

RE: P21-02006 - JOB 2021-130

Site Information:

Project Customer: **Project Name:** Lot/Block: Model: Address: City: General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014 Wind Code: N/A Wind Speed: 130 mph Roof Load: 40.0 psf

Mean Roof Height (feet): 12

Subdivision:

State:

Trenco 818 Soundside Rd Edenton, NC 27932

Design Program: MiTek 20/20 8.3 Design Method: User defined Floor Load: N/A psf

Exposure Category: B

Seal#	Truss Name	Date
144879604	T01	2/19/21
144879605	T01A	2/19/21
144879606	T01GE	2/19/21
144879607	T02	2/19/21
144879608	T03	2/19/21
144879609	V01	2/19/21
144879610	V02	2/19/21
144879611	V04	2/19/21
144879612	V05	2/19/21
144879613	V06	2/19/21
144879614	V07	2/19/21
	Seal# 144879604 144879605 144879605 144879607 144879609 144879609 144879610 144879611 144879612 144879613 144879614	Seal# Truss Name I44879604 T01 I44879605 T01A I44879606 T01GE I44879607 T02 I44879608 T03 I44879609 V01 I44879610 V02 I44879611 V04 I44879613 V05 I44879614 V07

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Longleaf Truss Company.

Truss Design Engineer's Name: Johnson, Andrew My license renewal date for the state of North Carolina is December 31, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Johnson, Andrew

February 19,2021



L	8-1	1-7	1:	5-11-8		1	23-9-9			31-11-0	
1	8-1	1-7	7	'-10-1			7-10-1	1		8-1-7	
Plate Offsets (2	X,Y) [2:0-0-7,	<u>0-1-12], [2:0-1-11,0-0-9],</u>	[2:0-5-9,0-0-0],	[6:0-3-0,0-2-	-4], [7:0-0-	0,0-0-0], [10:0-0-7,0)-1-12], [10:1-0-	12,0-0-9],	, [10:0-5-9,0-0-	0], [12:0-5-0,0-3-0]	
LOADING (ps	f)	0040000	0.0.0	001		DEEL	in (1.5.5)	1/-1-6	1.74		
TCLL (roof)	20.0	SPACING-	2-0-0	CSI.		DEFL.	In (IOC)	I/defi	L/d	PLATES	GRIP
Snow (Pf/Pa)	11 6/15 0	Plate Grip DOL	1.15	IC	0.93	Vert(LL)	-0.16 12-13	>999	240	M120	244/190
TCDI	10.0	Lumber DOL	1.15	BC	0.62	Vert(CT)	-0.36 12-13	>999	180		
PCU	0.0 *	Rep Stress Incr	YES	WB	0.37	Horz(CT)	0.12 10	n/a	n/a		
BOLL	0.0	Code IRC2018/TI	PI2014	Matrix	-S					Weight: 151 lb	FT = 20%
DUDL	10.0										
LUMBER- TOP CHORD BOT CHORD	2x4 SP No.1 2x4 SP No 1				 -	BRACING- TOP CHORD BOT CHORD	Sheathed. Rigid ceiling dir	ectly appli	ied or 10-0-0 o	ic bracing	
WERS	2x4 SP No 3					WERS	1 Row at midat	could apply	8-12 /-12	o braoing.	
		2 4 4 0 10 Pight 2v4 SI				WLDO	i Row at mupt		0-12, 4-12		
SLIDER	Len 2x4 SF NU.	3 -11 4-0-10, Right 224 3	- 110.3 -11 4-0-10)							
REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=74(LC 11) Max Uplift 2=-26(LC 12) Max Grav 2=1330(LC 2), 10=1264(LC 2)											
FORCES. (Ib TOP CHORD BOT CHORD	 Max. Comp./N 2-4=-2929/19, 2-13=0/2697, 	lax. Ten All forces 250 4-6=-2039/48, 6-8=-2039 12-13=0/2697, 11-12=0/2	(lb) or less exce 9/48, 8-10=-2936 2705, 10-11=0/2	pt when sho 6/22 705	own.						

WEBS 6-12=0/787, 8-12=-949/45, 8-11=0/330, 4-12=-940/42, 4-13=0/328

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=32ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Snow (Pf/Pg) TCDL BCLL BCDL	11.6/15.0 10.0 0.0 * 10.0	Lumber DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2018/TPI2014	BC 0.67 WB 0.42 Matrix-S	Ver((LL) Vert(CT) Horz(CT	-0.16 9-11 -0.29 8-9) 0.08 8	>999 >999 n/a	240 180 n/a	Weight: 233 lb	FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS SLIDER	2x6 SP No.1 2x6 SP No.1 2x4 SP No.3 Left 2x4 SP No	.3 -H 7-2-3, Right 2x4 SP No.3 -H 7-2-3		BRACING- TOP CHORD BOT CHORD	Sheathed or 4- Rigid ceiling di	3-10 oc pu rectly appl	urlins. ied or 10-0-0	oc bracing.	
REACTIONS.	. (size) 2=0	-3-8, 8=0-3-8							

Max Uplift 2=-26(LC 12) Max Grav 2=1339(LC 24), 8=1284(LC 25) FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3357/59, 3-5=-2970/9, 5-7=-2974/0, 7-8=-3361/63

BOT CHORD 2-11=-11/3187, 9-11=0/2084, 8-9=-15/3139

Max Horz 2=74(LC 11)

WEBS 5-9=0/1104, 7-9=-600/146, 5-11=0/1100, 3-11=-596/142

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=32ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

10) Load case(s) 3, 4, 5, 6, 7, 19, 20, 21, 22, 23, 24, 25, 26, 27, 32, 33 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-5=-43, 5-8=-43, 2-8=-20

Continued on page 2

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Job	Truss	Truss Type		Qty	Ply	JOB 2021-130	14 1070005
P21-02006	T01A	Common		6	1		144879605
						Job Reference (optional)	
Longleat Truss Company,	West End, NC - 27	376,	ID:nFpdx	3t3Hro3	8.330 s C ezFKpvk7s	ot 7 2020 Millek Industries, Inc. Fri Fet nziwGw-9bgZOemVQpftU7dv0A6LTzN	0 19 08:38:56 2021 Page 2 WMnwEcXLm9Sz?nvzidVz
			12 iii pas				
LOAD CASE(S) Standar	d ////////////////////////////////////				_		
3) Dead + 0.75 Root Live	(balanced) + 0.75 Uni	nhab. Attic Storage: Lumber Incre	ease=1.15, Plate Increa	se=1.1	2		
Vert: 1-5=-50,	5-8=-50, 2-14=-20, 14	I-15=-50, 8-15=-20					
4) Dead + 0.75 Snow (bal	anced) + 0.75 Uninha	b. Attic Storage: Lumber Increase	=1.15, Plate Increase=	1.15			
Uniform Loads (plf)	F 0 07 0 4 4 00 4 4						
5) Dead + 0 75 Snow (Un	5-8=-37, 2-14=-20, 14 hal left) + 0.75 Uninh	15=-50, 8-15=-20 ab. Attic Storage: Lumber Increa	se=1 15 Plate Increase	=1 15			
Uniform Loads (plf)		azi / illo elerager zamzer merea					
Vert: 1-12=-37	, 5-12=-53, 5-8=-25, 2	-14=-20, 14-15=-50, 8-15=-20					
6) Dead + 0.75 Snow (Unl	bal. Right) + 0.75 Unir	hab. Attic Storage: Lumber Increa	ase=1.15, Plate Increas	se=1.15			
Vert: 1-5=-25.	5-13=-53, 8-13=-37, 2	2-14=-20, 14-15=-50, 8-15=-20					
7) Dead + Uninhabitable A	Attic Without Storage:	Lumber Increase=1.25, Plate Incr	rease=1.25				
Uniform Loads (plf)							
Vert: 1-5=-20, 3	5-8=-20, 2-14=-40, 14 Attic Storage: Lumbe	15=-80, 8-15=-40 or Increase-0.90. Plate Increase-(0.90 Plt_metal=0.90				
Uniform Loads (plf)	Auto Otorage. Europe						
Vert: 1-5=-20	, 5-8=-20, 2-14=-20, 1	4-15=-60, 8-15=-20					
20) Dead + 0.75 Snow (ba	al.) + 0.75 Uninhab. A	ttic Storage + 0.75(0.6 MWFRS W	Vind (Neg. Int) Left): Lur	nber In	crease=1.6	60, Plate Increase=1.60	
Uniform Loads (pif) Vert: 1-2=-38	2-5=-41 5-8=-32 2-	14=-20 14-15=-50 8-15=-20					
Horz: 1-2=1, 2	2-5=4, 5-8=6						
21) Dead + 0.75 Snow (ba	al.) + 0.75 Uninhab. A	ttic Storage + 0.75(0.6 MWFRS W	Vind (Neg. Int) Right): L	umber l	ncrease=1	.60, Plate Increase=1.60	
Uniform Loads (plf)	25-2259-412	14-20 14 15-50 8 15-20					
Horz: 1-2=-20	2-5=-6, 5-8=-4	14=-20, 14-13=-30, 0-13=-20					
22) Dead + 0.75 Snow (ba	al.) + 0.75 Uninhab. A	ttic Storage + 0.75(0.6 MWFRS W	Vind (Neg. Int) 1st Paral	lel): Lu	mber Incre	ase=1.60, Plate Increase=1.60	
Uniform Loads (plf)		14 00 14 45 50 8 45 00					
Vent: 1-2=-34 Horz: 1-2=-3	, 2-5=-38, 5-8=-38, 2- 2-5=1 5-8=-1	14=-20, 14-15=-50, 8-15=-20					
23) Dead + 0.75 Snow (ba	al.) + 0.75 Uninhab. A	ttic Storage + 0.75(0.6 MWFRS W	Vind (Neg. Int) 2nd Para	llel): Lu	mber Incre	ease=1.60, Plate Increase=1.60	
Uniform Loads (plf)							
Vert: 1-2=-34	, 2-5=-38, 5-8=-38, 2- 2-5-1 5-81	14=-20, 14-15=-50, 8-15=-20					
24) Dead + 0.75 Roof Live	e (bal.) + 0.75 Uninhal	b. Attic Storage + 0.75(0.6 MWFR	RS Wind (Neg. Int) Left):	Lumbe	er Increase	=1.60, Plate Increase=1.60	
Uniform Loads (plf)		C .	, ,				
Vert: 1-2=-51	, 2-5=-54, 5-8=-44, 2-	14=-20, 14-15=-50, 8-15=-20					
25) Dead + 0.75 Roof Live	2-5=4, 5-6=6 e (bal.) + 0.75 Uninhal	b. Attic Storage + 0.75(0.6 MWFR	S Wind (Nea. Int) Right	t): Lum	per Increas	e=1.60. Plate Increase=1.60	
Uniform Loads (plf)	()		······································	.,			
Vert: 1-2=-41	, 2-5=-44, 5-8=-54, 2-	14=-20, 14-15=-50, 8-15=-20					
Horz: 1-2=-9, 26) Dead + 0 75 Roof Live	2-5=-6, 5-8=-4 (hal) + 0 75 Uninhal	h Attic Storage + 0 75(0 6 MWER	S Wind (Neg. Int) 1st P	arallel)	l umber lr	crease=1.60 Plate Increase=1.60	
Uniform Loads (plf)				aranor			
Vert: 1-2=-47	, 2-5=-51, 5-8=-51, 2-	14=-20, 14-15=-50, 8-15=-20					
Horz: 1-2=-3,	2-5=1, 5-8=-1 (bal) + 0.75 Uninbal	h Attic Storage L 0 75(0 6 MM/EE	S Wind (Nog. Int) 2nd I	Darallal). Lumbor I	noroaco-1.60. Plato Incroaco-1.60	
Uniform Loads (plf)	- (bai.) + 0.75 Official	3. Auto Storage + 0.75(0.0 MWI K	to wind (Neg. int) zhd i	aranci		nciease=1.00, 1 late increase=1.00	
Vert: 1-2=-47	, 2-5=-51, 5-8=-51, 2-	14=-20, 14-15=-50, 8-15=-20					
Horz: 1-2=-3,	2-5=1, 5-8=-1	75 Uninhoh Attis Otarazza I	har Ingradas 1 15 Dist	o locar -	1 15		
Uniform Loads (olf)	Live (unbalanced) + (J. 75 Oninnab. Auto Storage: Lum	uei increase=1.15, Plate		196=1.19		
Vert: 1-5=-50	, 5-8=-20, 2-14=-20, 1	4-15=-50, 8-15=-20					

33) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-20, 5-8=-50, 2-14=-20, 14-15=-50, 8-15=-20





1		31-11-0						1
I		31-11-0						
Plate Offsets (X,Y) [13:0-0-0	,0-0-0], [25:0-2-8,0-3-0]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2018/TPI2014	CSI. TC 0.14 BC 0.09 WB 0.07 Matrix-S	DEFL. Vert(LL) -0.0 Vert(CT) 0.0 Horz(CT) 0.0	in (loc) 00 1 00 1 00 18	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 159 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1		BR/ TOF BOT	\CING- ² CHORD Shea F CHORD Rigid	thed or 6-(ceiling dir)-0 oc pur ectly appl	lins. ied or 10-0-0	oc bracing.	

BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.3

REACTIONS. All bearings 31-11-0.

Max Horz 2=74(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 26, 27, 28, 29, 30, 31, 24, 23, 22, 21, 20, 19

Max Grav All reactions 250 lb or less at joint(s) 2, 25, 26, 27, 28, 29, 30, 24, 23, 22, 21, 20, 18 except 31=310(LC 30), 19=324(LC 31)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=32ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 1.5x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 11) will fit between the bottom chord and any other members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 26, 27, 28, 29, 30, 31, 24, 23, 22, 21, 20, 19,
- 13) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 18.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED WITHS KRETERENCE PAGE MIL-74/3 fev. or 19/2/2/2 DEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



	8-1-7 8-1-7				23-9-9 2 15-8-1 2-					31-9-8 5-0-4	<u>31-1</u> 1-0 0-1-8	
Plate Offsets (X,Y) [2:0-2-9,0	0-1-8], [2:Edge,0-2-1], [8:0-2-6,0-1-8], [′	10:Edge,0-2-	1], [10:0-:	3-9,0-1-8], [13:0-5-0,0)-3-0]					
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 11.6/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/	2-0-0 1.15 1.15 YES TPI2014	CSI. TC BC WB Matri	0.75 0.55 0.67 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.10 -0.24 0.06	(loc) 2-14 2-14 11	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 157 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS SLIDER	2x4 SP No.1 2x4 SP No.1 2x4 SP No.3 Left 2x4 SP No.3	3 -H 4-0-10, Right 2x4	SP No.3 -H 4-0- ⁻	10		BRACING- TOP CHORD BOT CHORD WEBS	Sheathe Rigid ce 6-0-0 oc 1 Row a	d or 3-9 iling dire bracine t midpt	9-14 oc pr ectly appl g: 10-11.	urlins. ied or 10-0-0 8-13, 4-13	oc bracing, Except:	

REACTIONS. (size) 2=0-3-8, 10=0-3-8, 11=0-3-8 Max Horz 2=74(LC 11) Max Uplift 2=-33(LC 12), 10=-45(LC 30) Max Grav 2=1117(LC 2), 10=148(LC 31), 11=1392(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-4=-2322/39, 4-6=-1395/67, 6-8=-1400/67, 8-10=0/454

- BOT CHORD 2-14=0/2128, 13-14=0/2128, 12-13=-32/1008, 11-12=-32/1008, 10-11=-383/0
- WEBS 6-13=0/467, 8-13=-2/425, 4-13=-972/42, 4-14=0/327, 8-11=-1845/0

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=32ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) All plates are 3x4 MT20 unless otherwise indicated.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.
 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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L 8-1	-7	23-9-9		26-9-4	31-9-8	31-11-0		
8-1	-7	15-8-1		2-11-11	5-0-4	0-1 ¹ -8		
Plate Offsets (X,Y) [7:0-2-10,0-1-8], [8:0-2-6,Edge], [8:0-0-0,0-0-15], [12:0-5-0,0-3-0]								
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. DE TC 0.76 Ve BC 0.71 Ve WB 0.75 Ho Matrix-S Ho Ho	FL. in (loc) //defi rt(LL) -0.12 2-13 >999 rt(CT) -0.29 2-13 >999 rz(CT) 0.06 10 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 147 lb	GRIP 244/190 FT = 20%		
LUMBER- TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WEBS 2x4 SP No.3 WEDGE		BRACING- TOP CHOR BOT CHOR WEBS	D Sheathed or 2-8-10 oc D Rigid ceiling directly ap 6-0-0 oc bracing: 8-10. 1 Row at midpt	purlins. pplied or 10-0-0 3-12	oc bracing, Except:			

Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 8=0-3-8, 10=0-3-8 Max Horz 2=74(LC 11) Max Uplift 2=-28(LC 12), 8=-90(LC 30) Max Grav 2=1091(LC 2), 8=94(LC 31), 10=1551(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2304/24, 3-5=-1319/51, 5-7=-1320/51, 7-8=-9/843

BOT CHORD 2-13=0/2099, 12-13=0/2099, 11-12=0/803, 10-11=0/803, 8-10=-706/30

WEBS 5-12=0/455, 7-12=0/493, 3-12=-1017/45, 3-13=0/341, 7-10=-2058/23

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=32ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.
 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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2x4 📁

2x4 🗢

0-p-12 0-b-12 Plate Offsets (X,Y) [2:0-2-0,E	[dge]	4-8-0 4-7-4						1
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2018/TPI2014	CSI. TC 0.02 BC 0.07 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 11 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1		BRA TOF BOT	ACING- P CHORD SI F CHORD RI	heathed or 4-8 igid ceiling dire	-0 oc purli ctly applie	ins. ed or 10-0-0	oc bracing.	

REACTIONS. (size) 1=4-6-8, 3=4-6-8 Max Horz 1=5(LC 11)

Max Grav 1=111(LC 2), 3=111(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

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