

Trenco 818 Soundside Rd Edenton, NC 27932

# Re: 2001007-2001007A ADAMS JOHNSON JOB

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I43675746 thru I43675766

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



November 19,2020

## Sevier, Scott

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 9, and 11. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





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7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



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November 19,2020

ENGINEERING BY REENCED A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932

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TOP CHORD2x6 SP No.2BOT CHORD2x6 SP No.2OTHERS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** All bearings 24-0-0.

(lb) - Max Horz 2=-218(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 21, 22, 23, 19, 18, 17 except 24=-117(LC 12), 16=-116(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 14, 20, 21, 22, 23, 19, 18, 17 except 24=320(LC 19), 16=320(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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## LUMBER-

TOP CHORD2x6 SP No.2BOT CHORD2x6 SP No.2OTHERS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** All bearings 22-0-0.

(lb) - Max Horz 1=-220(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 22, 23, 24, 18, 17, 16 except 25=-123(LC 12), 15=-107(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 21, 19, 22, 23, 24, 18, 17, 16, 15, 13 except 25=267(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-255/158

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



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5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.



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LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.05 BC 0.03 WB 0.09 Matrix-S	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.00         14         n/r         120           Vert(CT)         -0.00         14         n/r         90           Horz(CT)         0.00         14         n/a         n/a	PLATES         GRIP           MT20         244/190           Weight: 192 lb         FT = 20%
LUMBER-			BRACING-	

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 OTHERS 2x4 SP No.3 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 21-7-0.

(lb) - Max Horz 2=-223(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 22, 23, 24, 19, 18, 17, 16 except 25=-101(LC 12) Max Grav All reactions 250 lb or less at joint(s) 2, 21, 20, 22, 23, 24, 25, 19, 18, 14, 17, 16

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



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 Satisfies
 Ansi/TPH Qu

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 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Plate Offsets (X,Y)	[1:0-0-0,0-0-10], [5:Edge,0-0-10], [6:0-4	-12,0-1-8], [7:0-4-0,0-4-4],	[9:0-4-12,0-1-8]				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.22 BC 0.97 WB 0.96 Matrix-MS	<b>DEFL.</b> in Vert(LL) -0.07 Vert(CT) -0.15 Horz(CT) 0.05	(loc) l/d 7-9 >9 7-9 >9 5 ı	lefi L/d 99 240 99 180 n/a n/a	PLATES MT20 Weight: 471 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x4 SP	No.2 No.2 No.3		BRACING- TOP CHORD BOT CHORD	Structural v Rigid ceiling	vood sheathing dire g directly applied o	ectly applied or 6-0-0 o r 10-0-0 oc bracing.	oc purlins.
REACTIONS. (size Max H Max U Max G	a) 1=0-3-8, 5=0-3-8 brz 1=200(LC 9) plift 1=-772(LC 12), 5=-808(LC 13) rav 1=6607(LC 1), 5=6906(LC 1)						
FORCES.         (lb) - Max.           TOP CHORD         1-2=-           BOT CHORD         1-9=-           WEBS         3-7=-	Comp./Max. Ten All forces 250 (lb) or 8999/1148, 2-3=-6185/879, 3-4=-6188/8 896/7153, 7-9=-896/7153, 6-7=-836/715 910/6968, 4-7=-2958/505, 4-6=-326/328	less except when shown. 879, 4-5=-9039/1153 97, 5-6=-836/7197 87, 2-7=-2901/497, 2-9=-32	21/3221				
<ul> <li>NOTES-</li> <li>1) 3-ply truss to be con Top chords connected Bottom chords connected Bottom chords connected as</li> <li>2) All loads are conside ply connections have</li> <li>3) Unbalanced roof live</li> <li>4) Wind: ASCE 7-10; V gable end zone and forces &amp; MWFRS for</li> <li>5) This truss has been will fit between the b</li> <li>7) Two H2.5A Simpson connection is for upil</li> <li>8) Hanger(s) or other c 2-0-12, 1181 lb down lb down and 149 lb u and 149 lb up at 16- chord. The design/s</li> </ul>	nected together with 10d (0.120"x3") na ed as follows: 2x6 - 2 rows staggered at ected as follows: 2x6 - 2 rows staggered follows: 2x4 - 1 row at 0-9-0 oc. red equally applied to all plies, except if a been provided to distribute only loads loads have been considered for this de ult=130mph Vasd=103mph; TCDL=6.0p C-C Exterior(2) zone; cantilever left and reactions shown; Lumber DOL=1.60 pl designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on to ottom chord and any other members. Strong-Tie connectors recommended t ft only and does not consider lateral for onnection device(s) shall be provided su n and 149 lb up at 4-0-12, 1181 lb dowr up at 10-0-12, 1193 lb down and 149 lb up a election of such connection device(s) is lard	ils as follows: 0-9-0 oc. at 0-5-0 oc. i noted as front (F) or back noted as (F) or (B), unless sign. osf; BCDL=6.0psf; h=30ft; 1 right exposed ; end vertic ate grip DOL=1.60 e load nonconcurrent with he bottom chord in all area o connect truss to bearing ces. fficient to support concent n and 149 lb up at 6-0-12, up at 18-0-12, and 1193 lb do the responsibility of others	(B) face in the LOAD C otherwise indicated. Cat. II; Exp B; Enclosed al left and right exposed any other live loads. as where a rectangle 3-6 walls due to UPLIFT at rated load(s) 1181 lb do 1193 lb down and 149 l wn and 149 lb up at 14 wn and 149 lb up at 20 s.	ASE(S) sect ; MWFRS (e ;C-C for mer 3-0 tall by 2-( jt(s) 1 and 5. jt(s) 1 and 5. jt(s) 1 and 149 b up at 8-0- -0-12, 1193 J -0-12 on bot	ion. Ply to nvelope) mbers and D-0 wide . This Ib up at 12, 1193 b down tom	SE 0449	AROL NEER, HAL SEVIEN SEVIEN 19,2020
WARNING - Verify of	design parameters and READ NOTES ON THIS ANI	D INCLUDED MITEK REFERENCE	PAGE MII-7473 rev. 5/19/2020	) BEFORE USE.		ENGINE	ERING BY

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Job	Truss	Truss Type	Qty	Ply	ADAMS JOHNSON JOB	
					14367	75757
2001007-2001007A	DGR	Common Girder	1	3		
					Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.	420 s Oct	9 2020 MiTek Industries, Inc. Wed Nov 18 10:34:12 2020 Page	2

8.420 s Oct 9 2020 MiTek Industries, Inc. Wed Nov 18 10:34:12 2020 Page 2 ID:s7vO\_9sUp1hEcely5XVLEiyI?LB-nNHOS2aULAz84?15IaHaw8Owb6lN6aip10LmemyHzIv

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 10-13=-20

Concentrated Loads (lb)

Vert: 6=-1179(B) 16=-1179(B) 17=-1179(B) 18=-1179(B) 19=-1179(B) 20=-1179(B) 21=-1179(B) 22=-1179(B) 23=-1179(B) 24=-1179(B) 2

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	0-0 <u>-6</u> 0-0-6		<u>7-0-2</u> 6-11-12						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IBC2015/TPI2014	CSI. TC 0.29 BC 0.16 WB 0.03 Matrix-P	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	<b>GRIP</b> 244/190 FT = 20%
	Code IRC2015/TPI2014	Matrix-P	BRACING-					Weight: 24 lb	F1 = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD

2x4 SP No.3 OTHERS

REACTIONS. 1=6-11-6, 3=6-11-6, 4=6-11-6 (size) Max Horz 1=-50(LC 10) Max Uplift 1=-29(LC 12), 3=-35(LC 13) Max Grav 1=130(LC 1), 3=130(LC 1), 4=225(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





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🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid to use only design parameters and READ NOTES ON THIS AND INCLUDED WITH REPORT PAGE MIT 475 169 (2020 BEFORE USE). Design valid for use only with MITEK deconnectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent colleges with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component 
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 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road

Edenton, NC 27932



BRACING-

BOT CHORD

LUMBER-	
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BCDL

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3 OTHERS 2x4 SP No.3

10.0

OTHERS 2x4 SP No.3 OTHERS 2x4 SP No.3

REACTIONS. (size) 1=8-0-4, 3=8-0-4, 4=8-0-4 Max Horz 1=-67(LC 8) Max Uplift 1=-34(LC 12), 3=-43(LC 13) Max Grav 1=159(LC 1), 3=159(LC 1), 4=258(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code IRC2015/TPI2014

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-P

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



Weight: 29 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

FT = 20%

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2x4 //

2x4 🚿

Structural wood sheathing directly applied or 4-6-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

0-015			4-6-4								
Plate Offsets (X,Y)	[2:0-2-0,Edge]				4-3-14						
_OADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	n/a	-	n/a	999	MT20	244/190
CDL 10.0	Lumber DOL	1.15	BC	0.27	Vert(CT)	n/a	-	n/a	999		
3CLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
3CDL 10.0	Code IRC2015/TP	12014	Matrix	κ-P						Weight: 14 lb	FT = 20%

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3

REACTIONS. (size) 1=4-5-9, 3=4-5-9 Max Horz 1=-34(LC 10) Max Uplift 1=-15(LC 12), 3=-15(LC 13) Max Grav 1=146(LC 1), 3=146(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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