

RE: J0121-0603 Lot 52 Sierra Village Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:Customer:Project Name: J0121-0603Lot/Block:ModAddress:SubCity:Stat

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.3 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 18 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E15377763	A01	2/9/2021
•		-	
2	E15377764	A02	2/9/2021
3	E15377765	A03	2/9/2021
4	E15377766	A04	2/9/2021
5	E15377767	A05	2/9/2021
6	E15377768	A06	2/9/2021
7	E15377769	B01	2/9/2021
8	E15377770	B02	2/9/2021
9	E15377771	B03	2/9/2021
10	E15377772	B04	2/9/2021
11	E15377773	G01	2/9/2021
12	E15377774	G02	2/9/2021
13	E15377775	M01	2/9/2021
14	E15377776	M02	2/9/2021
15	E15377777	M03	2/9/2021
16	E15377778	PS-8	2/9/2021
17	E15377779	PS-8G	2/9/2021
18	E15377780	V01	2/9/2021

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

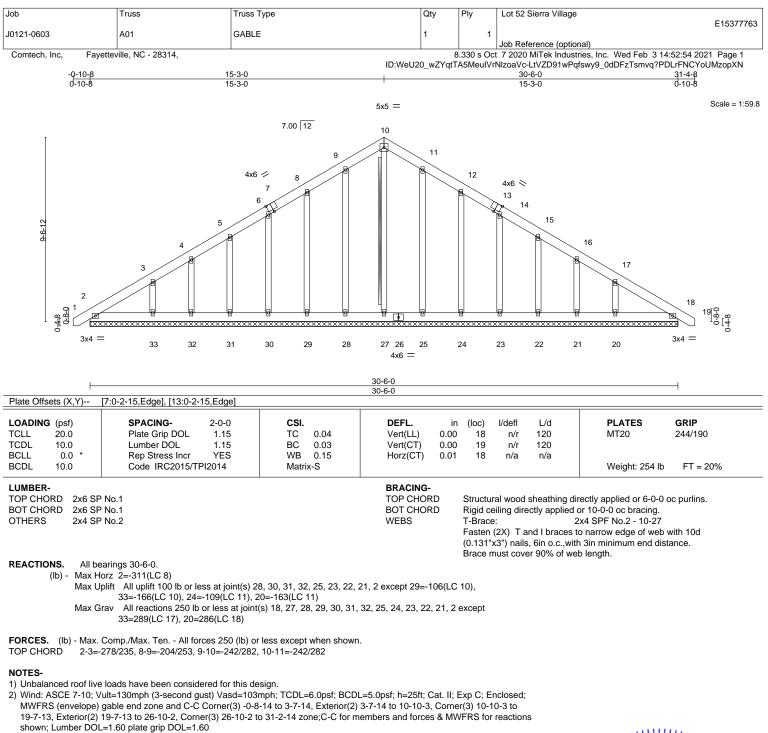
Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2021

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



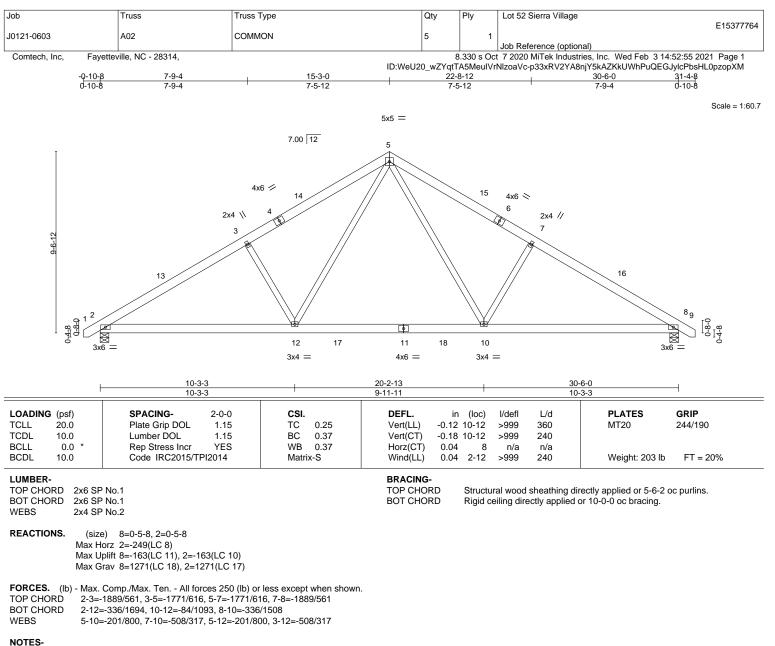


- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 28, 30, 31, 32, 25, 23, 22, 21, 2 except (jt=lb) 29=106, 33=166, 24=109, 20=163.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



ENGINEERING BY ERENCO A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932



1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-14 to 3-7-14, Interior(1) 3-7-14 to 10-10-3, Exterior(2) 10-10-3 to 19-7-13, Interior(1) 19-7-13 to 26-10-2, Exterior(2) 26-10-2 to 31-2-14 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

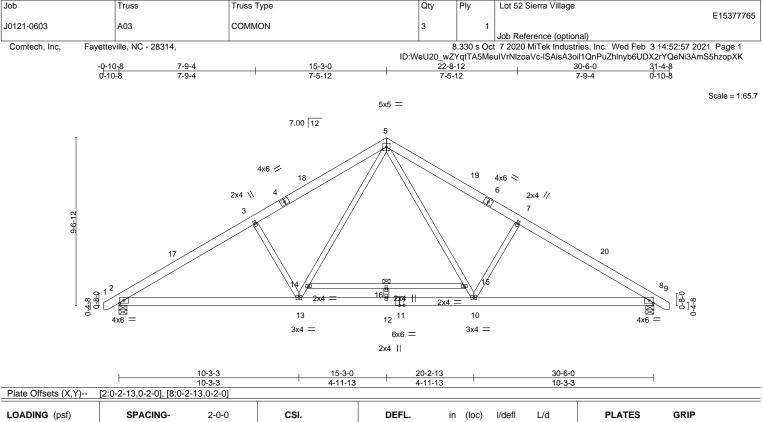
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=163, 2=163.







LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.28 BC 0.71 WB 0.42 Matrix-S	DEFL. i Vert(LL) -0.1 Vert(CT) -0.3 Horz(CT) 0.0 Wind(LL) 0.1	1 12 5 8	>999 n/a	L/d 360 240 n/a 240	PLATES GRIP MT20 244/190 Weight: 218 lb FT = 20%
LUMBER-			BRACING-				

TOP CHORD

BOT CHORD

WFBS

 LUMBER

 TOP CHORD
 2x6 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x4 SP No.2

REACTIONS. (size) 2=0-5-8, 8=0-5-8 Max Horz 2=-249(LC 8) Max Uplift 2=-212(LC 10), 8=-212(LC 11) Max Grav 2=1537(LC 1), 8=1537(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2427/750, 3-5=-2336/805, 5-7=-2336/805, 7-8=-2427/750

BOT CHORD 2-13=-496/2028, 12-13=-245/1476, 10-12=-245/1476, 8-10=-496/1972

WEBS 5-15=-310/1086, 10-15=-250/895, 7-10=-478/306, 13-14=-250/895, 5-14=-310/1086,

3-13=-478/306, 12-16=-304/104

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 10-10-3, Exterior(2) 10-10-3 to 19-7-13, Interior(1) 19-7-13 to 26-11-11, Exterior(2) 26-11-11 to 31-4-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=212, 8=212.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-60, 5-9=-60, 2-8=-20, 14-15=-60



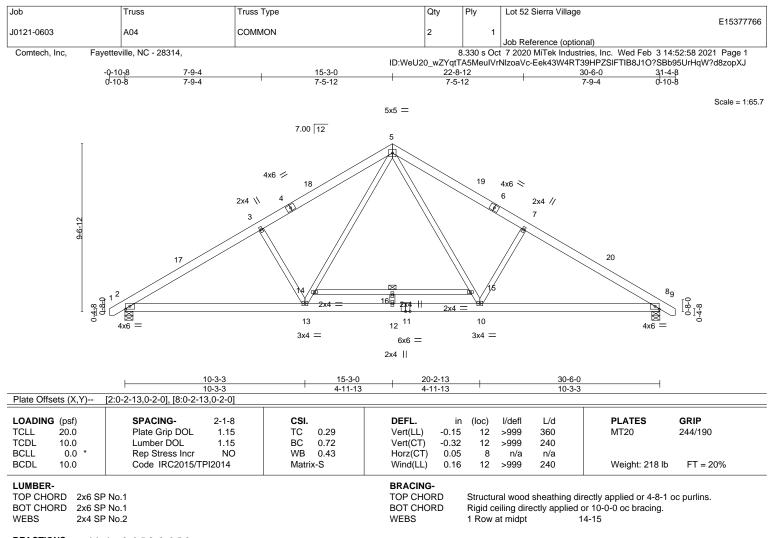
818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 4-9-11 oc purlins.

14-15

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt



REACTIONS. (size) 8=0-5-8, 2=0-5-8 Max Horz 2=-265(LC 8) Max Uplift 8=-222(LC 11), 2=-222(LC 10) Max Grav 8=1615(LC 1), 2=1615(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-2544/785, 3-5=-2446/844, 5-7=-2446/844, 7-8=-2544/785
- BOT CHORD 2-13=-517/2125, 12-13=-251/1543, 10-12=-251/1543, 8-10=-517/2066
- WEBS 5-15=-323/1134, 10-15=-262/940, 7-10=-510/326, 13-14=-262/940, 5-14=-323/1134,
- 3-13=-510/326, 12-16=-304/104

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 10-10-3, Exterior(2) 10-10-3 to 19-7-13, Interior(1) 19-7-13 to 26-11-11, Exterior(2) 26-11-11 to 31-4-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=222, 2=222.

LOAD CASE(S) Standard

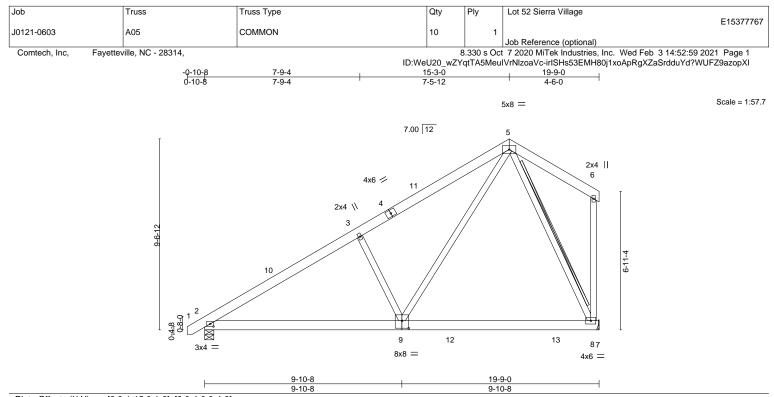
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-64, 5-9=-64, 2-8=-21, 14-15=-60







OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.25	Vert(LL) -0	.12 8-9	>999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.35	Vert(CT) -0	.17 8-9	>999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.43	Horz(CT) 0	.01 8	n/a n/a	
3CDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0	.03 2-9	>999 240	Weight: 148 lb FT = 20%

TOP CHORD

BOT CHORD

WEBS

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

WEBS 2x4 SP No.2

REACTIONS.	(size)	8=Mechanical, 2=0-5-8
	Max Horz	2=280(LC 10)
	Max Uplift	8=-139(LC 10), 2=-94(LC 10)
	Max Grav	8=875(LC 17), 2=842(LC 17)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-1097/233, 3-5=-947/312

- BOT CHORD 2-9=-367/967, 8-9=-109/316
- WEBS 3-9=-544/334, 5-9=-226/883, 5-8=-718/259

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-14 to 3-7-14, Interior(1) 3-7-14 to 10-10-3, Exterior(2) 10-10-3 to 19-5-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 8=139.

7) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c.,with 3in minimum end distance.

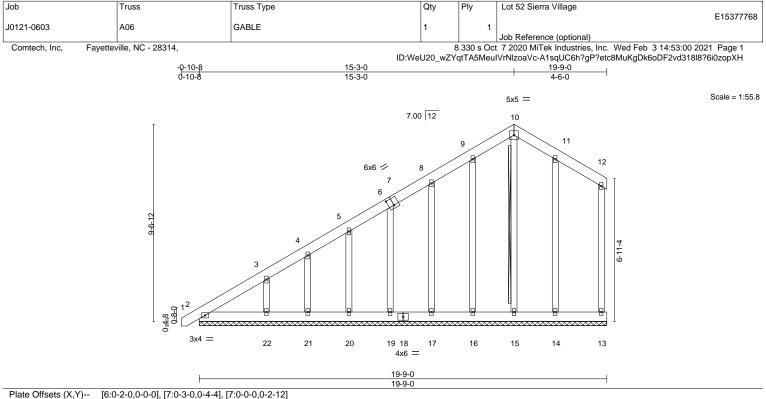
2x4 SPF No.2 - 5-8

Rigid ceiling directly applied or 10-0-0 oc bracing.

Brace must cover 90% of web length.

except end verticals.

T-Brace:



LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.06 BC 0.02 WB 0.17 Matrix-S	DEFL. i Vert(LL) -0.0 Vert(CT) 0.0 Horz(CT) 0.0	0 1	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 177 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x6 SF			BRACING- TOP CHORD		ıral wood end verti	•	rectly applied or 6-0-0 o	oc purlins,
WEBS 2x4 SF OTHERS 2x4 SF			BOT CHORD WEBS	T-Brac Fasten	е: (2X) Та	and I braces t	or 10-0-0 oc bracing. 2x4 SPF No.2 - 10-15 o narrow edge of web h 3in minimum end dist	

Brace must cover 90% of web length.

REACTIONS. All bearings 19-9-0. (lb) - Max Horz 2=411(LC

- Max Horz 2=411(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 13, 2, 16, 19, 20, 21, 14 except 17=-101(LC 10), 22=-172(LC 10)
- Max Grav All reactions 250 lb or less at joint(s) 13, 2, 15, 16, 17, 19, 20, 21, 14 except 22=290(LC 17)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.

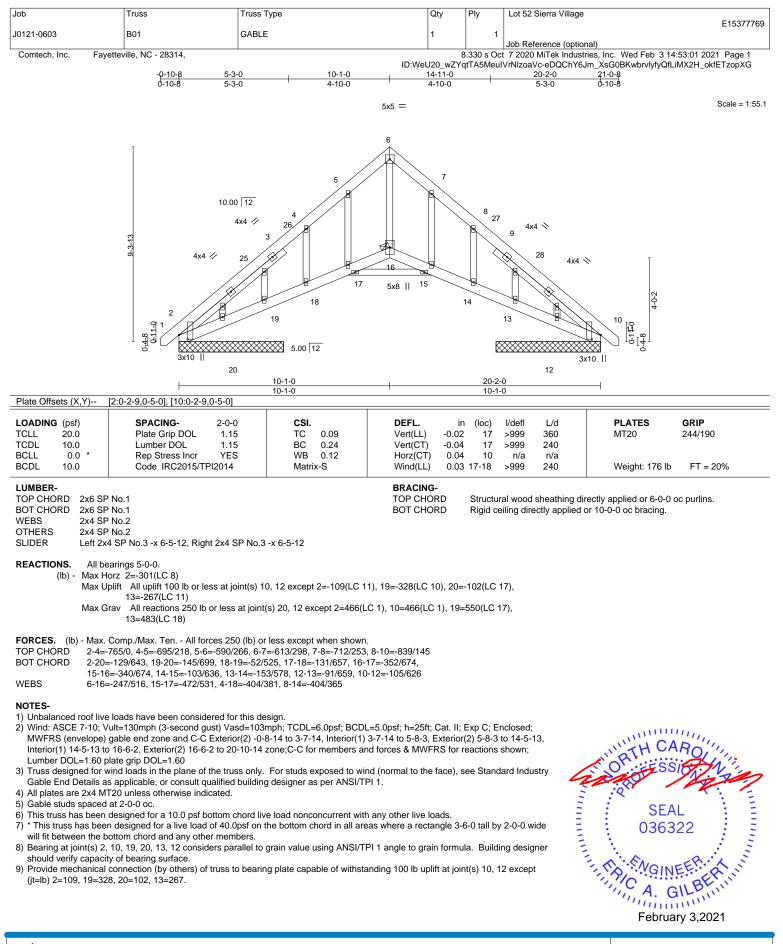
TOP CHORD 2-3=-421/293, 3-4=-301/185

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-14 to 3-7-14, Exterior(2) 3-7-14 to 10-10-3, Corner(3) 10-10-3 to 19-5-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13, 2, 16, 19, 20, 21, 14 except (jt=lb) 17=101, 22=172.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



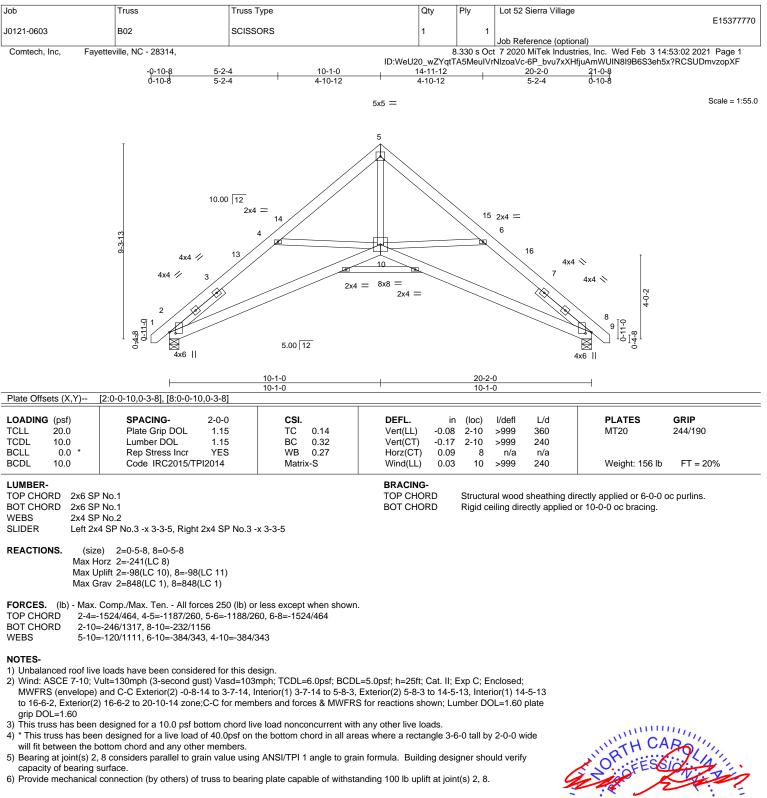




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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818 Soundside Road Edenton, NC 27932

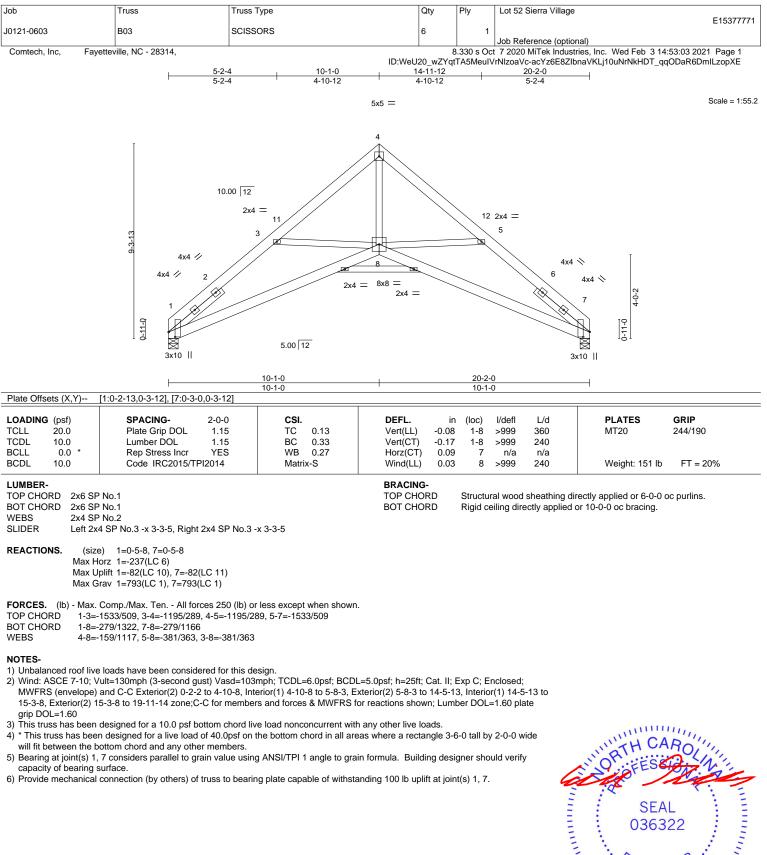




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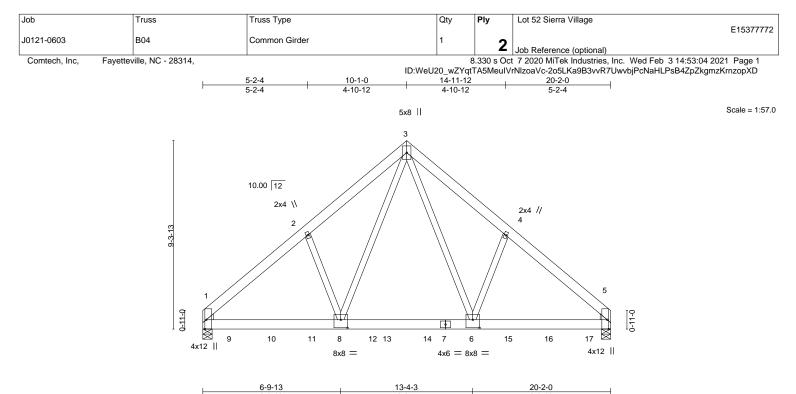




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Edenton, NC 27932



	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.62	Vert(LL)	-0.09	1-8	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.91	Vert(CT)	-0.17	1-8	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.39	Horz(CT)	0.03	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	912014	Matri	x-S	Wind(LL)	0.08	1-8	>999	240	Weight: 302 lb	FT = 20%

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS.	(size)	1=0-5-8, 5=0-5-8
	Max Horz	1=235(LC 26)
	Max Uplift	1=-819(LC 8), 5=-829(LC 9)
	Max Grav	1=4614(C2) $5=4673(C2)$

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-5031/915, 2-3=-4853/1004, 3-4=-4804/994, 4-5=-4980/905

BOT CHORD 1-8=-686/3704, 6-8=-418/2588, 5-6=-599/3606

WEBS 3-6=-666/3067, 4-6=-276/372, 3-8=-689/3174, 2-8=-276/375

NOTES-

 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=819, 5=829.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 779 lb down and 159 lb up at 1-4-4, 779 lb down and 159 lb up at 3-4-4, 779 lb down and 159 lb up at 5-4-4, 779 lb down and 159 lb up at 7-0-12, 754 lb down and 159 lb up at 11-0-12, 779 lb down and 159 lb up at 13-0-12, 774 lb down and 159 lb up at 11-0-12, 779 lb down and 159 lb up at 13-0-12, 779 lb down and 159 lb up at 15-0-12, and 779 lb down and 159 lb up at 15-0-12, and 779 lb down and 159 lb up at 15-0-12, and 779 lb down and 159 lb up at 15-0-12, and 780 lb down and 157 lb up at 19-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	Lot 52 Sierra Village	
						E15377772
J0121-0603	B04	Common Girder	1	2		
				_	Job Reference (optional)	
Comtech, Inc,	Fayetteville, NC - 28314,			8.330 s Oc	t 7 2020 MiTek Industries, Inc. Wed Fe	eb 3 14:53:04 2021 Page 2
			ID:WeU20_wZYqt	TA5MeuIV	rNlzoaVc-2o5LKa9B3vvR7UwvbjPcNaF	HLPsB4ZpZkgmzKrnzopXD

LOAD CASE(S) Standard

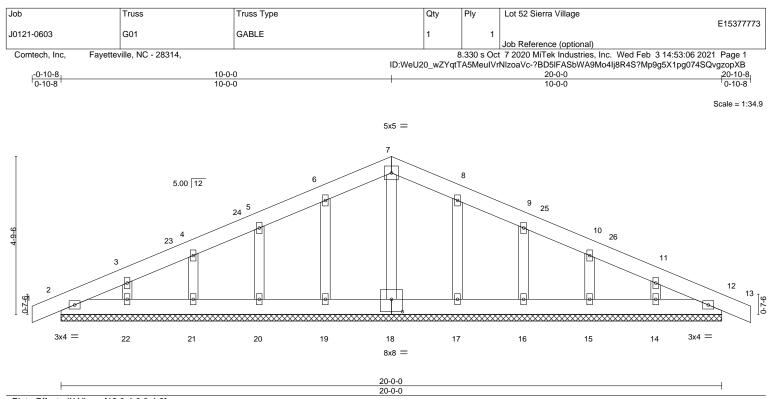
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 1-5=-20

Concentrated Loads (lb)

Vert: 6=-754(B) 8=-754(B) 9=-754(B) 10=-754(B) 11=-754(B) 13=-754(B) 14=-754(B) 15=-754(B) 16=-754(B) 17=-755(B)





LOADING (psf) FCLL 20.0 FCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.03 BC 0.01 WB 0.03	DEFL. in (loc) //defl L/d Vert(LL) -0.00 12 n/r 120 Vert(CT) -0.00 12 n/r 120 Horz(CT) 0.00 12 n/a n/a	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 130 lb FT = 20%

BOT CHORD

REACTIONS. All bearings 20-0-0.

2x4 SP No.2

(lb) -Max Horz 2=54(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 12, 19, 20, 21, 22, 17, 16, 15, 14 Max Grav All reactions 250 lb or less at joint(s) 2, 12, 18, 19, 20, 21, 22, 17, 16, 15, 14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

OTHERS

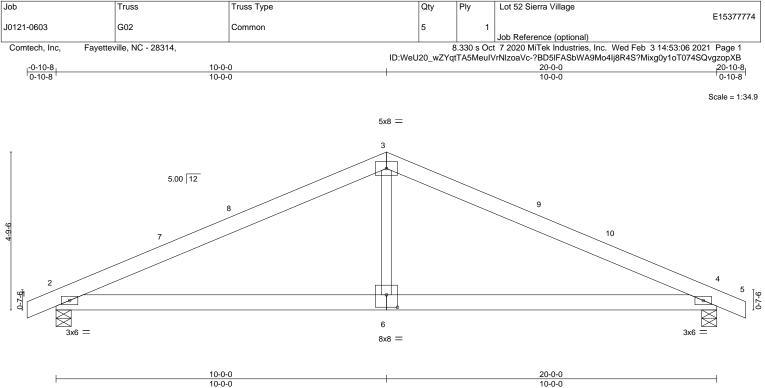
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 10-0-0, Corner(3) 10-0-0 to 14-4-13, Exterior(2) 14-4-13 to 20-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12, 19, 20, 21, 22, 17, 16, 15, 14.



818 Soundside Road Edenton, NC 27932



LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.49	Vert(LL)	-0.05	2-6	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.37	Vert(CT)	-0.12	2-6	>999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.11	Horz(CT)	0.02	4	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL)	0.04	2-6	>999	240	Weight: 108 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2

REACTIONS. (size) 2=0-5-8, 4=0-5-8 Max Horz 2=-54(LC 13) Max Uplift 2=-65(LC 12), 4=-65(LC 13) Max Grav 2=848(LC 1), 4=848(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1232/289, 3-4=-1232/289

BOT CHORD 2-6=-132/1025, 4-6=-132/1025

WEBS 3-6=0/474

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 10-0-0, Exterior(2) 10-0-0 to 14-4-13, Interior(1) 14-4-13 to 20-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

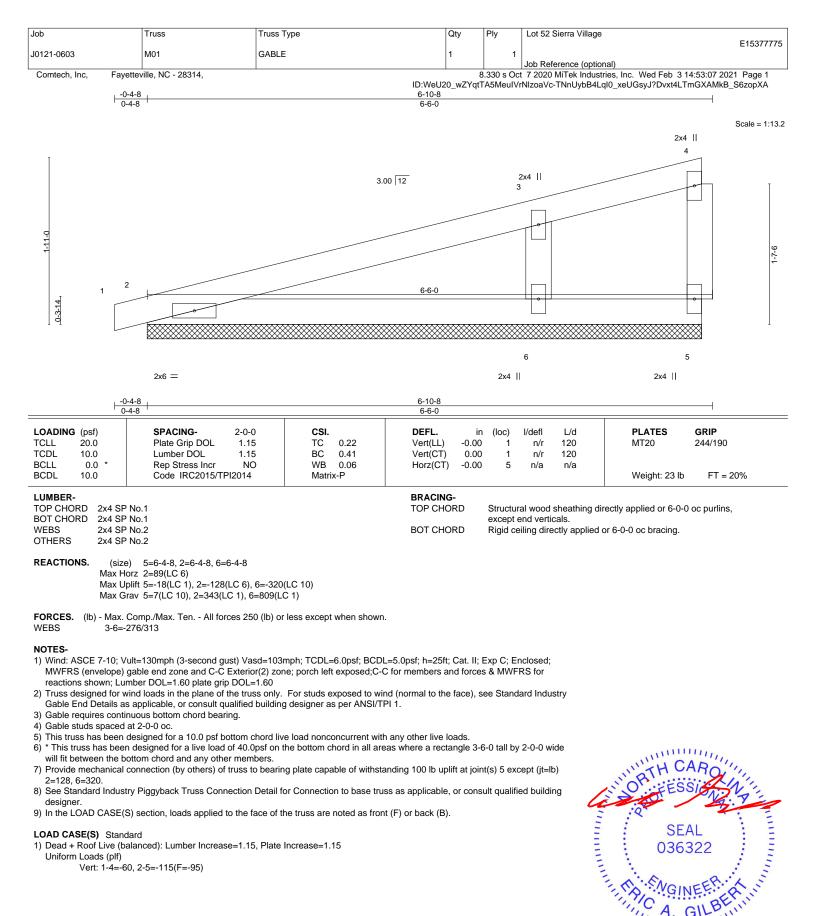
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

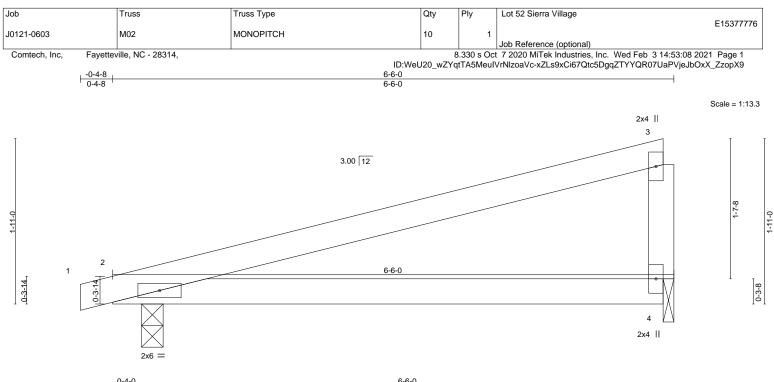




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems. see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

February 3,2021





		0-4-0				6-2-0						
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.64	Vert(LL)	-0.08	2-4	>946	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.82	Vert(CT)	-0.16	2-4	>473	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matrix	к-Р	Wind(LL)	0.19	2-4	>385	240	Weight: 22 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 4=0-1-8, 2=0-3-0

Max Horz 2=62(LC 6) Max Uplift 4=-128(LC 6), 2=-136(LC 6)

Max Grav 4=247(LC 1), 2=280(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=128, 2=136.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

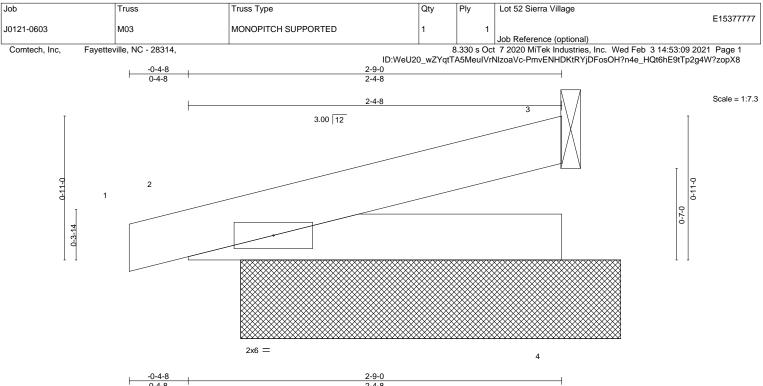


Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





		0-4-8				2-4-8						
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.22	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	-0.00	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-P	Wind(LL)	0.00	2	****	240	Weight: 8 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-4-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 2-5-0.

(lb) - Max Horz 2=36(LC 6)

Max Uplift All uplift 100 lb or less at joint(s) 3, 2 Max Grav All reactions 250 lb or less at joint(s) 3, 3, 2, 4

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

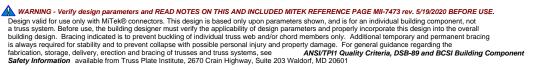
NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

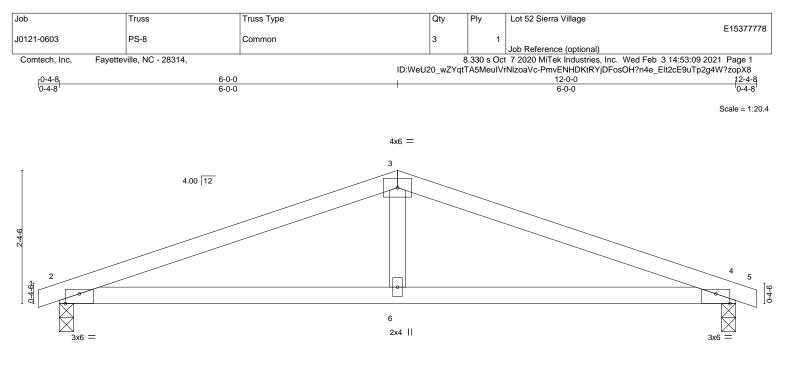
3) Gable studs spaced at 2-0-0 oc.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.









late Offsets (X,Y)	6-0-0 6-0-0 [2:0-3-0,Edge], [4:0-3-0,Edge]		<u>12-0-0</u> 6-0-0					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.39 BC 0.31 WB 0.06 Matrix-S	DEFL. in (loc) I/defl L/d Vert(LL) 0.10 2-6 >999 240 MT20 244/190 Vert(CT) -0.07 2-6 >999 240 MT20 244/190 Horz(CT) -0.01 4 n/a n/a MZ20 244/190					
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	? No.1		BRACING-TOP CHORDStructural wood sheathing directly applied or 5-10-12 oc purlins.BOT CHORDRigid ceiling directly applied or 5-10-11 oc bracing.					

EACTIONS. (size) 2=0-3-0, 4=0-3-0 Max Horz 2=28(LC 10) Max Uplift 2=-236(LC 6), 4=-236(LC 7) Max Grav 2=500(LC 1), 4=500(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-873/1166, 3-4=-873/1166

 BOT CHORD
 2-6=-1022/771, 4-6=-1022/771

BOT CHORD 2-6=-1022/771, 4-6=-1022/771 WEBS 3-6=-430/283

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

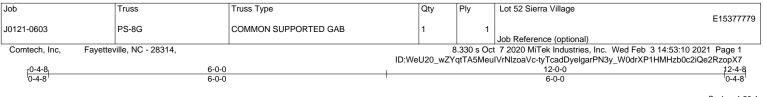
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=236, 4=236.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

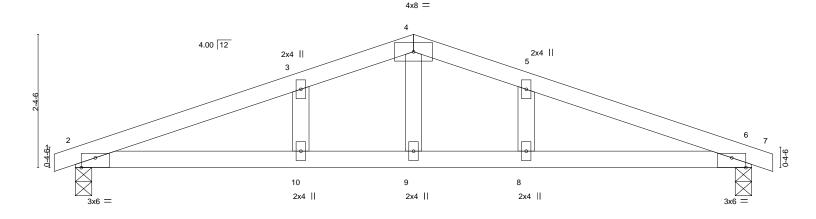
SEAL 036322 February 3,2021

TRENGINEERING BY A MITGE Affiliate 818 Soundside Road

Edenton, NC 27932



Scale = 1:20.4



OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. ii	n (loc) l/defl	L/d	PLATES	GRIP	
CLL 20.0	Plate Grip DOL 1.15	TC 0.42	Vert(LL) 0.11			MT20	244/190	
CDL 10.0 SCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr YES	BC 0.41 WB 0.07	Vert(CT) -0.09		-			
3CDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	Matrix-S	Horz(CT) -0.02	2 6 n/a	n/a	Weight: 44 lb	FT = 20%	
UMBER-			BRACING-					
TOP CHORD 2x4 SP No.1			TOP CHORD					
BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2			BOT CHORD	BOT CHORD Rigid ceiling directly applied or 5-0-8 oc bracing.				
OTHERS 2x4 SF	N0.2							
	e) 2=0-3-8, 6=0-3-8 orz 2=48(LC 14) plift 2=-325(LC 6), 6=-325(LC 7)							
	rav 2=500(LC 1), 6=500(LC 1)							

BOT CHORD 2-10=-1259/786, 9-10=-1259/786, 8-9=-1259/786, 6-8=-1259/786

WEBS 4-9=-559/291

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

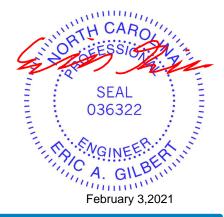
4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

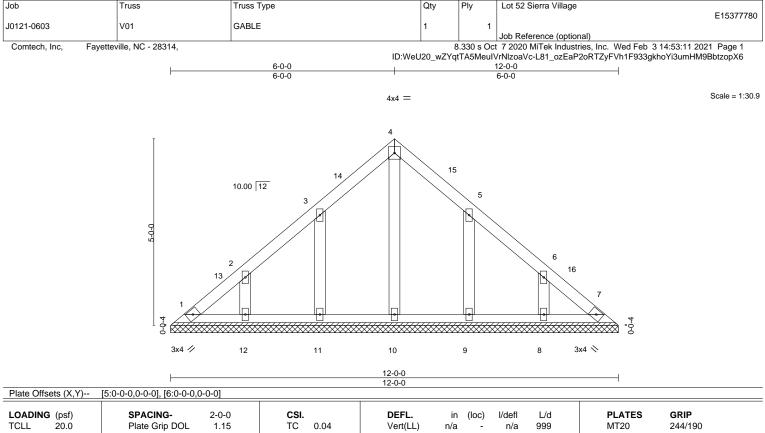
6) * This truss has been designed for a live load of 40.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=325, 6=325.

8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







LUMBER- TOP CHORD 2x4 SP No.1					BRACING- TOP CHOR					ectly applied or 6-0-0	
BCDL 10.0	0 Code IRC2015/TF	912014	Matrix	k-S						Weight: 58 lb	FT = 20%
BCLL 0.	.0 * Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	7	n/a	n/a		
TCDL 10.0	0 Lumber DOL	1.15	BC	0.02	Vert(CT)	n/a	-	n/a	999		
TCLL 20.0	0 Plate Grip DOL	1.15	TC	0.04	Vert(LL)	n/a	-	n/a	999	MT20	244/190
LOADING (pst	f) SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 12-0-0.

(lb) - Max Horz 1=-112(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 11, 12, 9, 8 Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 11, 12, 9, 8

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-0-0, Exterior(2) 6-0-0 to 10-4-13, Interior(1) 10-4-13 to 11-7-3 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11, 12, 9, 8.



