

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0221-0977 Lot 5 Pope Road

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15423688 thru E15423715

My license renewal date for the state of North Carolina is December 31, 2021.

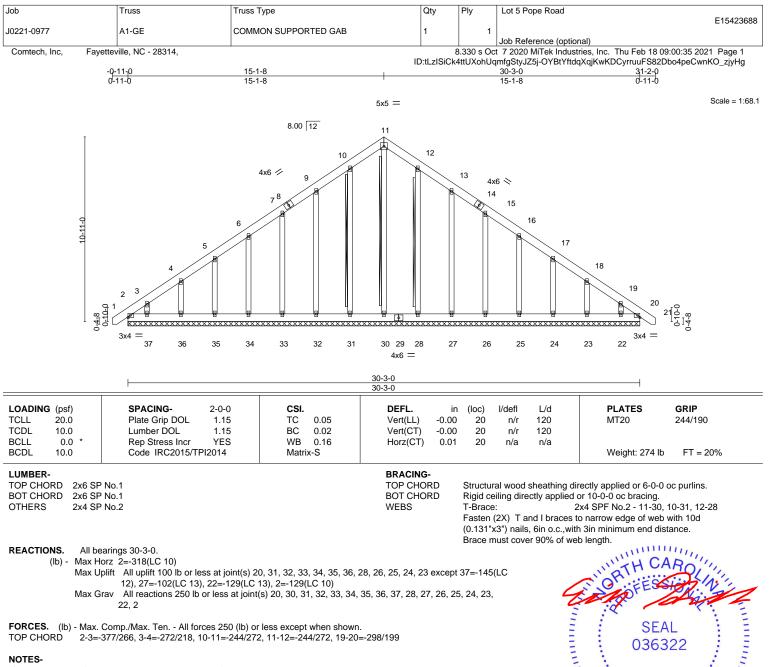
North Carolina COA: C-0844



February 18,2021

## Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20, 31, 32, 33, 34, 35, 36, 28, 26, 25, 24, 23 except (jt=lb) 37=145, 27=102, 22=129, 2=129.

10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



## February 18,2021

818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems. see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Typ	be		Qty	Ply	Lot 5 P	ope Road		<b>-</b> /-/000000
J0221-0977	A2	СОММО	N		7	1				E15423689
								erence (optional)		
Comtech, Inc, Fayette	eville, NC - 28314,			10					s, Inc. Thu Feb 18 09 4QaEUcJMtJzJKIRso	
-(	-11-0 7-8-5 -11-0 7-8-5		15-1-8	1	22-6-	11	qriigSty5	30-3-1 7-8-5	0 31-2-0 0-11-0	
(	-11-0 7-8-5	1	7-5-3	I	7-5-	3	1	7-8-5	0-11-0	
										Scale = 1:66.9
				5x5 =						
			8.00 12							
I			0.00   12	5						
		4	.6 1/	$// \leq$	$\sim$					
		47	14			15	4x6 📎			
		2x4 \\	4	// \\		$\searrow$	6	0.4.4		
					<u>\</u>	Ð	$\mathbf{X}$	2x4 //		
0-11-0		3	//		//					
10-1		1	、    //					$\sim$		
			$\backslash$ //							
	13						//		16	
					/	$\langle   \rangle$	/		$\sim$	
	2					\\ //				
9-1	1		$\bigvee$			$\mathbb{W}$			8	19
0-4-8 0-10-0				Þ		121				0-1-0 -4-8 -4-8
0	3x4 =	17	18 12 19	11	20	10	21	22	3x4 =	0
			3x4 =	4x6 =		3x4 =				
	l.	10-2-1		20-0-15				30-3-0		
		10-2-1		9-10-14		1		10-2-1		
Plate Offsets (X,Y) [2:	0-0-0,0-0-3], [8:Edge,0-	-0-3]		1						
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	ir	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.25	Vert(LL)		10-12	>999	360	MT20	244/190
TCDL 10.0 BCLL 0.0 *	Lumber DOL Rep Stress Incr	1.15 YES	BC 0.48 WB 0.37	Vert(CT Horz(CT		10-12 8	>999 n/a	240 n/a		
BCDL 10.0	Code IRC2015/TP		Matrix-S	Wind(LL		2-12	>999	240	Weight: 211 lb	FT = 20%
					· 					
LUMBER- TOP CHORD 2x6 SP N	o 1			BRACIN TOP CH		Structur	ral wood	choothing direct	ly applied or 5-5-4 c	
BOT CHORD 2x6 SP N				BOT CH					0-0-0 oc bracing.	c putilits.
WEBS 2x4 SP N						9	<b>J</b>		5	
REACTIONS. (size)	2 0 2 0 0 0 0 0 0									
()	2=0-3-8, 8=0-3-8 2 2=-254(LC 10)									
	t 2=-75(LC 12), 8=-75(									
	/ 2=1425(LC 19), 8=14	25(LC 20)								
Max Grav	mp/Max Ten - All for	ces 250 (lb) or le	ss except when shown	h						
Max Grav FORCES. (Ib) - Max. Co	omp./Max. Ten All for 32/357, 3-5=-1802/453			1.						
Max Grav FORCES. (lb) - Max. Co TOP CHORD 2-3=-19 BOT CHORD 2-12=-1		, 5-7=-1802/453, 3, 8-10=-161/150	7-8=-1933/357 4	1.						

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-7 to 3-7-6, Interior(1) 3-7-6 to 15-1-8, Exterior(2) 15-1-8 to 19-6-5, Interior(1) 19-6-5 to 31-0-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

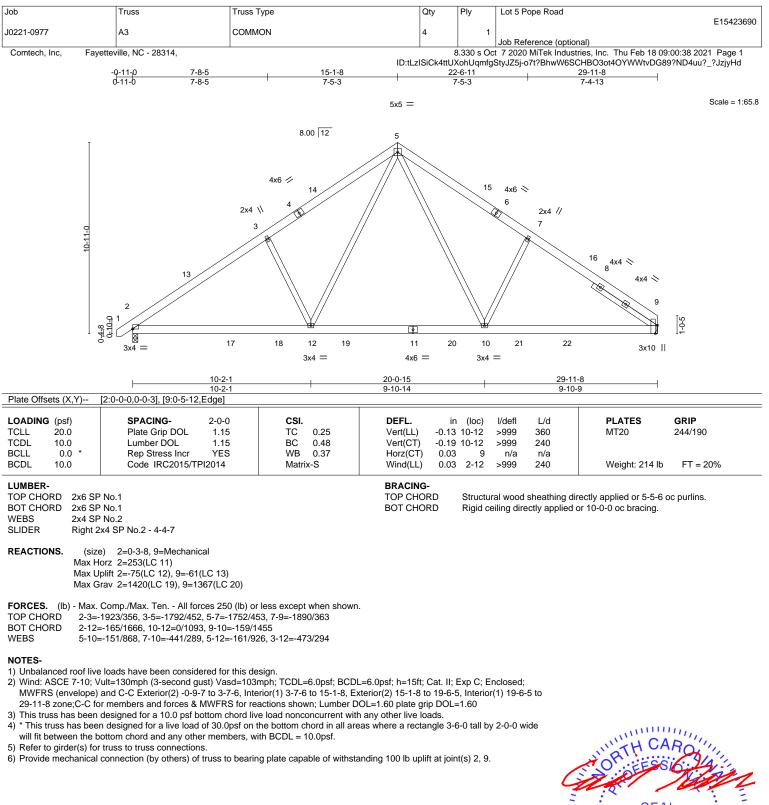
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

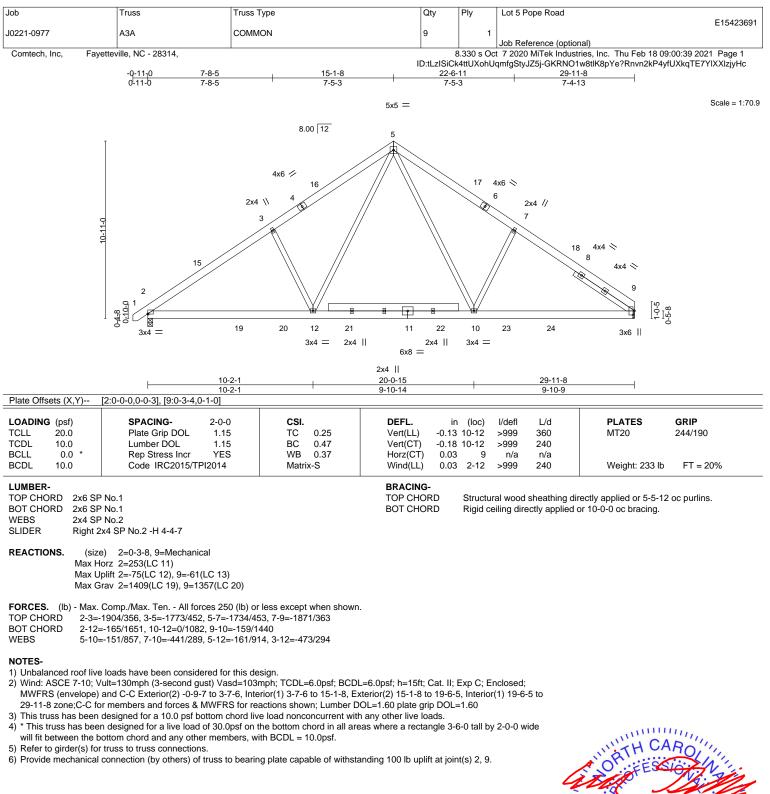


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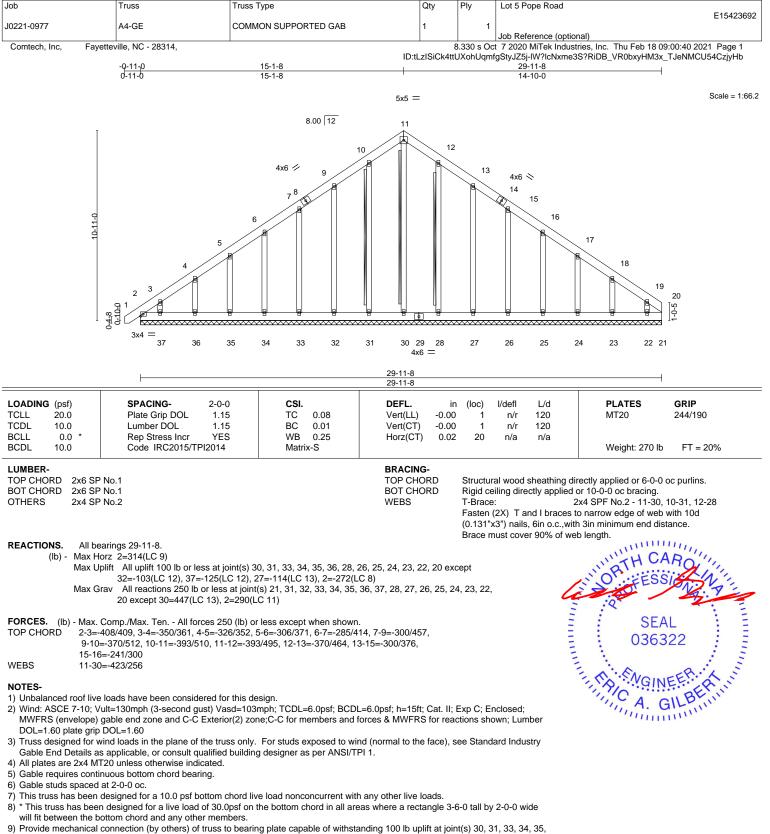


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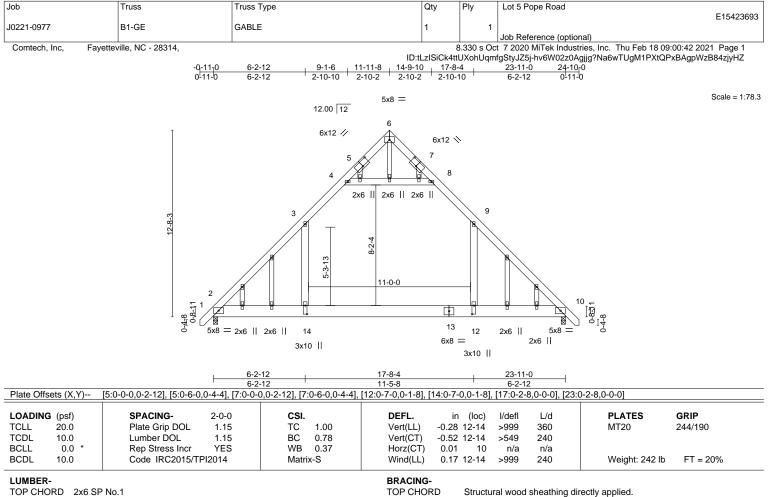
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 30, 31, 33, 34, 35 36, 28, 26, 25, 24, 23, 22, 20 except (jt=lb) 32=103, 37=125, 27=114, 2=272.

10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

February 18,2021

ENGINEERING BY **TRENCO** AMITEK Affiliate 818 Soundside Road Edenton, NC 27932

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BOT CHORD

Rigid ceiling directly applied or 9-6-8 oc bracing.

TOP CHORD2x6 SP No.1BOT CHORD2x10 SP No.1WEBS2x6 SP No.1OTHERS2x4 SP No.2

## REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=-382(LC 10)

Max Grav 2=1531(LC 20), 10=1531(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-2032/8, 3-4=-1129/181, 4-6=-9/300, 6-8=-9/300, 8-9=-1129/181, 9-10=-2032/8
- BOT CHORD 2-14=0/1264, 12-14=0/1264, 10-12=0/1264
- WEBS 4-8=-1498/263. 3-14=0/905. 9-12=0/904

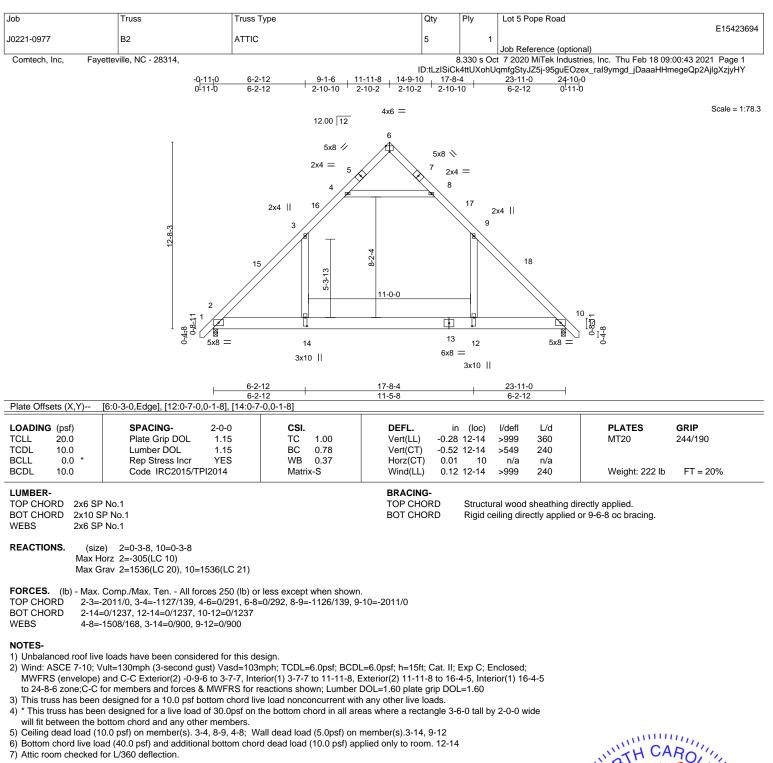
#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Ceiling dead load (10.0 psf) on member(s). 3-4, 8-9, 4-8; Wall dead load (5.0psf) on member(s).3-14, 9-12
- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
- 10) Attic room checked for L/360 deflection.



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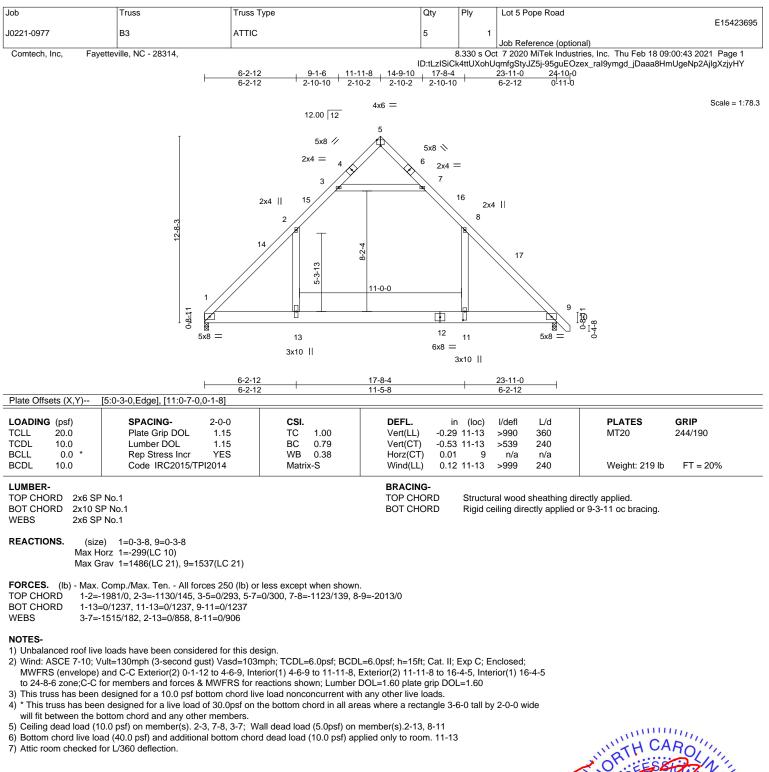






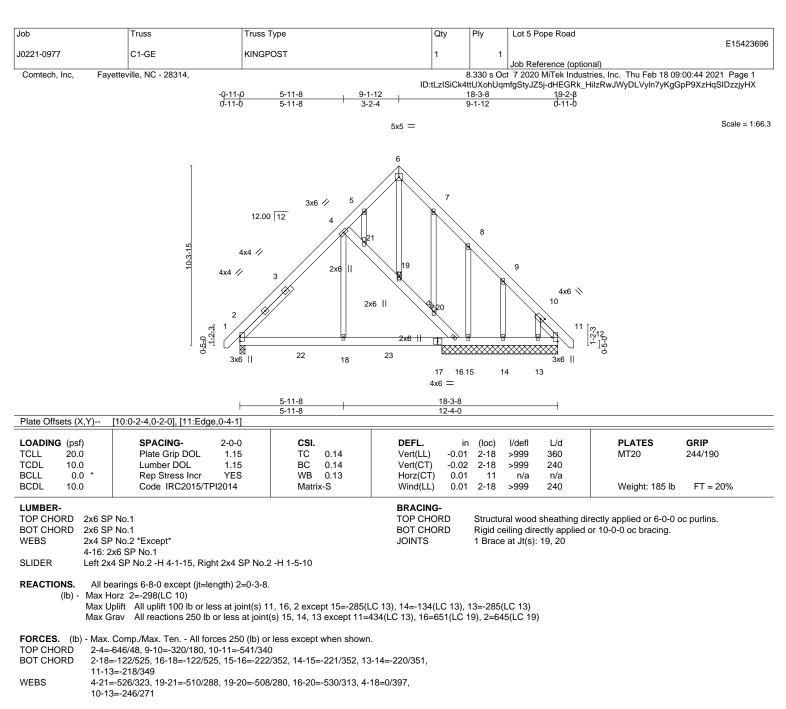
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A MiTek Aft 818 Soundside Road Edenton, NC 27932





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#### NOTES-

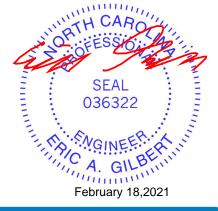
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

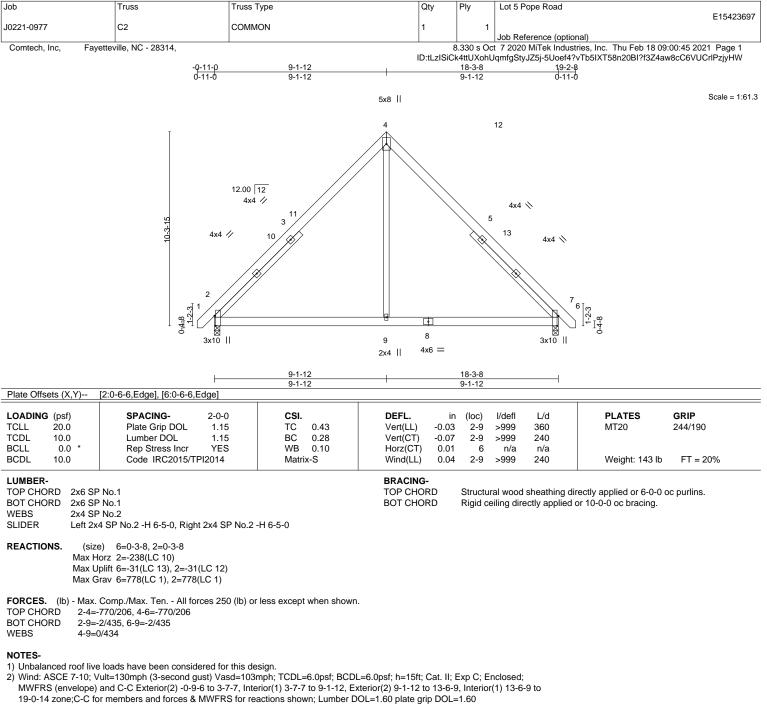
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 16, 2 except (jt=lb) 15=285, 14=134, 13=285.



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A MiTek Affilia 818 Soundside Road Edenton, NC 27932



3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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Job	Truss	Truss Type	Qty	Ply	Lot 5 Pope Road	
J0221-0977	С3	Common Girder	1	2		E15423698
Comtech, Inc, Faye	etteville, NC - 28314,			3.330 s Oc		tries, Inc. Thu Feb 18 09:00:47 2021 Page 1
	F	4-8-10 9-1-12	13-6-14	JXohUqmi	18-3-8	PL0nmFXvT3fNQIMmuDYcM2PznhypIzjyHU
		4-8-10 4-5-2	4-5-2		4-8-10	Scale = 1:61.3
			5x8			State = 1.01.5
	10-3-15 12-1 12-2-12	12.00 12 3x10 // 2 2 10 11 9 12	3 13 8 14 7 15	3x 4 6	10 × 5 16 5x12	1-2-3
Plate Offsets (X,Y)	F	4x12    <u>4-8-10 9-1-12</u> <u>4-8-10 4-5-2</u> [5:0-0-14,0-3-3], [5:0-0-7,0-0-7], [:	10x10 = 6x8 = $13-6-14$ $4-5-2$ 8:0-5-0,0-6-4]	4x12	<u>18-3-8</u> 4-8-10	
LOADING (psf) TCLL 20.0 TCDL 10.0 PCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.14 Lumber DOL 1.14	TC 0.58 BC 0.46	<b>DEFL.</b> in Vert(LL) -0.05 Vert(CT) -0.10	6-8 6-8	l/defl L/d >999 360 >999 240	PLATES         GRIP           MT20         244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr NC Code IRC2015/TPI2014	WB 0.72 Matrix-S	Horz(CT) 0.02 Wind(LL) 0.03		n/a n/a >999 240	Weight: 335 lb FT = 20%
WEBS 2x4 SP WEDGE Left: 2x6 SP No.1 , Righ	2400F 2.0E No.2 nt: 2x6 SP No.1		BRACING- TOP CHORD BOT CHORD		iling directly applied	irectly applied or 6-0-0 oc purlins. or 10-0-0 oc bracing.
Max Ho Max U Max G FORCES. (Ib) - Max. TOP CHORD 1-2=-1 BOT CHORD 1-9=-2	6520/410, 2-3=-4384/381, 3-4=- 296/4231, 8-9=-297/4240, 6-8=-	?) 0 (Ib) or less except when shown. 4382/381, 4-5=-6457/405	21/2988			SEAL 036322
Top chords connecte Bottom chords conne Webs connected as 2) All loads are conside ply connections have 3) Unbalanced roof live 4) Wind: ASCE 7-10; V/ MWFRS (envelope); 5) This truss has been 6) * This truss has been will fit between the bo	e been provided to distribute onl loads have been considered fo ult=130mph (3-second gust) Va Lumber DOL=1.60 plate grip D designed for a 10.0 psf bottom of designed for a live load of 30.0. bottom chord and any other mem	ered at 0-9-0 oc. aggered at 0-7-0 oc. except if noted as front (F) or back / loads noted as (F) or (B), unless this design. sd=103mph; TCDL=6.0psf; BCDL= DL=1.60 hord live load nonconcurrent with psf on the bottom chord in all area	otherwise indicated. =6.0psf; h=15ft; Cat. II; any other live loads. as where a rectangle 3-f	Exp C; Er 6-0 tall by	nclosed; 2-0-0 wide	SEAL 036322
8) Hanger(s) or other co 1-10-4, 1274 lb dowr down and 81 lb up at	n and 81 lb up at 3-10-4, 1274 l t 9-10-4, 1274 lb down and 81 l and 1281 lb down and 74 lb up f others.	vided sufficient to support concent o down and 81 lb up at 5-10-4, 12 o up at 11-10-4, 1274 lb down an at 18-1-12 on bottom chord. The	74 lb down and 81 lb up d 81 lb up at 13-10-4, a	o at 7-10 and 1274	-4, 1274 lb b down and	February 18,2021

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Job	Truss	Truss Type	Qty	Ply	Lot 5 Pope Road	]
					E15423698	
J0221-0977	C3	Common Girder	1	2		
				2	Job Reference (optional)	
Comtech, Inc, Fayettev	ille, NC - 28314,		6	3.330 s Oct	7 2020 MiTek Industries, Inc. Thu Feb 18 09:00:47 2021 Page 2	-
Comtech, Inc, Fayettev	ille, NC - 28314,				7 2020 MiTek Industries, Inc. Thu Feb 18 09:00:47 2021 Page 2	

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## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

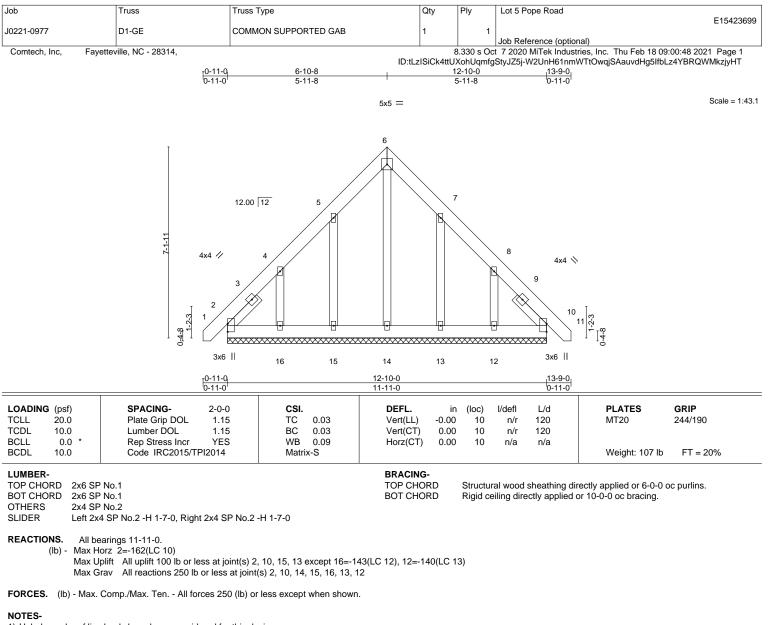
Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 1-5=-20

Concentrated Loads (lb)

Vert: 6=-1172(F) 5=-1179(F) 10=-1172(F) 11=-1172(F) 12=-1172(F) 13=-1172(F) 14=-1172(F) 15=-1172(F) 16=-1172(F)

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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-9-6 to 3-7-7, Exterior(2) 3-7-7 to 5-11-8, Corner(3) 5-11-8 to 10-4-5, Exterior(2) 10-4-5 to 12-8-6 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 15, 13

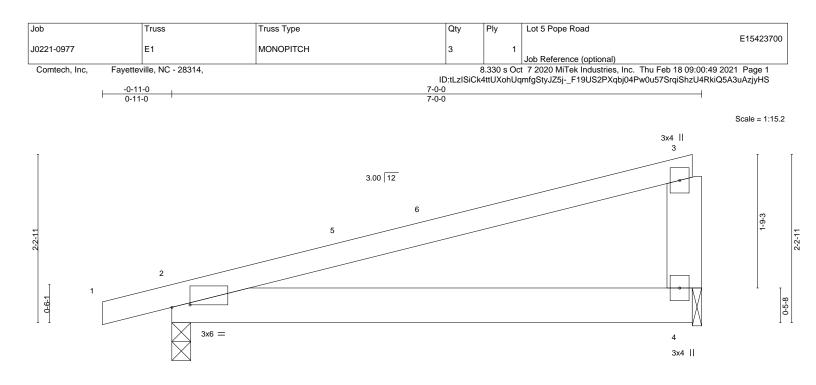
except (jt=lb) 16=143, 12=140.
10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



ENGINEERING BY AMITEK Affiliate 818 Soundside Road

Edenton, NC 27932

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OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
FCLL	20.0	Plate Grip DOL	1.15	TC	0.63	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.18	Vert(CT)	-0.05	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	κ-P	Wind(LL)	0.06	2-4	>999	240	Weight: 32 lb	FT = 20%

WEBS	2x6 SP No.1	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
BOT CHORD	2x6 SP No.1		except end verticals.
TOP CHORD	2x4 SP No.1	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=64(LC 8) Max Uplift 2=-135(LC 8), 4=-108(LC 8) Max Grav 2=333(LC 1), 4=261(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

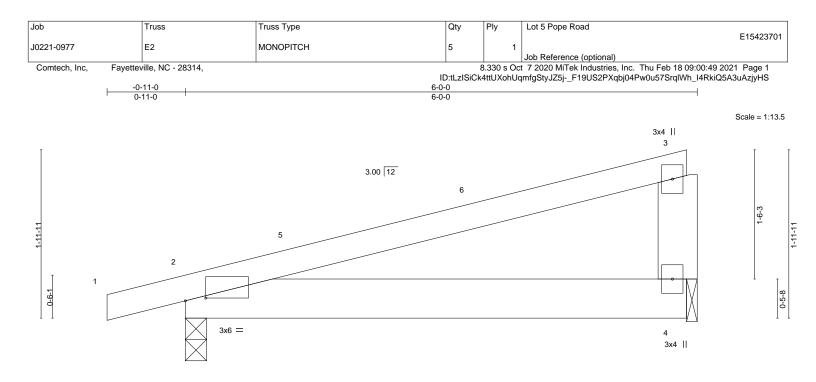
## NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 6-9-4 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=135, 4=108.



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	CLL 20.0 CDL 10.0 CLL 0.0 * CDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.44 BC 0.12 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.01 -0.03 0.00 0.03	(loc) 2-4 2-4 2-4	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 27 lb	<b>GRIP</b> 244/190 FT = 20%
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 BOT CHORD
 2x4 SP No.1
 TOP CHORD
 Structural wood snearining directly applied of 6-0-0 oc puring except end verticals.

 BOT CHORD
 2x6 SP No.1
 BOT CHORD
 Rigid ceiling directly applied or 10-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=56(LC 8) Max Uplift 2=-121(LC 8), 4=-91(LC 8) Max Grav 2=294(LC 1), 4=220(LC 1)

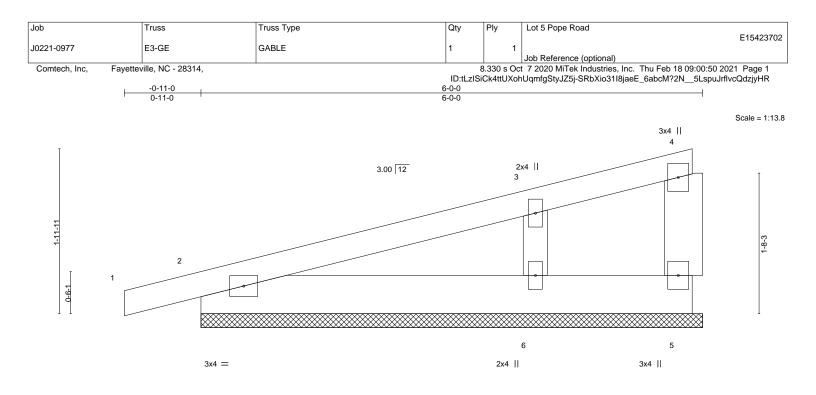
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 5-9-4 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=121.



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LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.13 BC 0.04 WB 0.04 Matrix-P	<b>DEFL.</b> ii Vert(LL) -0.00 Vert(CT) 0.00 Horz(CT) 0.00	0 1 n/r 120	PLATES         GRIP           MT20         244/190           Weight: 28 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x6 SF OTHERS 2x4 SF	P No.1 P No.1		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing of except end verticals. Rigid ceiling directly applied	directly applied or 6-0-0 oc purlins, d or 10-0-0 oc bracing.

REACTIONS. (size) 5=6-0-0, 2=6-0-0, 6=6-0-0

Max Horz 2=80(LC 8)

Max Uplift 5=-5(LC 8), 2=-76(LC 8), 6=-102(LC 12) Max Grav 5=8(LC 1), 2=194(LC 1), 6=315(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

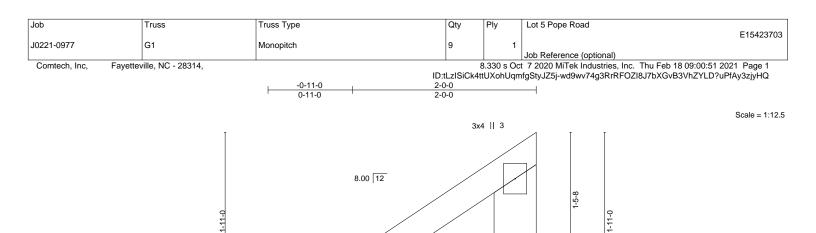
#### NOTES-

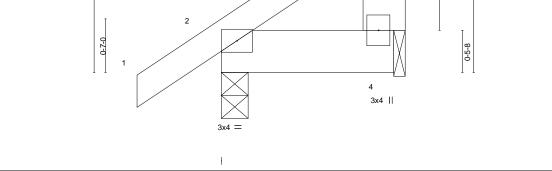
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2 except (jt=lb) 6=102.



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OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (l	loc) l/c	lefl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL) -	-0.00	2 >9	99 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.01	Vert(CT) -	-0.00	2 >9	99 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	0.00		n/a n/a	
3CDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL)	0.00	2 *	**** 240	Weight: 12 lb FT = 20%

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x4 SP No.1 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

REACTIONS. (size) 2=0-3-8, 4=0-1-8

Max Horz 2=55(LC 12) Max Uplift 2=-8(LC 12), 4=-20(LC 12)

Max Grav 2=142(LC 1), 4=62(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

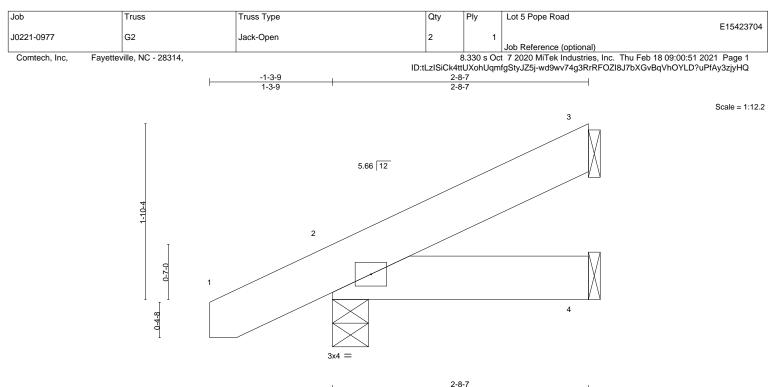


Structural wood sheathing directly applied or 2-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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						1	2-8-7					
	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.02	Vert(CT)	-0.00	2	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matrix	-P	Wind(LL)	0.00	2	****	240	Weight: 17 lb	FT = 20%

## LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

**REACTIONS.** (size) 3=Mechanical, 2=0-4-9, 4=Mechanical

Max Horz 2=55(LC 12)

Max Uplift 3=-31(LC 12), 2=-23(LC 12)

Max Grav 3=51(LC 1), 2=201(LC 1), 4=49(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.

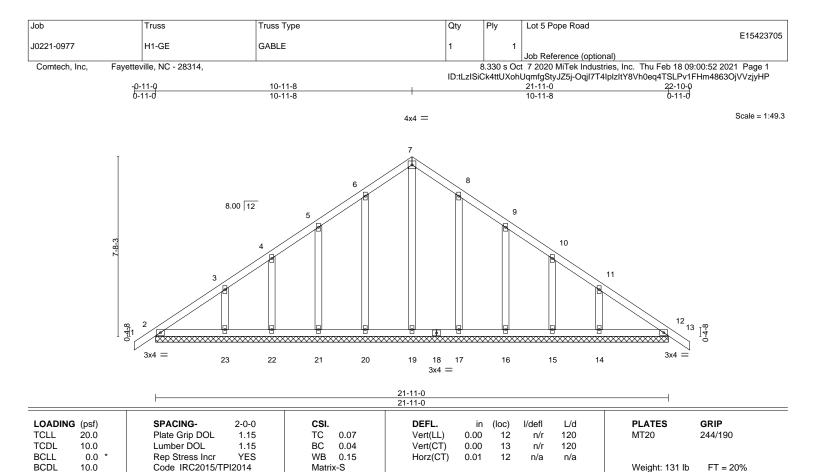


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 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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Structural wood sheathing directly applied or 2-8-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



BRACING-

TOP CHORD

BOT CHORD

# FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

LUMBER-

OTHERS REACTIONS.

TOP CHORD

BOT CHORD

(lb) -

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

14=254(LC 20)

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Max Uplift All uplift 100 lb or less at joint(s) 2, 20, 21, 22, 17, 16, 15 except 23=-126(LC 12), 14=-125(LC 13)

All reactions 250 lb or less at joint(s) 2, 12, 19, 20, 21, 22, 17, 16, 15 except 23=255(LC 19),

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.

2x4 SP No.1

2x4 SP No.1

2x4 SP No.2

Max Grav

All bearings 21-11-0. Max Horz 2=-237(LC 10)

- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 21, 22, 17, 16, 15 except (jt=lb) 23=126, 14=125.

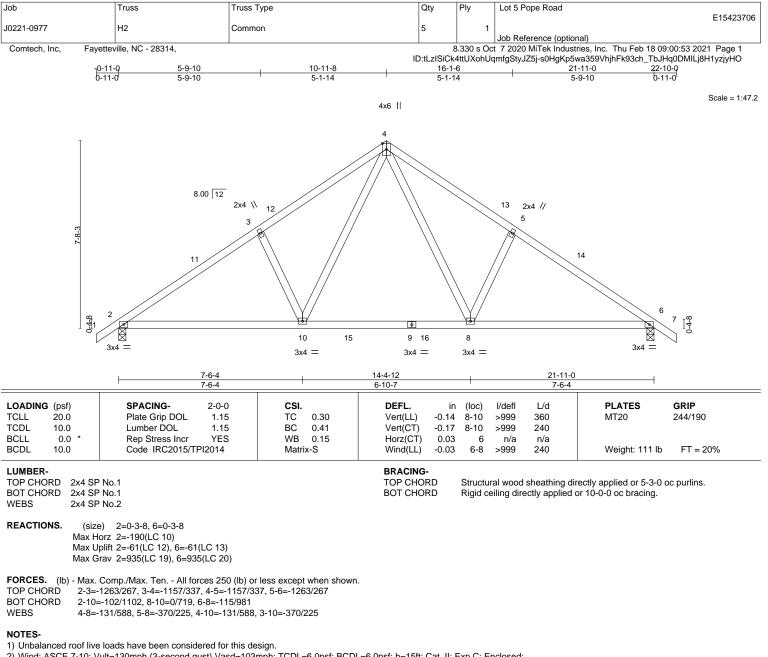


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 10-11-8, Exterior(2) 10-11-8 to 15-4-5, Interior(1) 15-4-5 to 22-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

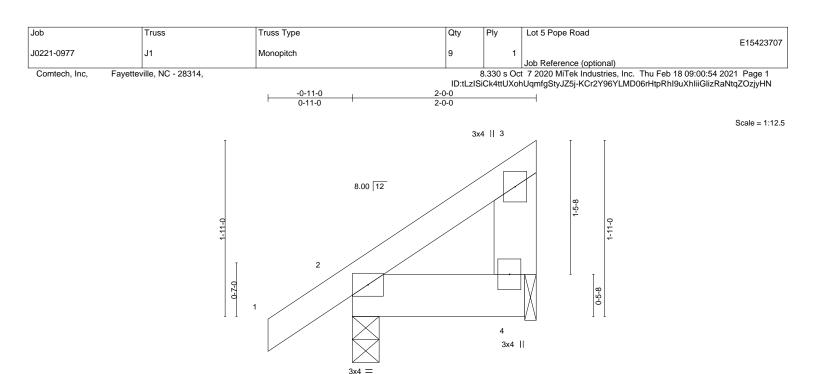
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.



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LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.01	Vert(CT)	-0.00	2	>999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	0.00		n/a	n/a		
3CDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL)	0.00	2	****	240	Weight: 12 lb	FT = 20%

TOP CHORD

BOT CHORD

ł

## LUMBER-

TOP CHORD 2x4 SP No.1 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

REACTIONS. (size) 2=0-3-8, 4=0-1-8

Max Horz 2=55(LC 12) Max Uplift 2=-8(LC 12), 4=-20(LC 12)

Max Grav 2=142(LC 1), 4=62(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

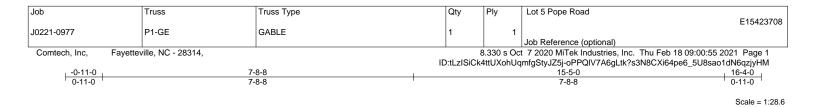


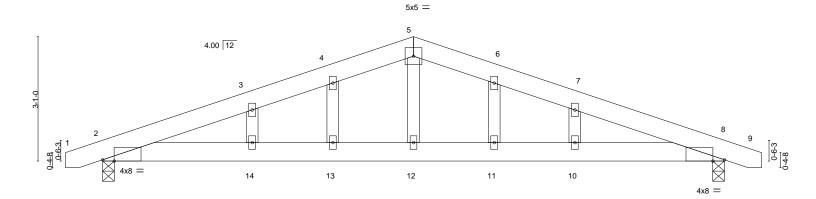
Structural wood sheathing directly applied or 2-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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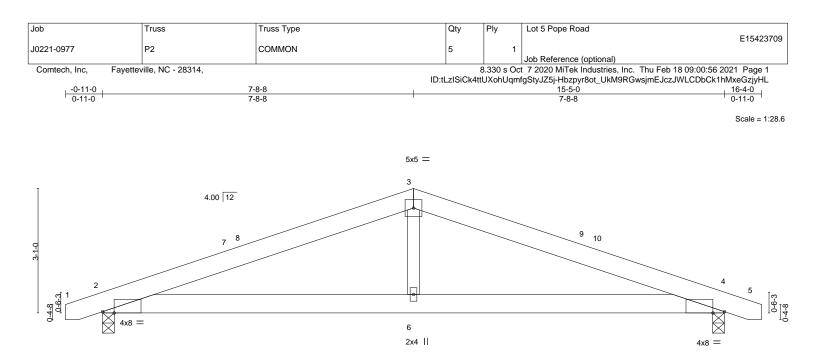




Ļ	7-8-8				15-5-0		
Plate Offsets (X,Y) [	7-8-8 [2:0-3-7,Edge], [8:0-3-7,Edge]				7-8-8		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.20 BC 0.29 WB 0.09 Matrix-S	DEFL. in Vert(LL) -0.04 Vert(CT) -0.07 Horz(CT) 0.01 Wind(LL) 0.07	14 × 14 × 8	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 89 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x4 SP OTHERS 2x4 SP	No.1 No.2		BRACING- TOP CHORD BOT CHORD			irectly applied or 6-0-0 or 7-2-9 oc bracing.	oc purlins.
Max Ho Max Up	e) 2=0-3-8, 8=0-3-8 brz 2=-57(LC 17) plift 2=-356(LC 8), 8=-356(LC 9) rav 2=659(LC 1), 8=659(LC 1)						
TOP CHORD 2-3=-1 7-8=-1 BOT CHORD 2-14=- 10-11:	Comp./Max. Ten All forces 250 (lb) or 1140/1230, 3-4=-1080/1233, 4-5=-1066 1140/1230 1090/1024, 13-14=-1090/1024, 12-13= =-1090/1024, 8-10=-1090/1024 484/357	/1253, 5-6=-1066/1253, 6	6-7=-1080/1234,				
<ul> <li>2) Wind: ASCE 7-10; Vu MWFRS (envelope) of for reactions shown;</li> <li>3) Truss designed for w Gable End Details as</li> <li>4) All plates are 2x4 MT</li> <li>5) Gable studs spaced at</li> <li>6) This truss has been of 7) * This truss has been will fit between the bor</li> </ul>	loads have been considered for this de ult=130mph (3-second gust) Vasd=103i gable end zone and C-C Exterior(2) zor Lumber DOL=1.60 plate grip DOL=1.60 vind loads in the plane of the truss only. s applicable, or consult qualified building 120 unless otherwise indicated. at 2-0-0 oc. designed for a 10.0 psf bottom chord liv n designed for a live load of 30.0psf on t ottom chord and any other members. connection (by others) of truss to bearin	mph; TCDL=6.0psf; BCD e; porch left and right ex For studs exposed to wi designer as per ANSI/T e load nonconcurrent with he bottom chord in all are	posed;C-C for members nd (normal to the face), s Pl 1. h any other live loads. eas where a rectangle 3-6	and forces see Standa 6-0 tall by 2	& MWFRS rd Industry 2-0-0 wide	SEA 0363	
							UEER. AT INT



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7-8-8	
DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         0.07         4-6         >999         240           Vert(CT)         -0.06         2-6         >999         240           Horz(CT)         0.01         4         n/a         n/a	PLATES         GRIP           MT20         244/190           Weight: 82 lb         FT = 20%
	ectly applied or 6-0-0 oc purlins. r 7-7-12 oc bracing.
	Vert(LL)         0.07         4-6         >999         240           Vert(CT)         -0.06         2-6         >999         240           Horz(CT)         0.01         4         n/a         n/a           BRACING-         TOP CHORD         Structural wood sheathing direction         Structural wood sheathing direction

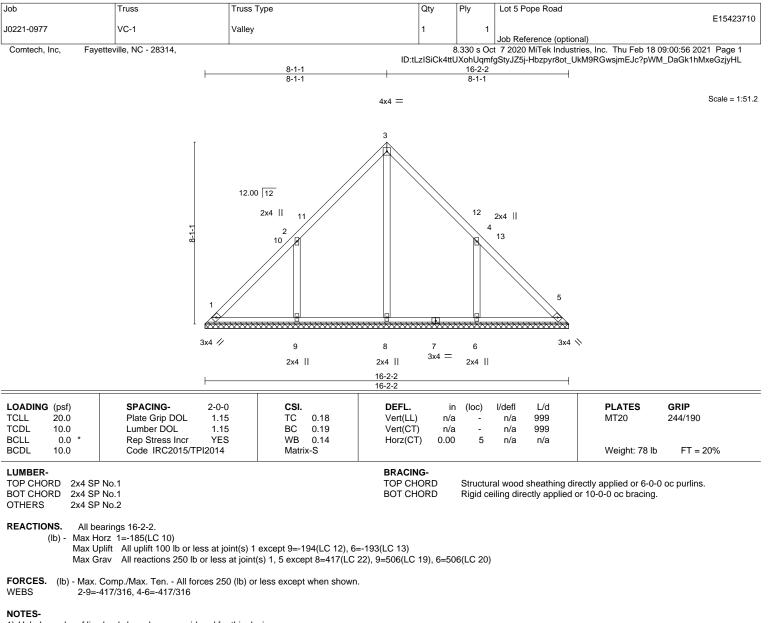
MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 3-7-12, Interior(1) 3-7-12 to 7-8-8, Exterior(2) 7-8-8 to 12-1-5, Interior(1) 12-1-5 to 16-2-1 zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=249, 4=249.



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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 8-1-1, Exterior(2) 8-1-1 to 12-5-14, Interior(1) 12-5-14 to 15-9-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

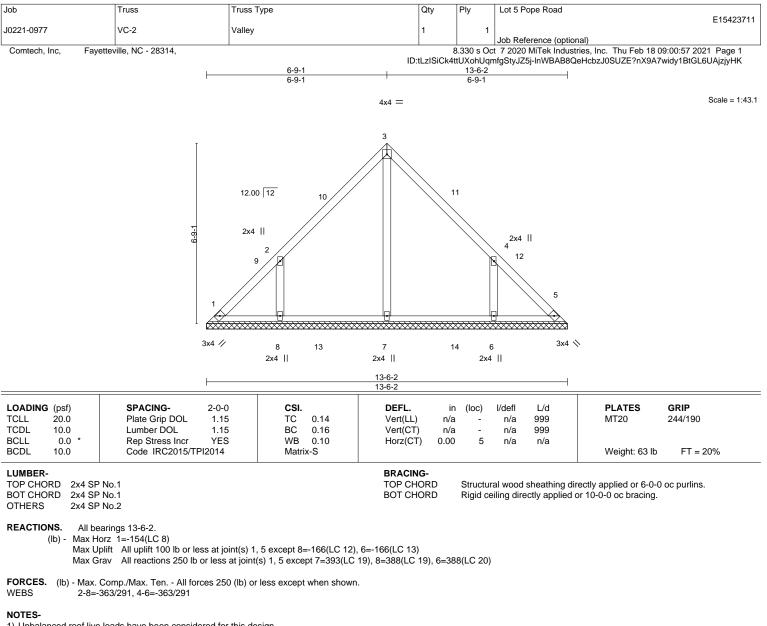
5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=194, 6=193.



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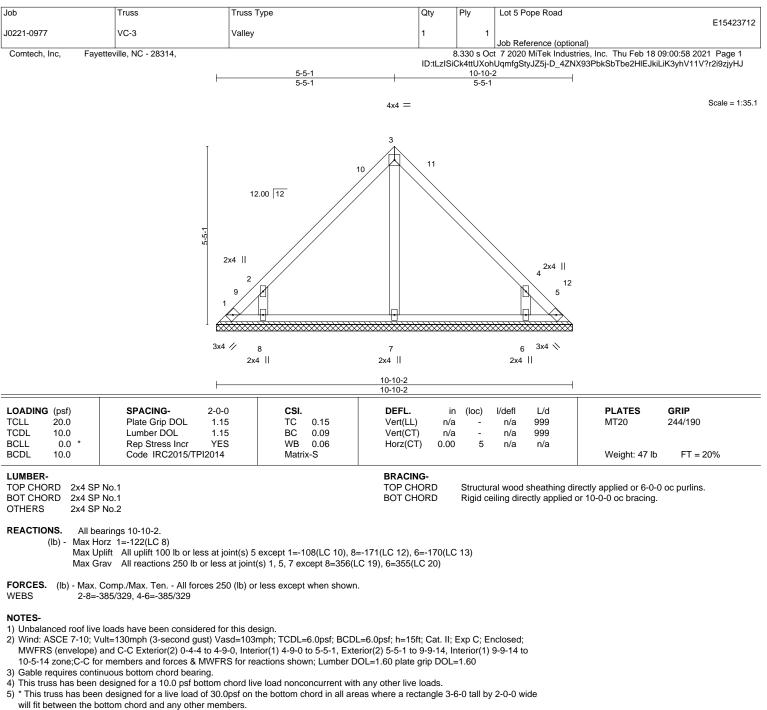


- Unbalanced roof live loads have been considered for this design.
   Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 6-9-1, Exterior(2) 6-9-1 to 11-1-14, Interior(1) 11-1-14 to 13-1-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=166, 6=166.



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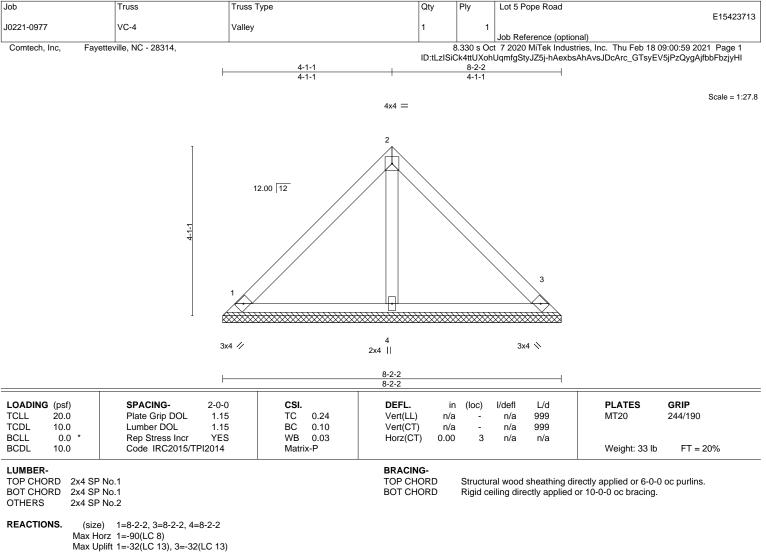


6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 1=108, 8=171, 6=170.



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Max Grav 1=182(LC 1), 3=182(LC 1), 4=234(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

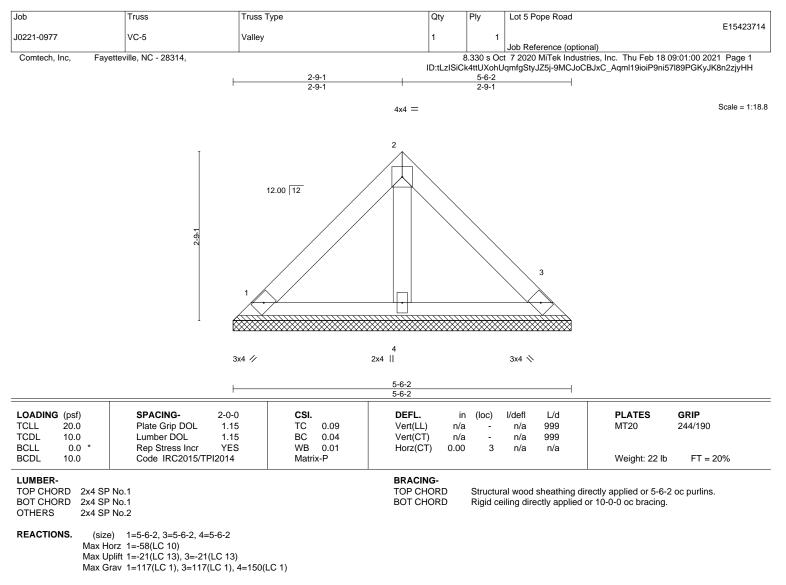
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

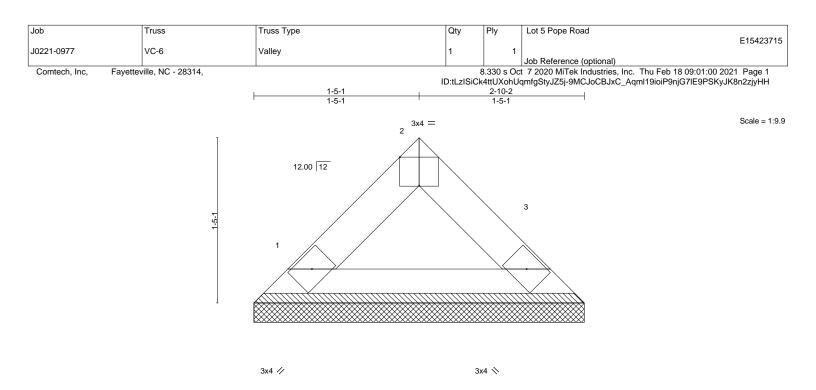
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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<u>2-10-2</u> 2-10-2

DEFL.

Vert(LL)

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

in (loc)

n/a

n/a

0.00

l/defl

n/a

n/a

n/a

3

L/d

999

999

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

PLATES

Weight: 9 lb

MT20

Structural wood sheathing directly applied or 2-10-2 oc purlins.

GRIP

244/190

FT = 20%

BOT CHORD 2x4 SP No.1

Plate Offsets (X,Y)--

20.0

10.0

10.0

0.0 \*

LOADING (psf)

TCLL

TCDL

BCLL

BCDL

LUMBER-

TOP CHORD

**REACTIONS.** (size) 1=2-10-2, 3=2-10-2

2x4 SP No.1

Max Horz 1=26(LC 9) Max Uplift 1=-3(LC 12), 3=-3(LC 13)

[2:0-2-0,Edge]

SPACING-

Plate Grip DOL

**Rep Stress Incr** 

Code IRC2015/TPI2014

Lumber DOL

Max Grav 1=86(LC 1), 3=86(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

CSI.

0.02

0.04

0.00

тс

BC

WB

Matrix-P

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

2-0-0

1.15

1.15

YES

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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