

#ONE VENT MUST BE WITHIN 3'-O' OF EVERY CRNR.

EDITION NC RES. CODES

THE WALLER

57

#12

1257 120 120 400

FIRST FLOOR FRONT PORCH WOOD DECK GARAGE

C

HEATHER or JOHNATHAN HALL 165 HEATHERSTONE CT BENSON NC 27504 (919) 207–1403

H SQUARED HOME DESIGN, INC.

PROP. MANG. JRT

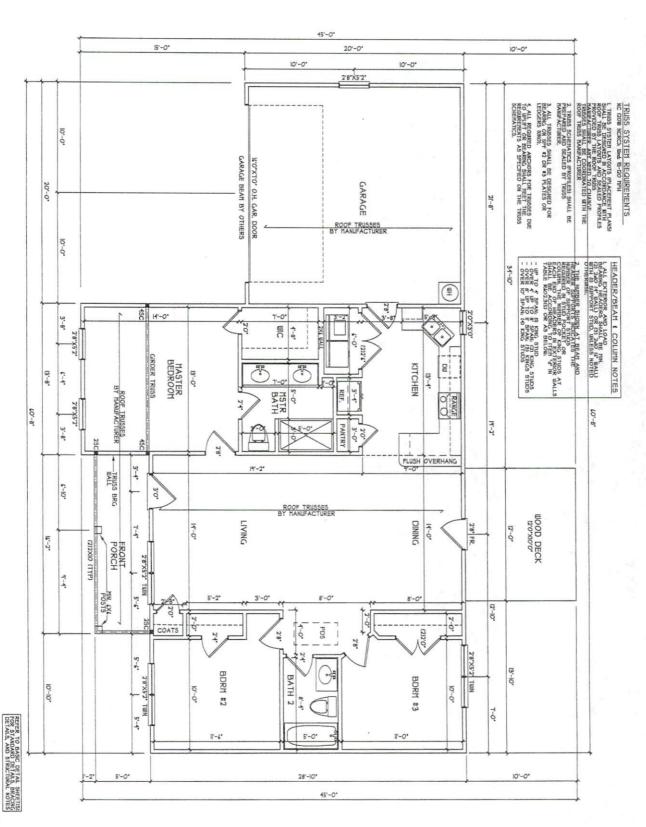
THIS PLAN IS TO ONLY BE BUILT BY THE ABOVE CITED BUILDER OR HOMEOWNER, NOT FOR MULTIPLE BUILDS UNLESS APPROVED BY H SQUARED. 11/11/2020

ANY DEVIATION OF THE SPECIFED HEASUREMENTS OR DIRECTIONS VOIDS H SQUARED HOME DESIGN, INC.'S LIABALITY. THIS PLAN HAS BEEN DRAWN
IN ACCORDANCE WITH HORTH
CAROLINA STATE RESIDENTIAL
BULDING CODES 2018 EDITION.

I STORY

010120.2

SCALE 1/4' = 1'-0'



SCALE IV4" = 1"-O"

1 STORY

PILE

O 10 120.2

THIS PLAN IS TO ONLY OF BE BILL!
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ONLY BE BILL!
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OR FOR MILLIFILE
BUILDS INICESS
AFPROVED BY
H SQUARED.



H SQUARED HOME
HOME
DESIGN, INC.

HEATHER or JOHNATHAN HALL
185 HEATHERSTONE CT
BENSON NC 27504
(1819) 207-1403

FIRST FLOOR
FRONT PORCH
WOOD DECK
GARAGE

= 1257 = 80 = 120 #1257 THE WALLER II

JRT MANG. PROP.

4. ALL REQUIRED ANCHORS FOR TRUSSES DUE TO UPLIFT OR BEACHIG SHALL HEET THE REQUIREMENTS AS SPECIFIED ON THE TRUSS SCHEMATICS. 3. ALL TRUSSES SHALL BE DESIGNED FOR BEARING ON SPF #2 OR #3 PLATES OR LEDGERS (UNO). 2. TRUSS SCHEMATICS (PROFILES) SHALL BE PREPARED AND SEALED BY TRUSS HANUFACTURER. L TRUSS SYSTEM LAYOUTS (PLACEMENT PLANS)
SHALL BE DESKARD IN ACCORDANCE BITH
SECOND BY THE RECOVERY SLED PROPILES
RECOVERED BY THE RECOVERY ANY MEET TO CHANGE
TRUSSES SHALL BE CORRINATED WITH THE
ROOF TRUSS HAMIFACTURER. NC 12018 NCRC): What IIS-120 HPH BY MANUFACTURER GIRDER TRUSS ROOF TRUSSES BY MANUFACTURER BY MANUFACTURER TRUSS_SUPPORT WALL BELOW. ROOF TRUSSES BY MANUFACTURER

REFER TO BASIC DETAIL SHEETIS)
FOR STANDARD DETAILS, BRACING
DETAILS, AND STRUCTURAL NOTES SCALE 1/4" = 1'-0"

THIS PLAN IS TO ONLY BE BUILT BY THE ABOVE OTTED BUILDER OR HOMEOWIELE BUILDS WILES BUILDS WILLS WILES BUILDS WILES BUILDS WILES BUILDS WILES BUILDS WILES BUILDS WILLDS W ANY DRANTON OF THE SPECIFIC DESCARSION OF CRESSORS VICES OF CRESSORS VICES IN SECURITY OF THE SPECIFIC CARGON AT THE SESSION VILLE OF COORS AGO SESSION AS THE SESSION VILLE OF COORS AGO SESTION OF COORS AGO SESTION 010120.2 H SQUARED HOME DESIGN, INC. I STORY

HEATHER OF JOHNATHAN HALL 165 HEATHERSTONE CT BENSON NC 27504 (919) 207-1403

FIRST FLOOR FRONT PORCH WOOD DECK GARAGE

= 1257 = 80 = 120 = 400

#1257

THE WALLER II GARAGE LEFT

JRT MANG. PROP.

2)	DESIGN LOADS (R3OL4)	LIVE LOAD	DEAD LOAD (PSF)	DEFLECTION (LL)
	ROOMS OTHER THAN SLEEPING RO	OM5 40	10	1/340
	SLEEPING ROOMS	30	Ю	L/340
	ATTIC WITH PERMANENT STAIR	40	Ю	L/340
	ATTIC WITH OUT PERMANENT STAIR	10	10	L/340
	ATTIC WITH OUT STORAGE	10	10	L/240
	STAIRS	10		L/340
	EXTERIOR BALCONIES	40	10	L/340
	DECKS	10	ю	L/340
	GUARDRAILS AND HANDRAILS	200	-	
	PASSENGER VEHICLE GARAGES	50	Ю	L/340
	FIRE ESCAPES	10	Ю	L/340
	SNOU	20		
	EIND LOAD IBASED ON IS/120 HI	HY CHILL HE	OCITY I EXPOS	SURE B)

3) WALL BRACING: BRACED WALL PANELS SHALL BE CONSTRUCTED ACCORDING TO SECTION PLOSIOS

SECTION RIGOZOJ. THE AROMET AND LOCATION OF BRACING SHALL CORPLY WITH TABLE RIGOZOJ. THE LENGTH OF BRACING SHALL BE DETERMINED BY SECTION RIGOZOJ. LATERAL BRACING SHALL BE STATISHED PER HEITHOD 3 BY CONTRIBUOUSLY SHEATHING WALLS WITH STRUCTURAL SHEATHING PER SECTION RIGOZOJ. NOTE THAT ARM SPECIMIC BRACED WALL DETER EN SECTION RIGOZOJ. SHEATHING PER S

NOTE THAT ANY SPECIFIC DRACED WALL DEFAIL SHALL BE INSTALLED AS SPECIFIED.

4. CONCRETE SHALL HAVE A RIMBHUT 38 DAY STRENGTH OT 3000 PS AND A
MANDING SLUPP OF S RICHES UNLESS NOTED OTHERSES BIND, AIR ENTRANDED
PER TABLE FOOZ, ALL CONCRETE SHALL BE PROPORTIONED, INSEED, HANDLED,
SAMPLES FOR PRIMING, SHALL BE TAKEN FRONT THE EXIT END OF THE PAIN',
SAMPLES FOR PRIMING, SHALL BE TAKEN FRONT THE EXIT END OF THE PAIN',
SAMPLES FOR PRIMING, SHALL BE TAKEN FRONT THE EXIT END OF THE PAIN',
SAMPLES FOR PRIMING, SHALL BE TAKEN FRONT THE STRUCTURAL ENGINEER FOR AND ALL CONTRACTOR
MUST CONTACT A GEOTECHNICAL ENGINEER AND THE STRUCTURAL ENGINEER FOR AND
MACKENT TO THE POWERATION WALL SHALL SHALL BE PROVIDED MAY FRONT FOR AND
ADDITIONAL SHALL SHALL SHALL BE PROVIDED MAY FRONT CHIRD FROM THE STRUCTURAL ENGINEER FOR AND
ADDITIONAL SHALL SHALL SHALL BE PROVIDED MAY FRONT CHIRD FROM THE STRUCTURAL SHALL SH

4) ALL FRAMING LUMBER SHALL BE SPF #2 (Fb = 815 PSO UNLESS NOTED OTHERWISE (INO). ALL TREATED LUMBER SHALL BE SYP # 2 (Fb=115 PS). PLATE MATERIAL MAY BE SPF # 3 OR SYP #3 (Fdpmp) = 425 PSI - MINJ.

NOTALL ALL CORRECTIONS PER THANDACTURERS INSTRUCTIONS.

9 ALL ROOF TRUSS AND HOST LAYOUTS SHALL BE PREPARED IN ACCORDANCE
BITH ANY SEALED STRUCTURAL DRAWINGS, TRUSSES AND HOSTS SHALL BE
NISTALED ACCORDING TO THE HANDFACTURES SPECIFICATIONS, ANY CHANGE IN
TRUSS OR HOSTS LAYOUT SHALL BE COORDINATED WITH DESIGNER OR ENGINEER.

TRUSS OR 1-JOIST LAYOUT SHALL BE COORDINATED WITH DESCARER OR ENGAGE
OF ALL STRUCTURAL STEELS SHALL BE ASTIN A-3. STEEL BEAMS SHALL BE
SUPPORTED AT LACK EIGH BITH A HINNIN BEARING LENGTH OF 3 V2* INCHES
AND PILL HANGE WITH PROVIDE SOLD BEARING FRON BEAM SUPPORT WITH TO
FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO LAG
SCREES OF "O MARTER F." I LONGL LATERAL SUPPORT IS CONSIDERED
ADEQUATE PROVIDED THE JOST ARE TOR MALED TO THE SOLE PLATE, AND
SOLE PLATE IS MALED OR BOLTED TO THE BEAM FLANGE 8 49' O.C. .
ALL STEEL TURNING SHALL BE ASTIN ASOO.

D REBAR SHALL BE DEFORMED STEEL, ASTMAS, GRADE 40.

I) FLITCH BEAMS SHALL BE BOLTED TOGETHER USING (2) ROUS OF V2* DAMETER BOLTS (ASTR ASOT) WITH MASHESS PLACED WADER THE THREADED END OF BOLT. BOLTS SHALL BE SPACED AT 24* O.C. (MAX), AND STAGGERED AT THE TOP AND BOTTOM OF BEAM 12* EOGE DISTANCED, WITH 2 BOLTS LOCATED AT 14* FROM EACH END.

BRICK LINTELS SHALL BE 3 V2'-3 V2'-4/4" STEEL ANGLE FOR UP TO 6"-0" SPAN AND 6"x1"x5/6" STEEL ANGLE WITH 6" LEG VERTICAL FOR SPANS UP TO 1"-0" (UNO).

H) THE POSITIVE AND NEGATIVE DESIGN PRESSURE FOR DOORS AND UNDOUS FOR A MEAN ROOF HEIGHT OF 35 FEET OR LESS SHALL BE 25 PSF.

5) THE POSITIVE AND NEGATIVE DESIGN PRESSURES REQUIRED FOR ANY ROOF OR EALL CLADDING APPLICATION NOT SPECIFICALLY ADDRESSED IN THE NORTH CARCIAN STATE RESIDENTIAL CODE - 20th EDITION SHALL BE AS FOLLOUS.

ROOF: 45.4 PSF - 2.25.12 PITCH OR LESS 34.8 PSF - 2.25.12 TO 1:12 PITCH 21 PSF - 1:12 TO 12:12 PITCH

WALLS: 24J PSF - WALLS

HEADER/BEAM (COLUMN NOTES I. ALL EXTERIOR AND LOAD BEARING HEADERS SHALL BE MIN. (2) 23/0 (4" WALL) OR (3) 23/0 (6" WALL) WITH (1) SUPPORT STUD, UNLESS NOTED OTHERWISE.

2. THE NUMBER SHOWN AT BEATH AND HEADER SUPPORTS INDICATES THE WHIBER OF SUPPORTS OF THE OR COLUMN. THE NUMBER OF KING STUDS AT EACH BHO OF HEADERS IN EXTERIOR WALLS SHALL BE ACCORDING TO ITEM "6" IN TABLE RIOZJESS OF AS BELOW.

TO 4' SPAN: (I) KING STUD ER 4' UP TO 8' SPAN: (2) KING STUDS ER 8' UP TO II' SPAN: (3) KINGS STUDS ER II' SPAN: (4) KING STUDS

FOUNDATION STRUCTURAL NOTES:

(1) (3) 240 SYP #2 OR SPF#2 GIRDER, TYPICAL UNO.

② CONCRETE BLOCK PIER SIZE SHALL BE-SIZE HOLLOW MASORRY SOLID MASORRY 8 × k UP TO 32" HIGH UP TO 5"-0" HIGH 12 × k UP TO 46" HIGH UP TO 5"-0" HIGH | CONCRETE BLOCK PIER SIZE SHALL Bis | SIZE | HOLLOW | HASONSY | SOLID | HASONS | 8 × k | UP TO 32" | HIGH | UP TO 5"-0" | HI | 12 × k | UP TO 48" | HIGH | UP TO 12"-0" | HI | 14 × k | UP TO 48" | HIGH | UP TO 12"-0" | HI | 24 × 24 | UP TO 48" | HIGH | UP TO 12"-0" | HIGH | UP TO 13" | HIGH | UP TO 12'-O' HIGH

(3) WALL FOOTING AS FOLLOUS: DEPTH: 8' - UP TO 2-1/2 STORY

SIDING FOR EQUAL) MDTH-

SIDING (OR EQUAL)
- 14" - 19" TO 2-1/2 STORY
- 20" - 3 STORY

BRICK VENEER
- 14" - 1 STORY
- 20" - 2 STORY
- 21" - 3 STORY

FOR FOUNDATION WALL HEIGHT AND BACKFILL REGURENENTS, REFER TO NORTH CAROLINA RESIDENTIAL CODE TABLE RO(41) If THEU 10 NOTE: ASSURED SOIL BEARING CAPACITY = 2000 PSF. CONTRACTOR MUST VERIFY SITE CONDITIONS AND CONTACT SOLS ENGINEER IF HARGINAL OR UNSTABLE SOILS ARE ENCOUNTERED.

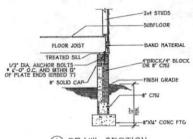
(4) (4) 240 SYP#2 OR SPF#2 GIRDER.

(5) (2) L'ISX4.25 LVL OR LSL GIRDER

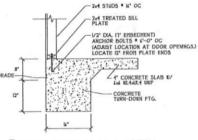
(1) (3) LTSX4.25 LVL OR LSL GIRDER

1. "M" DESIGNATES A SIGNIFICANT POINT LOAD TO HAVE SOLID BLOCKING TO PIER. SOLID BLOCK ALL BEAM BEARING POINTS NOTED TO HAVE THREE OR HORE STUDS TO FND, TYPICAL

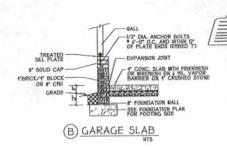
8. ABBREVIATIONS: "SJ" - SINGLE JOIST
"DJ" - DOUBLE JOIST "TJ" = TRIPLE JOIST

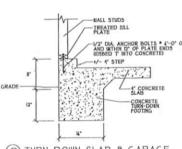




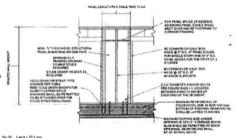


C TURN DOWN SLAB FOOTING

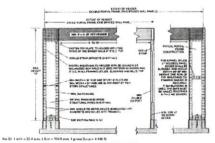




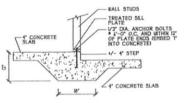




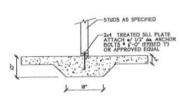
PIGURE RICO.10.3.2 ALTERNATE BRACED WALL PANEL



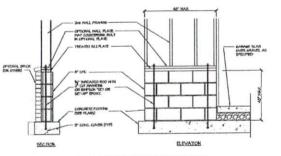
PROURE RIGHT TELLS METHOD FFH: PORTAL FRAME WITH HOLD D



(E) THICKENED SLAB & GARAGE



(F) TYPICAL THICKENED SLAB



GARAGE WING WALL! REINFORCING

כי BUILDIN SHEET DETAIL BASIC

TON THAT NOTE A *PLEASE } ALL DET, EVERY !

HEATHER HALL S HEATHERSTONE C BENSON NC 27504 (919) 207-1403

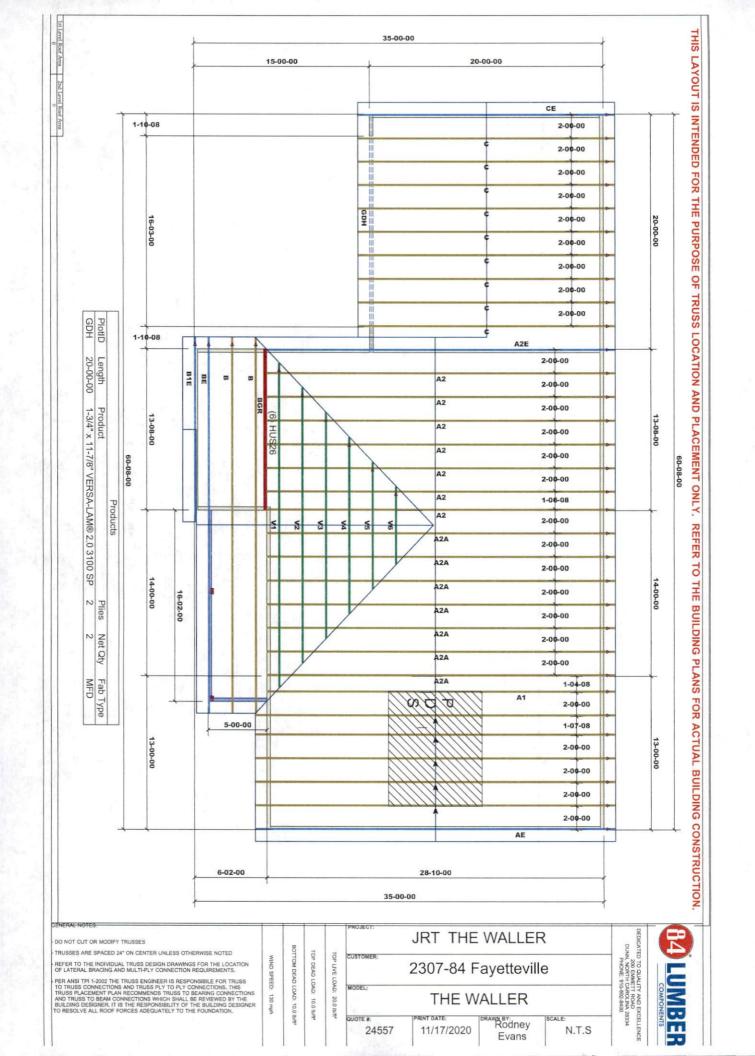
CT

H SQUARED HOME DESIGN, INC.

THE PLAY HAS BEEN DRAWN N ACCORDANCE WITH WORTH CAROLBA STATE RESIDENTAL SMEDRIC CODES JON EGITOR. **大型の大型**

DATE

FILE:









Roof\Dropped Beams\GDH(i19) (Dropped Beam) Dry | 1 span | No cant.

BC CALC® Member Report

Build 7493

File name:

August 5, 2020 15:26:20

Job name: Address:

Company:

2000612A.mmdl Roof\Dropped Beams\GDH(i19)

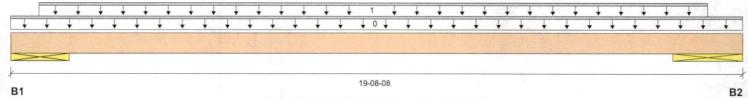
City, State, Zip:

Description: Specifier:

Customer: Code reports:

ESR-1040

Designer:



Total Horizontal Product Length = 19-08-08

Reaction Summary (Down / Uplift) (Ibs)

Bearing	Live	Dead	Snow	Wind	Roof Live	
B1, 19"		1987 / 0		613 / 1512	1982 / 0	
B2, 22-1/2"		2007 / 0		615 / 1662	1994 / 0	

Loa	ad Summary						Live	Dead	Snow	Wind	Roof Live	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	19-08-08	Тор		12				00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-09-04	18-09-04	Тор		208			221	n\a

Controls Summary	Value	% Allowable	Duration	Case	Location
Pos. Moment	14743 ft-lbs	56.9%	125%	1	09-09-04
End Shear	3122 lbs	31.6%	125%	1	16-10-02
Total Load Deflection	L/273 (0.719")	87.8%	n\a	1	09-09-04
Live Load Deflection	L/546 (0.36")	65.9%	n\a	98	09-09-04
Max Defl.	0.719"	71.9%	n\a	1	09-09-04
Span / Depth	16.5				

Bear	ing Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Wall/Plate	19" x 3-1/2"	3968 lbs	14.0%	8.0%	Spruce-Pine-Fir
B2	Wall/Plate	22-1/2" x 3-1/2"	4001 lbs	12.0%	6.8%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Design meets arbitrary (0.75") Maximum live load deflection criteria.

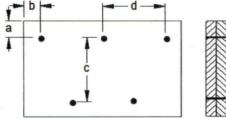
Calculations assume unbraced length of Top: 01-10-08, Bottom: 01-10-08.

BC CALC® analysis is based on IBC 2012.

Wind loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Connection Diagram: Full Length of Member







Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

August 5, 2020 15:26:20

Roof\Dropped Beams\GDH(i19) (Dropped Beam)

BC CALC® Member Report

Build 7493

Job name: Address: Dry | 1 span | No cant.

1992

., | . = | -----

File name:

2000612A.mmdl

City, State, Zip:

Description:

Roof\Dropped Beams\GDH(i19)

City, State, Zi

Specifier:

Customer: Code reports:

ESR-1040

Designer: Company:

Connection Diagram: Full Length of Member

a minimum = 2"

c = 7-7/8"

b minimum = 3"

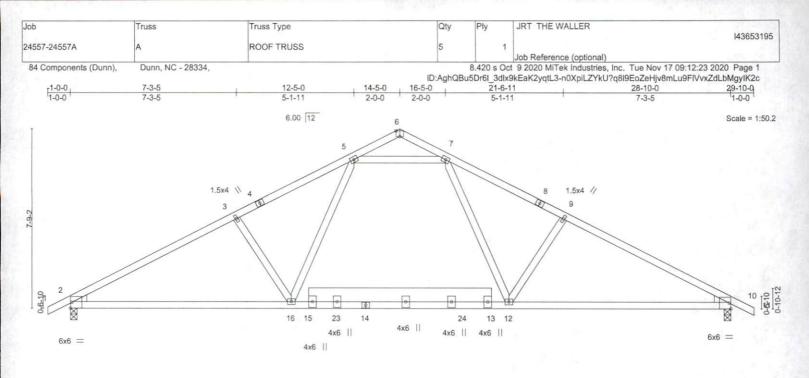
d = 24"

Connectors are: 3-1/4 in. Pneumatic Gun Nails

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



	9-7-14			1	19-2-2					28-10-0				
	9-7-14				9-6-4					9-7-14				
Plate Off	sets (X,Y)	[6:0-2-0,Edge]							10.					
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP		
TCLL	20.0	Plate Grip DOL	1.15	TC	0.62	Vert(LL)	-0.25 13	2-22	>999	240	MT20	197/144		
TCDL	10.0	Lumber DOL	1.15	BC	0.84	Vert(CT)	-0.40 1	2-22	>857	180				
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.47	Horz(CT)	0.05	10	n/a	n/a				
BCDL	10.0	Code IRC2015/T	PI2014	Matrix	k-MS						Weight: 160 lb	FT = 20%		

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2

2x4 SP No.2 or 2x4 SPF No.2 *Except* **BOT CHORD**

13-15: 2x8 SP No.2

WEBS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 10=0-3-8

Max Horz 2=-129(LC 13)

Max Uplift 2=-156(LC 12), 10=-156(LC 13) Max Grav 2=1213(LC 1), 10=1213(LC 1)

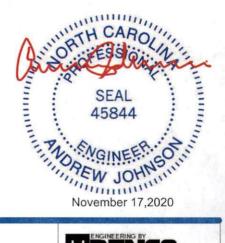
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1947/494, 3-5=-1720/496, 7-9=-1720/496, 9-10=-1947/494

BOT CHORD 2-16=-323/1654, 12-16=-159/1261, 10-12=-325/1654

WEBS 7-12=-96/587, 9-12=-420/247, 5-16=-96/587, 3-16=-420/247, 5-7=-1209/456

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 3x4 MT20 unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
- 7) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



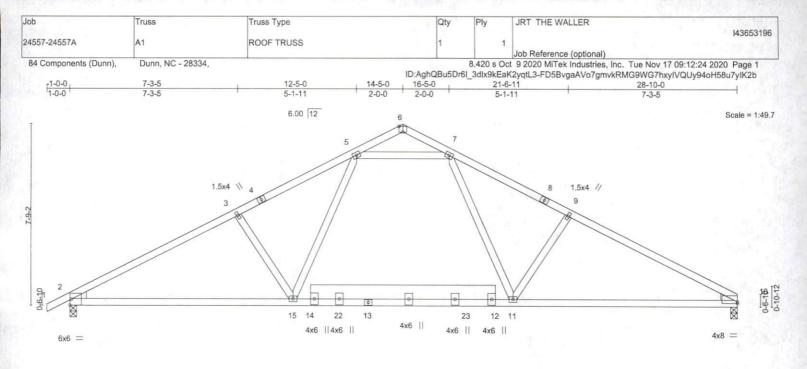
Structural wood sheathing directly applied or 3-5-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component

818 Soundside Road Edenton, NC 27932



	9-7-14			19-2-2		1	1.1				
		9-7-14		9-6-4		9-7-14					
Plate Off	sets (X,Y) [6:0-2-0,Edge], [10:0-0-0,0-0-	-13]								416
LOADIN	G (psf)	SPACING- 2-	-0-0 CSI.	DEFL	. in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL 1	1.15 TC	0.63 Vert(L	L) -0.25 1	1-21	>999	240	MT20	197/144	
TCDL	10.0	Lumber DOL 1	1.15 BC	0.85 Vert(0	T) -0.41 1	1-21	>841	180			
BCLL	0.0 *	Rep Stress Incr Y	YES WB	0.47 Horz(CT) 0.05	10	n/a	n/a			
BCDL	10.0	Code IRC2015/TPI20	14 Matri	ix-MS					Weight: 158 lb	FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except*

12-14: 2x8 SP No.2

WEBS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 10=0-3-8

Max Horz 2=137(LC 16)

Max Uplift 2=-156(LC 12), 10=-135(LC 13) Max Grav 2=1214(LC 1), 10=1152(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1949/496, 3-5=-1722/498, 7-9=-1727/500, 9-10=-1954/498

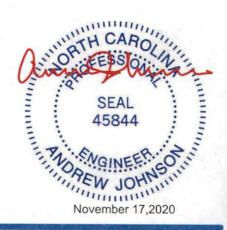
BOT CHORD 2-15=-346/1656, 11-15=-180/1264, 10-11=-348/1661

WEBS 7-11=-99/593, 9-11=-425/249, 5-15=-95/587, 3-15=-420/247, 5-7=-1210/457

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 3x4 MT20 unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
- 7) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



Structural wood sheathing directly applied or 3-5-2 oc purlins.

Rigid ceiling directly applied or 9-9-6 oc bracing.

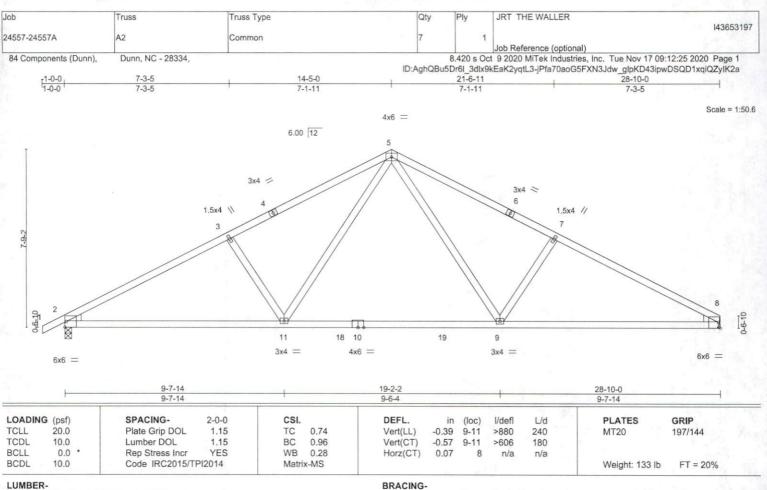
🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 **BOT CHORD** 2x4 SP No.2 or 2x4 SPF No.2

2x4 SP No.3 WEBS

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 8=Mechanical

Max Horz 2=137(LC 16)

Max Uplift 2=-156(LC 12), 8=-135(LC 13) Max Grav 2=1214(LC 1), 8=1152(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1970/504, 3-5=-1737/513, 5-7=-1742/515, 7-8=-1974/506

BOT CHORD 2-11=-358/1679, 9-11=-130/1122, 8-9=-360/1684

WEBS 5-9=-145/667, 7-9=-425/282, 5-11=-142/662, 3-11=-422/280

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 7) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 2-2-0 oc bracing.

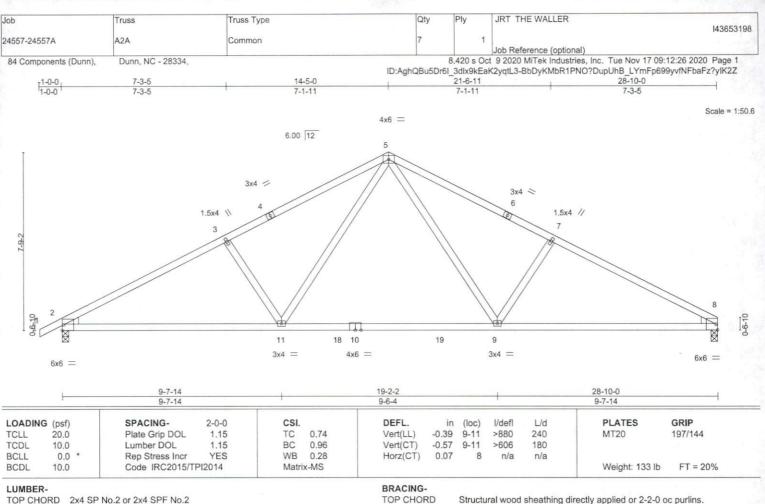
▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Edenton, NC 27932



BOT CHORD

Rigid ceiling directly applied or 2-2-0 oc bracing.

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2

BOT CHORD WEBS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 8=0-3-8

Max Horz 2=137(LC 16)

Max Uplift 2=-156(LC 12), 8=-135(LC 13) Max Grav 2=1214(LC 1), 8=1152(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

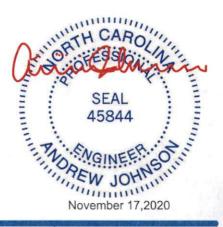
2-3=-1970/504, 3-5=-1737/513, 5-7=-1742/515, 7-8=-1974/506 TOP CHORD

BOT CHORD 2-11=-358/1679, 9-11=-130/1122, 8-9=-360/1684

WEBS 5-9=-145/667, 7-9=-425/282, 5-11=-142/662, 3-11=-422/280

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

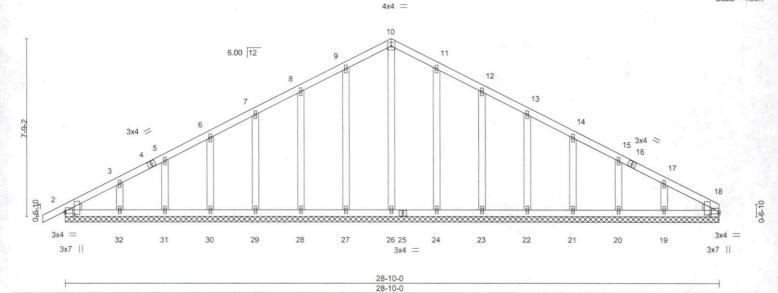
ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qu Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Roa Edenton, NC 27932

JRT THE WALLER Job Truss Truss Type Qty Ply 143653199 24557-24557A A2F Common Supported Gable Job Reference (optional) 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:28 2020 Page 1 84 Components (Dunn), Dunn, NC - 28334 ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-8_Lil2dhZ0d6EX1Cb6ESQzrlov3tQrHgjv3M1uylK2X 1-0-0 28-10-0 14-5-0 14-5-0

Scale = 1:50.7



[2:0-0-0,0-1-1], [2:0-1-11,0-4-13], [18:Edge,0-1-1], [18:0-1-11,0-4-13] Plate Offsets (X,Y)-LOADING (psf) SPACING-CSI DEFL PLATES GRIP I/def TCLL 20.0 Plate Grip DOL 1.15 TC 0.07 Vert(LL) -0.00 n/r 120 MT20 197/144 TCDL 10.0 Lumber DOL 1.15 BC 0.04 Vert(CT) 0.00 n/r 90 n/a n/a

TOP CHORD

BOT CHORD

BCLL 0.0 Rep Stress Incr YES WB 0.14 Horz(CT) 0.01 18 BCDL Code IRC2015/TPI2014 Matrix-S 10.0 BRACING-

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 171 lb

FT = 20%

OTHERS 2x4 SP No.3

BOT CHORD WEDGE

LUMBER-

Left: 2x4 SP No.3, Right: 2x4 SP No.3

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2

REACTIONS. All bearings 28-10-0.

(lb) - Max Horz 2=133(LC 12)

2x4 SP No.2 or 2x4 SPF No.2

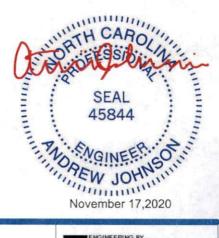
Max Uplift All uplift 100 lb or less at joint(s) 2, 27, 28, 29, 30, 31, 32, 24, 23, 22, 21, 20, 19

Max Grav All reactions 250 lb or less at joint(s) 2, 26, 27, 28, 29, 30, 31, 32, 24, 23, 22, 21, 20, 19, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



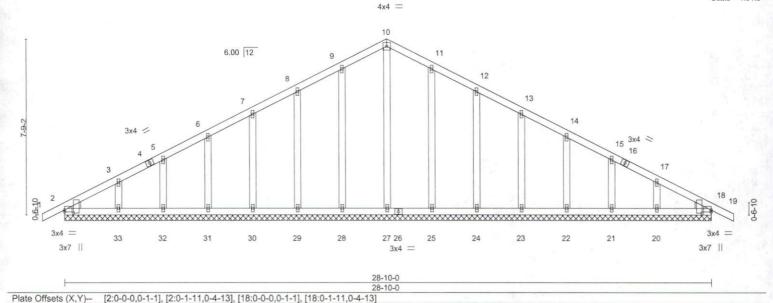
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Qu Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Edenton, NC 27932 Job Qty JRT THE WALLER Truss Truss Type 143653200 Common Supported Gable 24557-24557A AF Job Reference (optional) Dunn, NC - 28334, 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:30 2020 Page 1 84 Components (Dunn). ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-4MSTAkex5etqUqBajXGwWOx5IjlOulkzADYT6myIK2V 28-10-0 14-5-0 1-0-0 14-5-0 14-5-0 1-0-0

Scale = 1:51.3



LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.15	TC	0.07	Vert(LL)	-0.00	18	n/r	120	MT20	197/144	
TCDL	10.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	-0.00	18	n/r	90			
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.14	Horz(CT)	0.01	18	n/a	n/a			
BCDL	10.0	Code IRC2015/Ti	PI2014	Matri	x-S						Weight: 173 lb	FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2

BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

OTHERS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. All bearings 28-10-0.

(lb) - Max Horz 2=129(LC 16)

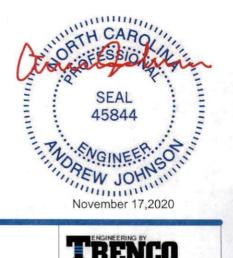
Max Uplift All uplift 100 lb or less at joint(s) 2, 28, 29, 30, 31, 32, 33, 25, 24, 23, 22, 21, 20

Max Grav All reactions 250 lb or less at joint(s) 2, 27, 28, 29, 30, 31, 32, 33, 25, 24, 23, 22, 21, 20, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing
- 6) Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) n/a
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 18.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

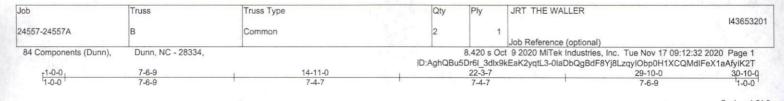
Rigid ceiling directly applied or 10-0-0 oc bracing.

▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





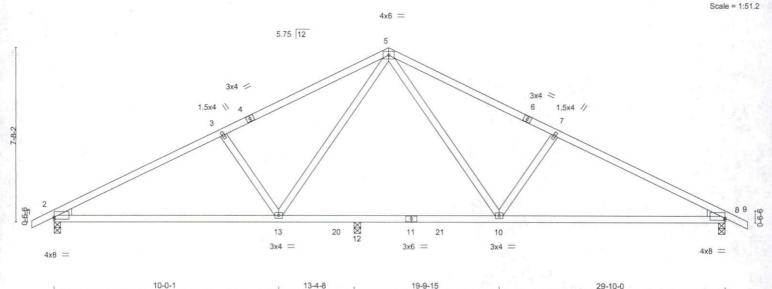


Plate Offsets (X,Y)-[2:0-0-0,0-1-0], [8:Edge,0-1-0] LOADING (psf) SPACING-CSI DEFL. 2-0-0 (loc) I/def L/d **PLATES** GRIP TC TCLL Plate Grip DOL 1.15 20.0 0.75 Vert(LL) -0.22 13-16 >730 240 MT20 197/144 TCDL BC 10.0 Lumber DOL 1.15 0.90 Vert(CT) -0.49 13-16 >332 180 0.0 * BCLL Rep Stress Incr YES WB 0.27 Horz(CT) 0.06 n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Weight: 138 lb FT = 20% BRACING-

6-5-7

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 **BOT CHORD**

WEBS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

(size) 2=0-3-8, 8=0-3-8, 12=0-3-8 REACTIONS.

Max Horz 2=-127(LC 13)

Max Uplift 2=-169(LC 12), 8=-169(LC 13)

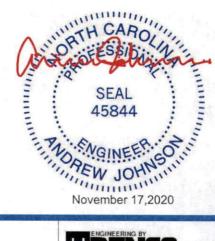
10-0-1

Max Grav 2=1181(LC 1), 8=1193(LC 1), 12=252(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1929/535, 3-5=-1678/532, 5-7=-1711/532, 7-8=-1962/534 **BOT CHORD** 2-13=-367/1662, 12-13=-131/1110, 10-12=-131/1110, 8-10=-371/1691 **WEBS** 5-10=-143/643, 7-10=-452/287, 5-13=-143/599, 3-13=-455/287

1) Unbalanced roof live loads have been considered for this design

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=169, 8=169.



10-0-1

Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 9-5-14 oc bracing.

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932

Truss Type Job JRT THE WALLER Truss 143653202 24557-24557A B1E Common Supported Gable Job Reference (optional) Dunn, NC - 28334. 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:33 2020 Page 1 84 Components (Dunn), ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-Ux8bolhqOYGPLlw9Ofpd70ZcRwmt579PsBm7i5yIK2S 14-8-0 -1-0-0 6-10-0 13-8-0 1-0-0 6-10-0 1-0-0 Scale = 1:28.2 4x4 = 5 5.75 12 14 13 12 11 10 3×4 = 3x7 | 3x4 = 3x7 ||

Plate Off	sets (X,Y)	[2:0-0-0,0-1-0], [2:0-1-7,0	0-5-8], [8:Edge,	0-1-0], [8:0-	1-7,0-5-8]							
OADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.07	Vert(LL)	0.00	8	n/r	120	MT20	197/144
CDL	10.0	Lumber DOL	1.15	BC	0.06	Vert(CT)	0.00	9	n/r	90	No processor	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	8	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-S						Weight: 64 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

13-8-0

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD

2x4 SP No.3 **OTHERS**

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. All bearings 13-8-0.

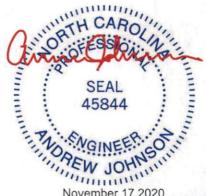
(lb) - Max Horz 2=-63(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 14, 11, 10 Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) n/a
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 8.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Ply JRT THE WALLER Qty Joh Truss Truss Type 143653203 GABLE 24557-24557A BF Job Reference (optional) 84 Components (Dunn). Dunn, NC - 28334. 8,420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:34 2020 Page 1 ID:AghQBu5Dr6I 3dlx9kEaK2yqtL3-y8iz?5hS9sOGzSVMyNLsgE5dXKuuqWoY5rWgFYyIK2R 29-10-0 1-0-0 7-6-9 7.4.7 7.4.7 7-6-9 1-0-0

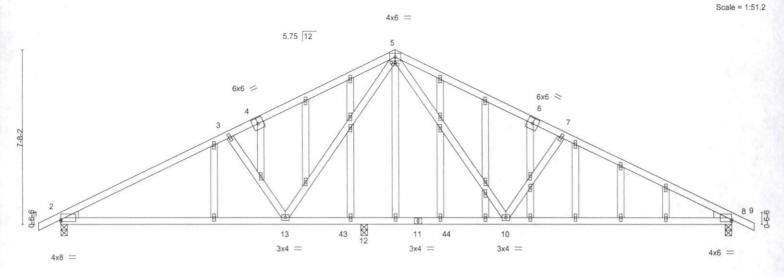


Plate Offs	sets (X,Y)	10-0-1 [2:0-0-0,0-1-0], [5:0-2-0,0	0-0-0], [8:0-0-0		3-4-7	6-5-	7				10-0-1		
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.15	TC	0.75	Vert(LL)	-0.22	13-39	>730	240	MT20	197/144	
TCDL	10.0	Lumber DOL	1.15	BC	0.90	Vert(CT)	-0.49	13-39	>332	180			
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.27	Horz(CT)	0.06	8	n/a	n/a			
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-MS						Weight: 204 lb	FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

13-4-8

19-9-15

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 **BOT CHORD** 2x4 SP No.2 or 2x4 SPF No.2

2x4 SP No.3 WEBS 2x4 SP No.3 OTHERS

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 8=0-3-8, 12=0-3-8

Max Horz 2=-127(LC 13)

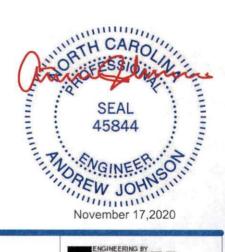
Max Uplift 2=-169(LC 12), 8=-169(LC 13)

10-0-1

Max Grav 2=1181(LC 1), 8=1193(LC 1), 12=252(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1929/535, 3-5=-1678/532, 5-7=-1711/532, 7-8=-1962/534 **BOT CHORD** 2-13=-367/1662, 12-13=-131/1110, 10-12=-131/1110, 8-10=-371/1691 **WEBS** 5-10=-143/643, 7-10=-452/287, 5-13=-143/599, 3-13=-455/287

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces



29-10-0

Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 9-5-14 oc bracing.

Edenton, NC 27932

JRT THE WALLER Job Truss Type Ply Truss Qtv 143653204 24557-24557A BGR Monopitch Girder Job Reference (optional) 84 Components (Dunn), Dunn, NC - 28334 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:36 2020 Page 1 ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-vWpkQnjigTe_Clfk3oNKlfB208h8IGPrY9?nJQylK2P 5-0-13 13-8-0 4-5-6 Scale = 1:42.1 1.5x4 || 4x8 = 5.75 12 3x7 =

> 4-1-14 4-5-6

> > BRACING-

TOP CHORD

BOT CHORD

14

15

except end verticals.

6

8x8 =

16

4x4 =

Structural wood sheathing directly applied or 4-6-15 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

Plate Offsets	(X,Y)	[1:0-3-0,0-3-9],	[6:0-3-8,0-4-8],	[7:0-4-8,0-1-8]

LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defI	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.15	TC	0.43	Vert(LL)	-0.06	7-10	>999	240	MT20	197/144	
TCDL	10.0	Lumber DOL	1.15	BC	0.47	Vert(CT)	-0.12	7-10	>999	180	No. 15 cong.		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.90	Horz(CT)	0.02	5	n/a	n/a			
BCDL	10.0	Code IRC2015/TR	PI2014	Matri	x-MS						Weight: 178 lb	FT = 20%	

13

3x6 ||

12

LUMBER-

REACTIONS.

2x4 SP No.2 or 2x4 SPF No.2 TOP CHORD

BOT CHORD 2x6 SP DSS

WEBS 2x4 SP No.3

> (size) 1=0-3-8, 5=0-3-8 Max Horz 1=250(LC 12)

Max Uplift 1=-457(LC 12), 5=-622(LC 12)

Max Grav 1=3786(LC 1), 5=4090(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-6310/799, 2-3=-3400/405

BOT CHORD 1-7=-947/5655, 6-7=-947/5655, 5-6=-497/3021

WEBS 2-7=-299/2620, 2-6=-3096/528, 3-6=-554/4345, 3-5=-4374/717

NOTES-

1) 2-ply truss to be connected together with 10d (0.120"x3") nails as follows:

Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-7-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.
- 7) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1132 lb down and 155 lb up at 2-0-12, 1132 lb down and 155 lb up at 4-0-12, 1132 lb down and 155 lb up at 6-0-12, 1132 lb down and 155 lb up at 8-0-12, and 1132 lb down and 155 lb up at 10-0-12, and 1132 lb down and 155 lb up at 12-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

SEA 45844 WGINEER SO November 17,2020

Continued on page 2

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932

Truss Type Job Qty Ply JRT THE WALLER Truss 143653204 24557-24557A BGR Monopitch Girder Job Reference (optional)

84 Components (Dunn),

Dunn, NC - 28334,

8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:36 2020 Page 2 ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-vWpkQnjigTe_Clfk3oNKlfB208h8IGPrY9?nJQylK2P

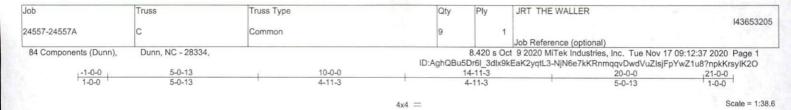
LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-4=-60, 5-8=-20

Concentrated Loads (lb)

Vert: 11=-1132(B) 12=-1132(B) 13=-1132(B) 14=-1132(B) 15=-1132(B) 16=-1132(B)





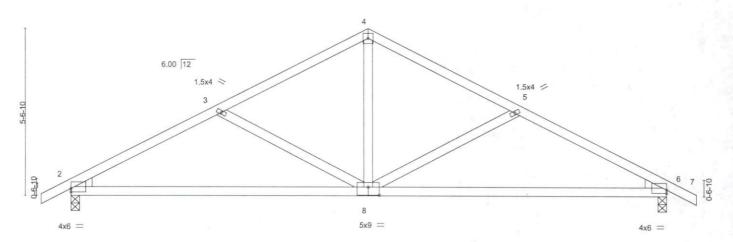


Plate Offse	ets (X,Y)-	[2:0-0-0,0-1-5], [6:0-0-0,0		10-0-0								
LOADING TCLL TCDL	(psf) 20.0 10.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15	CSI. TC BC	0.30	DEFL. Vert(LL) Vert(CT)	in -0.15 -0.31	8-14	l/defl >999 >770	L/d 240 180	PLATES MT20	GRIP 197/144
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code IRC2015/T	YES PI2014	WB Matrix	0.23 c-MS	Horz(CT)	0.03	6	n/a	n/a	Weight: 92 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

20-0-0

Structural wood sheathing directly applied or 5-0-2 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 **BOT CHORD** 2x4 SP No.2 or 2x4 SPF No.2

2x4 SP No.3 WEBS

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 6=0-3-8

Max Horz 2=92(LC 16)

Max Uplift 2=-115(LC 12), 6=-115(LC 13)

Max Grav 2=860(LC 1), 6=860(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1294/358, 3-4=-969/260, 4-5=-969/260, 5-6=-1294/358

TOP CHORD **BOT CHORD** 2-8=-229/1101, 6-8=-232/1101

WEBS 4-8=-69/561, 5-8=-358/216, 3-8=-358/216

NOTES-

1) Unbalanced roof live loads have been considered for this design

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

10-0-0

- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932

JRT THE WALLER Job Truss Type Truss 143653206 24557-24557A CE Common Supported Gable Job Reference (optional) 84 Components (Dunn), Dunn, NC - 28334 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:38 2020 Page 1 ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-rvxUrTkyC5uhR3o7BDPoq4GTGxTQmO280TUuOJyIK2N 1-0-0 10-0-0 20-0-0 21-0-0

4x4 =

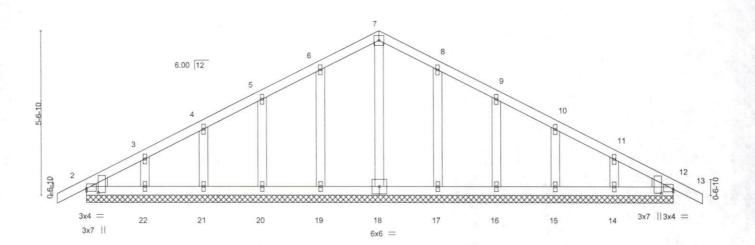


Plate Offsets (X,Y)--[2:0-0-0,0-1-1], [2:0-1-11,0-4-13], [12:0-0-0,0-1-1], [12:0-1-11,0-4-13] LOADING (psf) SPACING-2-0-0 CSI DEFL I/defl **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.07 -0.00 120 Vert(LL) 13 n/r MT20 197/144 TCDL 10.0 Lumber DOL 1.15 BC 0.03 Vert(CT) -0.00 13 n/r 90 BCLL 0.0 Rep Stress Incr YES WB 0.06 Horz(CT) 0.00 12 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Weight: 106 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

20-0-0

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD

OTHERS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

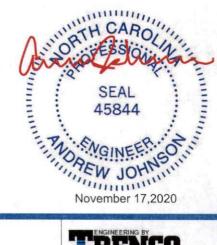
REACTIONS. All bearings 20-0-0.

(lb) - Max Horz 2=92(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 20, 21, 22, 17, 16, 15, 14, 12 Max Grav All reactions 250 lb or less at joint(s) 2, 18, 19, 20, 21, 22, 17, 16, 15, 14, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) n/a
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 12.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Scale = 1:39.2

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



JRT THE WALLER Job Truss Truss Type 143653207 24557-24557A Valley Job Reference (optional) 84 Components (Dunn), Dunn, NC - 28334 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:39 2020 Page 1 ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-J5Vs3plbzO0Y3DNJkww1NHoaHLm?VplHF7DRwlylK2M 27-4-13 13-8-6 13-8-6 4x4 = 5.75 12

-	27-4-4 27-4-4												4-13 1-0-8
LOADING TCLL	20.0	SPACING- Plate Grip DOL	2-0-0 1.15	CSI.	0.30	DEFL. Vert(LL)	in n/a	(loc)	l/defl n/a	L/d 999	PLATES MT20	GRIP 197/144	
BCLL BCDL	10.0 0.0 * 10.0	Lumber DOL Rep Stress Incr Code IRC2015/TR	1.15 YES PI2014	BC WB Matri	0.20 0.16 x-S	Vert(CT) Horz(CT)	n/a 0.00	7	n/a n/a	999 n/a	Weight: 111 lb	FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

10

6x6 =

9

8

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

REACTIONS.

3x4 =

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2

BOT CHORD

2x4 SP No.3 **OTHERS**

> All bearings 27-3-12. (lb) - Max Horz 1=-104(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 11, 9 except 12=-124(LC 12), 8=-123(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=402(LC 22), 11=325(LC 25), 12=424(LC 1),

11

9=321(LC 26), 8=425(LC 1)

12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-12=-308/203, 6-8=-308/203 WEBS

1) Unbalanced roof live loads have been considered for this design

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) All plates are 1.5x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



3x4 >

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



JRT THE WALLER Job Truss Truss Type Qty 143653208 24557-24557A V2 Valley Job Reference (optional) 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:40 2020 Page 1 ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SByIK2L 84 Components (Dunn), Dunn. NC - 28334 23-2-11 11-7-6 Scale = 1:38.0 4x4 = 5.75 12 3x4 = 10 15 8 3x4 = 23-2-3 LOADING (psf) SPACING-DEFL. I/defl L/d **PLATES** GRIP in (loc) TCLL 20.0 Plate Grip DOL 1.15 TC 0.20 Vert(LL) n/a n/a 999 MT20 197/144

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

n/a

0.00

n/a

n/a

999

n/a

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 **BOT CHORD** 2x4 SP No.2 or 2x4 SPF No.2

2x4 SP No.3 **OTHERS**

10.0

10.0

0.0

REACTIONS. All bearings 23-1-10. (lb) - Max Horz 1=88(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 13, 8 except 12=-106(LC 12), 9=-106(LC 13)

1.15

YES

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=372(LC 19), 12=344(LC 23), 13=304(LC 1),

BC

WB 0.11

Matrix-S

0.18

9=344(LC 24), 8=304(LC 1)

Lumber DOL

Rep Stress Incr

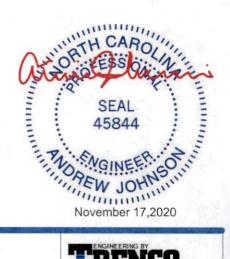
Code IRC2015/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

3-12=-264/174, 5-9=-264/174 WEBS

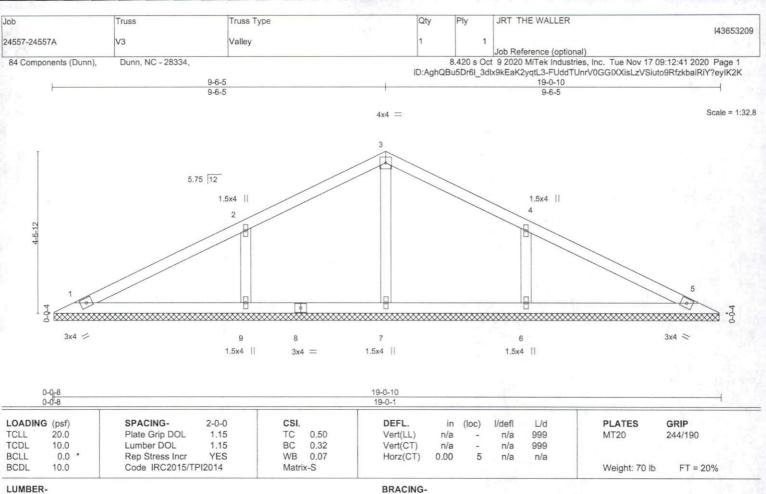
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 1.5x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



FT = 20%

Weight: 91 lb



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.3 2x4 SP No.3 **BOT CHORD**

2x4 SP No.3 OTHERS

REACTIONS. All bearings 18-11-9. (lb) - Max Horz 1=71(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=-131(LC 12), 6=-131(LC 13) All reactions 250 lb or less at joint(s) 1, 5, 7 except 9=435(LC 23), 6=435(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-9=-320/208, 4-6=-320/208

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.

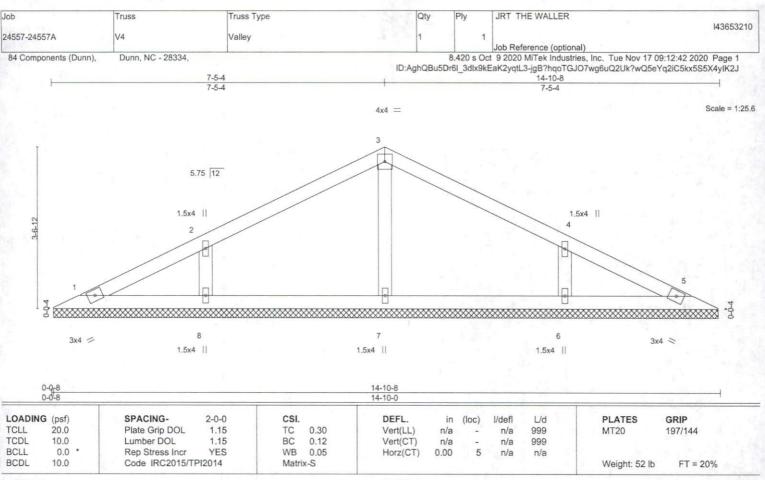
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



LUMBER-

TOP CHORD 2x4 SP No.3

BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

2x4 SP No.3 **OTHERS**

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 14-9-8.

(lb) - Max Horz 1=54(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-101(LC 12), 6=-101(LC 13)

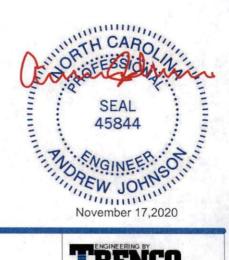
Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=284(LC 1), 8=323(LC 23), 6=323(LC 24)

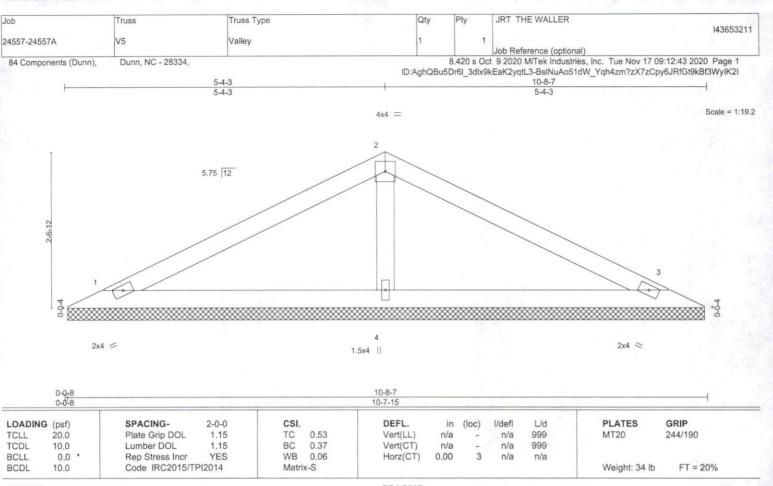
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.





LUMBER-TOP CHORD

2x4 SP No.3

2x4 SP No.3 BOT CHORD 2x4 SP No.3 **OTHERS**

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=10-7-6, 3=10-7-6, 4=10-7-6

Max Horz 1=-38(LC 17)

Max Uplift 1=-34(LC 12), 3=-41(LC 13), 4=-20(LC 12) Max Grav 1=171(LC 23), 3=171(LC 24), 4=414(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-4=-276/149 WEBS

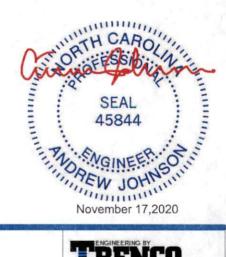
NOTES-

1) Unbalanced roof live loads have been considered for this design

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

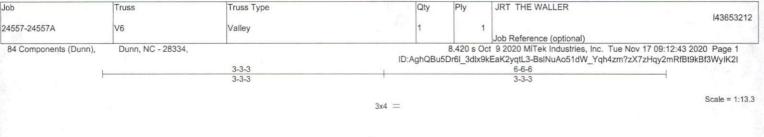


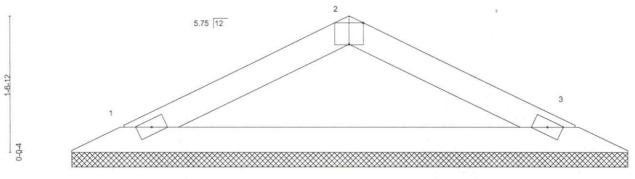
▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932





		F				6-5-13 6-5-13						6-6-6 0-0-8	
Plate Of	fsets (X,Y)	[2:0-2-0,Edge]	17			0-3-13						0-0-0	
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	n/a	-	n/a	999	MT20	244/190	
TCDL	10.0	Lumber DOL Rep Stress Incr	1.15 YES	BC WB	0.60	Vert(CT) Horz(CT)	n/a 0.00	- 2	n/a n/a	999 n/a			

BRACING-

TOP CHORD

BOT CHORD

2x4 >

Weight: 18 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

FT = 20%

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3

(size) 1=6-5-5, 3=6-5-5

Max Horz 1=21(LC 12) Max Uplift 1=-25(LC 12), 3=-25(LC 13) Max Grav 1=209(LC 1), 3=209(LC 1)

2x4 =

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code IRC2015/TPI2014

NOTES-

BCDL

LUMBER-

REACTIONS.

1) Unbalanced roof live loads have been considered for this design.

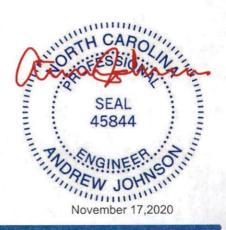
Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-P

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



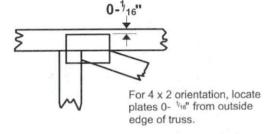
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

 4×4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal

Plate Connected Wood Truss Construction.

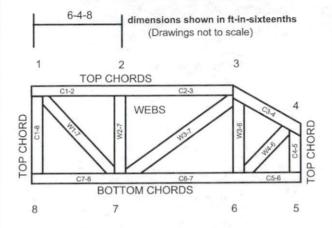
Design Standard for Bracing DSB-89:

BCSI:

Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate

Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g.

- diagonal or X-bracing, is always required. See BCSI.
- 2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- 3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- 4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- 6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- 7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- 8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- 10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- 16. Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- 18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- 19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.