RIDGE VENT AS REQUIRED 12

RIDGE VENT AS REQUIRED COMPOSITION SHINGLES AS COMPOSITION SHINGLES AS

FRONT ELEVATION WITH SIDE LOAD GARAGE SCALE 1/8" = 1'-0"

PLANS DESIGNED TO THE **2018 NORTH CAROLINA STATE** RESIDENTIAL BUILDING CODE

MEAN ROOF HEIGHT 25'-8	" HEIGHT TO RIDGE 30'-0"					
CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A			
FENESTRATION U-FACTOR	0.35	0.35	0.35			
SKYLIGHT U-FACTOR	0.55	0.55	0.55			
GLAZED FENESTRATION SHGC	0.30	0.30	0.30			
CEILING R-VALUE	38 or 30cl	38 or 30cl	38 or 30d			
WALL R-VALUE	15	15	19			
FLOOR R-VALUE	19	19	30			
* BASEMENT WALL R-VALUE	5/13	10/15	10/15			
** SLAB R-VALUE	0	10	10			
* CRAWL SPACE WALL R-VALUE	5/13	10/15	10/19			

"10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION ** INSULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF FOOTING: INSULATION DEPTH WITH STEM WALL SLAB 24° OR TO BOTTOM OF FOUNDATION WALL DESIGNED FOR WIND SPEED OF 120 MPH, 3 SECOND GUST (93 FASTEST MILE) EXPOSURE "8"

						HULLU		
MEAN ROOF	UPT	O 30'	30'-1"	TO 35"	35'-1"	TO 40'	40'-1"	TO 45'
ZONE 1	14.2	-15.0	14.9	-15.8	15.5	-16.4	15.9	-16.8
ZONE 2	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	-21.8	17.4	-22.4
DESIGNED FOR WIN								
COMPONENT								
	& CLA	DDING	DESIG		R THE	FOLLO	WING	LOADS
COMPONENT	& CLA	DDING	DESIG	NED FO	R THE	FOLLO TO 40'	WING 40'-1"	TO 45
COMPONENT MEAN ROOF	& CLA	O 30'	DESIG 30'-1"	NED FC TO 35' -18.9	35'-1" 18.2	FOLLO TO 40' -19.6	WING 40'-1" 18.7	TO 45' -20.2
COMPONENT MEAN ROOF ZONE 1	8 CLA UP 1 16.7	ODING O 30' -18.0	DESIG 30'-1' 17.5	NED FO TO 35' -18.9 -22.1	35'-1" 18.2 18.2	FOLLO TO 40' -19.6 -22.9	WING 40'-1" 18.7 18.7	TO 45' -20.2 -23.5
COMPONENT MEAN ROOF ZONE 1 ZONE 2	8 CLA UP 1 16.7 16.7	ODING O 30' -18.0 -21.0	30'-1" 17.5 17.5 17.5	NED FO TO 35' -18.9 -22.1	35'-1" 18.2 18.2	FOLLO TO 40' -19.6 -22.9 -22.9	WING 40'-1" 18.7 18.7 18.7	TO 45' -20.2 -23.5

AIR LEAKAGE

Company, Inc\200128B Gaston II\200128B Gaston II.aed

N1102.4.1 Building thermal envelope. The building thermal envelope shall be durably sealed with an air barrier system to limit infiltration. The sealing methods between dissimilar materials shall allow for differential expansion and contraction. For all homes, where present, the following shall be caulked, gasketed, weather stripped or otherwise sealed with an air barrier material or solid material consistent with Appendix E-2.4 of this code:

material consistent with Appendix = 2-2 or this code:

1. Blocking and sealing floor/ceiling systems and under knee walls open to unconditioned or exterior space.

2. Capping and sealing shafts or chases, including flue shafts.

3. Capping and sealing soffit or dropped ceiling areas.

ROOF VENTILATION

SECTION R806

R805.1 Ventilation required. Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or snow. Ventilation openings shall have a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Ventilation openings having a least dimension larger than 1/4 inch (6.4 mm) shall be provided with corrosion-resistant wire cloth screening, hardware dolb, or similar material with openings having a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Openings in roof framing members shall conform to the requirements of Section R802.7.

PER CODE

requirements of Section R802.7.

R805.2 Minimum area. The total net free ventilating area shall not be less than 1/150 of the area of the space ventilated except that reduction of the total area to 1/300 is permitted provided that at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above the eave or comice vents with the balance of the required ventilation provided by eave or comice vents. As an alternative, the rectified cross-ventilation area may be reduced to 1/300 when a Class I or II vapor retarder is installed on the warm-in-winter side of the ceiling.

Exceptions:

1. Enclosed ettic/rafter spaces requiring less than 1 square foot (0.0929 m2) of ventilation may be vented with continuous soffit ventilation only.

2. Enclosed attic/rafter spaces over unconditioned space may be vented with

continuous soffit vent only. SOUARE FOOTAGE OF ROOF TO BE VENTED = 1558 SO.FT.

NET FREE CROSS VENTILATION NEEDED:

WITHOUT 50% TO 80% OF VENTING 3'-0" AROVE FAVE = 10.39 SO FT. WITH 50% TO 80% OF VENTING 3'-0' ABOVE EAVE; OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CELLING = 5.16 SQ.FT.

GUARD RAIL NOTES

SECTION R312

R312.1 Where required. Guards shall be located along open-sided walking surfaces, Including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or grade below at any point inches (162 mm) measured vertically to the notor or grade below at any poin within 36 inches (914 mm) horizontally to the edge of the open side. Insect screening shall not be considered as a guard. R312.2 Height. Required guards at open-sided walking surfaces, including

stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads.

Exceptions:

1. Guards on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the

stairs, the top of the guard shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

R312.3 Opening limitations. Required guands shall not have openings from the

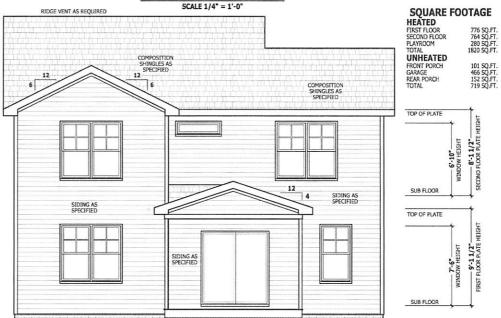
walking surface to the required guard height which allow passage of a sphere 4 inches (102 mm)in diameter.

1. The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153 mm) in diameter

2. Guards on the open sides of stairs shall not have openings which allow passage of a sphere 4 3/8 inches (111 mm) in diameter



FRONT ELEVATION



REAR ELEVATION

SCALE 1/4" = 1'-0"

CODES AND CONDITIONS MAY VARY WITH LOCATION. A LOCAL DESIGNER, ARCHITECT OR ENGINEER SHOULD BE CONSULTE BEFORE CONSULT BEFORE TO STRUCTION. THESE DRAWING ARE UNSTRUMENTS OF SERVICE AND

REAR ELEVATIONS GASTON H ంభ FRONT

| SQUARE FOOTAGE | HEATED | 175 SQ FT. | 1800 H.0.008 | 745 SQ FT. | 1800 H.0.008 | 745 SQ FT. | 1800 H.0.008 | 1800 SQ FT. | 1707A. | 1800 SQ FT. | 1800 H.0.008 | 1800 H.0.00

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STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code.

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no

liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		
Guardrail in-fill components	50		
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x105 PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strain lumber (LSL) Ph=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be

prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. boits at 2-4" on entire for spans up to 18-5" unless noced otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing. ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick. CONCRETE AND SOILS: See foundation notes.

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer falls to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics.

BEARING. All trusses shall be designed for bearing on SPF #2 plates or ledgers unless nated atherwise

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

EXTERIOR HEADERS

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16' KING STUD(S) 1 2 3 5 6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE LADDER FRAMED

BRACE WALL PANEL NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless

GYPSUM: All interior sides of exterior walls and both sides Interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

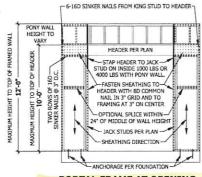
REQUIRED LENGTH OF BRACING: Required brace wall length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length. HD: 800 lbs hold down hold down device fastened to the edge

of the brace wall panel closets to the corner.

Methods Per Table R602.10.1

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d(2 1/2" long x 0.113" diameter). CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing

GB: Interior walls show as GB are to have minimum 1/2' gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws. PF: Portal fame per figure R602.10.1



PORTAL FRAME AT OPENING (METHOD PE PER EIGURE AND SECTION 8602 10.1.) SCALE 1/4" = 1'-0"

FIRST FLOOR STRUCTURAL

SCALE 1/4" = 1'-0"

-4 X 4 TREATED POST OR EQUIVALENT TYPICAL (2) 2 X 10 ATTACH RAFTERS TO HEADER WITH HURRICANE CONNECTORS (SIMPSON H2.5 OR EQUIVALENT). ATTACH HEADER TO POST AND POST TO BASE WITH **PORCH** POST CAP, METAL STRAPS, AND/OR POST BASE. (2) 50 ROOF TRUSSES FLOOR TRUSSES BY MANUFACTURER BY MANUFACTURER (3) SC DINING ROOM (2) 1.75" X 9.25" LVL (2) 1.75" X 14" LVL (2) 95 3 JACKS EACH END (2) 2 X 8 (3) SC **FAMILY ROOM** KITCHEN (3) SC (2) 1.75" X 14" LVL (3) 50 FOYER TRUSS GB GB (4) 2X6 SC ii (2) SC FLOOR TRUSSES BY MANUFACTURER PORCH **DOUBLE GARAGE** (3) 2 x 10 (3) SC (3) 1.75" X 24" LVL TOP FLUSH (8) SC 4 X 4 TREATED POST OR EQUIVALENT TYPICAL, ATTACH RAFTERS TO HEADER LINE OF FLOOR WITH HURRICANE CONNECTORS (SIMPSON H2.5 OR EQUIVALENT). ABOVE ATTACH HEADER TO POST AND POST TO BASE WITH POST CAP, METAL STRAPS, AND/OR POST BASE. (4) 2X6 SC (3) SC PF **PF** (3) SC (2) 1.75" X 11.875" LVL SIDE LOAD GARAGE WINDOWS W/ SIDE LOAD GARAGE

CODES AND CONDITIONS MAY VARY WITH LOCATION. A LOCAL DESIGNER, ARCHITECT OR NGINEER SHOULD BE CONSULTI BEFORE CONSULTI BEFORE CONSULTI THESE DRAWING ARE INSTRUMENTS OF SERVICE AN

STRUCTURAL GASTON FLOOR ш Ξ FIRST

SQUARE FOOTAGE 776 SQ 71 764 SQ 71 280 SQ 71 1820 SQ 71 UNHEATED

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All construction shall conform to the latest requirements of the 2018 North Carolina Residental Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code. 308 SITE PRACTICES AND SAFETY: Havnes Home Place

Jule 3112 PROFILES AND SOFTEN THE PARTY TO PROFILE AS A TO PROFILE AS A TO PROCEED AS A TO PROCEED AS A TO PROCEED AS A TO PROFILE AS A TO PROPOSED AS A TO PROFILE AS A TO PR braced in accordance with good construction practice and

are treasured concer.			
DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(IL)
Attics without storage	10	10	1/240
Attics with Emited storage	20	10	1/360
Attics with flood stairs	. 40	10	L/360
Balconies and decks	.40	10	1/360
Per escapes	\$	10	1,/360
Guardraits and handraits	200	-	-
Quardrall in-fill components	50	-	-
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	1/360
Sleeping rooms	30	10	1/360
States	40	-	L/360
Snow	20	-	

FRANCING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless cod other wit

BICDREES WOOD BEAVES:

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist Inuses and evolves increases, increases, and of use an expen-inguist shall be propered in accordance with this document. Trusses and 1-joists shall be installed according to the menufecture's specifications. Any change in truss or 1-joist layout shall be coordinated with Haynes Hones Plants, Inc. LIRITELS: Brick Inust shall be 3 1/2* x 3 1/2* x 1/4* state angle for up to 6"-0" span. 6" x 4" x 5/16" steel angle with 6" log vertical for spans up to 9"-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" stool angle with 1/2" bods at 2"-0" on center for spans up to 18"-0" unless noted otherwise. FLOOR SHEATHONG: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist specing, minimum 5/8" thick for 19.2" on center jobst spacing, and minimum 3/4" thick for 24" on centur joist specing. RDOF SHEATHING: OSB or CDX roof sheathing minimum

3/8" thick. CONCRETE AND SOILS: See foundation notes.

ATTIC ACCESS

SECTION R807

R807.1 Attic access. An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have a vortical height of 60 inches (1524 mm) or greater. The net clear opening shall not be less than 20 inches by 30 inches (508 ozar opering stea not be less than all inches by a more spoken mm by 752 mm) and shall be located in a halway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headrown in the attic spece shall be provided at some point above the access opening. See Section MI 1305.1.3 for access requirements where mobalished requirement is located for access requirements where mobalished requirement is located.

1. Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access. Pull down stair treads, stringers, handralls, and hardware may produce into the not clear opening.

EXTERIOR HEADERS

(2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE

NING STUDS EACH END PER TABLE RELOW HEADER SPAN < 3 | 3-4 | 4-8 | 8-12 | 12-16 XDNG STUD(S) 1 2 3 5 6

INTERIOR HEADERS

LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END URLESS NOTED OTHERWISE NON LOAD BEARING HEADERS TO BE

LADDED EDAMED

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordan with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. ICREE WALL AND CELLING REIGHTS. All finished knee wall heights and osting heights are shown furred down 10° from nod decking for insulation. If for any reason the truss manufacturer falls to meet or exceed designated heel heights, finished knee wall heights, or finished cetting heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any varietion due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required enchars for trusses due to uplift or bearing

shall meet the recuirements as specified on the burst schematics. BEARDING. All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights

WALL THICKNESSES

Exterior walls and walls adjacent to a garage area are drawn as 4° or as noted 2 X 6 are drawn as 6° to include 1/2" shosthing or gypsum. Subtract 1/2" for

are drawn as 5 1/2°, and do not include gypsum.

Interior walls are drawn as 3 1/2" or as noted 2 X 6

EXTERIOR WINDOWS AND DOORS

SECTION R612

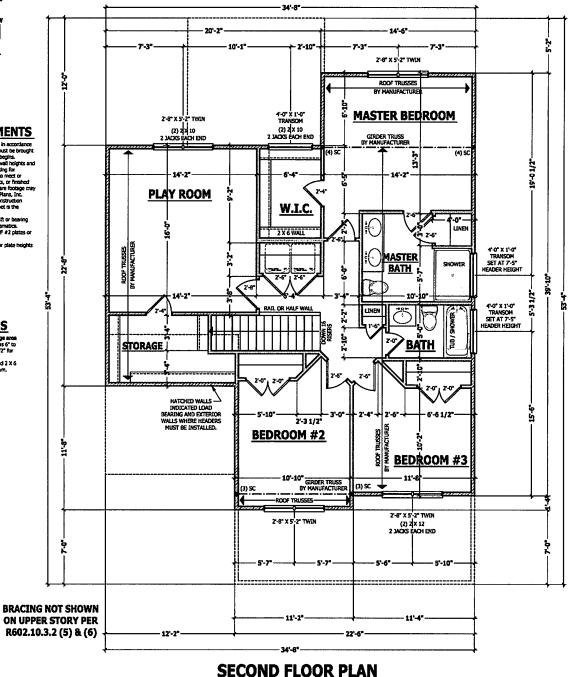
R\$12.1 General. This section prescribes performance and construction requirements for extentor windows and doors installed in walks. Windows and doors shall be installed and windows and door of the fenetization installations are the status installation installation. Window and door openings shall be flashed in accordance with Section R7013. Writton in the status installation in the status of the status in installation instructions shall be provided by the fenestration manufacturer for each window

R\$12.2 Window sills. In awaiting units, where the opening of an operable window is NOLLE Windows Mad. In including similar, which are beginning in an upcase without an located more than 72 inches (1829 mm) above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches (510 mm) above the finished floor of the room in which the window is located. Openable sections windows shall not permit openings that allow passage of a 4 inch (120 mm) disminion sphere. there such openings are located within 24 inches (610 mm) of the finished floor

1. Windows whose cornings will not allow a 4-inch diameter (102 mm) sphere to pass through the opening when the opening is in its largest opened position.

2. Openings that are provided with window fall prevention devices that comply with Section

3. Openings that are provided with fall prevention devices that comply with ASTM F 2090. Windows that are provided with opening limiting devices that comply with Section RI R612.3 Window fall prevention devices. Window fall prevention devices and window guards, where provided, shall comply with the requirements of ASTM F 2090.



HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR INTRACTORS PRACTICES AND PROCEDURES

THESE DRAWING AND ISTRUMENTS OF SERVICE AS SUCH SHALL ROMAIN PROPERTY OF THE DESIGN

PLAN GASTON SECOND FLOOR 开

SOUARE FOOTAGE PRIST PLOOR SECTING PLOOR RANTECON TOTAL UNIVERSITY PROST POPCH GARAGE REAL POPCH 12 SOT. 12 SOT. 12 SOT.

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PLACHASER MUST VOICTY AL EDMENSIONS AND CONDITION BUTURE CONSTRUCTION BOOK HAYNES HOME PLANS, DIC ACQUIRES HO LIABILITY PO DINTRACTORS PRACTICES A PROCEDURES.

II GASTON ROOF PLAN 里

SQUARE FOOTAGE
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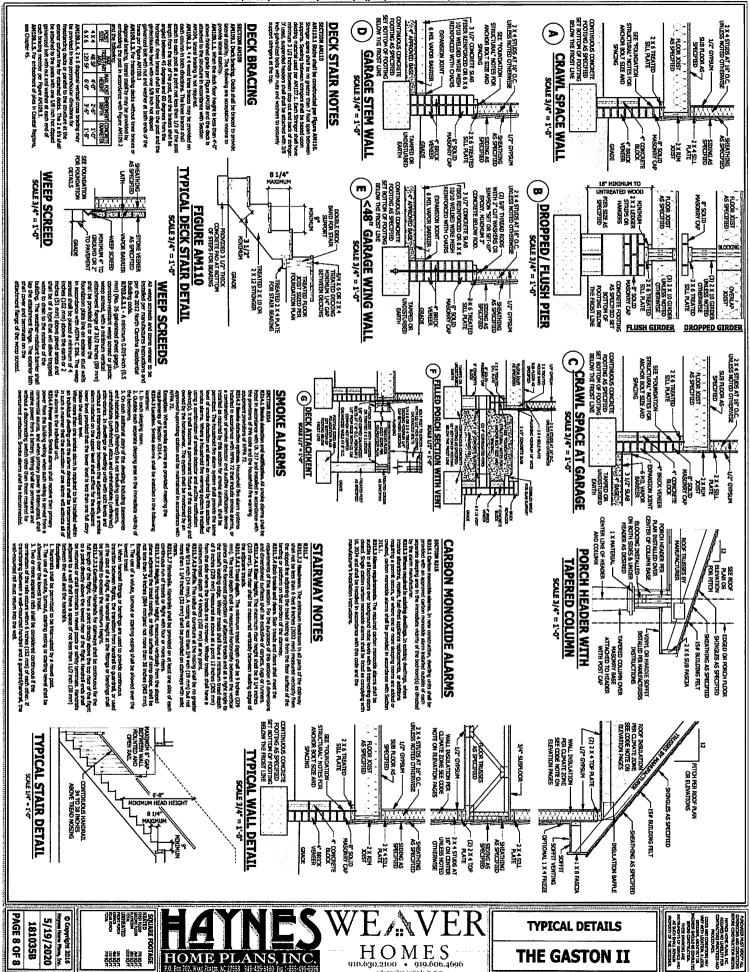
ledgers unless noted otherwise.

Plate Heights & Floor Systems. See eand floor system thicknesses.

HEEL HEIGHT ABOVE

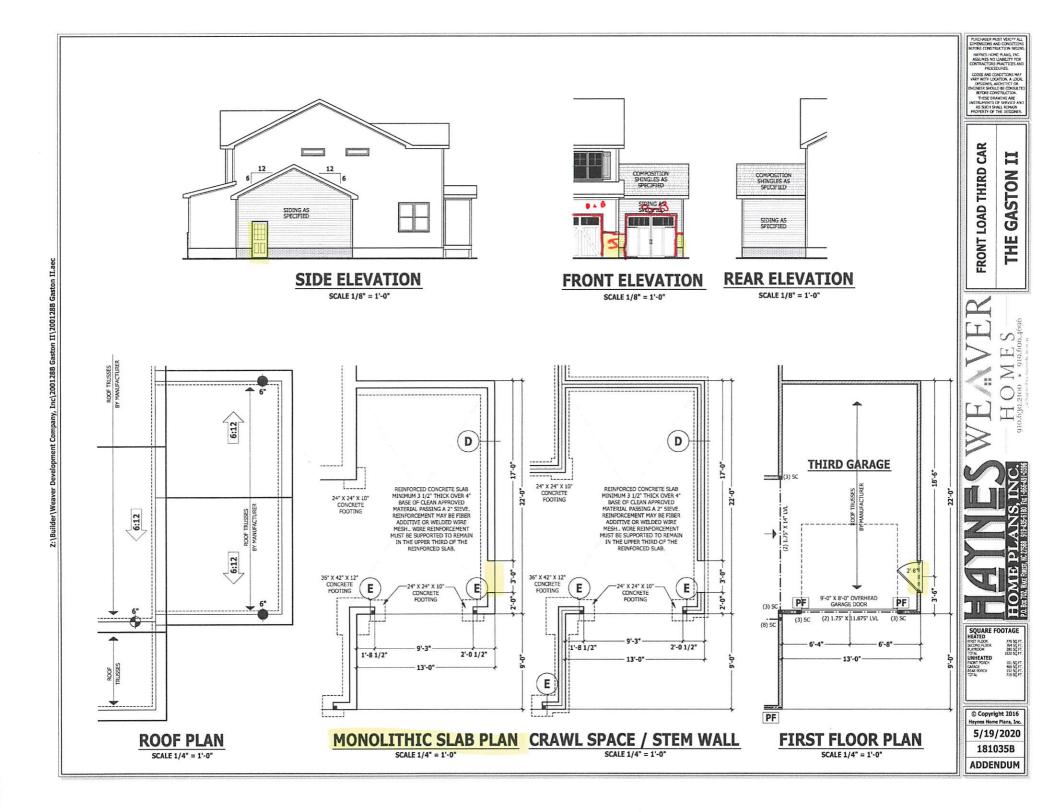
HEEL HEIGHT ABOVE SECOND PLOOR PLATE

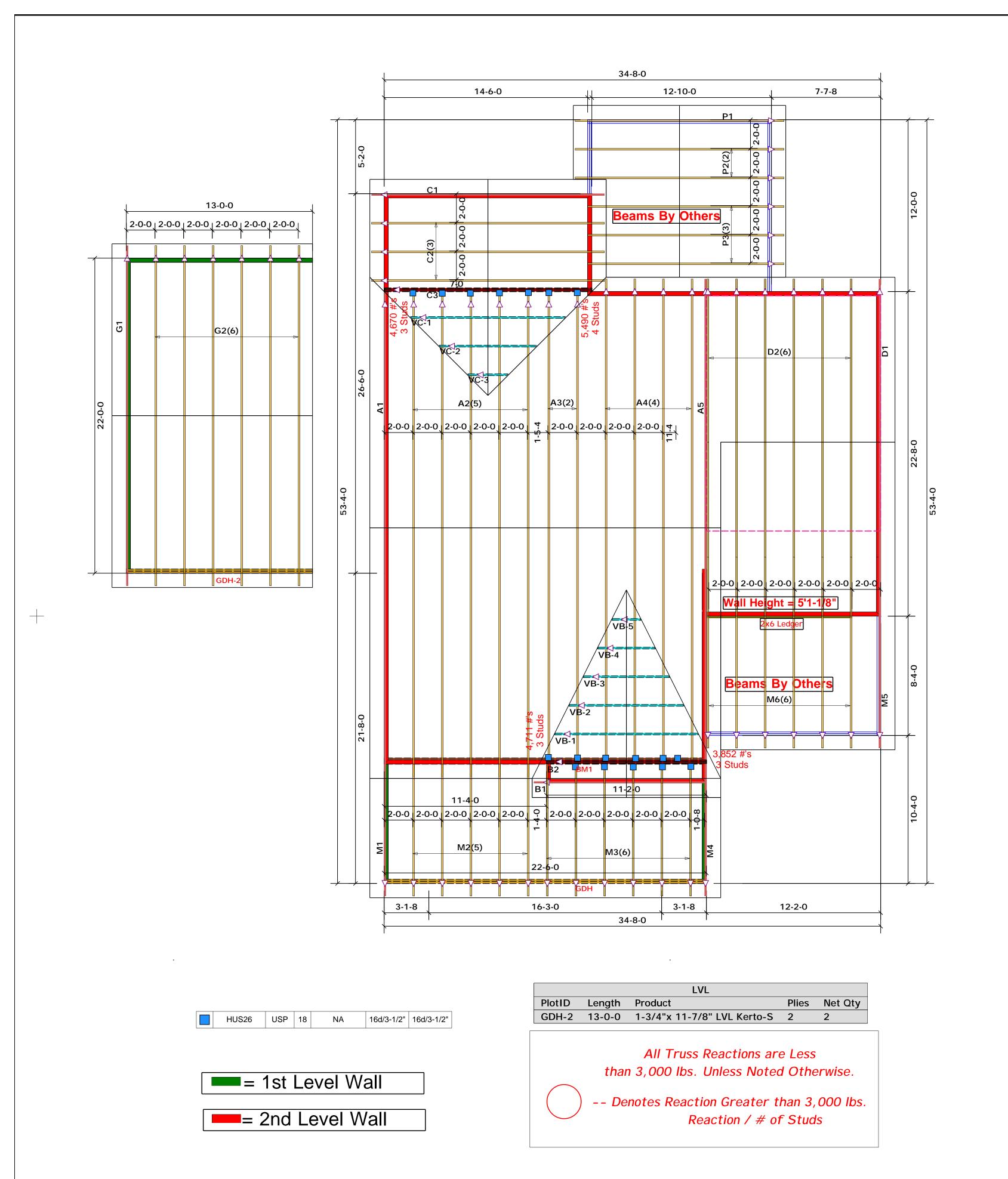
SCALE 1/4" = 1'-0"



THE GASTON II

HOME





Truss Placement Plan SCALE: 1/4"=1'

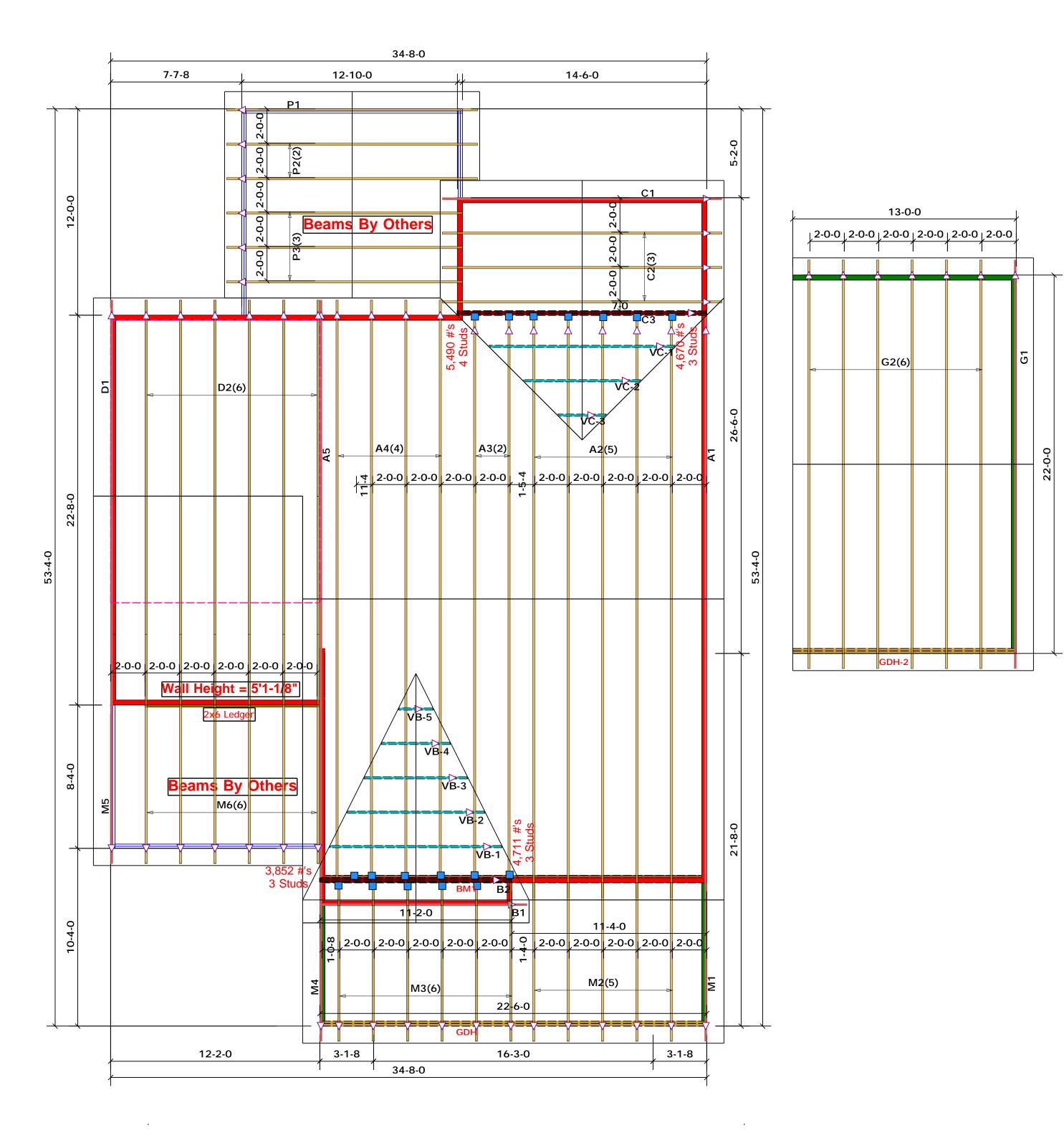
= Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

LOAD CHART FOR JACK STUDS **BUILDER COUNTY** Harnett Weaver Development Co. Inc. (045Eb ON 140LES 8502.5(1) & (6)) NUMBER OF JACK STUDG REQUIRED IN CALCUD OF FEADER/ROOF. Lot 2 Byrd Farm OF TO PERCONNECTORN (A) TO PER **JOB NAME ADDRESS** Lot 2 Byrd Farm PLAN Gaston II (181035B) w/ 3rd Car MODEL Roof 1700 1 3400 2 3400 2550 1 5100 2 6800 2 / / **SEAL DATE** N/A DATE REV. 5100 3 7650 3 10200 3 6800 4 8500 5 10200 6 11900 7 13600 8 10200 4 13600 4 12750 5 17000 5 QUOTE # DRAWN BY Marshall Naylor 15300 6 J1220-5850 **SALESMAN** Lenny Norris JOB # 15300 9

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

Marshall Naylor





HUS26 USP 18 NA 16d/3-1/2" 16d/3-1/2"

= 1st Level Wall
= 2nd Level Wall

LVL
PlotID Length Product Plies Net Qty
GDH-2 13-0-0 1-3/4"x 11-7/8" LVL Kerto-S 2 2

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.
Reaction / # of Studs

Truss Placement Plan SCALE: 1/4"=1'

= Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

	MANAGE OF TAKES SOURCE & CACAD OF PERCENTION OF THE PROPERTY OF		BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS These tru the buildir sheets for		
			JOB NAME	Lot 2 Byrd Farm	ADDRESS	Lot 2 Byrd Farm	is respons the overal walls, and regarding		
		및 등 기 및 등 및 PLAN	PLAN	Gaston II (181035B) w/ 3rd Car	MODEL	Roof	or online (
	1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 6600 10200	2	SEAL DATE	N/A	DATE REV.	/ /	(derived foundation than 3000 be retained
	10200 6	10200 4 12750 5 15300 6	13600 17000		QUOTE #		DRAWN BY	Marshall Naylor	specified retained t
	11900 7 13600 8 15300 o				JOB#	J1220-5850	SALESMAN	Lenny Norris	

HIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
hese trusses are designed as individual building components to be incorporated into
e building design at the specification of the building designer. See individual design
leets for each truss design identified on the placement drawing. The building designer
responsible for temporary and permanent bracing of the roof and floor system and for
e overall structure. The design of the truss support structure including headers, beams,
alls, and columns is the responsibility of the building designer. For general guidance
garding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package
online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature Marshall Naylor





Project: Address: Weaver Homes

Gaston II (181035B)

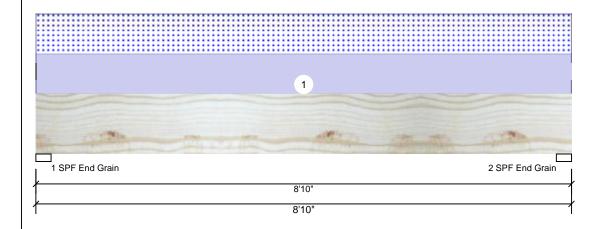
Date: 1/26/2021

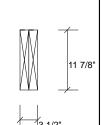
Input by: Marshall Naylor Job Name: Gaston II (181035B)

Project #:

1.750" X 11.875" GDH-2 **Kerto-S LVL** 2-Ply - PASSED

Level: Level





Page 1 of 1

WICHINGE ITTIOTTI	iation
Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Member Information

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)									
Brg	Live	Dead	Snow	Wind	Const				
1	0	1145	1104	0	0				
2	0	1145	1104	0	0				

Analysis Results Analysis Actual Comb. Case Location Allowed Capacity Moment 4554 ft-lb 4'5" 22897 ft-lb 0.199 (20%) D+S L Unbraced 4554 ft-lb 4'5" 10675 ft-lb 0.427 (43%) D+S L 1650 lb 1'2 1/8" 10197 lb 0.162 (16%) D+S Shear ī LL Defl inch 0.036 (L/2845) 4'5 1/16" 0.211 (L/480) 0.170 (17%) S TL Defl inch 0.073 (L/1397) 4'5 1/16" 0.282 (L/360) 0.260 (26%) D+S L

Bearings Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1145 / 1104 1 - SPF 3.000" 2249 L D+S End Grain 2 - SPF 3.000" 1145 / 1104 2249 L D+S End

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	250 PLF	0 PLF	250 PLF	0 PLF	0 PLF	G2
	Self Weight				9 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

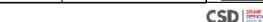
Grain

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS







Weaver Homes

Project:

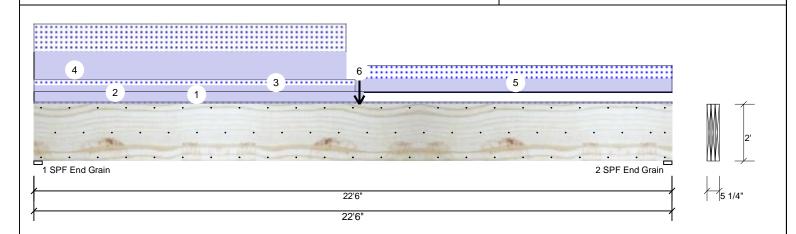
Address: Gaston II (181035B) Date: 1/26/2021

Input by: Marshall Naylor Job Name: Gaston II (181035B) Page 1 of 1

Project #:

Kerto-S LVL 3-Ply - PASSED 1.750" X 24.000" FB₂

evel: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Туре: Girder Application: Floor Brg Wind Const Live Dead Snow Plies: 3 Design Method: ASD 225 6536 5095 0 0 1 Moisture Condition: Dry **Building Code:** IBC 2012 225 0 2 4429 3676 0 Deflection LL: 480 Load Sharing: Yes Deflection TL: 360 Deck: Not Checked Importance: Normal Temperature: Temp <= 100°F Bearings

Bearing Length

1 - SPF 3.500"

2 - SPF 3.500"

End Grain

End Grain Cap. React D/L lb

6536 / 5095

4429 / 3676

Total Ld. Case

11631 L

8104 L

Ld. Comb. D+S

D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	65477 ft-lb	11'5 3/4"	131295 ft-lb	0.499 (50%)	D+S	L
Unbraced	65477 ft-lb	11'5 3/4"	65903 ft-lb	0.994 (99%)	D+S	L
Shear	10093 lb	2'2 5/8"	30912 lb	0.327 (33%)	D+S	L
LL Defl inch	0.226 (L/1171)	11'1 11/16"	0.552 (L/480)	0.410 (41%)	S	L
TL Defl inch	0.501 (L/528)	11' 7/8"	0.735 (L/360)	0.680 (68%)	D+S	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 3'11 5/8" o.c.

6 Bottom braced at bearings. 7 Lateral slenderness ratio based on single ply width.										
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Tie-In	0-0-0 to 22-6-0	0-6-0	Far Face	15 PSF	40 PSF	0 PSF	0 PSF	0 PSF	1' Floor
2	Part. Uniform	0-0-0 to 11-7-8		Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
3	Part. Uniform	0-0-0 to 11-4-0		Near Face	79 PLF	0 PLF	79 PLF	0 PLF	0 PLF	M2
4	Part. Uniform	0-0-0 to 11-0-0		Тор	341 PLF	0 PLF	341 PLF	0 PLF	0 PLF	A2
5	Part. Uniform	11-4-0 to 22-6-0		Near Face	164 PLF	0 PLF	164 PLF	0 PLF	0 PLF	M3
6	Point	11-5-12		Тор	2293 lb	0 lb	2293 lb	0 lb	0 lb	B2
	Self Weight				28 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

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This design is valid until 2/26/2023



Client: Project: Address:

Weaver Homes

Gaston II (181035B)

Date: 1/26/2021

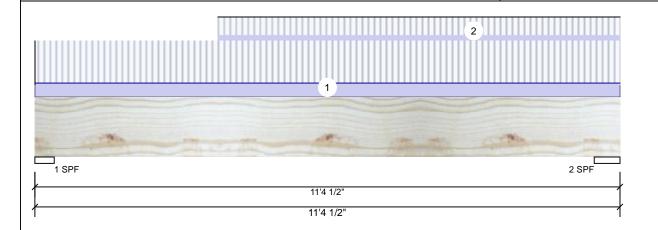
Marshall Naylor Job Name: Gaston II (181035B)

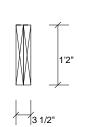
Project #:

Input by:

1.750" X 14.000" 2-Ply - PASSED **Kerto-S LVL** FB₁

Level: Level





Page 1 of 1

Member Infor	Member Information					Reactions UNPATTERNED Ib (Uplift)						
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const			
Plies:	2	Design Method:	ASD	1	2129	771	0	0	0			
Moisture Condition	n: Dry	Building Code:	IBC 2012	2	2523	904	0	0	0			
Deflection LL:	480	Load Sharing:	No									
Deflection TL:	360	Deck:	Not Checked									
Importance:	Normal											
Temperature:	Temp <= 100°F											
				Bearing	S							
				Bearing	Length	Cap. Re	eact D/L lb	Total Ld. Case	Ld. Comb.			
				1 - SPF	4.500"	43%	771 / 2129	2899 L	D+L			
				2 - SPF	6.000"	38%	904 / 2523	3426 L	D+L			

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8168 ft-lb	5'9 3/16"	26999 ft-lb	0.303 (30%)	D+L	L
Unbraced	8168 ft-lb	5'9 3/16"	10258 ft-lb	0.796 (80%)	D+L	L
Shear	2446 lb	9'9 1/4"	10453 lb	0.234 (23%)	D+L	L
LL Defl inch	0.090 (L/1419)	5'8 3/16"	0.266 (L/480)	0.340 (34%)	L	L
TL Defl inch	0.122 (L/1044)	5'8 3/16"	0.354 (L/360)	0.340 (34%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

					I						
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	106 PLF	318 PLF	0 PLF	0 PLF	0 PLF	F5	
2	Part. Uniform	3-6-8 to 11-4-8		Тор	44 PLF	132 PLF	0 PLF	0 PLF	0 PLF	F9	
	Self Weight				11 PLF						

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be out or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

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This design is valid until 2/26/2023



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Weaver Homes

Project: Address:

Gaston II (181035B)

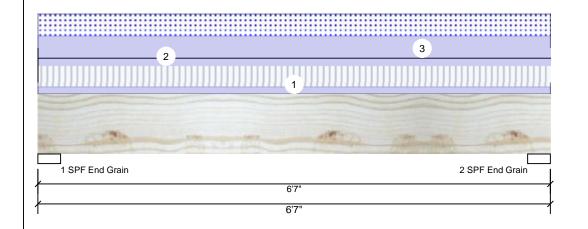
Date: 1/26/2021

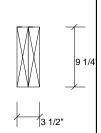
Input by: Marshall Naylor Job Name: Gaston II (181035B)

Project #:

Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED 6/0 SLIDER

Level: Level





Page 1 of 1

Member Inform	ation
Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F
I	

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked

Reacti	ons UNPAT	TERNED I	(Uplift)		
Brg	Live	Dead	Snow	Wind	Const
1	1060	1887	1113	0	0
2	1060	1887	1113	0	0
1					

Analysis Results Analysis Actual Comb. Case Location Allowed Capacity 3'3 1/2" 14423 ft-lb 0.347 (35%) D+0.75(L+S) L Moment 5009 ft-lb Unbraced 5009 ft-lb 3'3 1/2" 10451 ft-lb 0.479 (48%) D+0.75(L+S) L 2448 lb 1' 7943 lb 0.308 (31%) D+0.75(L+S) L Shear LL Defl inch 0.042 (L/1741) 3'3 1/2" 0.153 (L/480) 0.280 (28%) 0.75(L+S) L 3'3 1/2" 0.204 (L/360) 0.450 (45%) D+0.75(L+S) L TL Defl inch 0.091 (L/807)

Bearings

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1887 / 1629 1 - SPF 3.500" 3516 L D+0.75(L+S) End

Grain 2 - SPF 3.500" 1887 / 1629 3516 L D+0.75(L+S) End Grain

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 2. Top loads must be supported equally by all plice
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

3 Top loads must be supported equally by all piles.
4 Top braced at bearings.
C. Dottom broad at bearings

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	108 PLF	322 PLF	0 PLF	0 PLF	0 PLF	F4
2	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
3	Uniform			Тор	338 PLF	0 PLF	338 PLF	0 PLF	0 PLF	A4
	Self Weight				7 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals

Damaged Beams must not be used Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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This design is valid until 2/26/2023 CSD I

Manufacturer Info



Weaver Homes

Project:

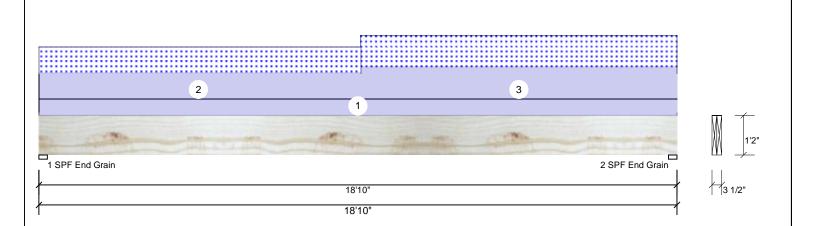
Address: Gaston II (181035B) Date: 1/26/2021

Input by: Marshall Naylor Job Name: Gaston II (181035B) Page 1 of 1

Project #:

Kerto-S LVL 1.750" X 14.000" Front GDH 2-Ply - PASSED Level: Level

Reactions UNPATTERNED Ib (Uplift)



Type:	Girder		Applicat	tion: FI	oor		Brg	Live	Dea	d Snow	Wir	nd	Const	
Plies:	2		Design	Method: A	SD		1	0	161	9 952		0	0	
Moisture Con	dition: Dry		Building	Code: IB	C 2012		2	0	172	0 1052		0	0	
Deflection LL:	: 480		Load Sh	naring: N	0									
Deflection TL	: 360		Deck:	N	ot Checked									
Importance:	Normal													
Temperature:	Temp <=	100°F												_
	•						Bearings	6						
							Bearing	Length	Cap.	React D/L lb	Total Lo	d. Case	Ld. Comb	
							1 - SPF	3.000"	28%	1619 / 952	2571 L		D+S	
							End							
Analysis Re	esults						Grain							
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2 - SPF	3.000"	30%	1720 / 1052	2772 L		D+S	
Moment	12090 ft-lb	9'8 7/8"	31049 ft-lb	0.389 (39%)) D+S	L	End Grain							
Unbraced	12090 ft-lb	9'8 7/8"	12111 ft-lb	0.998	D+S	L								_

TL Defl inch Design Notes

Shear

Member Information

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.

17'5 3/4" 12021 lb

- 3 Top loads must be supported equally by all plies.
- 4 Top must be laterally braced at a maximum of 8'7 7/8" o.c.
- 5 Bottom braced at bearings.

LL Defl inch 0.184 (L/1202)

2360 lb

0.491 (L/451)

6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	wall
2	Part. Uniform	0-0-0 to 9-6-0		Тор	96 PLF	0 PLF	96 PLF	0 PLF	0 PLF	M2
3	Part. Uniform	9-6-0 to 18-10-0		Тор	117 PLF	0 PLF	117 PLF	0 PLF	0 PLF	M3
	Self Weight				11 PLF					

ī.

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- Indicating & Installation

 I. VIL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 1. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

(100%)

9'6 3/16" 0.461 (L/480) 0.400 (40%) S

9'5 13/16" 0.615 (L/360) 0.800 (80%) D+S

0.196 (20%) D+S

6. For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

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This design is valid until 2/26/2023 CSD |



Project: Address:

Weaver Homes

Gaston II (181035B)

Date: 1/26/2021

Input by: Marshall Naylor Job Name: Gaston II (181035B)

Project #:

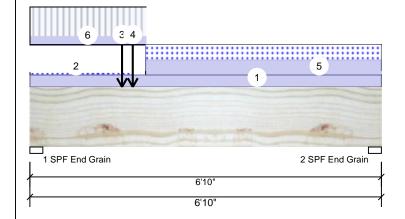
Bearings

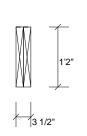
End

Grain

1.750" X 14.000" 2-Ply - PASSED Kerto-S LVL DB₁

Level: Level





Page 1 of 1

Member Inform	ation
Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked

Reaction	ons UNPAT	TERNED I	(Uplift)		
Brg	Live	Dead	Snow	Wind	Const
1	2861	3387	1990	0	0
2	873	1906	1168	0	0

Analysis Results Analysis Actual Location Allowed Comb. Case Capacity 11172 ft-lb 0.360 (36%) D+0.75(L+S) L Moment 2' 31049 ft-lb Unbraced 11172 ft-lb 2' 15735 ft-lb 0.710 (71%) D+0.75(L+S) L 6425 lb 1'4 1/4" 12021 lb 0.534 (53%) D+0.75(L+S) L Shear LL Defl inch 0.033 (L/2343) 2'7 5/8" 0.161 (L/480) 0.200 (20%) 0.75(L+S) 0.067 (L/1165) TL Defl inch 2'8 7/8" 0.215 (L/360) 0.310 (31%) D+0.75(L+S) L

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 3387 / 3638 7025 L 1 - SPF 3.000" D+0.75(L+S) End Grain 2 - SPF 3.000" 1906 / 1531 3437 L D+0.75(L+S)

- **Design Notes** 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details. 3 Top loads must be supported equally by all plies.

4 Top braced at bearings.	
5 Bottom braced at bearings.	
6 Lateral slenderness ratio based on single ply width.	

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
2	Tie-In	0-0-0 to 2-0-0	1-0-0	Тор	20 PSF	0 PSF	20 PSF	0 PSF	0 PSF	2' ROOF
3	Point	1-9-8		Тор	1040 lb	3115 lb	0 lb	0 lb	0 lb	F08
4	Point	2-0-0		Тор	2385 lb	0 lb	2385 lb	0 lb	0 lb	C3
5	Part. Uniform	2-3-0 to 6-10-0		Тор	160 PLF	0 PLF	160 PLF	0 PLF	0 PLF	C2
6	Part. Uniform	2-3-0 to 0-0-0		Тор	97 PLF	300 PLF	0 PLF	0 PLF	0 PLF	F07
	Self Weight				11 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

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Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS







Weaver Homes

Project:

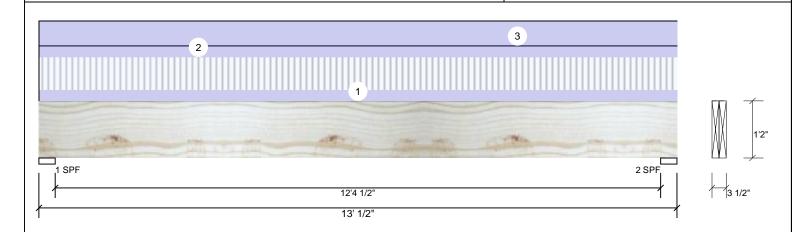
Address: Gaston II (181035B) Date: 1/26/2021

Input by: Marshall Naylor Job Name: Gaston II (181035B) Page 1 of 1

Project #:

1.750" X 14.000" 2-Ply - PASSED **Kerto-S LVL GCO**

Level: Level



Member Infor	rmation			Reaction	ons UNPAT	TERNED Ib	(Uplift)		
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	2374	3468	0	0	0
Moisture Condition	on: Dry	Building Code:	IBC 2012	2	2374	3468	0	0	0
Deflection LL:	480	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal								
Temperature:	Temp <= 100°F								
				Bearing	gs				
				Bearin	g Length	Cap. Read	t D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPI	4.000"	98% 346	8 / 2374	5842 L	D+L
				2 - SPI	4.000"	98% 346	8 / 2374	5842 L	D+L

Analysis Results

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Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	17498 ft-lb	6'6 1/4"	26999 ft-lb	0.648 (65%)	D+L	L
Unbraced	17498 ft-lb	6'6 1/4"	17570 ft-lb	0.996 (100%)	D+L	L
Shear	4554 lb	1'5 1/4"	10453 lb	0.436 (44%)	D+L	L
LL Defl inch	0.142 (L/1059)	6'6 1/4"	0.312 (L/480)	0.450 (45%)	L	L
TL Defl inch	0.349 (L/430)	6'6 1/4"	0.625 (L/240)	0.560 (56%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top must be laterally braced at a maximum of 5'6 3/4" o.c.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	122 PLF	364 PLF	0 PLF	0 PLF	0 PLF	F01
2	Uniform			Тор	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
3	Uniform			Тор	274 PLF	0 PLF	0 PLF	0 PLF	0 PLF	A1
	Self Weight				11 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Handling & Installation

 1. UVI beams must not be out or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

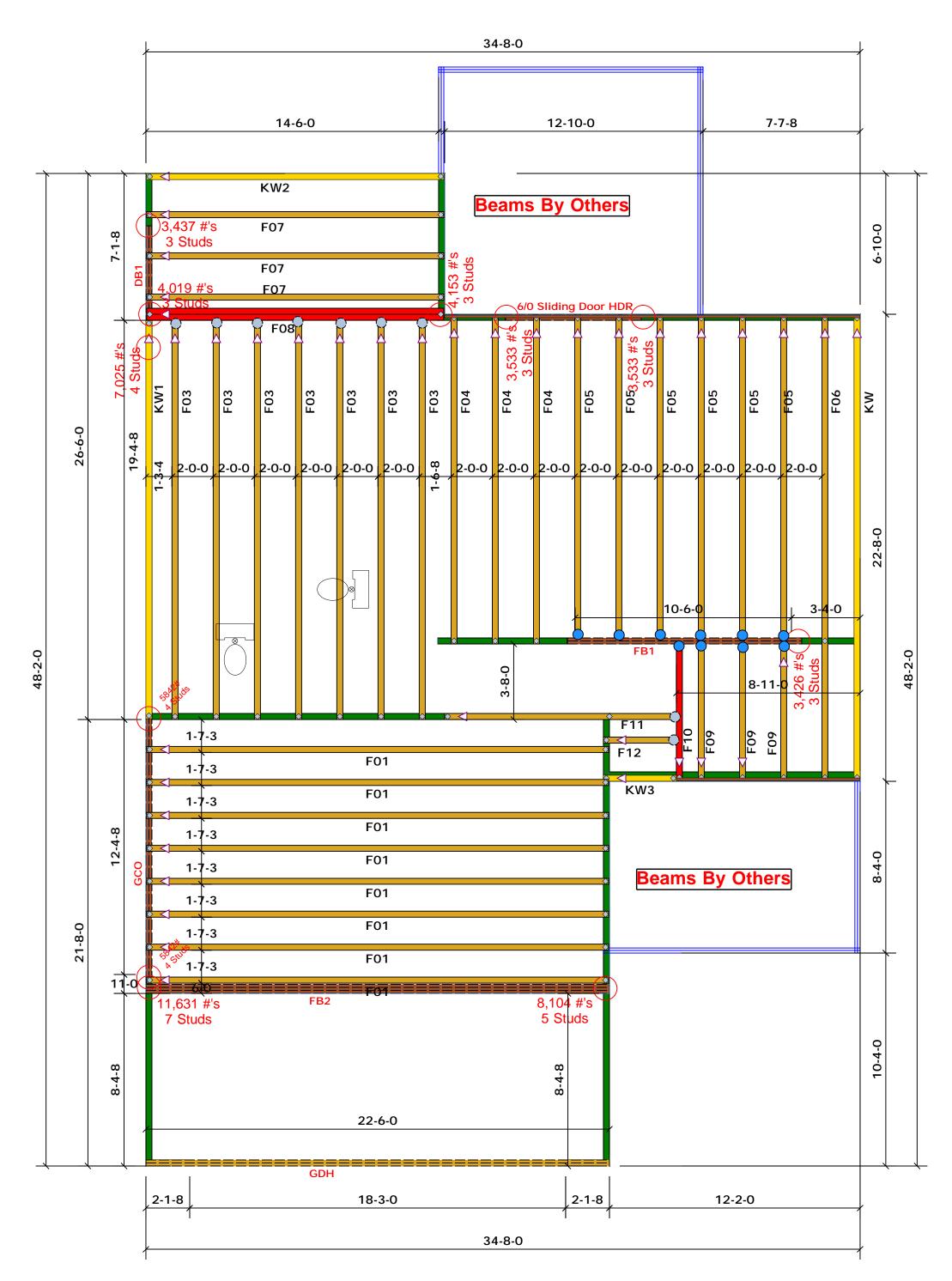
Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 2/26/2023





All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.
Reaction / # of Studs

HUS410	USP	10	NA	16d/3-1/2"	16d/3-1/2"
MSH422	USP	9	Varies	10d/3"	10d/3"

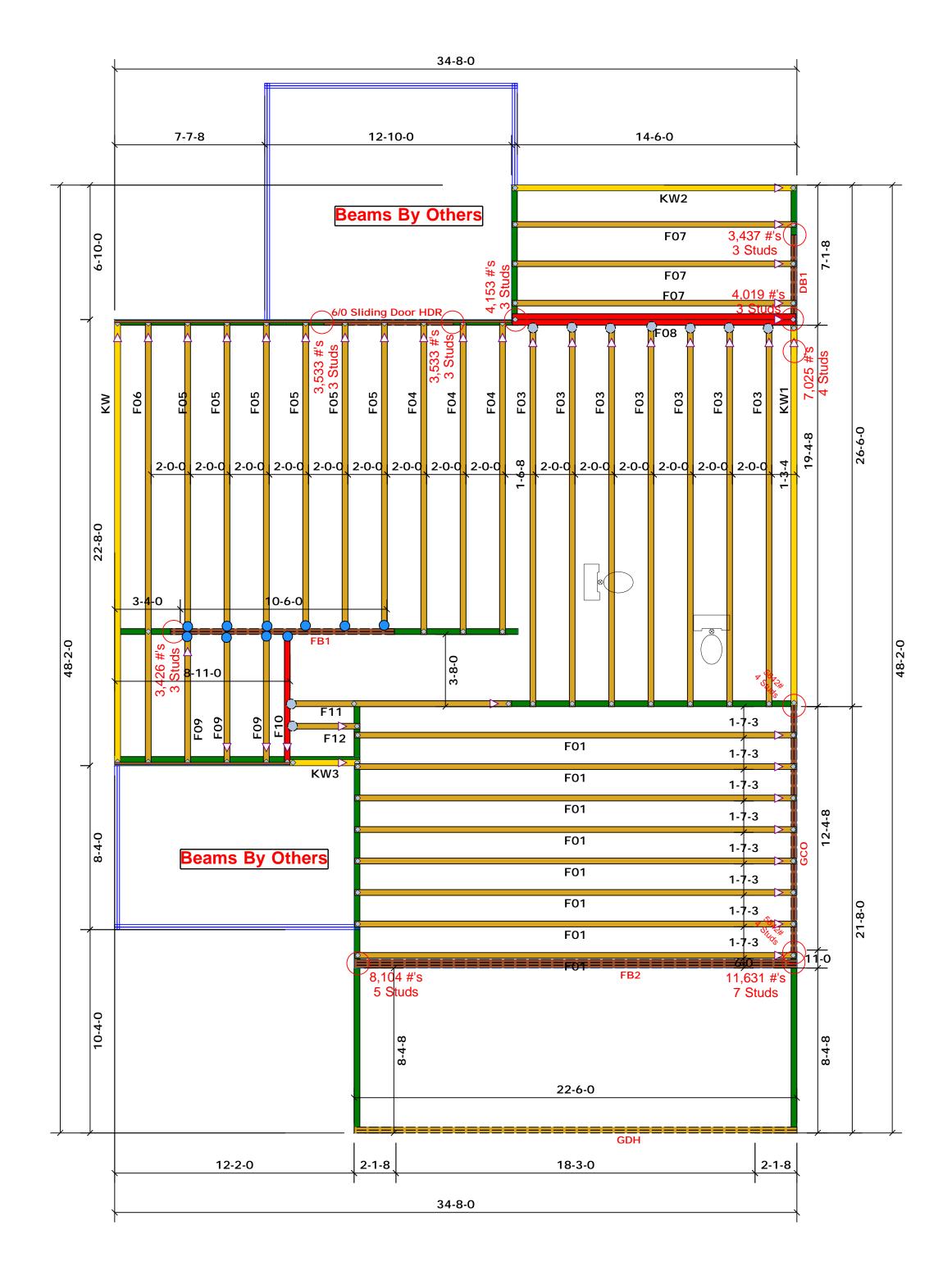
		Products		
PlotID	Length	Product	Plies	Net Qty
6/0 Sliding Door HDR	7-0-0	1-3/4"x 9-1/4" LVL Kerto-S	2	2
GDH	23-0-0	1-3/4"x 14" LVL Kerto-S	2	2
GCO	14-0-0	1-3/4"x 14" LVL Kerto-S	2	2
FB1	12-0-0	1-3/4"x 14" LVL Kerto-S	2	2
DB1	7-0-0	1-3/4"x 14" LVL Kerto-S	2	2
FB2	23-0-0	1-3/4"x 23-7/8" LVL Kerto-S	3	3

Truss Placement Plan SCALE: 1/4"=1'

= Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

							_
LOAD CHART FOR JACK STUDS (MANF) ON TABLES \$502.5() \$ (b)) SLANGE OF JACK STUDS \$(0) (MCD) \$(4.450.0)		502 5(1) 4 (6))	BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer
NO FE	FEADERVEIR Ž & Z Š	064 8	JOB NAME	Lot 2 Byrd Farm	ADDRESS	Lot 2 Byrd Farm	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package
GND REAC C *C)	#60 pm	Special Control of the	PLAN	Gaston II (181035B) w/3rd Car	MODEL	Floor	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables
1700 3400 5100	2 5100 a 3 7650 3	1 3400 1 2 6600 2 3 10200 3	SEAL DATE	N/A	DATE REV.	/ /	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those
6800 8500 10200	5 12750 5 6 15300 6	4 13600 4 5 17000 5	QUOTE #	Quote #	DRAWN BY	Marshall Naylor	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.
11900 7 13600 8 15300 9	8		JOB #	J1220-5851	SALESMAN	Lenny Norris	Marshall Naylor





All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

	HUS410	USP	10	NA	16d/3-1/2"	16d/3-1/2"
	MSH422	USP	9	Varies	10d/3"	10d/3"

		Products		
PlotID	Length	Product	Plies	Net Qty
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DB1	7-0-0	1-3/4"x 14" LVL Kerto-S	2	2
FB2	23-0-0	1-3/4"x 23-7/8" LVL Kerto-S	3	3

Truss Placement Plan SCALE: 1/4"=1'

= Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

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Ī	LOAD CHART FOR JACK STUDS (0.45% ON FAILS (2005)) 3 (6) STANDS OF JACK STUDS (C. COD OF	BUILDER	Weaver Development Co. Inc.	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer
	#EADERVETROES Z	JOB NAME	Lot 2 Byrd Farm	ADDRESS	Lot 2 Byrd Farm	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package
	SA CONTRACTOR OF THE PROPERTY	PLAN	Gaston II (181035B) w/3rd Car	MODEL	Floor	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables
	1700 1 2550 1 3400 1 3400 2 5100 2 6600 2 5100 3 7650 3 10200 3 6800 4 10200 4 13600 4	SEAL DATE	N/A	DATE REV.	/ /	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those
	8500 5 12750 5 17000 5 10200 6 15300 6	QUOTE #	Quote #	DRAWN BY	Marshall Naylor	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.
	11900 7 13600 8 15300 9	JOB #	J1220-5851	SALESMAN	Lenny Norris	Signature Marshall Naylor

соттесн **ROOF & FLOOR TRUSSES & BEAMS**