325 lb down and 96 lb up at 6-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 1-3=-20

ntinued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oullapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

1-0-3

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Job	Truss	Truss Type	Qty	Ply	Lot 1 Clark Point	
J0121-0465	D1-GR	Common Girder	1	2	E14898236	
				_	Job Reference (optional)	
Comtech, Inc, Fayettev	ille, NC - 28314,		8	330 s Jul 2	22 2020 MiTek Industries, Inc. Wed Sep 23 09:20:36 2020 Page 2	
		ID:1yUksKymplk2404ufZYCrxyoKUD-yxRgTP3KQuG2m4RUjKZvly4j70rU?SpTkB1UFGyao39				

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 5=-1261(B) 6=-1258(B) 7=-1258(B) 8=-1258(B)

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 OTHERS 2x4 SP No 2 WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS. All bearings 7-11-0.

(lb) - Max Horz 2=-118(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-152(LC 12), 8=-148(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=152.8=148.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

			5-0-0		
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.28	DEFL. in (loc) I/de Vert(LL) -0.01 2-4 >99	lefl L/d 99 360	PLATES GRIP MT20 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-P	Horz(CT) 0.00 r Wind(LL) 0.01 2-4 >99	99 240 n/a n/a 99 240	Weight: 24 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=63(LC 8) Max Uplift 2=-102(LC 8), 4=-79(LC 8)

Max Grav 2=255(LC 1), 4=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

- capacity of bearing surface. 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=102.

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This say INCLODED MITE REPERENCE PAGE MIT-14/3 feb. 37/93/200 BEPCHC OSE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **AVSUPTI Quelity Criteria, DSB-89 and BCSI Building Compor Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

	5-0-0								
LOADING (psf) SPACING- TCLL 20.0 Plate Grip DOL TCDL 10.0 Lumber DOL BCLL 0.0 * Rep Stress Incr	2-0-0 CS 1.15 TC 1.15 BC YES WB	0.09 0.09 0.02	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.01 -0.01 -0.00	(loc) 8 8 6	l/defl >999 >999 n/a	L/d 240 240 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0 Code IRC2015/TP	12014 Mat	rix-S						Weight: 27 lb	FT = 20%

LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1
OTHERS	2x4 SP No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD

REACTIONS. (size) 2=0-3-0, 6=0-1-8

Max Horz 2=90(LC 8) Max Uplift 2=-147(LC 8), 6=-115(LC 8)

Max Grav 2=255(LC 1), 6=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=147.6=115.

Design valid for use only with MTEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Comport Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	Lot 1 Clark Point
					E14898240
J0121-0465	M2	Half Hip	3	1	Ich Reference (optional)
Comtech, Inc,	Fayetteville, NC - 28314,		8	330 s Jul 2	22 2020 MiTek Industries, Inc. Wed Sep 23 09:20:38 2020 Page 2

8.330 s Jul 22 2020 MiTek Industries, Inc. Wed Sep 23 09:20:38 2020 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-vKZRu55axWWm0ObsrkbNNN95ppb4TSKICUWbK9yao37

LOAD CASE(S) Standard
Concentrated Loads (lb)
Vert: 9=-350
3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-20, 3-4=-20, 5-6=-40, 2-7=-40
Concentrated Loads (Ib)
Vert: 9=-300
4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-2=70, 2-3=56, 3-4=153, 5-6=12, 2-8=52, 8-10=115, 7-10=52
HOIZ: 1-282, 2-3-7(), 3-4=-105, 3-5=-55
Concentrated Loads (ib)
Vell. $s=340$
La forma and (all)
Unit 1.2-51 2.3-58 3.4-51 5.6-42 2.8-52 8.10-115 7.10-52
Hor: 1,2-63,2,3-70,3,4-63,3-5-55
Concentrated Loads (Ib)
Vert: 9=566
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51
Concentrated Loads (lb)
Vert: 9=-420
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51
Concentrated Loads (lb)
Ver: 9=-420
b) Dead + 0.5 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60
Unitorin Loads (pii) Vlatt 1 2-26 2 2-21 2 4-14 5 5- 11 2 9-10 9 10-22 7 10-10
Velt. 1-2=00, 2-3=21, 3-4=14, 5-0=-11, 2-0=10, 0-10=33, 7-10=10 Hore: 1-2=-48, 2-3=-23, 2-4, 2-3, 2-5=7
(1) (2, 1) = -40, (2, 2) = -30, (3, 2) = -1
Vert 9=154
9) Dead + 0.6 MWERS Wind (Pos_Internal) Right: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (olf)
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27
Concentrated Loads (lb)
Vert: 9=43
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2
Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34
Concentrated Loads (lb)
Vert: 9=-339
11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Velt. 1-2=-2, 2-3=-3, 3-4=-2, 3-0=-21, 2-1=-20
noiz. 1-2=-10, 2-3=-11, 3-4=-10, 3-3=-0
12) Dead + 0.6 MWERS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (olf)
Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12
Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
Concentrated Loads (lb)
Vert: 9=43
13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12
Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
Concentrated Loads (lb)
Vert: 9=43
14) Dead + 0.0 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60
voli, i-z=14, z-3=21, 3-4=14, 3-0=-11, z-1=-12 Harr 1, 2-28, 2-32, 3, 2, -38, 2, 5-30
1012. 172-720, 273-730, 574-720, 573-739 Concentrated Loads (Ih)
Vert: 9=43
15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60

Job	Truss	Truss Type	Qty	Ply	Lot 1 Clark Point
					E14898240
J0121-0465	M2	Half Hip	3	1	
					Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,	3314, 8.330 s Jul 22 2020 MiTek Industries, Inc. Wed Sep 23 09:20:38 2020 Pa			

8.330 s Jul 22 2020 MiTek Industries, Inc. Wed Sep 23 09:20:38 2020 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-vKZRu55axWWm0ObsrkbNNN95ppb4TSKICUWbK9yao37

LOAD CASE(S) Standard
Uniform Loads (plf)
Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12
Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
Vert: 9=43
Liniform Loads (off)
Vert 1-2=6 2-3=-1 3-4=6 5-6=-33 2-7=-20
Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12
Concentrated Loads (lb)
Vert: 9=-234
17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-21, 2-7=-20
HOTZ: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0
Veril 5=254
Uniform Loads (off)
Vert: 1-3-20, 3-4=-20, 5-6=-120, 2-7=-20
Concentrated Loads (lb)
Vert: 9=-200
19) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-8=-3, 8-10=13, 7-10=-3
Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26
20) Dead + 0.75 Roof Live (bal) + 0.75 Attic Floor + 0.75/0.6 MWERS Wind (Neg. Int) Right): Lumber Increase=1.60. Plate Increase=1.60
Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-86, 6-9=-116, 2-7=-20
Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0
Concentrated Loads (Ib)
Vert: 9=-375
21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (pit)
Vert. 1-2=-31, 2-3=-30, 3-4=-31, 3-3=-30, 0-3=-123, 2-7=-20 Hore: 1-2=-10, 2-2=-14, 2-4-10, 2-5=-0
$\frac{1}{100}$
Vert: 9=-375
22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-86, 6-9=-116, 2-7=-20
Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0
Concentrated Loads (lb)
Vert: 9=-375
Liniform Loads (nft
Vert: 1-3=-60. 3-4=-60. 5-6=-40. 2-7=-20
Concentrated Loads (lb)
Vert: 9=-400
24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-3=-20, 3-4=-20, 5-9=-40, 6-9=-80, 2-7=-20
Concentrated Loads (lb)
Vell. 5=-400 25) 3rd Dead + 0.75 Paof Live (upbalanced): Lumber Increase=1.15, Plate Increase=1.15
Liniform Loads (off)
Vert: 1-3=-50. 3-4=-50. 5-6=-100. 2-7=-20
Concentrated Loads (lb)
Vert: 9=-350
26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-3=-20, 3-4=-20, 5-9=-100, 6-9=-130, 2-7=-20
Concentrated Loads (lb)
ven. 9=-550

Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual Truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings with possible personal injury and provery damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Compose Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 1 Clark Point		
J0121-0465	M2-GR	HALF HIP	1	2	Job Reference (optional)	E14	898241
		•					-

Comtech, Inc, Fayetteville, NC - 28314,

LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 9=-400

Uniform Loads (plf)

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Vert: 1-3=-50, 3-4=-50, 5-9=-220, 6-9=-250, 2-7=-20
Concentrated Loads (lb)
Vert: 9=-350
3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-20, 3-4=-20, 5-6=-160, 2-7=-40
Concentrated Loads (lb)
4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber increase=1.60, Plate increase=1.60
Vert. 1-2=7(0, 2-0=30, 3-4=135, 3-0=100, 2-0=32, 0=10=113, 7=10=32 Horz: 1, 2=.82, 2-3=-70, 2-4=-165, 2-5=-55
$\frac{1}{102} - \frac{1}{2} - $
Vert 9=548
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (olf)
Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=-78, 2-8=52, 8-10=115, 7-10=52
Horz: 1-2=-63. 2-3=-70. 3-4=-63. 3-5=-55
Concentrated Loads (lb)
Vert: 9=566
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51
Concentrated Loads (lb)
Vert: 9=-420
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51
Concentrated Loads (lb)
Vert: 9=-420
8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber increase=1.60, Plate increase=1.60
Uniform Loads (pii) Vort $1, 2, 26, 2, 2-24, 2, 4-14, 5, 6-, 124, 2, 9-10, 9, 10-22, 7, 10-10$
Vert. $1-2=30$, $2-3=21$, $3-4=14$, $3-0=131$, $2-0=10$, $0-10=33$, $7-10=10$
$\frac{1}{1012.1-2=+40, 2-3=-50, 3-4=-20, 5-5=7}{Concentrated 1 ads (lb)}$
Vart 9–154
9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (olf)
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27
Concentrated Loads (lb)
Vert: 9=43
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2
Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34
Concentrated Loads (lb)
Vert: 9=-339
11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (pit)
Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20
HOTZ: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0
Concentrated Loads (ib)
Veil. 5=234
Linform Loads (off)
Vert 1-2-14 2-3-21 3-4-14 5-6131 2-712
Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
Concentrated Loads (b)
Vert: 9=43
13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12
Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
Concentrated Loads (lb)
Vert: 9=43
14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60

2) Dead + 0.75 Roof Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent ouclaul truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent ouclause with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, terection and bracing of trusses sand truss systems, see
 ANS/TPH1 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	Lot 1 Clark Point
					E14898241
J0121-0465	M2-GR	HALF HIP	1	2	
				-	Job Reference (optional)
Comtech, Inc, Fayettev	rille, NC - 28314,		8	330 s Jul 2	22 2020 MiTek Industries, Inc. Wed Sep 23 09:20:39 2020 Page 3

8.330 s Jul 22 2020 MiTek Industries, Inc. Wed Sep 23 09:20:39 2020 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-NW6p6Q6CipecdYA3PS6cvbilTDyqCvavQ8G8sbyao36

10	
20/	
	Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12
	Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
	Concentrated Loads (lb)
15)	verit 3=43 Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60. Plate Increase=1.60
,	Uniform Loads (plf)
	Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12
	Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
16)	veri. 5=45 Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60. Plate Increase=1.60
,	Uniform Loads (plf)
	Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20
	Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12
17)	Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60. Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-141, 2-7=-20
	Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0
	Voncentrated Loads (ID)
18)	Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
,	Uniform Loads (plf)
	Vert: 1-3=-20, 3-4=-20, 5-6=-240, 2-7=-20
	Concentrated Loads (lb)
19)	Vert. 3-200 Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60. Plate Increase=1.60
- /	Uniform Loads (plf)
	Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-8=-3, 8-10=13, 7-10=-3
	Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26
	Vince Quality (Vince Action)
20)	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-237, 2-3=-42, 3-4=-37, 5-9=-206, 6-9=-236, 2-7=-20
	H0TZ: 1-2Z=-13, 2-3Z=-8, 3-4Z=-13, 3-5Z=-0 Concentrated Loads (Ib)
	Vert: 9–375
21)	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-7=-20
	H072: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9 Concentrated Loads (Ib)
	Vert: 9375
22)	Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-3=-205, 6-3=-236, 2-7=-20 Hor: 1-2=-10, 2-3=-5, 2-4=-10, 2-5=-0
	Concentrated Loads (Ib)
	Vert: 9=-375
23)	1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
	Vett. 1-3=-00, 3-4=-00, 3-0=-100, 2-7=-20 Concentrated Loads (lb)
	Vert: 9=-400
24)	2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1-3=-20, 3-4=-20, 5-9=-160, 6-9=-200, 2-7=-20
	Vert: 9=-400
25)	3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1.3=-50, 3-4=-50, 5-6=-220, 2-7=-20
	Voncentrated Loads (in) Vert 9=-350
26)	4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1-3=-20, 3-4=-20, 5-9=-220, 6-9=-250, 2-7=-20
	Concentrated Loads (ID)

8-6-5 [7:0-3-11 Edge] [9:0-1-6 0-1-0] [10:0-2-0 0-0-10]

	[7.0-3-11,Luge], [3.0-1-0,0-1-0], [10.0-2	-0,0-0-10]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.06 BC 0.03 WB 0.08	DEFL. in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 7 n/a n/a	
LUMBER-		Matrix-S	BRACING-	

TOP CHORD

BOT CHORD

12-10-9

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

4-4-4

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No 2

REACTIONS. All bearings 12-10-9.

(lb) - Max Horz 1=231(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10 except 11=-112(LC 12), 12=-107(LC 12), 13=-133(LC 12), 8=-126(LC 13)

8-6-5

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 9, 11, 12, 13, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-295/189

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 9) Bearing at joint(s) 7, 9, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify
- capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10 except (jt=lb) 11=112, 12=107, 13=133, 8=126.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 7, 9, 8.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 14-10-0.

(lb) - Max Horz 1=-140(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-135(LC 12), 6=-135(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=400(LC 19), 8=393(LC 19), 6=393(LC 20)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 WEBS
 2-8=-338/246, 4-6=-338/246

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed;

WWERS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-5-0, Exterior(2) 7-5-0 to 11-9-13, Interior(1) 11-9-13 to 14-5-3 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

- will fit between the bottom chord and any other members, with BCDL = 10.0psf. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb)
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 ib uplint at joint(s) 1 except (t=ib) 8=135, 6=135.

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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 12-5-4.

(lb) - Max Horz 1=-116(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-123(LC 12), 6=-123(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=326(LC 19), 6=326(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-8=-312/241. 4-6=-312/241

WEBS 2-8=-312/241, 4-6=-312/241

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-2-10, Exterior(2) 6-2-10 to 10-7-7, Interior(1) 10-7-7 to 12-0-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123, 6=123.

6) Non Standard bearing condition. Review required.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=10-0-7, 3=10-0

VS. (size) 1=10-0-7, 3=10-0-7, 4=10-0-7 Max Horz 1=-92(LC 8)

Max Uplift 1=-22(LC 13), 3=-30(LC 13)

Max Grav 1=197(LC 1), 3=197(LC 1), 4=344(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate orip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 6) Non Standard bearing condition. Review required.

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. (size) 1=7-7-10, 3=7-7-10, 4=7-7-10

Max Horz 1=68(LC 9)

Max Uplift 1=-24(LC 13), 3=-30(LC 13)

Max Grav 1=158(LC 1), 3=158(LC 1), 4=230(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. (size) 1=5-2-13, 3=5-2-13, 4=5-2-13 Max Horz 1=44(LC 11)

Max Uplift 1=-15(LC 13), 3=-19(LC 13)

Max Grav 1=102(LC 1), 3=102(LC 1), 4=149(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

Structural wood sheathing directly applied or 5-2-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

3x4 🥢

3x4 📏

Plate Offsets (X,Y)	[2:0-2-0,Edge]		2-10-0		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.01 BC 0.03 WB 0.00 Matrix-P	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	(loc) l/defl L/d - n/a 999 - n/a 999 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 8 lb FT = 20%

2-10-0

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. (size) 1=2-10-0, 3=2-10-0

Max Horz 1=-20(LC 8) Max Uplift 1=-4(LC 12), 3=-4(LC 13) Max Grav 1=81(LC 1), 3=81(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

Structural wood sheathing directly applied or 2-10-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

	co	m	те	CH				
T R	ROOF & FLOOR TRUSSES & BEAMS Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444							
Bearing deemed requiren attached requiren size and reaction 15000#. retained reaction Tables.	reactions to compl hents. The I Tables (hents) to number s s greater to design that exce A register to design s that exce	e less that y with the e contract derived f determin of wood s than 3000 red desig n the supp ceds thos red design n the supp ceed 1500	n or equa prescrip tor shall r rom the p e the min studs requ D# but no n profess port syste e specifie n profess port syste 0#.	I to 3000# tive Code efer to the prescriptiv imum fou uired to so t greater t ional sha em for any od in the a ional shal em for all	e e Code ndation upport han II be / ttached I be			
Signatu	re[Davi Davi	d La	andr ndrv	y			
LO	AD CH.	ART FO	DR JA	CK STL	IDS			
80 (2) and 2) an	ee a of the source of the sour	HEADS3	Recovered AGE AGE <th>00155 0</th> <th>ог 89794414(1) 10 1 N 3 10 1 N 3 00 4 00 5</th>	00155 0	ог 89794414(1) 10 1 N 3 10 1 N 3 00 4 00 5			
Dunn / Harnett	Lot 1 Clark Point	Floor	11	David Landry	Lenny Norris			
CI TY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.			
Weaver Development Co. I nc.	E Lot 1 Clark Point	Hickory "A"	E Seal Date	Quote #	J0121-0466			
BUILDER	JOB NAME	PLAN	SEAL DAT	QUOTE #	JOB #			
THIS IS These to compore design See ind identified	A TRUSS russes an nents to k at the sp ividual de ed on the er is resp	S PLACEN re designo be incorpo ecification esign she placemen onsible fo	MENT DIA ed as indi prated into n of the b ets for ea nt drawing or tempor	GRAM ON vidual bu o the build uilding de ich truss g. The bui ary and	ILY. ilding ding signer. design Iding			

Dimension Notes All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes Plumbing drop locations shown are NOT exact.
 Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
 Adjust spacing as needed not to exceed 24"oc.

	Conne	Nail Info	ormation			
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
\bigcirc	HUS410	USP	14	NA	16d/3-1/2"	16d/3-1/2"

Products							
PlotID	Length	Product	Plies	Net Qty			
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2			
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2			
BM3	12' 0"	2x10 SPF No.2	2	2			
GDH	12' 0"	2x12 SPF No.2	2	2			

Truss Placement Plan Scale: 1/4"=1'

A= Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Ŀ	CO RO(DF 8	T e & FL		•				
R	RUS Eeilly R Fayett Phon Fax:	SES teville e: (910)	& B ndustr , N.C. 3 0) 864 864-4	EAN 28309 -8787 1444	ЛЅ ҡ				
Bearing deemed requirem attached requirem size and reactions 15000# retained reaction Tables/ retained reactions	earing reactions less than or equal to 3000# are semed to comply with the prescriptive Code iquirements. The contractor shall refer to the tached Tables (derived from the prescriptive Code iquirements) to determine the minimum foundation ze and number of wood studs required to support actions greater than 3000# but not greater than 5000#. A registered design professional shall be tained to design the support system for any eaction that exceeds those specified in the attached ables. A registered design professional shall be tained to design the support system for all tactions that exceed 15000#								
Signatur	re	<mark>Davi</mark> Davi	<mark>d L</mark> a d Lar	andr ndrv	<u>y</u>				
LO	AD CHA (BASER	ART FO	DR JAC	CK STL	IDS				
8000 1700 3400 5100 6800 8500 10200 11900 13600 15300		12750 15300	REQUIRED AGENERAL SALES REALIZED AND COLORS	021 021 022 022 022 022 022 022 022 022	30 30 30 30 4 300 4 300 4 300 5 30 4 300 5 30 4 300 5 30 4 300 5 30 4 300 5 30 4 300 5 30 4 300 5 30 4 30 5 30 4 30 5 30 4 30 5 30 4 30 5 30 4 30 5 30 4 30 5 30 4 30 5 30 4 30 5 30 4 30 3 30 4 30 5 30 4 30 3 30 4 30 3 30 4 30 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3				
Dunn / Harnett	Lot 1 Clark Point	Floor	01/27/21	David Landry	Lenny Norris				
CI TY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.				
DER Weaver Development Co. I nc.	NAME Lot 1 Clark Point	I Hickory "A"	- DATE Seal Date	TE # Quote #	# J0121-0466				
THIS IS These t	A TRUSS russes ar	DLACEN The designed	SEAL	LONO GRAM ON vidual bu	₩ BOC				
compor design See ind identifie designe perman for the support and col designe consult truss de	nents to b at the spe ividual de en on the ent brach overall st t structure umns is t er. For ge BCSI-B1 elivery pa	e incorpo ecification sign shee placemer onsible fo ng of the ructure. T e includin the respon neral guic and BCS ackage or	orated into n of the bi- ets for ea th drawing or tempora roof and 'he design g headers nsibility o lance reg online @	b the build uilding de ch truss of g. The build ary and floor syst n of the tr s, beams, f the build arding brai ided with sbcindus	ding signer. design lding em and uss walls, ding acing, the the ttry.com				

Dimension Notes All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes

Plumbing drop locations shown are NOT exact.
 Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
 Adjust spacing as needed not to exceed 24"oc.

	Conne	Nail Info	ormation			
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
\bigcirc	HUS410	USP	14	NA	16d/3-1/2"	16d/3-1/2"

Products							
PlotID	Length	Product	Plies	Net Qty			
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2			
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2			
BM3	12' 0"	2x10 SPF No.2	2	2			
GDH	12' 0"	2x12 SPF No.2	2	2			

Truss Placement Plan Scale: 1/4"=1'

RE: J0121-0466 Lot 1 Clark Point

Trenco 818 Soundside Rd Edenton, NC 27932

Site Information: Customer: Lot/Block: Project Name: J0121-0466 Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf

Design Program: MiTek 20/20 8.3 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 8 individual, dated Truss Design Drawings and 0 Additional Drawings.

No. 1 2 3 4 5 6 7	Seal# E14898249 E14898250 E14898251 E14898252 E14898253 E14898254 E14898255	Truss Name ET1 ET2 ET3 F1 F2 F3 F4	Date 1/27/2021 1/27/2021 1/27/2021 1/27/2021 1/27/2021 1/27/2021
7 8	E14898255 E14898256	F4 F5	1/27/2021 1/27/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2021 North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Gilbert, Eric

January 27, 2021

			8-7-0			
Plate Offsets (X,Y)	[5:0-1-8,Edge], [17:0-1-8,0-1-8], [18:0-1	-8,0-1-8]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.00 WB 0.05 Matrix-P	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	(loc) l/defl L/d - n/a 999 - n/a 999 9 n/a n/a	PLATES MT20 Weight: 54 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S OTHERS 2x4 S	P No.1(flat) P No.1(flat) P No.3(flat) P No.3(flat)	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	rectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,	

8-7-0

REACTIONS. All bearings 8-7-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 9-16=-10, 1-8=-100 Concentrated Loads (lb) Vert: 3=-71 6=-71 19=-71 20=-77

		1	11-1-0			
Plate Offsets (X,Y)	[4:0-1-8,Edge], [14:0-1-8,Edge]		11-1-0			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.01 WB 0.04 Matrix-S	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	(loc) l/defl L/d - n/a 999 - n/a 999 10 n/a n/a	PLATES MT20 Weight: 52 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) OTHERS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied or	ctly applied or 6-0-0 10-0-0 oc bracing.	oc purlins,

REACTIONS.

DNS. All bearings 11-1-0. (lb) - Max Grav All reactions 250 lb or less at joint(s) 18, 10, 17, 16, 15, 14, 13, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

 TOP CHORD
 2x4 SP No.1(flat)

 BOT CHORD
 2x4 SP No.1(flat)

 WEBS
 2x4 SP No.3(flat)

 OTHERS
 2x4 SP No.3(flat)

REACTIONS. All bearings 3-5-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

 	14-7-12				26-5-0				
Plate Offsets (Plate Offsets (X,Y) [4:0-1-8,Edge], [15:0-1-8,Edge], [16:0-1-8,Edge], [21:0-1-8,Edge]						11-9-	-4	
LOADING (ps TCLL 40 TCDL 10 BCLL 0 BCDL 5	f) O O .O .O	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.55 BC 0.59 WB 0.53 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.17 21-22 -0.25 21-22 0.03 14	l/defl >999 >697 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 129 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD BRACING- TOP CHORD BOT CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.3(flat) WEBS 2x4 SP No.3(flat) REACTIONS. (size) 22=0-3-8, 17=0-3-8, 14=0-3-8 Max Grav Max Grav 22=728(LC 10), 17=1669(LC 1), 14=562(LC 7)					sheathing dii icals. ectly applied o g: 17-19,16-17	rectly applied or 6-0-0 c or 10-0-0 oc bracing, F 7.	oc purlins, Except:		
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1987/0, 3-4=-1987/0, 4-5=-1740/0, 5-6=-1740/0, 6-8=0/1282, 8-9=0/1282, 9-10=-1183/0, 10-11=-1183/0, 11-12=-1183/0 BOT CHORD 21-22=0/1314, 20-21=0/1987, 19-20=0/1987, 17-19=-191/818, 16-17=-366/574, 15-16=0/1183, 14-15=0/954 WEBS 8-17=-284/0, 2-22=-1440/0, 2-21=0/745, 3-21=-260/0, 6-17=-1781/0, 6-19=0/1122, 4-19=-646/0, 9-17=-1465/0, 9-16=0/917, 10-16=-329/0, 12-14=-1044/0, 12-15=-86/311									
NOTES-	NOTES-								

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

			10-10-0			
Plate Offsets (X,Y)	[8:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.42 BC 0.35 WB 0.30 Matrix-S	DEFL. in Vert(LL) -0.08 Vert(CT) -0.11 Horz(CT) 0.02	(loc) l/defl L/d 9-10 >999 480 9-10 >999 360 7 n/a n/a	PLATES MT20 Weight: 56 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	rectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,	
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF REACTIONS (size	2 No.1(flat) 2 No.1(flat) 2 No.3(flat) 2 No.3(flat)	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	rectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,	

Max Grav 10=576(LC 1), 7=576(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1234/0, 3-4=-1234/0, 4-5=-1234/0

BOT CHORD 9-10=0/981, 8-9=0/1234, 7-8=0/982 2-10=-1073/0, 5-7=-1075/0, 5-8=0/485, 2-9=0/478, 3-9=-255/0, 4-8=-266/0 WEBS

NOTES-

 Unbalanced floor live loads have been considered for this design.
 Plates checked for a plus or minus 1 degree rotation about its center.
 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

				11-11-0			
Plate O	offsets (X,Y)	[8:0-1-8,Edge], [9:0-1-8,Edge]					
LOADI TCLL TCDL BCLL BCDL	NG (psf) 40.0 10.0 0.0 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.68 BC 0.56 WB 0.34 Matrix-S	DEFL. ir Vert(LL) -0.19 Vert(CT) -0.29 Horz(CT) 0.02	n (loc) l/defi L/d 9 9-10 >740 480 9 9-10 >490 360 2 7 n/a n/a	PLATES MT20 Weight: 59 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied o	rectly applied or 6-0-0 or 10-0-0 oc bracing.) oc purlins,	

REACTIONS. (size) 10=0-3-8, 7=0-3-8 Max Grav 10=635(LC 1), 7=635(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1508/0, 3-4=-1508/0, 4-5=-1508/0

BOT CHORD 9-10=0/1112, 8-9=0/1508, 7-8=0/1121

WEBS 2-10=-1219/0, 2-9=0/558, 5-7=-1228/0, 5-8=0/655, 4-8=-353/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

 Plates checked for a plus or minus 1 degree rotation about its center.
 Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ocliapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses sand truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

11-11-0

10

1.5x3 ||

9

3x6 =

H			<u>14-6-0</u> 14-6-0			
Plate Offsets (X,Y)	[4:0-1-8,Edge], [11:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.61 BC 0.84 WB 0.46 Matrix-S	DEFL. in Vert(LL) -0.20 Vert(CT) -0.25 Horz(CT) 0.03	(loc) l/defl L/d 9-10 >838 480 9-10 >684 360 8 n/a n/a	PLATES MT20 Weight: 73 lb	GRIP 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied or	ectly applied or 6-0-0 • 10-0-0 oc bracing.	oc purlins,

REACTIONS. (size) 12=0-3-8, 8=Mechanical

Max Grav 12=778(LC 1), 8=784(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2243/0, 3-4=-2243/0, 4-5=-2186/0, 5-6=-2186/0

BOT CHORD 11-12=0/1424, 10-11=0/2243, 9-10=0/2243, 8-9=0/1429 WEBS 2-12=-1561/0, 2-11=0/958, 3-11=-303/0, 6-8=-1573/0, 6-9=0/

EBS 2-12=-1561/0, 2-11=0/958, 3-11=-303/0, 6-8=-1573/0, 6-9=0/836, 5-9=-271/41, 4-9=-428/186

NOTES-

12

=

3x6 =

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

11

3x4 =

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

8

3x6 =

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

11-7-8								
11-7-8								
Plate Offsets (X,Y) [1:Edge,0-1-8], [8:0-1-8,Edge], [9:0-1-8,Edge]								
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.60 BC 0.51 WB 0.33 Matrix-S	DEFL. in Vert(LL) -0.16 Vert(CT) -0.25 Horz(CT) 0.02	(loc) I/defi L/d 9-10 >846 480 9-10 >540 360 7 n/a n/a	PLATES MT20 2 Weight: 58 lb	GRIP 244/190 FT = 20%F, 11%E		
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied	rectly applied or 6-0-0 o or 10-0-0 oc bracing.	oc purlins,		
REACTIONS. (size) 10=Mechanical, 7=0-3-8 Max Grav 10=626(LC 1), 7=619(LC 1)								

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1441/0, 3-4=-1441/0, 4-5=-1441/0

BOT CHORD 9-10=0/1081, 8-9=0/1441, 7-8=0/1087 WEBS 2-10=-1190/0, 2-9=0/515, 5-7=-1190/0, 5-8=0/606, 4-8=-323/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

