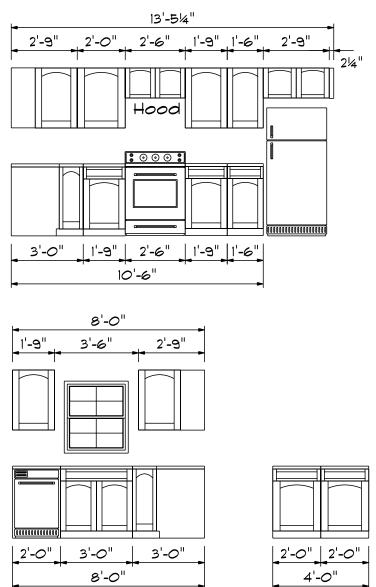


First Floor Plan

Kitchen Cabinets



Dimensions

Areas

SCALE 1/4
DRAWN BY
APPROVED

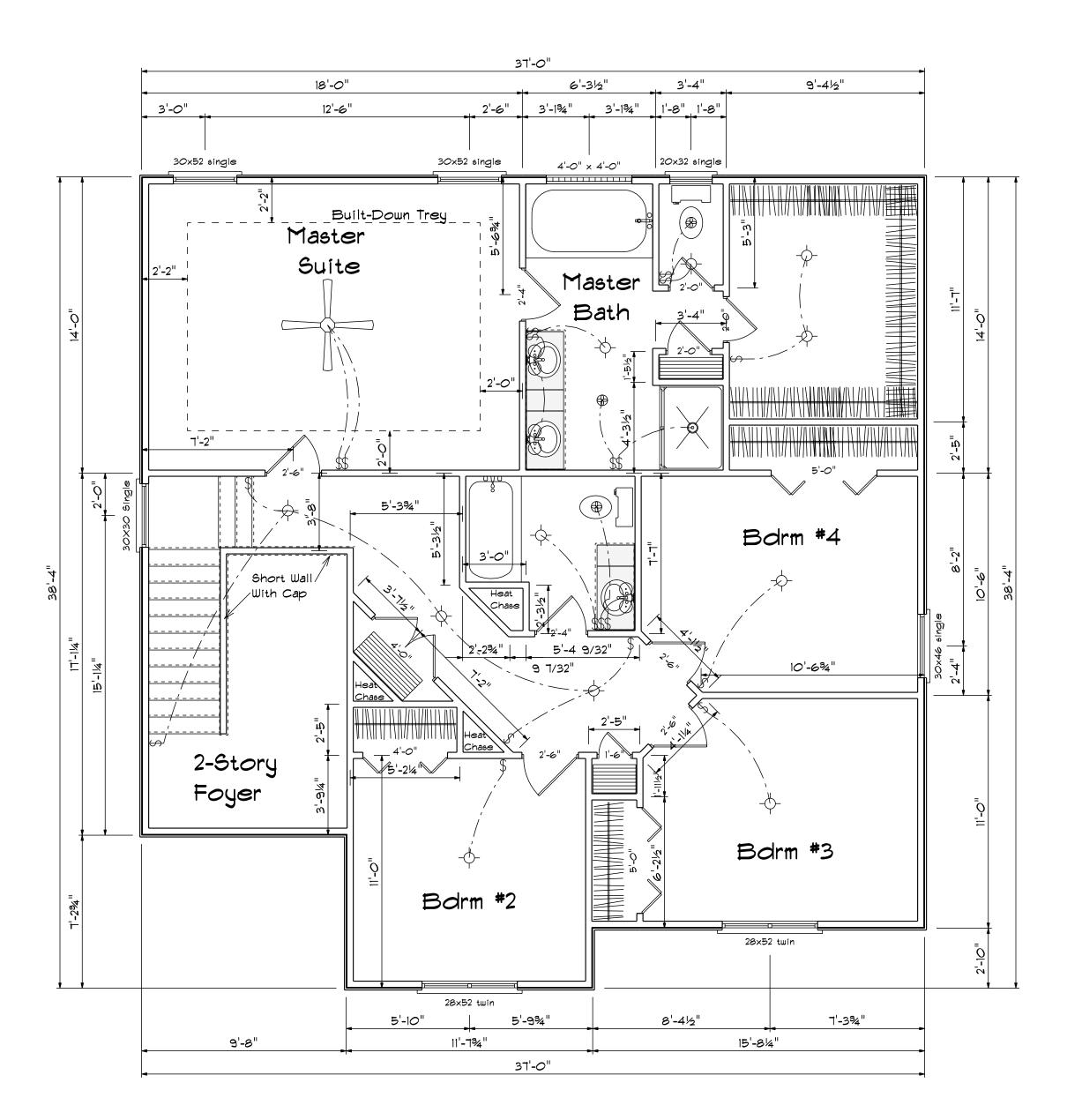
Exterior measurements are to outside of Sheathing on siding walls.

Interior measurements are to center of interior walls and outside of sheathing to exterior walls.

First Flr. Sq.ft. 1068
Second Flr. Sq.ft. 1165
Total Heated 2233
Garage 585
Porch 66

First Floor Openings

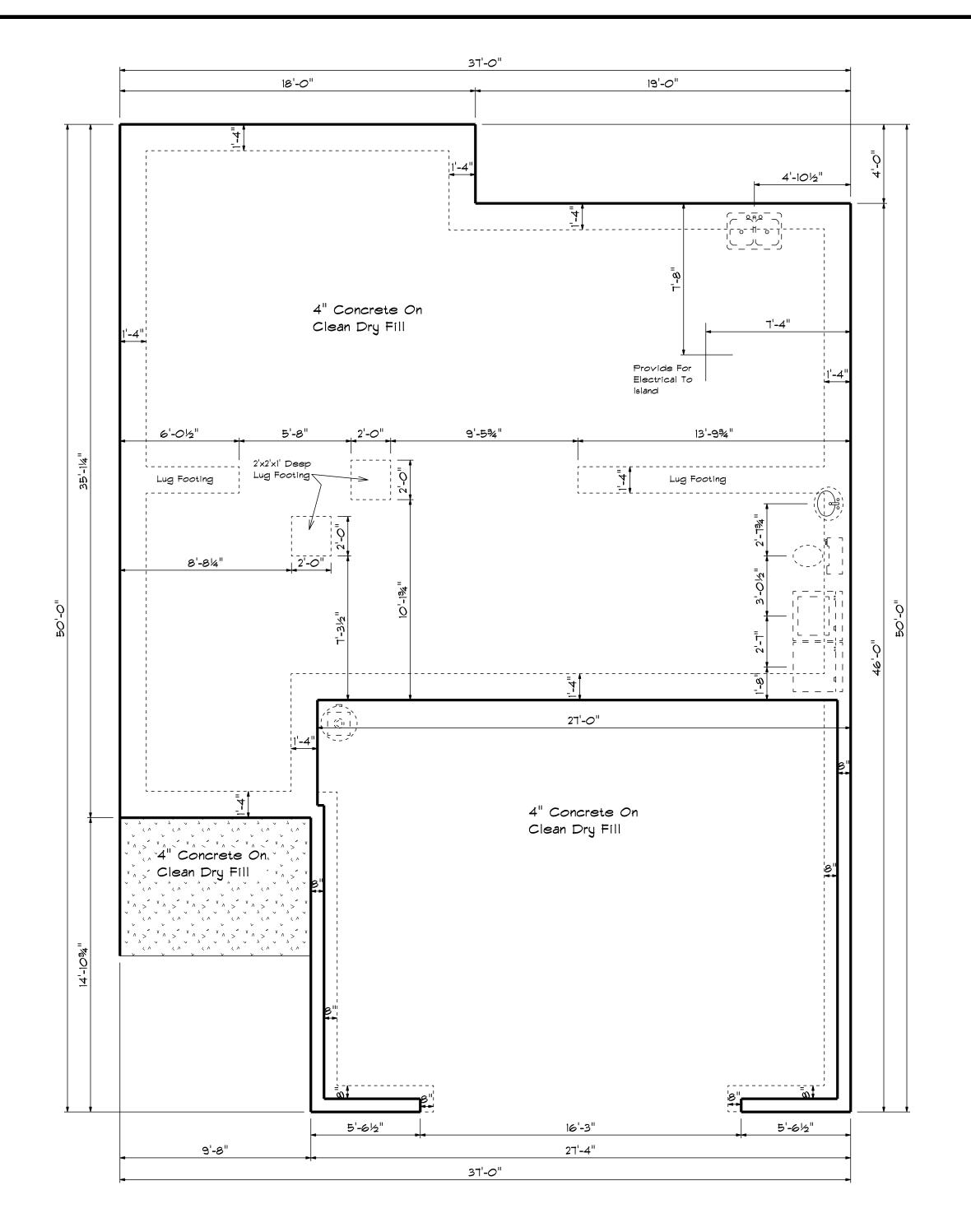
OPENING SC	HEDULE		
PRODUCT CODE	SIZE	HINGE	COUNT
30 with 1-sideRH	4'-3"	NA	1
32×80 FRENCH A 1	2'-8"	R	1
192×84 - 1 PANEL	16'-0"	и	1
36x80 BARN DOOR	2'-8"	L	1
2-0 Door Unit	2'-0"	L	1
2-0 Door Unit	2'-0"	R	1
2-4 Door Unit	2'-4"	L	1
2-8 Door Unit	2'-8"	R	1
20x32 single	2'-O" × 3'-2"	N	1
28×32 single	2'-8" × 3'-2"	N	1
28×52 twin	4'-6" × 5'-2'	NN	1
30x52 single	3'-0" × 5'-2'	' N	2
24×36 OYAL	2'-0" × 3'-0	" N	1

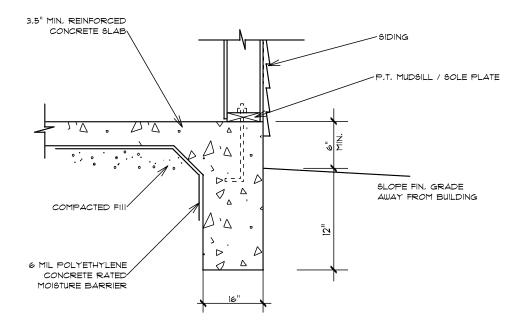


Second Floor Openings

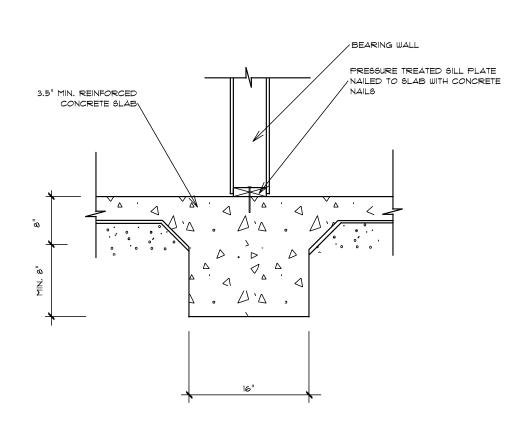
PRODUCT CODE	SIZE	HINGE DIRECTION	COUNT	R.O. WIDTH
40 Bifold	4'-0"	LR	1	4'-0"
50 Bifold	5'-0"	LR	2	5'-0"
1-6 Door Unit RH	1'-6"	R	1	1'-8"
2-0 Door Unit LH	2'-0"	L	1	2'-2"
2-0 Door Unit RH	2'-0"	R	2	2'-2"
2-4 Door Unit LH	2'-4"	L	2	2'-6"
2-6 Door Unit LH	2'-6"	L	3	2'-8"
2-6 Door Unit RH	2'-6"	R	1	2'-8"
4-0 Double Hung Door Unit	4'-0"	LR	1	4'-2"
30×30 Single	3'-0" × 3'-0"	N	1	3'-0"
28x46 single	2'-8" × 4'-6"	N	1	2'-8"
28x52 twin	4'-6" × 5'-2"	NN	2	4'-6"
30x46 single	3'-0" × 4'-6"	N	1	3'-0"
30x52 single	3'-0" × 5'-2"	N	2	3'-0"
20x32 single	2'-0" × 3'-2"	N	1	2'-0"
8X8 GLASS BLOCK	4'-0" × 4'-0"	N	1	4'-01/2"

Second Floor Plan





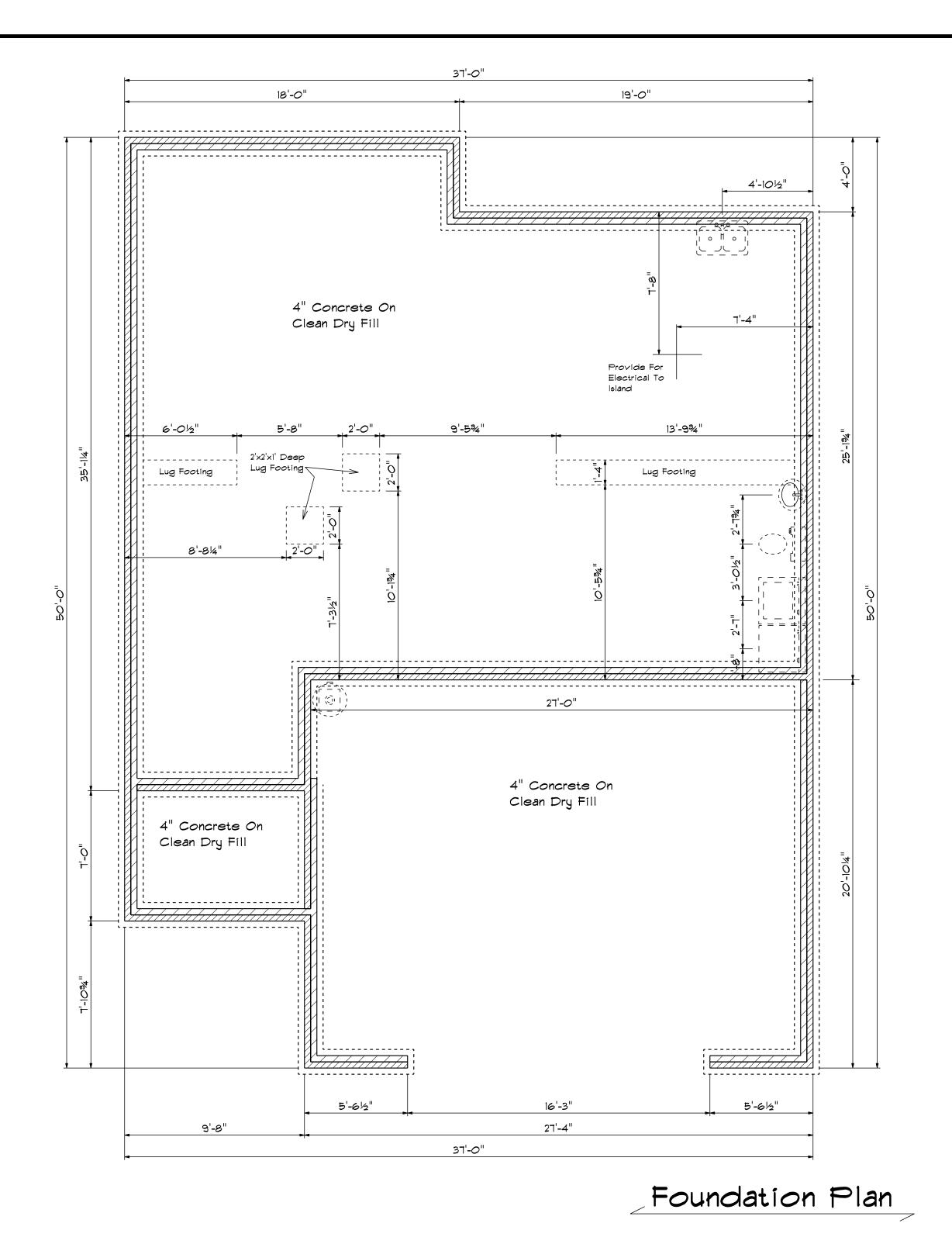
TURN-DOWN FOOTING DETAIL



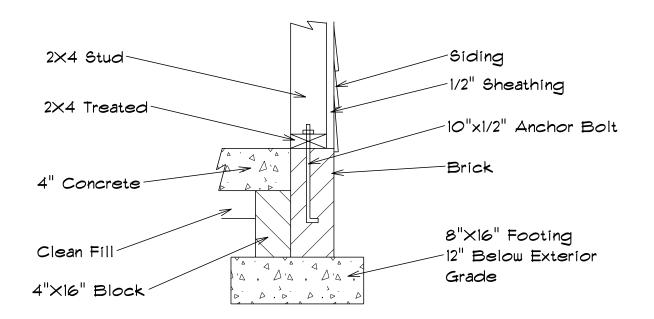
INTEGRAL SLAB FOOTING DETAIL AT BEARING WALL

Foundation Plan

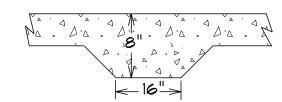
DATE Thursday, August 22, 2019	() ()	KEY GED	# ! !
8CALE 1/4"		DRAWN BY	
		カガオ-222	



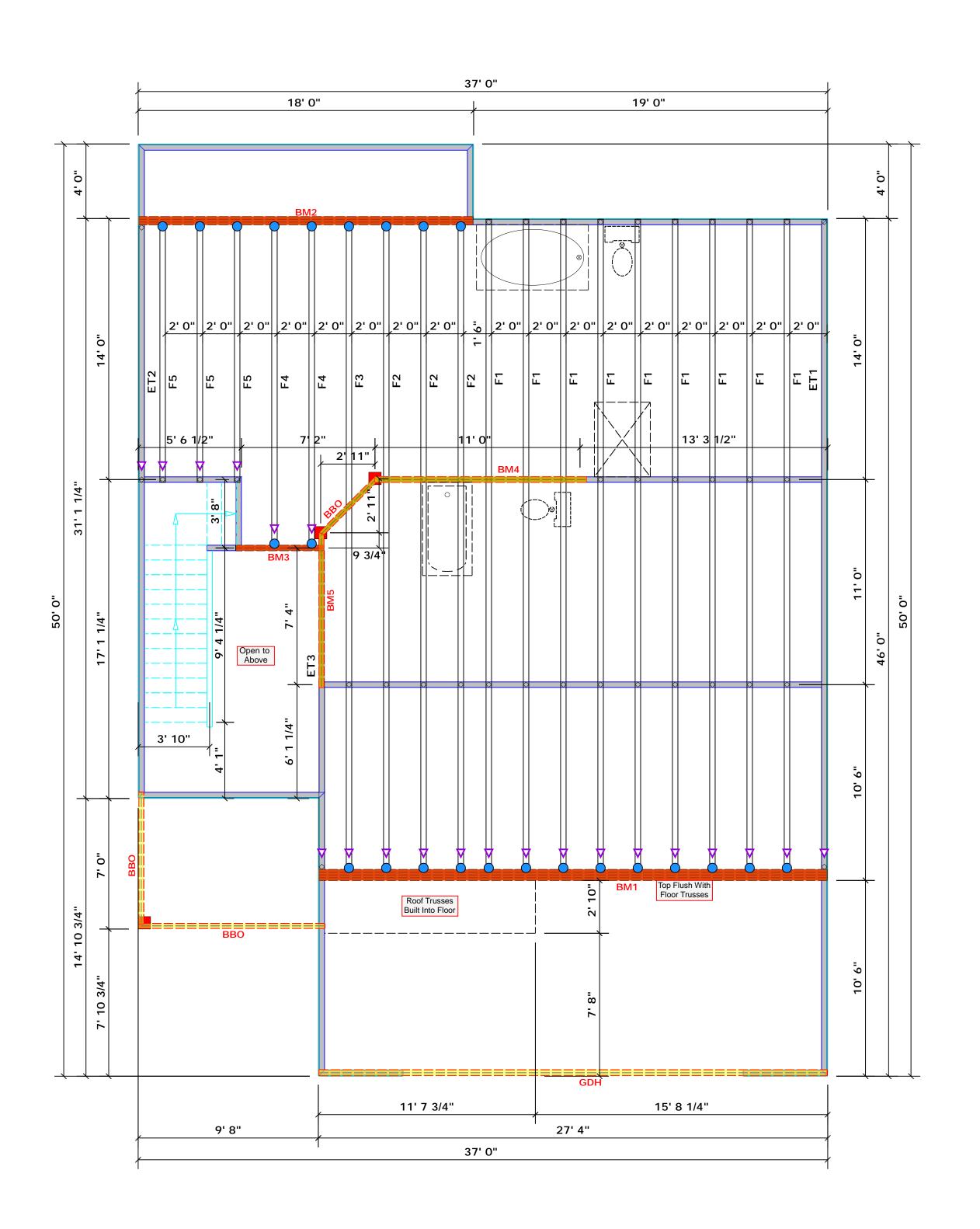
Foundation Detail Siding



Lug Footing Detail



BBH-2221



Dimension Notes

1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
2. All interior wall dimensions are to face of frame wall unless noted otherwise
3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

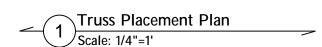
All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes

1. Plumbing drop locations shown are NOT exact.
2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
3. Adjust spacing as needed not to exceed 24"oc.

	Conne	ctor Info	rmati	ion	Nail Info	rmation	
Sym	Product	Manuf	Qty	Supported Member	Header	Truss	
	HUS410	USP	24	Varies	16d/3-1/2"	16d/3-1/2"	

		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	28' 0"	1-3/4"x 23-7/8" LVL Kerto-S	4	4
BM2	18' 0"	1-3/4"x 16" LVL Kerto-S	3	3
BM3	5' 0"	1-3/4"x 14" LVL Kerto-S	2	2
BM4	12' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
BM5	10' 0"	2x10 SPF No.2	2	2
GDH	28' 0"	1-3/4"x 16" LVL Kerto-S	2	2



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature_

David Landry

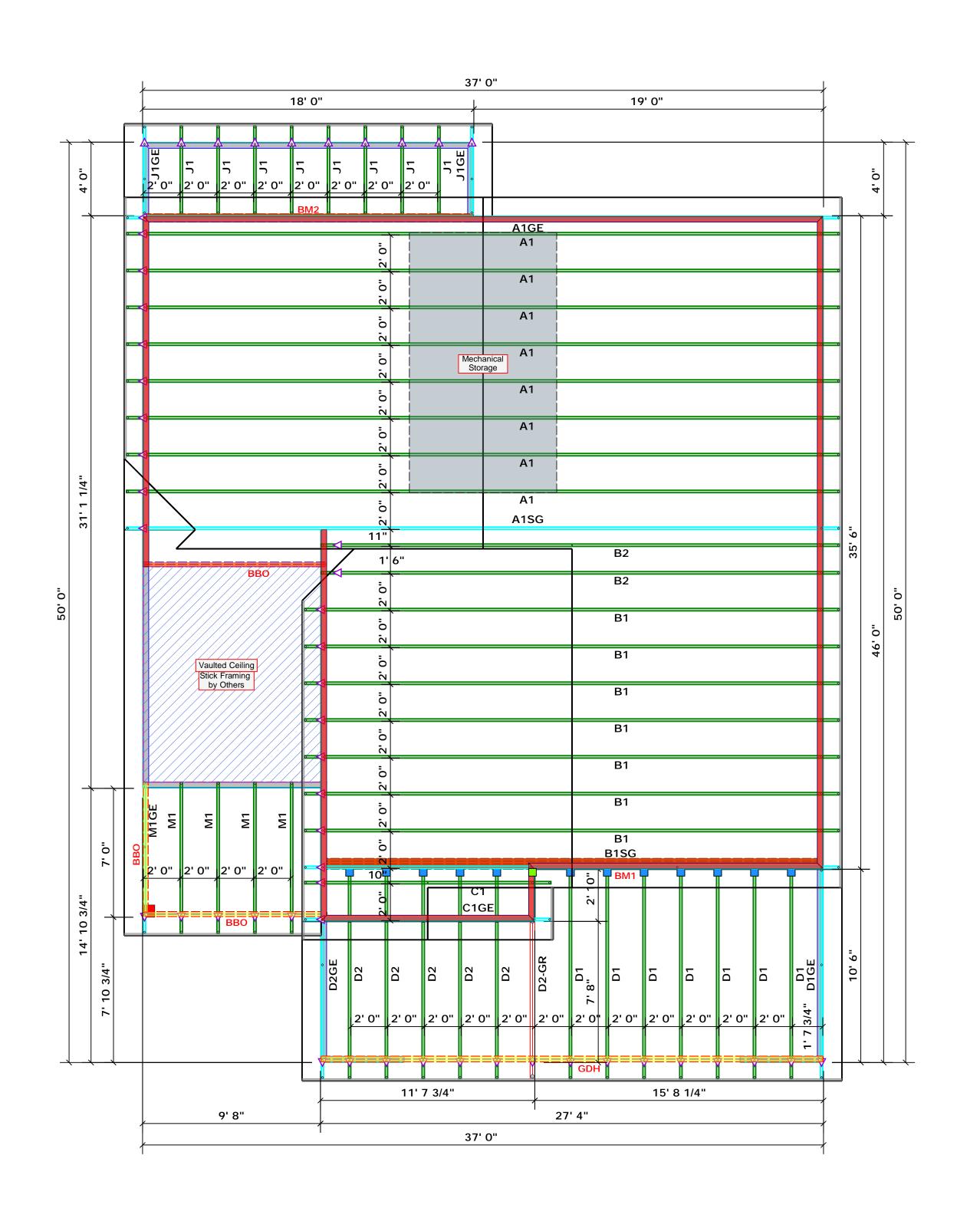
LOAD CHART FOR JACK STUDS (845ED ON TABLES RECED(I) & (b))

3400 2 5100 2 6800 2 5100 3 7650 3 10200 3 6800 4 13600 4 10200 4 8500 5 12750 5 17000 5 10200 6 15300 6 11900 7 13600 8 15300 9

COUNTYCumberlandADDRESS15 North Dakota CtMODELRoofDATE REV.01/07/21DRAWN BYDavid LandrySALESMANMarshall Naylor

BUILDERBen Stout Real EstateJOB NAMELot 1 Sierra VillasPLANWilmingtonSEAL DATEN/AQUOTE #Quote #JOB #JO121-0103

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



Dimension Notes

1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
2. All interior wall dimensions are to face of frame wall unless noted otherwise
3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Roof Area = 2292.76 sq.ft. Ridge Line = 61.53 ft. Hip Line = 0 ft. Horiz. OH = 168.47 ft. Raked OH = 216.04 ft. Decking = 79 sheets

Hatch Legend
Padded HVAC
Second Floor Walls
Vaulted Ceiling

	Conne	Connector Information						
Sym	Product	Manuf	Qty	Supported Member	Header	Truss		
	HUS26	USP	12	Varies	16d/3-1/2"	16d/3-1/2"		
	THD26-2	USP	1	Varies	16d/3-1/2"	10d/3"		

PlotID	Length	Product	Plies	Net Qty
BM1	28' 0"	1-3/4"x 23-7/8" LVL Kerto-S	4	4
BM2	18' 0"	1-3/4"x 16" LVL Kerto-S	3	3
ВМ3	5' 0"	1-3/4"x 14" LVL Kerto-S	2	2
BM4	12' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
BM5	10' 0"	2x10 SPF No.2	2	2
GDH	28' 0"	1-3/4"x 16" LVL Kerto-S	2	2

Truss Placement Plan
Scale: 1/4"=1'



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature_

David Landry

LOAD CHART FOR JACK STUDS

(8ASÉD ON TABLÉS ROCES(1) & (b)) NUMBER OF JACK STUDS REQUIRED © EA END OF HEADER/GIRDER END REACTION (LE TO) REQ'D STUDS FOR (3) ALY HEADER 1700 1 2550 1 3400 1 3400 2 5100 2 6800 2 5100 3 7650 3 10200 3 6800 4 10200 4 13600 4 8500 5 12750 5 17000 5 10200 6 15300 6 11900 7

	SALESMAN Marshall Naylor	SALESMAN	
	DRAWN BY David Landry	DRAWN BY	
	01/07/21	DATE REV . 01/07/21	
	Roof	MODEL	
8 9	15 North Dakota Ct	ADDRESS	
13600 15300	Cumberland	COUNTY	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

SEAL DATE

J0121-0103

Quote #

#

QUOTE 7

Ben Stout Real Estate

BUILDER

Villas

Lot 1 Sierra

JOB NAME

Wilmington

PLAN

Client:

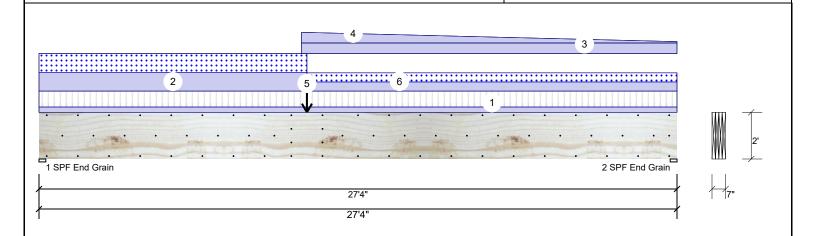
Project: Address: Ben Stout Real Estate

1/7/2021 Date:

David Landry Input by: Job Name: Lot 1 Sierra Villas Project #: J0121-0104

4-Ply - PASSED Kerto-S LVL 1.750" X 24.000" BM₁

Level: Level



Member Information Type: Plies: 4 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal Temperature: Temp <= 100°F

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: Yes Deck: Not Checked

Reactions	UNPATTI	Uplift)			
Brg	Live	Dead	Snow	Wind	Const
1	2460	5185	2893	0	0
2	2460	5403	2018	0	0

Analysis Results Analysis Actual Location Allowed Comb. Case Capacity 12'7 1/2" 175059 ft-lb Moment 64272 ft-lb 0.367 (37%) D+0.75(L+S) L Unbraced 64272 ft-lb 12'7 1/2" 64355 ft-lb 0.999 D+0.75(L+S) L (100%)7607 lb 25'1 3/8" 35840 lb 0.212 (21%) D+L Shear LL Defl inch 0.221 (L/1459) 13'3 7/8" 0.672 (L/480) 0.330 (33%) 0.75(L+S) TL Defl inch 0.553 (L/584) 13'6 3/16" 0.897 (L/360) 0.620 (62%) D+0.75(L+S) L

Bearings Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.500" 5185 / 4015 9200 L D+0.75(L+S) End Grain 5403 / 3359 D+0.75(L+S) 2 - SPF 3.500" 8762 L End

Design Notes

- 1 Fasten all plies using 3 rows of WS6 at 16" o.c. Maximum end distance not to exceed 8".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Simpson fasteners applied from a single side of the member use tip values where published.
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top loads must be supported equally by all plies.
- 7 Top must be laterally braced at a maximum of 5'6 3/8" o.c.
- 8 Bottom braced at bearings.
- 9 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	60 PLF	180 PLF	0 PLF	0 PLF	0 PLF	F1-F3
2	Part. Uniform	0-0-0 to 11-5-12		Near Face	212 PLF	0 PLF	212 PLF	0 PLF	0 PLF	D2
3	Part. Uniform	11-3-0 to 27-4-0		Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Above
4	Tapered Start	11-3-0		Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	B1SG

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Grain

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 1 of 13



Continued from page 1

Part. Uniform

6

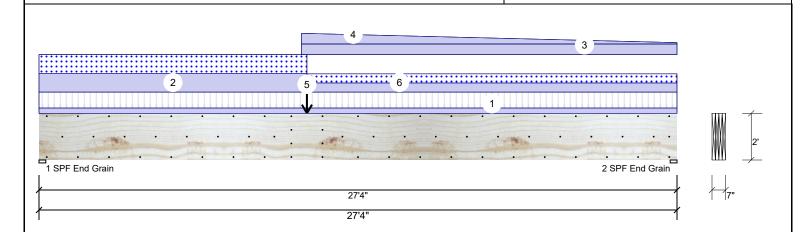
Client: Ben Stout Real Estate

Project: Address: Date: 1/7/2021

Input by: David Landry Job Name: Lot 1 Sierra Villas Project #: J0121-0104

1.750" X 24.000" **Kerto-S LVL** 4-Ply - PASSED BM₁

Level: Level



103 PLF

0 PI F

103 PLF

Continued from p	age i								
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25
	End	27-4-0			15 PLF	0 PLF	0 PLF	0 PLF	0 PLF
5	Point	11-5-12		Near Face	845 lb	0 lb	845 lb	0 lb	0 lb

Near Face

Self Weight 37 PLF

11-5-12 to 27-4-0

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- This design is valid until 2/26/2023

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS

Comments

D2-GR

0 PLF D1

0 PLF



Page 2 of 13



Client: Project: Address:

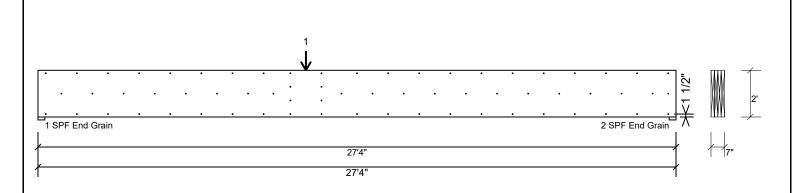
Ben Stout Real Estate

Date: 1/7/2021

Input by: David Landry Job Name: Lot 1 Sierra Villas Project #: J0121-0104

Kerto-S LVL 4-Ply - PASSED 1.750" X 24.000" BM₁

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of WS6 at 16" o.c.. except for regions covered by concentrated load fastening. Fasteners shall be replicated on both sides. Maximum end distance not to exceed 8"

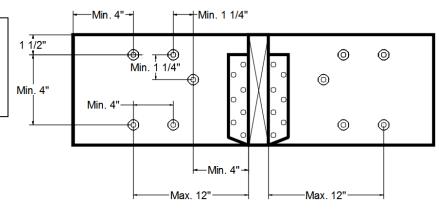
Capacity	67.9 %	
Load	318.0 PLF	
Yield Limit per Foot	468.3 PLF	
Yield Limit per Fastener	208.2 lb.	
Yield Mode	Lookup	
Edge Distance	1 1/2"	
Min. End Distance	4"	
Load Combination	D+S	
Duration Factor	1.15	

Concentrated Load

Fasten at concentrated side load at 11-5-12 with a minimum of (8) – WS6 in the pattern shown. All fasteners shall be installed with the head on the side of the annlied load

of the applied load.	
Capacity	76.1 %
Load	1267.5lb.
Total Yield Limit	1665.2 lb.
Cg	1.0000
Yield Limit per Fastener	208.2 lb.
Yield Mode	Lookup
Load Combination	D+S
Duration Factor	1.15

Min/Max fastener distances for Concentrated Side Loads



Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood Norwalk, CT 06851 (800) 622-5850

This design is valid until 2/26/2023

Manufacturer Info

301 Merritt 7 Building, 2nd Floor www.metsawood.com/us ICC-ES: ESR-3633

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 3 of 13



Client:

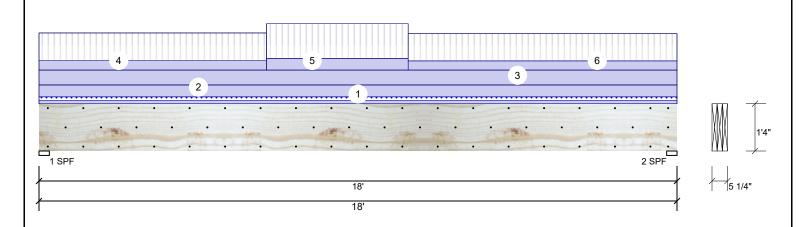
Project: Address: Ben Stout Real Estate

1/7/2021

Input by: David Landry Job Name: Lot 1 Sierra Villas Project #: J0121-0104

1.750" X 16.000" **Kerto-S LVL** 3-Ply - PASSED BM₂

Level: Level



Member Information					Reactions UNPATTERNED lb (Uplift)							
Туре:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wi	ind	Const		
Plies:	3	Design Method:	ASD	1	2662	3805	315		0	0		
Moisture Condition	on: Dry	Building Code:	IBC/IRC 2015	2	2621	3790	315		0	0		
Deflection LL:	480	Load Sharing:	Yes									
Deflection TL:	360	Deck:	Not Checked									
Importance:	Normal											
Temperature:	Temp <= 100°F											
				Bearings								
				Bearing	Length	Cap. Rea	ct D/L lb	Total L	d. Case	Ld. Comb.		
				1 - SPF	3.500"	83% 380	05 / 2662	6467 L		D+L		
				2 - SPF	3.500"	82% 379	90 / 2621	6410 L		D+L		

Analysis Results

	•						
ſ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	28237 ft-lb	8'11 3/16"	53922 ft-lb	0.524 (52%)	D+L	L
	Unbraced	28237 ft-lb	8'11 3/16"	28334 ft-lb	0.997 (100%)	D+L	L
l	Shear	5430 lb	1'6 5/8"	17920 lb	0.303 (30%)	D+L	L
l	LL Defl inch	0.197 (L/1069)	8'11 11/16"	0.439 (L/480)	0.450 (45%)	L	L
l	TL Defl inch	0.473 (L/445)	8'11 13/16"	0.585 (L/360)	0.810 (81%)	D+L	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 6'1 1/2" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	35 PLF	0 PLF	35 PLF	0 PLF	0 PLF	J1
2	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Above
3	Uniform			Тор	150 PLF	0 PLF	0 PLF	0 PLF	0 PLF	A1GE
4	Part. Uniform	0-0-0 to 6-5-0		Тор	94 PLF	280 PLF	0 PLF	0 PLF	0 PLF	F5
5	Part. Uniform	6-5-0 to 10-5-0		Тор	117 PLF	350 PLF	0 PLF	0 PLF	0 PLF	F4
6	Part. Uniform	10-5-0 to 18-0-0		Тор	92 PLF	275 PLF	0 PLF	0 PLF	0 PLF	F2 & F3
	Self Weight				19 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 2 Damaged Beams must not be used

- Danaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 4 of 13

Client:

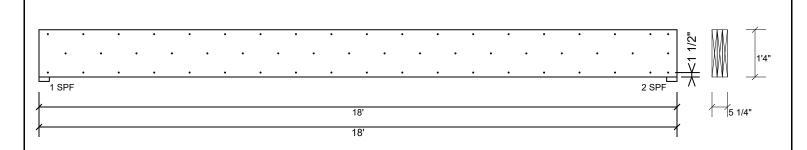
Project: Address: Ben Stout Real Estate

1/7/2021

Input by: David Landry Job Name: Lot 1 Sierra Villas Project #: J0121-0104

1.750" X 16.000" **Kerto-S LVL** 3-Ply - PASSED BM₂

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed

Capacity	16.5 %
Load	46.7 PLF
Yield Limit per Foot	282.4 PLF
Yield Limit per Fastener	94.1 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	D+S
Duration Factor	1 15

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used

- Danaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 5 of 13



Client:

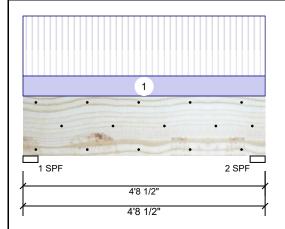
Project: Address: Ben Stout Real Estate

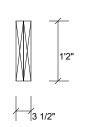
Date: 1/7/2021

Input by: David Landry Job Name: Lot 1 Sierra Villas Project #: J0121-0104

1.750" X 14.000" **Kerto-S LVL** 2-Ply - PASSED BM₃

Level: Level





D+I

Page 6 of 13

Member Information Reactions UNPATTERNED Ib (Uplift) Application: Brg Dead Snow Wind Type: Floor Live Const Plies: 2 Design Method: ASD 824 301 0 0 0 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 824 301 0 0 0 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal Temp <= 100°F Temperature: **Bearings** Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.500" 1125 L D+L 301 / 824

2 - SPF 3.500"

22%

301 / 824

1125 L

Analysis Results

,						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1079 ft-lb	2'4 1/4"	26999 ft-lb	0.040 (4%)	D+L	L
Unbraced	1079 ft-lb	2'4 1/4"	21231 ft-lb	0.051 (5%)	D+L	L
Shear	1003 lb	1'4 3/4"	10453 lb	0.096 (10%)	D+L	L
LL Defl inch	0.003 (L/14727)	2'4 5/16"	0.106 (L/480)	0.030 (3%)	L	L
TL Defl inch	0.005 (L/10786)	2'4 5/16"	0.142 (L/360)	0.030 (3%)	D+L	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings
- 5 Bottom braced at bearings.
- 6. Lateral clanderness ratio based on single ply width

o Lateral sienderness ratio based on single ply width.											
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Far Face	117 PLF	350 PLF	0 PLF	0 PLF	0 PLF	F4	
	Self Weight				11 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



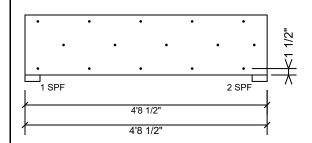
Client: Ben Stout Real Estate

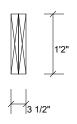
Project: Address: Date: 1/7/2021 Input by:

David Landry Job Name: Lot 1 Sierra Villas Project #: J0121-0104

1.750" X 14.000" 2-Ply - PASSED **Kerto-S LVL** BM₃

Level: Level





Page 7 of 13

Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

raster an piles asing 5 rows	or 100 box 110113 (.120x3) at
Capacity	95.1 %
Load	233.5 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	D+L
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





Client: Ben Stout Real Estate

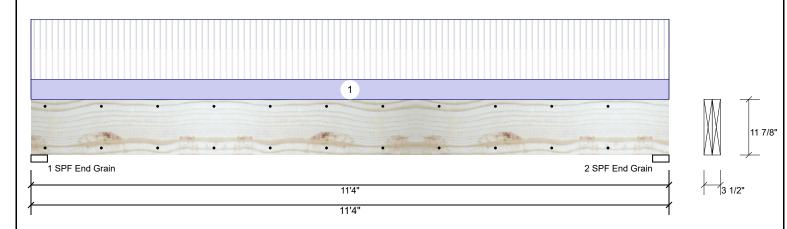
Project: Address:

Date: 1/7/2021

Input by: David Landry Job Name: Lot 1 Sierra Villas Project #: J0121-0104

evel: Level

1.750" X 11.875" 2-Ply - PASSED **Kerto-S LVL** BM4



Member Information Reactions UNPATTERNED Ib (Uplift) Application: Brg Snow Wind Type: Floor Live Dead Plies: 2 Design Method: ASD 3088 1084 0 0 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 3088 1084 0 0 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal Temp <= 100°F Temperature: **Bearings** Bearing Length Cap. React D/L lb Total Ld. Case 1-SPF 3.500" 1084 / 3088 4172 L End Grain

Ana	ly	sis	Resu	lts
_		-		

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10884 ft-lb	5'8"	19911 ft-lb	0.547 (55%)	D+L	L
Unbraced	10884 ft-lb	5'8"	10893 ft-lb	0.999 (100%)	D+L	L
Shear	3275 lb	10'1 3/8"	8867 lb	0.369 (37%)	D+L	L
LL Defl inch	0.198 (L/659)	5'8"	0.272 (L/480)	0.730 (73%)	L	L
TL Defl inch	0.267 (L/488)	5'8"	0.362 (L/360)	0.740 (74%)	D+L	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 8'1 1/8" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	182 PLF	545 PLF	0 PLF	0 PLF	0 PLF	F1 & F2
	Self Weight				9 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

1084 / 3088

4172 L

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 8 of 13

Const

0

0

Ld. Comb.

D+L

This design is valid until 2/26/2023 CSD DESIGN

2 - SPF 3.500"

End Grain

Client: Ben Stout Real Estate

Project: Address: Date: 1/7/2021

Input by: David Landry Job Name: Lot 1 Sierra Villas Project #: J0121-0104

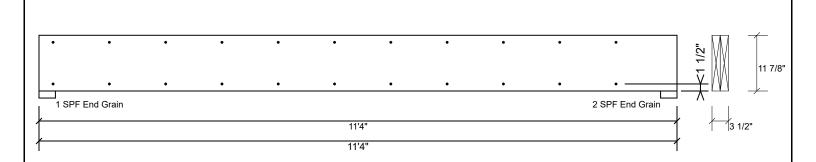
Page 9 of 13

Kerto-S LVL BM4

1.750" X 11.875"

2-Ply - PASSED

evel: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850

www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS







Client: Project: Address: Ben Stout Real Estate

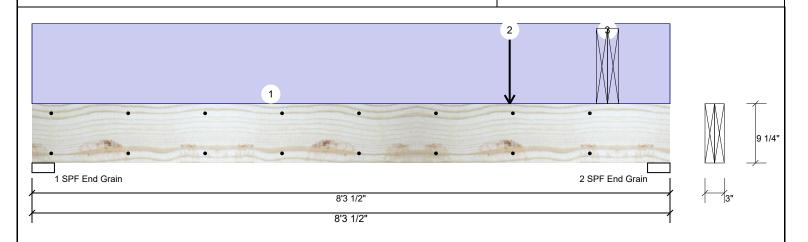
Date: 1/7/2021

Input by: David Landry Job Name: Lot 1 Sierra Villas Project #: J0121-0104

Page 10 of 13

2.000" X 10.000" 2-Ply - PASSED BM₅ S-P-F #2

Level: Level



Member Info	rmation	Reaction	Reactions UNPATTERNED lb (Uplift)						
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	61	662	142	0	0
Moisture Condition	on: Dry	Building Code:	IBC/IRC 2015	2	763	1234	458	0	0
Deflection LL:	480	Load Sharing:	No						
Deflection TL:	360	Deck:	Not Checked						
Importance:	Normal								
Temperature:	Temp <= 100°F								
				Bearin	as				

Analysis	Results
----------	---------

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2560 ft-lb	6'2 1/2"	3946 ft-lb	0.649 (65%)	D+0.75(L+S)	L
Unbraced	2560 ft-lb	6'2 1/2"	3281 ft-lb	0.780 (78%)	D+0.75(L+S)	L
Shear	1603 lb	7'3 1/2"	2498 lb	0.642 (64%)	D+L	L
LL Defl inch	0.028 (L/3410)	4'7 7/8"	0.196 (L/480)	0.140 (14%)	0.75(L+S)	L
TL Defl inch	0.093 (L/1009)	4'5 3/8"	0.261 (L/360)	0.360 (36%)	D+0.75(L+S)	L

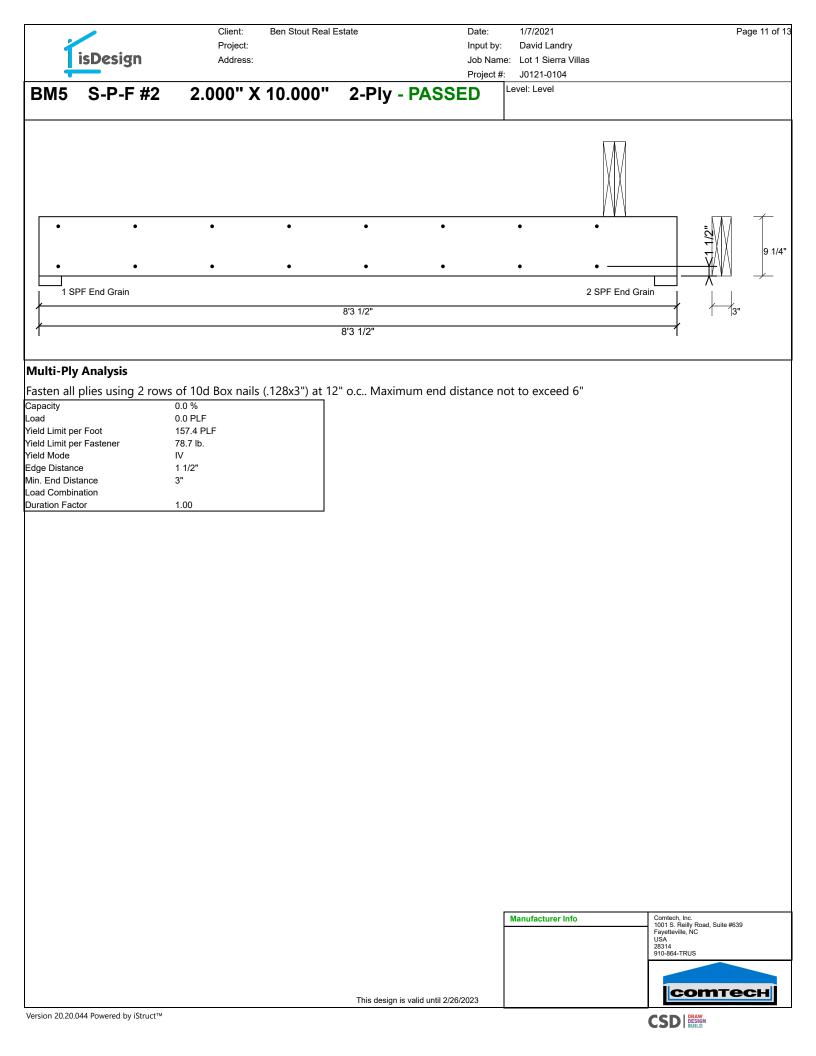
Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

Bearings	5						
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	_
1 - SPF End Grain	3.500"	18%	662 / 153	814	L	D+0.75(L+S)	
2 - SPF End Grain	3.500"	48%	1234 / 915	2150	L	D+0.75(L+S)	

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Above
2	Point	6-2-8		Тор	600 lb	0 lb	600 lb	0 lb	0 lb	Roof Load
3	Point	7-5-12		Тор	301 lb	824 lb	0 lb	0 lb	0 lb	B3 Brg 2

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS Manufacturer Info соттесн



Client: Ben Stout Real Estate

Project: Address: Date: 1/7/2021

2 - SPF 3.500"

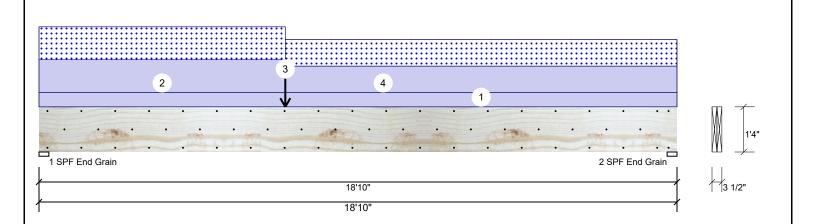
End

Grain

Input by: David Landry Job Name: Lot 1 Sierra Villas Project #: J0121-0104

Kerto-S LVL 2-Ply - PASSED 1.750" X 16.000" **GDH**

Level: Level



Member Information								
Туре:	Girder							
Plies:	2							
Moisture Condition:	Dry							
Deflection LL:	480							
Deflection TL:	360							
Importance:	Normal							
Temperature:	Temp <= 100°F							

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Live Wind Brg Dead Snow Const 2090 1408 0 0 0 1 2 0 1894 1212 0 0

Analysis Results Analysis Actual Location Allowed Comb. Case Capacity Moment 15592 ft-lb 8'4 5/16" 39750 ft-lb 0.392 (39%) D+S L Unbraced 15592 ft-lb 8'4 5/16" 15639 ft-lb 0.997 L (100%)2954 lb 1'6 5/8" 13739 lb 0.215 (22%) D+S Shear L ı

Bearings Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.500" 2090 / 1408 3498 L D+S End Grain

1894 / 1212

3105 L

LL Defl inch 0.170 (L/1298) 9'2 3/16" 0.460 (L/480) 0.370 (37%) S TL Defl inch 0.425 (L/520) 9'2 13/16" 0.613 (L/360) 0.690 (69%) D+S

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 7'6 3/4" o.c.
- 6 Bottom braced at bearings.

7 Lateral slende	erness ratio based o	n single ply width.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Above	
2	Part. Uniform	0-0-0 to 7-3-4		Тор	139 PLF	0 PLF	139 PLF	0 PLF	0 PLF	D2	
3	Point	7-3-4		Тор	314 lb	0 lb	314 lb	0 lb	0 lb	D2-GR	
4	Part. Uniform	7-3-4 to 18-10-0		Тор	112 PLF	0 PLF	112 PLF	0 PLF	0 PLF	D1	
	Self Weight				12 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



D+S

Page 12 of 13



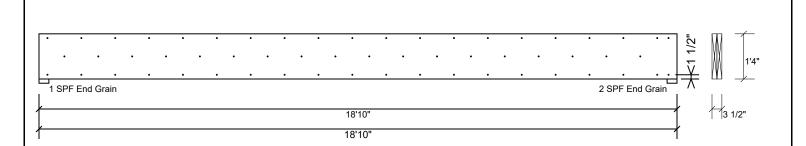
Client: Ben Stout Real Estate

Project: Address: Date: 1/7/2021

Input by: David Landry Job Name: Lot 1 Sierra Villas Project #: J0121-0104

Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED **GDH**

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

, ,		`	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	245.6 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is valid until 2/26/2023

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 13 of 13

