

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 2100217-2100217A Chelsea Roof

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I44904581 thru I44904607

My license renewal date for the state of North Carolina is December 31, 2021.

North Carolina COA: C-0844



February 22,2021

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13 and 9. This connection is for uplift only and does not consider lateral forces.













3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 177 lb uplift at joint 12.

One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8. This
connection is for uplift only and does not consider lateral forces.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system. See **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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Edenton, NC 27932



- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 2-0-0 oc.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.







- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 2-0-0 oc.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 9) will fit between the bottom chord and any other members.



2) Wind: ASCE 7-10; Vult=140mph Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12 and 8. This connection is for uplift only and does not consider lateral forces.

		· .		12-6-0
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.13	Vert(LL) -0.00 9 n/r 120 MT20 197/144
TCDL	10.0	Lumber DOL 1.15	BC 0.08	Vert(CT) -0.01 9 n/r 90
BCLL	0.0 *	Rep Stress Incr YES	WB 0.16	Horz(CT) 0.00 10 n/a n/a
BCDL	10.0	Code IRC2015/TPI2014	Matrix-R	Weight: 77 lb FT = 20%

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 12-6-0.

(lb) - Max Horz 16=209(LC 11)

 Max Uplift
 All uplift 100 b or less at joint(s) 16, 10, 14, 12 except 15=-154(LC 12), 11=-151(LC 13)

 Max Grav
 All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=140mph Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

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TRENCO AMITEK Affiliate 818 Soundside Road

Edenton, NC 27932

February 22,2021

Plate Offsets (X,Y)	[1:Edge,0-1-10], [5:0-3-8,Edge], [7:0-3-	8,0-5-0], [10:0-3-8,0-5-0]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCode IRC2015/TPI2014	CSI. TC 0.98 BC 0.54 WB 0.95 Matrix-MS	DEFL. ir Vert(LL) -0.12 Vert(CT) -0.23 Horz(CT) 0.03	n (loc) l/defl L/d 8-10 >999 240 8-10 >999 180 6 n/a n/a	PLATES GRIP MT20 197/144 MT18HS 244/190 Weight: 338 lb FT = 20%			
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF 3-8: 2x	P No.2 or 2x4 SPF No.2 P DSS P No.3 *Except* 4 SP No.2 or 2x4 SPF No.2, 1-11,5-6: 2	x4 SP No.1	BRACING- TOP CHORD BOT CHORD	Structural wood sheathir Rigid ceiling directly app	ng directly applied, except end verticals. lied or 10-0-0 oc bracing.			
REACTIONS. (siz Max H Max U Max G	REACTIONS. (size) 11=0-3-8 (req. 0-5-12), 6=0-3-8 (req. 0-5-4) SUPPLEMENTARY BEARING PLATES, SPECIAL ANCHORAGE, OR Max Horz 11=304(LC 34) OTHER MEANS TO ALLOW FOR THE MINIMUM REQUIRED SUPPORT Max Uplift 11=-1227(LC 12), 6=-1126(LC 13) WIDTH (SUCH AS COLUMN CAPS, BEARING BLOCKS, ETC.) Max Grav 11=7311(LC 2), 6=6668(LC 2) OTHER MEANS TO ALLOW FOR THE RUSS MANUFACTURER							
FORCES. (lb) - Max. TOP CHORD 1-2=- 5-6=- BOT CHORD 10-1 WEBS 3-8=- 1-10=-	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-7700/1408, 2-3=-5448/1142, 3-4=-5448/1142, 4-5=-7620/1398, 1-11=-5632/1056, 5-6=-5583/1051 BOT CHORD 10-11=-492/1438, 8-10=-1055/5840, 7-8=-979/5778, 6-7=-282/1216 WEBS 3-8=-1287/6578, 4-8=-2356/618, 4-7=-438/2785, 2-8=-2443/628, 2-10=-451/2898, 1-10=-700/4583, 5-7=-710/4604							
 NOTES- 2-bjt truss to be connected together with 10d (0.120*x3*) nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-7-0 oc. Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 co. All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been considered for this design. Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=140mph Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions show; Lumber DOL=1.60 plate grip DOL=1.60 All plates are MT20 plates unless otherwise indicated. This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. WARNING: Required bearing size at joint(s) 11, 6 greater than input bearing size. Bearing at joint(s) 11, 6 greater than input bearing size. Bearing at joint(s) 11, 6 greater than input bearing size. Bearing surface. WARNING: Required bearing size at joint(s) 11, 6 greater than input bearing size. Bearing at joint(s) 11, 6 greater than input bearing size. Bearing at joint(s) 11, 6 greater than input bearing size. Bearing at joint(s) 11, 6 greater than input bearing size. WarkING: Required bearing size at joint(s) 11, 6 greater than input bearing size. WarkING: Required bearing size at joint(s) 11, 6 greater than input bearing size. WarkING: Required bearing size at joint(s) 11, 6 greater than input bearing size. WarkING: Required bearing size at joint(s) 11, 6 greater than input bearing size. Bearing at joint(s) 11, 6 greater than input beari								
Design valid for use o a truss system. Before building design. Brac is always required for	nly with MiTek® connectors. This design is based e use, the building designer must verify the applica ing indicated is to prevent buckling of individual tru stability and to prevent collarse with possible pers	only upon parameters shown, an bility of design parameters and p ss web and/or chord members or onal injury and property damage	d is for an individual building co roperly incorporate this design hly. Additional temporary and p For general guidance regard	omponent, not into the overall permanent bracing no the	TRENCO A Mi Tek Atfiliate			

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building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	Chelsea Roof	
				-		144904592
2100217-2100217A	BGR	Common Girder	1	2		
				_	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.4	430 s Feb	12 2021 MiTek Industries, Inc. Mon Feb 22 11:26:43 2021	Page 2
		ID:vah	UOS6Oz\	/uZlvVsvut	OcUzv8L2-t8BrdYW3hszp_Bdifp7cex5BvxO9pRsW?bAw5h	Izibma

NOTES-

10) H10A-2 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 11. This connection is for uplift only and does not consider lateral forces.

- 11) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6. This connection is for uplift only and does not consider lateral forces.
- 12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1133 lb down and 195 lb up at 0-11-4, 1131 lb down and 197 lb up at 2-11-4, 1131 lb down and 197 lb up at 6-11-4, 1131 lb down and 197 lb up at 10-11-4, 1129 lb down and 197 lb up at 10-11-4, 1129 lb down and 197 lb up at 10-11-4, 1129 lb down and 197 lb up at 12-11-4, 1119 lb down and 197 lb up at 14-11-4, 1119 lb down and 197 lb up at 16-11-4, and 1119 lb down and 197 lb up at 18-11-4, an

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-5=-60, 6-11=-20 Concentrated Loads (lb)

Vert: 9=-1082(B) 7=-1080(B) 12=-1084(B) 13=-1082(B) 14=-1082(B) 15=-1082(B) 16=-1080(B) 17=-1080(B) 18=-1080(B) 19=-1080(B) 20=-1080(B)

February 22,2021

ENGINEERING BY ERENCED A MITEK Affiliate 818 Soundside Road Edenton, NC 27932

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 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not Design valid for Use only with with exerconnectors. This design is based only upon parameters shown, and is to an individual outing component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Plate Offsets (X,Y)	[7:0-3-8,0-4-12], [8:0-4-0,0-4-12], [10:0-	-3-8,0-4-12]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.78 BC 0.57 WB 0.88 Matrix-MS	DEFL. in Vert(LL) -0.09 Vert(CT) -0.18 Horz(CT) 0.02	n (loc) I/defl L/d 7-8 >999 240 7-8 >999 180 6 n/a n/a	PLATES GRIP MT20 197/144 Weight: 331 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF 5-6: 2x OTHERS 2x4 SF REACTIONS. (siz Max H Max U Max C	 No.2 or 2x4 SPF No.2 DSS No.3 *Except* (4 SP No.1 No.3 (e) 11=0-3-8, 6=0-3-8 forz 11=-372(LC 8) Jplift 11=-1099(LC 13), 6=-1044(LC 13) Grav 11=6375(LC 2), 6=6230(LC 2) 		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing Rigid ceiling directly appli	g directly applied, except end verticals. ed or 10-0-0 oc bracing.
FORCES. (lb) - Max. TOP CHORD 1-2= 5-6= BOT CHORD 10-1 WEBS 2-10. 1-10:	Comp./Max. Ten All forces 250 (lb) o -3277/711, 2-3=-3552/849, 3-4=-3619/8 -4507/856 1=-286/345, 8-10=-503/2465, 7-8=-778/ =-875/254, 2-8=-226/619, 3-8=-916/425 =-693/3811, 5-7=-510/3480	r less except when shown 23, 4-5=-6204/1138, 1-11 4692, 6-7=-287/1241 5, 4-8=-2797/697, 4-7=-4	n. =-5095/968, 92/3333,		
 NOTES- 1) 2-ply truss to be cor Top chords connect Bottom chords connect Bottom chords connect Webs connected as 2) All loads are consid ply connections hav 3) Unbalanced roof live 4) Wind: ASCE 7-10; \ gable end zone and shown; Lumber Dol 5) This truss has been will fit between the back of the provided for the back of the back of	nnected together with 10d (0.120"x3") na ted as follows: 2x4 - 1 row at 0-9-0 oc. nected as follows: 2x6 - 2 rows staggered follows: 2x4 - 1 row at 0-9-0 oc. ered equally applied to all plies, except i re been provided to distribute only loads loads have been considered for this de /ult=140mph Vasd=111mph; TCDL=6.0 I C-C Exterior(2) zone; end vertical left a L=1.60 plate grip DOL=1.60 designed for a 10.0 psf bottom chord live in designed for a live load of 20.0psf on pottom chord and any other members.	ails as follows: d at 0-5-0 oc. f noted as front (F) or bar noted as (F) or (B), unles ssign. psf; BCDL=6.0psf; h=30f nd right exposed;C-C for re load nonconcurrent wit the bottom chord in all ar ng ANSI/TPI 1 angle to g	ck (B) face in the LOAD C ss otherwise indicated. t; Cat. II; Exp B; Enclosed members and forces & M th any other live loads. eas where a rectangle 3-1 rain formula. Building de	CASE(S) section. Ply to I; MWFRS (envelope) IWFRS for reactions 6-0 tall by 2-0-0 wide signer should verify	SEAL 044925
capacity of bearing 8) Two H2.5A Simpson Continue:tionpis:	surface. n Strong-Tie connectors recommended lift only and does not consider lateral for	to connect truss to bearin	g walls due to UPLIFT at	jt(s) 11 and 6. This	February 22,2021
WARNING - Verify Design valid for use o a truss system. Befor building design. Brac	design parameters and READ NOTES ON THIS AN only with MITek® connectors. This design is based e use, the building designer must verify the applica- ing indicated is to prevent buckling of individual tru	D INCLUDED MITEK REFEREN only upon parameters shown, a bility of design parameters and ss web and/or chord members of	CE PAGE MII-7473 rev. 5/19/202 nd is for an individual building co properly incorporate this design only. Additional temporary and p	0 BEFORE USE. omponent, not into the overall permanent bracing	

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Job	Truss	Truss Type	Qty	Ply	Chelsea Roof	
2100217-2100217A	CGR	Common Girder	1	2	144	4904595
					Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.	430 s Feb	12 2021 MiTek Industries, Inc. Mon Feb 22 11:26:58 2021 Pa	age 2

NOTES-

ID:vahUOS6OzVuZlyVsvutOcUzy8L2-x0bWngiT9TshHVHa2Su8l5DoD_WDgEijSQID7vzibmR 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1124 lb down and 192 lb up at 0-7-4, 1120 lb down and 197 lb up at

2-7-4, 1120 lb down and 197 lb up at 4-7-4, 1120 lb down and 197 lb up at 6-7-4, 1120 lb down and 197 lb up at 8-7-4, 1131 lb down and 197 lb up at 10-7-4, 1131 lb down and 197 lb up at 12-7-4, 1131 lb down and 12-7-4, 1131 lb do chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 6-11=-20

Concentrated Loads (lb)

Vert: 12=-1085(F) 13=-1082(F) 14=-1082(F) 15=-1082(F) 16=-1082(F) 17=-1082(F) 18=-1082(F) 19=-1082(F) 20=-1082(F) 21=-1084(F)

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

 BOT CHORD
 2X4 SP No.2 of 2X4 SPF N

 WEBS
 2X4 SP No.3

 OTHERS
 2X4 SP No.3

 SLIDER
 Left 2x4 SP No.3 -t 1-6-0

REACTIONS. (size) 5=Mechanical, 6=0-3-8, 2=0-3-8 Max Horz 2=164(LC 9) Max Uplift 5=-69(LC 12), 6=-195(LC 12), 2=-12(LC 8) Max Grav 5=100(LC 1), 6=375(LC 1), 2=221(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 4-6=-327/378

NOTES-

 Wind: ASCE 7-10; Vult=140mph Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 2. This connection is for uplift only and does not consider lateral forces.

SEAL 044925 WGINEEPHIERUM February 22,2021

 IMREP.	_
	-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

 SLIDER
 Left 2x4 SP No.3 -t 1-6-0

 BRACING

 TOP CHORD
 Structural wood sheathing directly applied or 4-9-8 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=Mechanical, 6=0-3-8, 2=0-3-8 Max Horz 2=164(LC 9) Max Uplift 5=-69(LC 12), 6=-195(LC 12), 2=-12(LC 8) Max Grav 5=100(LC 1), 6=375(LC 1), 2=221(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 4-6=-327/378

NOTES-

1) Wind: ASCE 7-10; Vult=140mph Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 2. This connection is for uplift only and does not consider lateral forces.

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REACTIONS. (size) 1=7-8-11, 3=7-8-11, 4=7-8-11 Max Horz 1=-83(LC 8) Max Uplift 1=-45(LC 13), 3=-56(LC 13) Max Grav 1=158(LC 1), 3=158(LC 1), 4=241(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=140mph Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.

ENGINEERING BY

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February 22,2021

(lb) -Max Horz 1=114(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-156(LC 12), 6=-156(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=261(LC 19), 6=261(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

2) Wind: ASCE 7-10; Vult=140mph Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

¹⁾ Unbalanced roof live loads have been considered for this design.

REACTIONS. (size) 1=6-8-11, 3=6-8-11, 4=6-8-11 Max Horz 1=71(LC 11) Max Uplift 1=-39(LC 13), 3=-48(LC 13) Max Grav 1=136(LC 1), 3=136(LC 1), 4=206(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=140mph Vasd=111mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

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