isDesign

Client: WEAVER

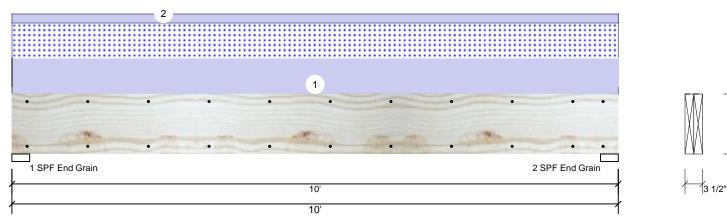
Project: Address: Date: 2/11/2021

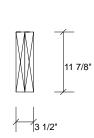
Input by: Lenny Norris Job Name: LINDSEY B 3-CAR

Project #:

Kerto-S LVL 1.750" X 11.875" GDH9' 3-car 2-Ply - PASSED

Level: Level





Page 1 of 1

Member Information								
Type:	Girder							
Plies:	2							
Moisture Condition:	Dry							
Deflection LL:	480							
Deflection TL:	360							
Importance:	Normal							
Temperature:	Temp <= 100°F							

Actual

6091 ft-lb

6091 ft-lb

2024 lb

LL Defl inch 0.052 (L/2209)

TL Defl inch 0.119 (L/962)

Application: Floor ASD Design Method: **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Brg Live Wind Const Dead Snow 0 1511 1165 0 0 1 0 0 2 1511 1165 0

Capacity Comb. Case 0.266 (27%) D+S L 0.627 (63%) D+S L 0.198 (20%) D+S ī 5' 0.239 (L/480) 0.220 (22%) S

Dear ing	•							
Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.		
1 - SPF End Grain	3.500"	25%	1511 / 1165	2676	L	D+S		
2 - SPF End Grain	3.500"	25%	1511 / 1165	2676	L	D+S		

Design Notes

Analysis Results

Analysis

Moment

Shear

Unbraced

1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".

Location Allowed

5'

1'2 5/8" 10197 lb

5' 22897 ft-lb

9721 ft-lb

5' 0.318 (L/360) 0.370 (37%) D+S

- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

	0 1 7											
ID	D Load Type		Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments		
1	Uniform			Тор	233 PLF	0 PLF	233 PLF	0 PLF	0 PLF	D1 TRUSS		
2	Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	FRAME DOWN WALL		
	Self Weight				9 PLF							

L

Rearings

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





isDesign

Client: WEAVER

Project: Address:

Date: Input by:

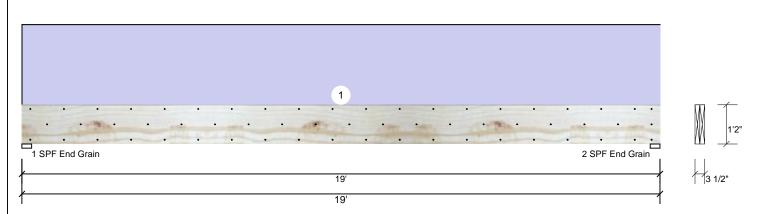
Project #:

Lenny Norris Job Name: LINDSEY B 3-CAR Page 1 of 1

1.750" X 14.000" 2-Ply - PASSED **Kerto-S LVL GDH18'**

Level: Level

2/11/2021



vlember Inf	ormation						Reaction	ns UNPAT	TERNE	D lb (Uplift)			
Type:	Girder		Application	on: F	loor		Brg	Live	Dea	d Snow		Wind	Const
Plies:	2		Design M	lethod: A	SD		1	0	200	0		0	0
Moisture Cond	ition: Dry		Building (Code: II	BC/IRC 2015		2	0	200	0		0	0
Deflection LL:	480		Load Sha	aring: N	lo								
Deflection TL:	360		Deck:	N	lot Checked								
Importance:	Normal												
Temperature:	Temp <= 10	0°F					Bearings	s					
							Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF End	3.500"	19%	2003 / 0	2003	Uniform	D
Analysis Res	sults						Grain						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2-SPF	3.500"	19%	2003 / 0	2003	Uniform	D
Moment	9063 ft-lb	9'6"	24299 ft-lb	0.373 (37%	b) D	Uniform	End Grain						
Unbraced	9063 ft-lb	9'6"	9068 ft-lb	0.999 (100%)	D	Uniform							
Shear	1709 lb	1'4 3/4"	9408 lb	0.182 (18%	b) D	Uniform							
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)									

Design Notes

1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".

9'6 1/16" 0.618 (L/360) 0.600 (60%) D

- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 12'3 3/4" o.c.
- 6 Bottom braced at bearings.

TL Defl inch 0.372 (L/599)

7 Lateral slenderness ratio based on single ply width.

1 Eatoral olorido	mode ratio basea em emigio	p.,a									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	200 PLF	0 PLF	0 PLF	0 PLF	0 PLF	FRAME DOWN WALL & GABLE	
	Self Weight				11 PLF						

Uniform

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Handling & Installation

 1. UVI beams must not be out or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

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