MEAN ROOF HEIGHT: 19'-9	" HEIGHT TO RIDGE: 27'-					
CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A			
FENESTRATION U-FACTOR	0.35	0.35	0.35			
SKYLIGHT U-FACTOR	0.55	0.55	0.55			
GLAZED FENESTRATION SHGC	0.30	0.30	0.30			
CEILING R-VALUE	38 or 30ci	38 or 30ci	38 or 30ci			
WALL R-VALUE	15	15	19			
FLOOR R-VALUE	19	19	30			
* BASEMENT WALL R-VALUE	5/13	10/15	10/15			
** SLAB R-VALUE	0	10	10			
* CRAWL SPACE WALL R-VALUE	5/13	10/15	10/19			

FOUTING, INSUL				ALL SLAD	Z4 UK IC		OF FOUN	DATION W
DESIGNED FOR WIN	D SPEED	OF 120 MF	PH, 3 SECO	OND GUST	(93 FAST	EST MILE)	EXPOSUR	E "B"
COMPONENT	`& CLA	DDING	DESIG	NED FC	DR THE	FOLLO	WING I	OADS
MEAN ROOF	UP T	O 30'	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45'
ZONE 1	14.2	-15.0	14.9	-15.8	15.5	-16.4	15.9	-16.8
ZONE 2	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	-21.8	17.4	-22.4
DESIGNED FOR WIN	d speed	OF 130 MF	PH, 3 SECO	OND GUST	(101 FAS	TEST MILE) EXPOSU	RE "B"
COMPONENT	& CLA	DDING	DESIG	NED FC	DR THE	FOLLO	WING I	OADS
MEAN ROOF	UP T	O 30'	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45'
ZONE 1	16.7	-18.0	17.5	-18.9	18.2	-19.6	18.7	-20.2
ZONE 2	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 3	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5

surfaces, including stairs, ramps and landings, that are located more than 30 within 36 inches (914 mm) horizontally to the edge of the open side. Insect

R312.2 Height. Required guards at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads.

and bottom rail of a *guard*, shall not allow passage of a sphere 6 inches (153

passage of a sphere 4 3/8 inches (111 mm) in diameter.

ROOF VENTILATION

SQUARE FOOTAGE OF ROOF TO BE VENTED = 2,477 SQ.FT.









FOUNDATION STRUCTURAL

115 to 130 mph wind zone (1 1/2 to 2 1/2 story) CONTINUOUS FOOTING: 16" wide and 8" thick minimum. 20" wide minimum at brick veneer. Must extended 2" to either side of supported wall. **GIRDERS:** (3) 2 X 10 girder unless noted otherwise.

PIERS: 16" X 16" piers with 8" solid masonry cap on 30" X 30" X 10" concrete footing with maximum pier height of 64" with hollow masonry and 160" with solid masonry.

POINT LOADS: designates significant point load and should have solid blocking to pier, girder or foundation wall.

minimum 7", maximum 6'-0" on center, within 12" of plate ends, and minimum two anchor bolts per plate.

130 MPH ANCHORS BOLTS: 1/2" diameter anchor bolts embedded minimum 15", maximum 4'-0" on center, within 12" of plate ends, and minimum two anchor bolts per plate.

CONCRETE: Concrete shall have a minimum 28 day strength of 3000 psi and a maximum 5" slump. Air entrained per table 402.2. All concrete shall be in accordance with ACI standards. All samples for pumping shall be taken from the exit end of the pump.

SOILS: Allowable soil bearing pressure assumed to be 2000 PSF. The contractor must contact a geotechnical engineer and a structural engineer if unsatisfactory subsurface conditions are encountered. The surface area adjacent to the foundation wall shall be provided with adequate drainage, and shall be graded so as to drain surface water away from foundation walls.







STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code.

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and

braced in accordance with good construction practice and the building code. DESIGN LOADS

DESIGN LOADS			DELECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10	10	L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		
Guardrail in-fill components	50		
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20		

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS :

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x106 PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. **LINTELS:** Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4"

thick for 24" on center joist spacing. ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on

center rafters.

CONCRETE AND SOILS: See foundation notes.

ROOF TRUSS REQUIREMENTS

UNLESS NOTED OTHERWISE

UNLESS NOTED OTHERWISE

LADDER FRAMED

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or

exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

BRACE WALL PANEL NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise.

GYPSUM: All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

REQUIRED LENGTH OF BRACING: Required brace wall length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length. **HD:** 800 lbs hold down hold down device fastened to the edge of the brace wall panel closets to the corner.

Methods Per Table R602.10.1

PONY WALL

HEIGHT TO

VARY

: 16D 3" O.

Р®

ROWS NAILS

TWO

.

Ч

TOP .

р**о**

HEIGHT -

Σ

PF

Ь

-Ф Г

12.

HEIGHT

MUMIXAM

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or $8d(2 1/2" \log x 0.113" diameter)$. **CS-SFB:** Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing nails.

GB: Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws. **PF**: Portal fame per figure R602.10.1

- 6-16D SINKER NAILS FROM KING STUD TO HEADER-

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. **KNEE WALL AND CEILING HEIGHTS.** All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code. **JOB SITE PRACTICES AND SAFETY:** Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code

construction practice and the bu	inung coue.		
DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		
Guardrail in-fill components	50		
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20		

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS :

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x10⁶ PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x10⁶ PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x10⁶ PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. **LINTELS:** Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. **FLOOR SHEATHING:** OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing. **ROOF SHEATHING:** OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters.

ATTIC ACCESS

CONCRETE AND SOILS: See foundation notes.

SECTION R807

R807.1 Attic access. An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have a vertical height of 60 inches (1524 mm) or greater. The net clear opening shall not be less than 20 inches by 30 inches (508 mm by 762 mm) and shall be located in a hallway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

Exceptions:

1. Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access.

2. Pull down stair treads, stringers, handrails, and hardware may protrude into the net clear opening.

EXTERIOR HEADERS

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE

 - KING STUDS EACH END PER TABLE BELOW

 HEADER SPAN
 < 3'</td>
 3'-4'
 4'-8'
 8'-12'
 12'-16'

 KING STUD(S)
 1
 2
 3
 5
 6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE

LADDER FRAMED



ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. **KNEE WALL AND CEILING HEIGHTS.** All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.



HEEL HEIGHT ABOVE SECOND FLOOR PLATE







R315.1 Carbon monoxide alarms. In new construction, dwelling units shall be provided with an approved carbon monoxide alarm installed outside of each separate sleeping area in the immediate vicinity of the bedroom(s) as directed

requiring a permit occurs, or where one or more sleeping rooms are added or created, carbon monoxide alarms shall be provided in accordance with Section

audible in all bedrooms over background noise levels with all intervening doors closed. Single station carbon monoxide alarms shall be listed as complying with

requirements of this section. For the purposes of this section all dimensions R311.7.4.1 Riser height. The maximum riser height shall be 8 1/4 inches (210 mm). The riser shall be measured vertically between leading edges o

R311.7.4.2 Tread depth. The minimum tread depth shall be 9 inches (229 mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a

R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greater more than 1 1/4 inches (32 mm) shall be provided on stairways with solid

R311.7.7 Handrails. Handrails shall be provided on at least one side of each R311.7.7.1 Height. Handrail height, measured vertically from the sloped

plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm)and not more than 38 inches (965 mm).

1. The use of a volute, turnout or starting easing shall be allowed over the

transition between flights, the transition from handrail to guardrail, or used at the start of a flight, the handrail height at the fittings or bendings shall

R311.7.7.2 Continuity. Handrails for stairways shall be continuous for the full length of the flight, from a point directly above the top riser of the flight to a point directly above the lowest riser of the flight. Handrail ends shall be returned or shall terminate in newel posts or safety terminals. Handrails

2. The use of a volute, turnout, starting easing or starting newel shall be

termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the







	-		Client:	Weaver Deve	elopment		D	ate:	5/10/2	2021				Page	1 of 10
	D		Project:				In	nput by:	Curtis	Quick					
	Design		Address:				Jo	ob Name:	: The La	auren H	Beams				
					<u> </u>			roject #:	evel: Le	vel					
GDH	Kerto-S L	_VL 1	1.750"	X 14.00	0. 2-	Ply - P	ASSE	ן ט		VCI					
•	•	•			1	•			•			11. Contractor 2.1		$\pi \prec$	
	· · ··································			14. V.	attin m		·		-		-	•	•	1'2"	,
		1000 A	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1		•		Service Service				- stille				
1 SPF En	d Grain										2 SPF	End Gra	in		
1					18'10"								\neg	3 1/2"	
1					18'10"								\neg		
/lember In	formation						Reaction	ns UNP	PATTER	RNED	lb (Uplif	t)			
Туре:	Girder		Applicat	tion: F	loor		Brg	Live	•	Dead	Snov	V	Wind	Const	
Plies: Maiatura Can	2 dition: Dr/		Design	Method: A	SD		1	0)	2457	()	0	0	
Deflection LL	360		Load St	naring: N	BC 2012		2	0		2457	()	0	0	
Deflection TL	: 240		Deck:	N N	lot Checked										
Importance:	Normal														
Temperature:	Temp <= 10	00°F					-								
							Bearing	S							
							Bearing	Length	ı C	ap. Re	act D/L lb	Total	Ld. Case	Ld. Comb).
							1 - SPF	3.500"	2	23%	2457 / 0	2457	Uniform	D	
nalysis Re	sults						Grain								
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2 - SPF	3.500"	2	23%	2457 / 0	2457	Uniform	D	
Moment	11011 ft-lb	9'5"	24299 ft-lb	0.453 (45%	5) D	Uniform	Grain								
Unbraced	11011 ft-lb	9'5"	11013 ft-lb	1.000	D	Uniform									
Shear	2093 lb	1'4 3/4"	9408 lb	(100%)	D (Uniform									
LI Deflinch	0.000 (L/999)	0	999.000 (L/0	0.000 (0%)	., 0	onnorm									
TL Defl inch	0.444 (L/497)	9'5 1/16"	0.919 (L/240) 0.480 (48%	5) D	Uniform									
)esian No	tes			, ,			1								
1 Fasten all	plies using 3 rows	of SDW2233	8 at 24" o.c. N	Aximum end	distance not	to exceed	1								
12".	at many of colouist	iana fan faata.		for an exified la	a da										
3 Simpson fa	asteners applied from	om a single s	ide of the mer	nber use tip v	alues where	published.									
4 Girders are	e designed to be si	upported on t	he bottom edg	ge only.											
5 Top loads	must be supported	l equally by al	l plies. n of 0'7 1/2" o	•											
7 Bottom bra	iced at bearings.		110197 112 0	.0.											
8 Lateral sle	nderness ratio bas	ed on single	ply width.												
ID	Load Type		Location	Trib Width	Side	Dead 0.9	Live	1 Snov	w 1.15	Winc	1.6 Con	st. 1.25	Comment	ts	
1	Uniform				Тор	250 PLF	0 PL	F	0 PLF	0	PLF	0 PLF			
	Self Weight					11 PLF									
									Manufact	turer Info		С	omtech, Inc.		
Votes Calculated Structured	Designs is responsible on	chem Iv of the Handli	icals ng & Installati	on	 For flat pondir 	at roofs provide p ng	roper drainage to	prevent	Metsä Wo	ood			01 S. Reilly Road ayetteville, NC	I, Suite #639	
tructural adequacy esign criteria an	of this component based d loadings shown. It	on the 1. LVL b is the 2. Refer	eams must not be contracture	ut or drilled er's product infor	mation				301 Merri	tt 7 Buildi	ng, 2nd Floo	28	SA 3314		
esponsibility of the insure the compo-	customer and/or the contr nent suitability of the i	ractor to regar intended faster	ding installation ning details, beam	requirements, m strength values, and	ulti-ply d code				(800) 622	-5850		91	U-864-1RUS		
.umber	, are amensions and 1080	3. Dama 4 Decid	ovals aged Beams must no in assumes top edge	ot be used	d				www.mets ICC-ES: E	sawood.c ESR-3633	om/us B				
 Dry service condi LVL not to be tree 	tions, unless noted otherwis ated with fire retardant or o	se 5. Provi corrosive latera	de lateral support a l displacement and r	at bearing points to rotation	avoid	design is valid	until 2/26/202						con	ntec	H
	D				INIS	uesign is valid	unui 2/20/202								

CSD DESIGN

	-		Client:	Weaver Developm	ent	Date	e:	5/10/2021	Page 2 of 10
			Project:			Inpu	ıt by:	Curtis Quick	
	IsDesign		Address:			Job	Name:	The Lauren H Beams	
		0.1.7/1	4 = = 0 !!	X 4 4 000		Proje	ect #:		
GDH	i Kerto-	5 LVL	1.750"	X 14.000"	2-Piy	PASSED			
									-
•	•		•	• •	•	•		• •	
	•	•	•	•	•	•	•	• •	• 1'2"
	•		•	• •	•	٠		• • • • • • • • • • • • • • • • • • •	
15	PF End Grain							2 SPF End (
					18'10"				3 1/2"
/					18'10"				
Multi-	Dly Analysis								
Footon					مثام اممد مد		امممم	100	
Capacity	an plies using :	0 10WS 01	SDVV22336 a	t 24 O.C Maxim	ium ena aist	ance not to exe	ceed	12	
Load		0.0	PLF						
Yield Limi	t per Foot	382	2.5 PLF						
Yield Limi	t per Fastener	25 Lo	5.0 lb. okup						
Edge Dist	ance	1 1	/2"						
Min. End I	Distance	6"							
Load Com	ibination Factor	1.0	0						
L									
Notes			chemicals		6. For flat roofs prov	vide proper drainage to pre	event	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated S structural ac	structured Designs is responsion dequacy of this component	sible only of the based on the	Handling & Installa	ation e cut or drilled	ponding			Metsä Wood 301 Merritt 7 Building 2nd Floor	Fayetteville, NC USA
design crite responsibility	eria and loadings show of the customer and/or th	n. It is the e contractor to	2. Refer to manufact regarding installatio	turer's product information n requirements, multi-ply			[]	Norwalk, CT 06851	28314 910-864-TRUS
ensure the application, a	component suitability of and to verify the dimensions a	ine intended and loads.	fastening details, bea approvals	m strength values, and code				www.metsawood.com/us	
Lumber 1. Dry servi	ce conditions, unless noted o	otherwise	J. Damaged Beams must Design assumes top er Provide lateral surgers	t not be used dge is laterally restrained rt at bearing points to overit				ICC-ES: ESR-3633	
2. LVL not t	to be treated with fire retards	ant or corrosive	lateral displacement ar	nd rotation	This design is	valid until 2/26/2023			соттесн



LisDesign	Client: Weaver Developmen Project:	t Date: Input by:	5/10/2021 Curtis Quick	Page 4 of 10
	Address:	Job Name Project #:	E: The Lauren H Beams	
GDH-1 Kerto-S LVL	1.750" X 11.875"	2-Ply - PASSED	Level: Level	
• •	•	•	• •	= M 1
• •	•	•	• • —	
1 SPF End Grain			2 SPF End Grain	
	10'			3 1/2"
1	10'		1	
Multi-Ply Analysis				
Fasten all plies using 2 rows of SDW	/22338 at 24" o.c Maximu	m end distance not to exceed	112"	
Capacity 0.0 %				
Yield Limit per Foot 255.0 PL	F			
Yield Limit per Fastener 255.0 lb. Yield Mode Lookup				
Edge Distance 1 1/2"				
Load Combination				
Duration Factor 1.00				
		1	Manufacturer Info	Comtech. Inc.
Notes cherr Calculated Structured Designs is responsible only of the Handlin	nicals 6.	For flat roofs provide proper drainage to prevent ponding	Metsä Wood	1001 S. Reilly Road, Suite #639 Fayetteville, NC
design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to any or the composed with the state based on the regar	peams must not be cut or drilled r to manufacturer's product information rding installation requirements, multi-ply		301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (200) 622 5850	28314 910-864-TRUS
application, and to verify the dimensions and loads. Lumber 3. Dama	ning details, beam strength values, and code ovals aged Beams must not be used		www.metsawood.com/us	
4 Desig	agoa Boarno maorinor bo aboa		ICC-LO. LOI - 3033	



Project: Address: BM1 Kerto-S LVL 1.750" X 9.250" 2-Ply - F • • • • •	Input by: Curtis Quick Job Name: The Lauren H Beams Project #: PASSED Level: Level
Address: BM1 Kerto-S LVL 1.750" X 9.250" 2-Ply - F • • • • •	Job Name: The Lauren H Beams Project #: PASSED Level: Level
BM1 Kerto-S LVL 1.750" X 9.250" 2-Ply - F	Project #: PASSED Level
BM1 Kerto-S LVL 1.750" X 9.250" 2-Ply - F	• •
• • •	• • • • • • • • • • • • • • • • • • •
• • •	•
• • •	• • • • • • • • • • • • • • • • • • •
• • •	• • • 2 SPF 3 1/2"
• • •	• • • 2 SPF 3 1/2"
• • •	•
• • •	•
• • •	•
• • •	• 9 1/ 2 SPF 3 1/2"
• • •	• Ý 9 1/ 2 SPF 3 1/2"
• • •	• / / / / / / / / / / / / / / / /
	2 SPF
	3 1/2"
7'6"	
	1
1 7'6"	
Multi-Ply Analysis	
Easten all plies using 2 rows of SDW22338 at 24" o.c. Maximum and distance	a not to avcoud 12"
Load 0.0 PLF	
Yield Limit per Foot 255.0 PLF	
Yield Limit per Fastener 255.0 lb.	
Yield Mode Lookup Edge Distance 1 1/2"	
Min. End Distance 6"	
Load Combination	
Duration Factor 1.00	
Notes chemicals 6. For flat roofs provide proponding Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. 1. UX beams must not be cut or drilled 6. For flat roofs provide proponding 1. UX beams must not be cut or drilled application, and to verify the dimensions and loads. 3. Refer to manufacturer's product information requirements, multi-ply fastering details, beam strength values, and code approvals 6. For flat roofs provide proponding 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive 6. Provide lateral support at bearing points to avoid lateral displacement and rootation	per drainage to prevent Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633 Comtech_Inc. Comtech_Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS

		Client: Weaver Development		D	ate:	5/10/2021			Page 7 of 10
~		Project:		In	put by:	Curtis Quick	ζ.		-
i	isDesign	Address:		Je	b Name:	The Lauren	H Beams		
				Р	roject #:				
RM2	Korto-SIVI	1 750" X 9 250"	2-Plv -	DVCC		evel: Level			
		1.750 X 5.250	Z-1 1y -						
		1							
•		•	•						
	a state .	attern attern							/ / 9 1/
1	International Contraction of the			and the second second					
	F End Grain		2 SPE End Gra	in i					
		C14 1							
		5'4"							3 1/2"
1		5'4"		1					
lember l	nformation			Reaction) lh (Unliff)	
	Girdor	Application: Elect		Bro				/	Const
rype. Plies:	2	Design Method [,] ASD			Live	1650	3110W	vvina	ounsi
Moisture Co	ndition: Drv	Building Code: IBC 2012			0	1009	1040	U	0
Deflection L	L: 360	Load Sharing: No		2	0	1059	1040	0	0
Deflection T	L: 240	Deck: Not Check	ked						
Importance:	Normal								
Temperature	e: Temp <= 100°F								
	·			Bearing	s				
				Bearing	Length	Cap. I	React D/L lb	Total Ld. C	ase Ld. Comb.
				1 - SPF	3.500"	31%	1659 / 1640	3299 L	D+S
				End					
nalysis R	lesults			Grain	0.500"	0.40/	4050 / 4040		D .0
Analysis	Actual Locat	ion Allowed Capacity Comb	. Case	2 - SPF	3.500"	31%	1659 / 1640	3299 L	D+S
Moment	3675 ft-lb	2'8" 14423 ft-lb 0.255 (25%) D+S	L	Grain					
Unbraced	3675 ft-lb	2'8" 11811 ft-lb 0.311 (31%) D+S	L						
Shear	2062 lb	4'4" 7943 lb 0.260 (26%) D+S	L						
LL Defl incl	h 0.023 (L/2497)	2'8" 0.162 (L/360) 0.140 (14%) S	L						
TL Defl inc	h 0.047 (L/1241)	2'8" 0.244 (L/240) 0.190 (19%) D+S	L						
esian Na	otes			1					
1 Fasten al	I plies using 2 rows of SDW2	2338 at 24" o.c. Maximum end distance	not to exceed	1					
12".									
2 Refer to I	last page of calculations for fa	asteners required for specified loads.	are nublished						
4 Girders a	re designed to be supported	on the bottom edge only.	ere published.						
5 Top loads	s must be supported equally I	by all plies.							
6 Top brace	ed at bearings.								
/ BOTTOM bi	raced at bearings. lenderness ratio based on sin	ale ply width							
	Load Type	Location Trib Width Side	Dead 0 0	Live	1 Snow	1 15 Wi	nd 1.6 Cons	t 1.25 Comr	ments
1	Liniform		615 DI E		E 610	5 PI F		0 PLF 42	
1		100	01311	012	1 010		01 El	UTLI AZ	
	Seir Weight		/ PLF						
latar		chemicals 0.1	For flat roofs provide	roper drainage **	prevent	lanufacturer li	nfo	Comtech, Inc	•
NOTES Calculated Structur	red Designs is responsible only of the Ha	andling & Installation	ponding	noper urainage to	M	letsä Wood		1001 S. Reilly Fayetteville, N	/ Road, Suite #639 NC
structural adequac design criteria a	cy of this component based on the 1. and loadings shown. It is the 2.	LVL beams must not be cut or drilled Refer to manufacturer's product information			30	01 Merritt 7 Bu	ilding, 2nd Floor 351	USA 28314	IC
esponsibility of the nsure the comp	e customer and/or the contractor to ponent suitability of the intended	regarding installation requirements, multi-ply fastening details, beam strength values, and code			(8	300) 622-5850		910-864-1RU	10
umber	vering are dimensions and loads.	approvals Damaged Beams must not be used			w IC	/ww.metsawoo CC-ES: ESR-36	a.com/us 633		
 Dry service con LVL not to be to 	nditions, unless noted otherwise 5.	Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and retation	.						отесн
			i nis design is valid	untii 2/26/202	3				

	Client: Weaver Developm	ent Date:	5/10/2021	Page 8 of 10
lisDesign	Project:	Input by	r: Curtis Quick	
	Audress.	Project	#:	
BM2 Kerto-S LVL	1.750" X 9.250"	2-Plv - PASSED	Level: Level	
				,
•	•	•		\overline{M} 1
		1 1 1		
•	•	•¥		
1 SPE End Grain				
	5'4"			3 1/2"
	54			5 1/2
I	54	Ι		
Fasten all plies using 2 rows of S	DW22338 at 24" o.c Maxim	ium end distance not to excee	ed 12"	
Load 0.0 P	° PLF			
Yield Limit per Foot 255.0 Yield Limit per Fastener 255.0) PLF			
Yield Mode Look	up			
Edge Distance 1 1/2' Min End Distance 6"				
Load Combination				
Duration Factor 1.00				
Nataa	chemicals	6 For flat roofe provide proper drainage to accurat	Manufacturer Info	Comtech, Inc.
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the	andling & Installation	ponding	Metsä Wood	1001 S. Reilly Road, Suite #639 Fayetteville, NC USA
design criteria and loadings shown. It is the 2. responsibility of the customer and/or the contractor to	LVL peams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-plv		Norwalk, CT 06851	28314 910-864-TRUS
ensure the component suitability of the intended application, and to verify the dimensions and loads.	fastening details, beam strength values, and code approvals Damaged Beams must not be used		(000) 022-0850 www.metsawood.com/us	
Dry service conditions, unless noted otherwise LVL not to be treated with fire retardant or corrosive	Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid		100-ES: ESK-3033	сотесн
	lateral displacement and rotation	This design is valid until 2/26/2023		



		Client:	Weaver Developmer	nt	Date:	5/10/2021	Page 10 of 10
		Project:			Input by:	Curtis Quick	5
isDesig	ิย	Address:			Job Nam	e: The Lauren H Beams	
					Project #		
DM2 Kowto		4 750			DACOED	Level: Level	
BINI3 Kerto	0-5 LVL	1.750	X 9.250	2-Piy	- PASSED		
•		•		٠	•	-	
						1/2	V V
						$\overline{\Sigma}$	9 1/4
•		٠		•	• -	— <u> </u>	
1 SPF End Grain					2 SPF End Grain		
1			6'3"				3 1/2"
<u>/</u>			6121			/	
			03			I	
Multi-Plv Analycic							
	0 10		o	1 11		1.400	
⊦asten all plies using	g 2 rows of SDV	v22338 at	24" o.c Maximu	m end dista	nce not to excee	d 12"	
Capacity	0.0 %						
Load	0.0 PLF	-					
Yield Limit per Foot	255.0 PL	_F					
Yield Mode	200.0 lb.						
Edge Distance	1 1/2"						
Min. End Distance	6"						
Load Combination							
Duration Factor	1.00						
							Orretta h las
Notes	cher	micals	6	. For flat roofs provide	e proper drainage to prevent	wanutacturer Info	Lomiech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Structured Designs is res structural adequacy of this comport	ponsible only of the Hand	beams must not be	cut or drilled			Metsä Wood 301 Merritt 7 Building 2nd Floor	Fayetteville, NC USA
design criteria and loadings si responsibility of the customer and/o	hown. It is the 2. Reference on the contractor to	er to manufactu arding installation	rer's product information requirements multi-ply			Norwalk, CT 06851	28314 910-864-TRUS
ensure the component suitability application, and to verify the dimension	of the intended faste	ening details, beam rovals	strength values, and code			(800) 622-5850 www.metsawood.com/us	
Lumber	3. Dan	naged Beams must r	not be used			ICC-ES: ESR-3633	
 Dry service conditions, unless not LVL not to be treated with fire ret 	ted otherwise tardant or corrosive	vide lateral support	at bearing points to avoid	This 1 1 1	11 June 11 0/00/00000		соттесн
	later			i nis design is va	ilia until 2/26/2023		





					Beam Legend						
					PlotID	Length	Product	Plies	Net Qty		
		All Truss Reactions are Le	ess		BM1	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2		
		than 3,000 lbs. Unless Noted O		BM3	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2			
= Denotes Left End	End a second			BM2	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2			
(Reference Engineered	Truss Drawing)	Denotes Reaction Greater th	Truss Placement Plan	GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2			
Do Not Erect Trusses Backwards			SCALE: 3/16" = 1'	GDH	23' 0"	1-3/4"x 14" LVL Kerto-S	2	2			
LOAD CHART FOR JACK STUDS (04565 CN1 ABLES (5025()) & (b)) MARKEY LACK STUDE (CO 100(0 & (A CM ST	BUILDER	Weaver Development Co. I nc.	CITY/CO.	Harnett Co. / Harnett		THIS IS A TRUSS F These trusses are desi the building design at th sheets for each truss do	LACEMENT DIAGRAM ONLY. gned as individual building components to be incorporated into e specification of the building designer. See individual design sign identified on the placement drawing. The building designer				

LOAD CHART FOR JACK STUDS MANFE ON TABLES (2003) 3 (b)) MANFE OF THE STUDY OF THE OF THE		BUILDER	Weaver Development Co. I nc.	CITY/CO.	Harnett Co. / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer		
(14) 100 100 100 100 100 100 100 100 100 10	FEADEWEEROUS POINTS ADDITION OF ADDITIONOO ADDITICOO ADDITICOO ADDITICOO ADDITICOO ADDITICOO ADDITICOO ADDITICOO ADDITICOO ADDITICOO ADDITIC	IND RIACTION 0.5 TO 0.5	JOB NAME	Lot 4 C.P. Stewart Rd.	ADDRESS	606 C.P. Stewart Rd.	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
			PLAN	The Lauren H / Elev. A / Cp / 3 Car	MODEL Roof	Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR	
1700 1 3400 2 5100 3	2650 1 5100 2 7650 3	5600 2 10200 3	SEAL DATE	2/24/20	DATE REV.	05/10/21	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studies required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#. <u>Curtis Quick</u> <u>Signature</u> <u>Curtis Quick</u>	Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444
8500 5 10200 6	10200 4 12750 5 15300 6	13000 4 17000 5		Quote #	DRAWN BY	Curtis Quick		
13600 8 15300 9			JOB #	J0521-2902	SALES REP.	Lenny Norris		

56' 0" 14' 0" 15' 2" 16' 0" 10' 10" P1GE P1 P1 10'0" 10'0" P1 P1 Floor, Roof, & Ceiling To Be Ladder Framed By Builder 16' 4" 20' 6'' 23' 0" 3 674 lbs. Cathedral Ceiling 3 Studs A6GE A4A A6 A5 A6 A5 A5 A5 A4 A4 A4 **В**М2 A1GE 2' 1 1/2" A2A 4' 9" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" АЗ АЗ A2 A2 A2 A2 A3 A2 Lite Attic Storage 16' 8" 30 lbs. Live Load 71' 6" 71' 6" PB4GE PB1GE PB3 PB3 PB3 PB3 PB4 PB3 PB2 PB2 PB1 PB4 PB2 PB1 PB1 PB1 PB1 PB1 PB1 PB1 PB1 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 2' 0" 5,371 lbs. 4 S<mark>t</mark>uds BM1 ____ Bonus Room Extend Beam To Allow Nailing Of Hanger Flange 40 lbs. Live Load 30, 0" Floor, Roof, & Ceiling To Be Ladder/Framed By Builder





					Beam Legend			
				PlotID	Length	Product	Plies	Net Qty
	All Truss Reactions are Le		BM1	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	
	than 3,000 lbs. Unless Noted O	therwise.		BM3	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
SS			BM2	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	
awing)	Denotes Reaction Greater th	Truss Placement Plan SCALE: 3/16" = 1'	GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	
ards			GDH	23' 0"	1-3/4"x 14" LVL Kerto-S	2	2	
DER	Weaver Development Co. Inc.	CITY/CO.	Harnett Co. / Harnett		THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design			
					sneets for each truss d	esign identified on the placement drawing. The building designer		

= Denotes Left End of Truss
(Reference Engineered Truss Drawing
Do Not Erect Trusses Backwards

LOAD CH	HART FOR JA EN ON 1 ABLES REO2 5 MARK STATES ACTURE	ACK STUDS (5) (10) (5) (10) (5) (10)	BUILDER	Weaver Development Co. I nc.	CITY/CO.	Harnett Co. / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	COMTECH ROOF & FLOOR TRUSSES & BEAMS Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787	
X Y	PEADEWERNER 2017		JOB NAME	Lot 4 C.P. Stewart Rd.	ADDRESS	606 C.P. Stewart Rd.			
			PLAN	The Lauren H / Elev. A / Cp / 3 Car	MODEL	Roof	or online @ sociadustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables		
	2650 1 5100 2 7650 3 10200 4 12750 5 15300 6		SEAL DATE	2/24/20	DATE REV.	05/10/21	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those		
			QUOTE #	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#. Curtis Ouick		
			JOB #	J0521-2902	SALES REP.	Lenny Norris	SignatureCurtis Quick	Fax: (910) 864-4444	