

H SQUARED.

DATE

IMUZOZO

I STORY

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H. SOUMER HOTE DEBAG.

R. SOURCE HOTE TO SHE TO SHE



H SQUARED HOME DESIGN, INC.

HEATHER or JOHNATHAN HALL 165 HEATHERSTONE CT BENSON NC 27504 (919) 207-1403

FIRST FLOOR FRONT PORCH WOOD DECK GARAGE

= 1257 = 80 = 120 = 400

#1257

13'-I' ASSUMED MEAN ROOF HGT

THE WALLER II

JRT MANG. PROP.

2.02F0F0 I STORY 0707/11/11 DATE THIS PLAN IS TO ONLY BE BUILT BY THE ABOVE BY ANOT FOR HULT PLE BOUNTER, AND TO SHAPP BUILT BUILDE BY A SOURKED BY A SOURK THE PLAN HAS BEEN DEATH TO THE PLAN STATE RESIDENTAL THE HORTH HOR MC.5 INVESTIGATE

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MC.3 INVESTI H SQUARED HOME DESIGN, INC. HEATHER OF JOHNATHAN HALL 185 HEATHERSTONE CT BENSON NC 27504 (919) 207-1403 FRST FLOOR FRONT PORCH WOOD DECK GARAGE 1257 120 120 12 5 JRT THE WALLER MANG. PROP. П

2CYFE 1/4. = 1-0. FOUNDATION PLAN CRAWL SPACE

MITHIN 3'-O' OF EVERY CRNR. * WITH VAPOR BARRIOR

EDITION NC RES. CODES FOR DAMP PROOFING (
MATER PROOFING REFER TO
SECTION 405 I 404 IN 2018

1257/150 = 8.4 50. FT. REO'D 8.4/.68 = 10 VENTS FND VENTS

REFER TO BASIC DETAILS, BRACING, POR STANDARD DETAILS, BRACING, DETAILS, AND STRUCTURAL NOTES DAMP PROOFING .8-,07 .01-,01 13.-8. .2-.71 30,-0. **JENT** CONC. FTG. KENT KERL CONC. FTG. YEN VI-1 .8-.71 1-10 Ks. PER DETAIL SEE SD SHEET CONC. FTG. ZY XZY XIO. 3XIO E1 C. COL .7-.81 .4-.41 2XIO E1 (B) FOR LIT REGNIKEHENS)

HOENEES (REOT'S) SEE REOT'S)

HOENEES (ON RIMERES) ON RIM

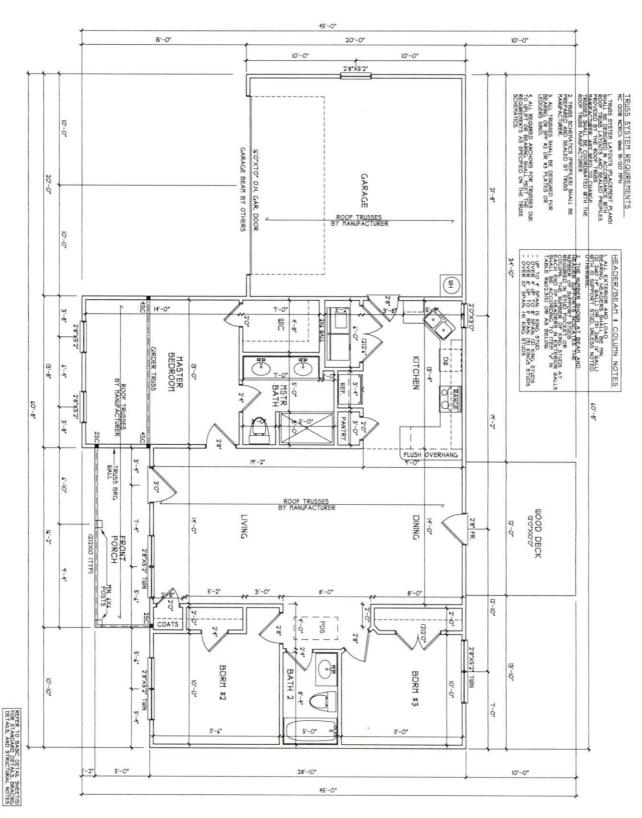
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HOENEES (REOT'S) SEE REOT'S)

HOENEES (REOT'S)

HOE (Y) ETG. LIK 33.X3t. -NENI VENT _YENT_ TO (B) (B) TAT SCLESSER (צ) ושוופאלש דאר סא דפר פואמפא (4) (4)3XIO SYP#2 OR SPP#2 GIRDER S FOUNDATION MALL AND FOOTING CONCRETE FTG. (TYP), BRACE POSTS PER APPENDIX M. 2) CONCRETE FOOTING, UND D (GYT) GNAB T CIRDER, TYPICAL UNO. FOUNDATION LEGEND (SEE SD SHEET FOR FULL FOUNDATION STRUCTURAL NOTES) 13,-10. 15,-0. *8-.OF 30.-0. .8-,07



FIRST FLOOR PLAN

1 STORY 010120.2	DATE:	THIS PLAN IS TO ONLY BE BUILT BY THE ABOVE CITED BUILDER OR HOHEOWINER, NOT FOR MILTIPLE BUILDS UNLESS APPROVED BY H SQUARED.	THE PLAN HAS BEEN DRAWN IN ACCORDANCE WITH HORTH CAROLINA STATE RESIDENTIAL BULLDING CODES 20W EDITION.	ANY DEVIATION OF THE SPECIFIED HEASUREHENTS OR DIFFENSIONS VOIDS H SQUARED HOME DESIGN, NC.'S LIABILITY.	J	H SQUARE HOME DESIGN, IN	
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ŀ	DESIGNED BY:
ı	HEATHER or
ı	JOHNATHAN HALL
ı	165 HEATHERSTONE CT
	BENSON NC 27504
ı	(919) 207-1403

WALL TOO THUS	
FIRST FLOOR	= 1257
FRONT PORCH	= 80
WOOD DECK	= 120
GARAGE	= 400

EATED FOOTAGE:				
#1257	THE WALLER II			
#1257		MANG.	PROP.	



3. ALL TRIBSES SIVALL BE DESIGNED FOR BLANKS ON BEFAVES ON EDICATED AND THATES ON EDICATE OF THATES ON EDICATE OF THATES OF THATES DESIGNED ANCIONS OF TRIBSES DIE TO URLIFT OF REVOKEINST AS SPECIFIED ON THE TRIBS SCIENTING. 2. TRUSS SCHEMATICS (PROFILES) SHALL BE PREPARED AND SEALED BY TRUSS HANUFACTURER. NC (2018 NCRC): Winds 115-120 HPH L TRUSS SYSTEM LAYOUTS (PLACEMENT PLAKS)
ROWS TRUSS SYSTEM AND COMMAKE SITH
ROWS TRUSS SYSTEM AND SEASON PROPULES
ROWS TRUSS TRUSS TRUSS TRUSS SYSTEM
ROWS TRUSS SYSTEM ECONOMATED SITH THE
ROOF TRUSS TAME ACTURER
ROOF TRUSS TAME ACTURER
ROOF TRUSS TAME ACTURER ROOF TRUSSES BY MANUFACTURER GIRDER TRUSS BY MANUFACTURER TRUSS SUPPORT WALL BELOW. ROOF TRUSSES BY MANUFACTURER

REFER TO BASIC DETAL SHEET(S)
FOR STANDARD DETALS, BRACHES,
DETALS, AND STRUCTURAL HOTES
ROOF PLAN
SCALE I/4" = T-O"

010120.2	II/II/2020	ONLY BE BUILT BY THE ABOVE CITED BUILDER OR HOMEOWIRE. NOT FOR MILTIPLE BUILDS WILESS APPROVED BY H SQUARED.	THE PLAN HAS BEEN DRAIN IN ACCORDANCE WITH HORTH CAROLINA STATE RESIDENTIAL BULLDING CODES SOW EDITION.	ANY DEVATION OF THE SPECIFIC PENANTS VOICE OF DESIGNAL VOICE DESIGN, NC.'S LIABILITY.	J	H SQUARED HOME DESIGN, INC.
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DESIGNED BY:
HEATHER or
JOHNATHAN HALL
165 HEATHERSTONE CT
BENSON NC 27504
(919) 207-1403

١	SQUARE FOOTAGE:
	FIRST FLOOR
1	FRONT PORCH
ı	WOOD DECK
١	GARAGE

	HEATED FOOTAGE:
= 1257 = 80 = 120 = 400	#1257

THE	WALLER	II
	RAGE LEFT	11
GA		

JRT MANG. PROP.

2)	DESIGN LOADS	(R3O1.4)	LIVE LOAD	DEAD LOAD	DEFLECTION

.,	DESIGN LOADS (RSOLA)	(PSF)	(PSF)	(LL)
	ROOMS OTHER THAN SLEEPING RO	OMS 40	10	L/340
	SLEEPING ROOMS	30	10	L/340
	ATTIC WITH PERMANENT STAIR	40	10	L/340
	ATTIC WITH OUT PERMANENT STAFF	20	10	L/340
	ATTIC WITH OUT STORAGE	10	10	L/240
	STAIRS	40		1/340
	EXTERIOR BALCONIES	60	Ю	L/340
	DECKS	40	10	L/340
	GUARDRAILS AND HANDRAILS	200		-
	PASSENGER VEHICLE GARAGES	50	10	L/340
	FIRE ESCAPES	50 40 20	10	L/340
	SNOW	20	-	

UIND LOAD (BASED ON IIS/120 MPH WIND VELOCITY (EXPOSURE B)

3) WALL BRACING: BRACED WALL PANELS SHALL BE CONSTRUCTED ACCORDING TO BALL BRACING: BRACED WALL PANELS SHALL BE CONSTRUCTED ACCORDING I SECTION ROZOLOS.

THE ANOINT AND LOCATION OF BRACING SHALL COMPLY WITH TABLE ROZOLO. THE LENGTH OF BRACED PANELS SHALL BE DETERMINED BY SECTION ROZOLO. LATERAL BRACING SHALL BE SATISHED PER HETHOD 3 BY CONTINUOUSLY SHEATHING WALLS WITH STRUCTURAL SHEATHING PER SECTION ROZOLO. NOTE THAT ANY SPECIFIC BRACED WALL DETAIL SHALL BE INSTALLED AS SPECIFIED

NOTE THAT ART SPECIFIC BRAZEU WALL DEFAIL SHALL BE INSTALLED AS SIN CONCRETE SHALL HAVE A HINDIN 28 DAYS STERNISH TO SOOD PS AND A MAXIMIT SLUTP OF 5 INCHES UNLESS NOTED OTHERWISE (INO), ARE SHITRANDED PER TABLE 4022, ALL CONCRETE SHALL BE PROPORTIONED, INDED, HANDLED SAMPLED, TESTED, AND PLACED IN ACCORDANCE WITH ACI STANDARDS, ALL SHAPLES FOR PUMPING SHALL BE TAKEN FROM THE EXIT END OF THE PUMP.

SARPLES FOR FURTHING SHALL BE TRACE FROM THE EXIT END UT HE PUTH.

3 ALLOHABLE SOIL BEARIN, PRESSURE ASSURED TO BE 2000 PSF. THE CONTRACTOR
HIST CONTACT A GEOTECHNICAL ENGINEER AND THE STRUCTUAL ENGINEER IF
HISTORY SUBSURFACE CONDITIONS ARE ENCOUNTERED, THE SURFACE AREA
ADJACENT TO THE FOUNDATION BIALL SHALL BE PROVIDED BITH ADEQUATE DRAINAGE,
AND SHALL BE GRADED SO AS TO DRAINSPRACE BATER MAY FROM FOUNDATION BIALLS.

ALL BOODEN BEAMS AND HEADERS SHALL HAVE THE FOLLOWING END SUPPORTS: (I) 224 STUD COLUMN FOR ϵ -0° MAX. BEAM SPAN (BNO), (2) 2X4 STUDS FOR BEAM SPAN GREATER THAN ϵ -0° (BNO).

SI LVL. SHALL BE LAMNATED VENEER LUMBER: Fb-2400 PS, Fv-265 PS, E-1-NO I P.S.L. SHALL BE LAMNATED VENEER LUMBER: Fb-2700 PS, Fv-700 PS, E-2-0-NO LSL. SHALL BE PARALLEL STRAND LUMBER: Fb-220 PS, Fv-700 PS, E-1-SHAU NSTALL ALL CONNECTIONS PER MARIFACTURERS INSTRUCTIONS.

NSTALL ALL CONNECTIONS PER HAMP/ACTURERS INSTRUCTIONS.

9 ALL ROOP TRUISS AND LOSSI LAYOUTS SHALL BE PERFARED IN ACCORDANCE BITH ANY SEALED STRUCTURAL DRABNOS. TRUISSES AND I-JOISTS SHALL BE NSTALLED ACCORDANC TO THE HAMP/ACTURES SPECIFICATIONS. ANY CHANGE IN TRUISS OR I-JOIST LAYOUT SHALL BE COORDINATED BITH DESIGNER OR ENGINEER.

9 ALL STRUCTURAL STEEL SHALL BE ASTIN 3-45. STEEL BEATS SHALL BE SUPPORTED AT EACH BIOL BITH A PRINTIP BEASING LENGTH OF JUT DISCUSSION OF THE STATE OF THE SHALL BE ASTIN ACCENT OF SHALL BITH TO LOS SCREEN (JZ DANTETER x 1 LONG. LATERAL SUPPORT BITH TO LAG SCREEN (JZ DANTETER x 1 LONG. LATERAL SUPPORT BITH TO LAG SCREEN (JZ DANTETER X 1 LONG. LATERAL SUPPORT BITH TO LAG SCREEN (JZ DANTETER X 1 LONG. LATERAL SUPPORT BITH THE SULP PLATE. AND SOLE PLATE IS MAILED OR BOLTED TO THE BEAT HANCE * 48" O.C. .

ALL STEEL TERMING SHALL BE ASTIN AECONE REASED.

D REBAR SHALL BE DEFORMED STEEL, ASTMAS, GRADE 40.

(2) FLITCH BEAMS SHALL BE BOLTED TOGETHER USING (2) ROUS OF 1/2" DIAMETER BOLTS (ASTM ASOT) WITH WASHERS PLACED UNDER THE THREADED END OF BOLT. BOLTS SHALL BE SPACED AT 2" O.C. (MAX). AND STAGGERED AT THE TOP AND BOTTOM OF BEAM (2" EDGE DISTANCE), WITH 2 BOLTS LOCATED AT ("FROM EACH END.

13) BRICK LINTELS SHALL BE 3 1/2"x3 1/2"x/4" STEEL ANGLE FOR UP TO SPAN AND SATS THELL ANGLE WITH & LEG VERTICAL FOR SPANS UP TO 1'-0' (UNO).

14) THE POSITIVE AND NEGATIVE DESIGN PRESSURE FOR DOORS AND WINDOWS FOR A MEAN ROOF HEIGHT OF 35 FEET OR LESS SHALL BE 25 PSF.

B) THE POSITIVE AND NEGATIVE DESIGN PRESSURES REQUIRED FOR ANY ROOF OR BALL CLADDING APPLICATION NOT SPECIFICALLY ADDRESSED IN THE NORTH CAROLINA STATE RESIDENTIAL CODE - 208 EDITION SHALL BE AS FOLLOWS. ROOF.

45.4 PSF - 2.25:12 PITCH OR LESS 34.8 PSF - 2.25:12 TO 1:12 PITCH 21 PSF - 1:12 TO 12:12 PITCH

WALLS: 24J PSF - WALLS

HEADER/BEAM & COLUMN NOTES

I. ALL EXTERIOR AND LOAD BEARING HEADERS SHALL BE MIN. (2) 240 (4" WALL) OR (3) 240 (6" WALL) WITH (1) SUPPORT STUD, UNLESS NOTED OTHERWISE.

2. THE NUMBER SHOWN AT BEATH AND HEADER SUPPORTS INDICATES THE REQUIRED IN STUD POCKET OR COLUMN. THE NUMBER OF KING STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS SHALL BE ACCORDING TO ITEM "O" IN TABLE RIOZJSIS OR AS BELOW.

UP TO 4' SPAN: (I) KING STUD OVER 4' UP TO 8' SPAN: (2) KING STUDS OVER 8' UP TO II' SPAN: (3) KINGS STUDS OVER II' SPAN: (4) KING STUDS

FOUNDATION STRUCTURAL NOTES: NC (2018 NCRC): Wmd: 15-120 HPH

(1) (3) 250 SYP #2 OR SPF#2 GIRDER, TYPICAL UNO.

(2) CONCRETE BLOCK PIER SIZE SHALL BE-HOLLOW MASONRY
UP TO 32" HIGH
UP TO 48" HIGH
UP TO 4" HIGH SOLID MASONRY
UP TO 5'-0' HIGH
UP TO 4'-0' HIGH
UP TO 12'-0' HIGH 16 x 16 UP TO 14" HIGH UP TO 12"-0" H 24 x 24 UP TO 14" HIGH WITH 30" x 30" x 10" CONCRETE FOOTING, UNO.

(3) WALL FOOTING AS FOLLOWS

DEPTH: 8" - UP TO 2-1/2 STORY 10" - 3 STORY WIDTH: SIDING IOR EGUAL)
- 'k' - 'UP TO 2-1/2 STORY
- 20' - 3 STORY
BRICK VENEER
- 'k' - 1 STORY
- 20' - 2 STORY
- 20' - 2 STORY
- 21' - 3 STORY

FOR FOUNDATION BALL HEIGHT AND BACKFILL REGURERENTS, REFER TO MORTH CAROLINA RESDERTIAL CODE TABLE REGULATION TO THE ASSUMEND SOIL BEARBING CAPACITY * 2000 PSF. CONTRACTOR THAT VERYEY SITE CONDITIONS AND CONTACT SOIL SENGMEET IF TARGINAL OR UNSTABLE SOILS ARE ENCONTREND.

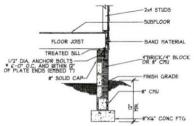
(4) (4) 240 SYP#2 OR SPF#2 GIRDER

(\$) (2) L15X4.25 LVL OR LSL GIRDER

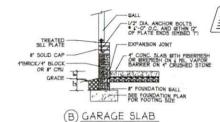
(I) (3) LTSX4.25 LVL OR LSL GIRDER

1. "M" DESIGNATES A SIGNIFICANT POINT LOAD TO HAVE SOLID BLOCKING TO PIER. SOLID BLOCK ALL BEAT BEARING POINTS NOTED TO HAVE THREE OR HORE STIDS TO FID, TYPICAL

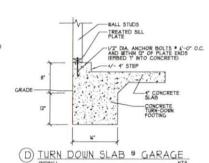
A ABBREVIATIONS "SJ" = SINGLE JOIST
"DJ" = DOUBLE JOIST
"TJ" = TRIPLE JOIST



(A) CRAWL SECTION

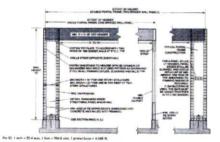


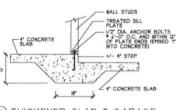
- 2-4 STUDS * K* OC 2x1 TREATED SILL PLATE 1/2" DIA. (1" EMBEDMENT) ANCHOR BOLTS * 6'-0" OC (ADJUST LOCATION AT DOOR OPENINGS.)
LOCATE 12" FROM PLATE ENDS CPADE 4" CONCRETE SLAB W/ 12* C TURN DOWN SLAB FOOTING



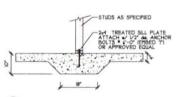
PRANTIC STUCK WILL B 12" O.C. AT STUDE UND MANNAN FOOTING BIZE LINDER OFFINANCIE 12'112" A TERRIDO GOIN GLAS BYALL SE PERMITTED AT BOOK OPENINGS. HERMOGENIG SHALL

PIGURE RICE TO 3.2

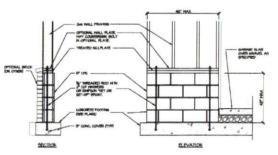




(E) THICKENED SLAB & GARAGE



(F) TYPICAL THICKENED SLAB



GARAGE WING WALL' REINFORCING PER INC FIGURE RE022/04/3

BUILDING SHEET DETAIL BASIC

TOT APPLY NOTE ALS A *PLEASE ALL DET EVERY

CT 370NE C 27504 1403 HEATHER HA BENSON NC 2 (919) 207-14 185

H SQUARED HOME DESIGN, INC.

CCORDANCE WTH NORTH CLORDANCE WITH NORTH COUNT STATE REDUCENTAL ANY DEVATOR OF THE SPECIAL DEASEMENTS OF DEPRESON YOUNGS H SOLMED HOTE DESCA. HAS LAST TO THE PARK SEEN DOAD IN A COORDINATE HIT WORLD CAROLING OF STATE RESIDENT SALLING COORD SOF ETTING.

DATE

FILE





Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

Roof\Dropped Beams\GDH(i19) (Dropped Beam)

Dry | 1 span | No cant.

August 5, 2020 15:26:20

Build 7493

Job name:

Address:

City, State, Zip:

Customer: Code reports: ESR-1040

BC CALC® Member Report

File name:

2000612A.mmdl

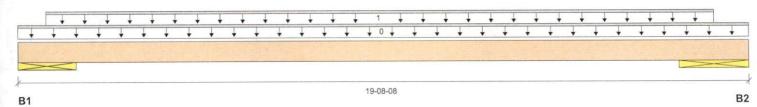
Description:

Roof\Dropped Beams\GDH(i19)

Specifier:

Designer:

Company:



Total Horizontal Product Length = 19-08-08

Reaction Summary (Down / Uplift) (lbs)

riouotion ou.	\					
Bearing	Live	Dead	Snow	Wind	Roof Live	
B1, 19"		1987 / 0		613 / 1512	1982 / 0	
B2, 22-1/2"		2007 / 0		615 / 1662	1994 / 0	

Loa	ad Summary						Live	Dead	Snow	Wind	Roof Live	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	19-08-08	Тор		12				00-00-00
1	Smoothed Load	Unf. Lin. (lb/ft)	L	00-09-04	18-09-04	Top		208			221	n\a

Controls Summary	Value	% Allowable	Duration	Case	Location
Pos. Moment	14743 ft-lbs	56.9%	125%	1	09-09-04
End Shear	3122 lbs	31.6%	125%	1	16-10-02
Total Load Deflection	L/273 (0.719")	87.8%	n\a	1	09-09-04
Live Load Deflection	L/546 (0.36")	65.9%	n\a	98	09-09-04
Max Defl.	0.719"	71.9%	n\a	1	09-09-04
Span / Depth	16.5				

Bear	ing Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Wall/Plate	19" x 3-1/2"	3968 lbs	14.0%	8.0%	Spruce-Pine-Fir
B2	Wall/Plate	22-1/2" x 3-1/2"	4001 lbs	12.0%	6.8%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Design meets arbitrary (0.75") Maximum live load deflection criteria.

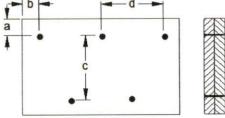
Calculations assume unbraced length of Top: 01-10-08, Bottom: 01-10-08.

BC CALC® analysis is based on IBC 2012.

Wind loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Connection Diagram: Full Length of Member











August 5, 2020 15:26:20

Roof\Dropped Beams\GDH(i19) (Dropped Beam)

BC CALC® Member Report

Build 7493 Job name:

Customer:

Address: City, State, Zip:

Code reports:

ESR-1040

Dry | 1 span | No cant.

File name:

2000612A.mmdl

Roof\Dropped Beams\GDH(i19) Description:

Specifier:

Designer:

Company:

Connection Diagram: Full Length of Member

a minimum = 2"

c = 7-7/8"

b minimum = 3"

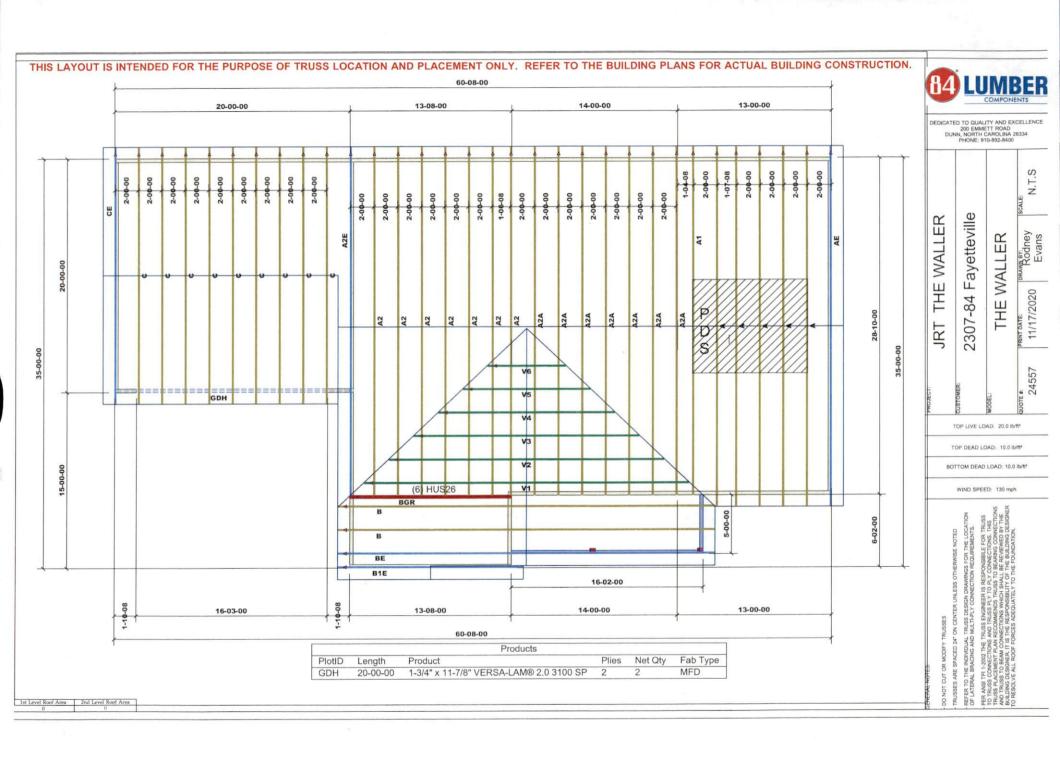
d = 24"

Connectors are: 3-1/4 in. Pneumatic Gun Nails

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™. ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





Trenco

818 Soundside Rd Edenton, NC 27932

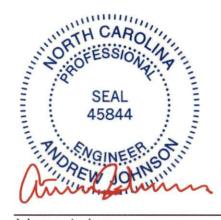
Re: 24557-24557A JRT THE WALLER

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I43653195 thru I43653212

My license renewal date for the state of North Carolina is December 31, 2020.

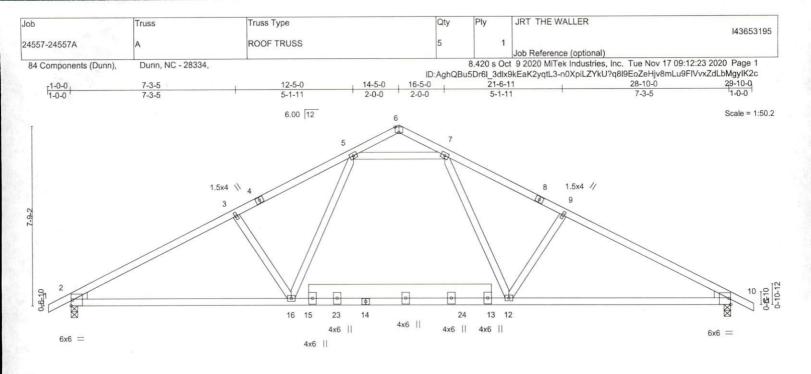
North Carolina COA: C-0844



November 17,2020

Johnson, Andrew

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



9-7-14 9-7-14					19-2-2 9-6-4					9-7-14				
Plate Offs	sets (X,Y)	[6:0-2-0,Edge]		,										
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP		
TCLL	20.0	Plate Grip DOL	1.15	TC	0.62	Vert(LL)	-0.25 1	2-22	>999	240	MT20	197/144		
TCDL	10.0	Lumber DOL	1.15	BC	0.84	Vert(CT)	-0.40 1	2-22	>857	180				
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.47	Horz(CT)	0.05	10	n/a	n/a				
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-MS						Weight: 160 lb	FT = 20%		

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except*

13-15: 2x8 SP No.2

WEBS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. (size)

(size) 2=0-3-8, 10=0-3-8

Max Horz 2=-129(LC 13)

Max Uplift 2=-156(LC 12), 10=-156(LC 13) Max Grav 2=1213(LC 1), 10=1213(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1947/494, 3-5=-1720/496, 7-9=-1720/496, 9-10=-1947/494

BOT CHORD 2-16=-323/1654, 12-16=-159/1261, 10-12=-325/1654

WEBS 7-12=-96/587, 9-12=-420/247, 5-16=-96/587, 3-16=-420/247, 5-7=-1209/456

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 3x4 MT20 unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
- 7) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE:



Structural wood sheathing directly applied or 3-5-15 oc purlins.

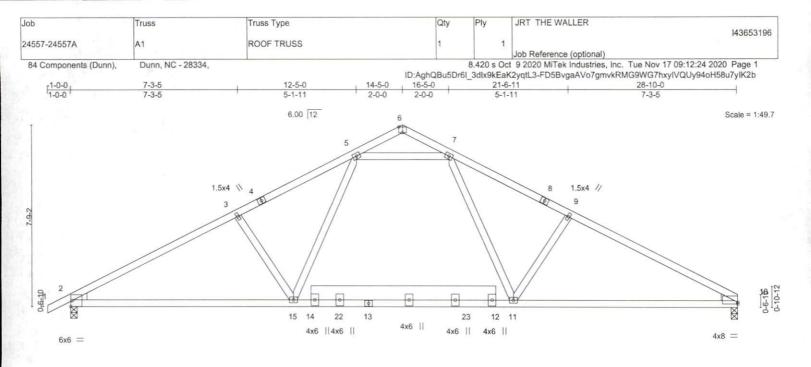
Rigid ceiling directly applied or 10-0-0 oc bracing.

ENCINEEDING BY

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSUTPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



9-7-14						19-2-2		7	28-10-0				
		9-7-14			9-6-4		,	-	9-7-14				
Plate Offs	sets (X,Y)	[6:0-2-0,Edge], [10:0-0-0	,0-0-13]										
OADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc) I/defl	L/d	PLATES	GRIP		
CLL	20.0	Plate Grip DOL	1.15	TC	0.63	Vert(LL)	-0.25 11-21	>999	240	MT20	197/144		
CDL	10.0	Lumber DOL	1.15	BC	0.85	Vert(CT)	-0.41 11-21	>841	180				
CLL	0.0 *	Rep Stress Incr	YES	WB	0.47	Horz(CT)	0.05 10	n/a	n/a				
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-MS					Weight: 158 lb	FT = 20%		

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2

2x4 SP No.2 or 2x4 SPF No.2 *Except* **BOT CHORD**

12-14: 2x8 SP No.2

WEBS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 10=0-3-8

Max Horz 2=137(LC 16)

Max Uplift 2=-156(LC 12), 10=-135(LC 13)

Max Grav 2=1214(LC 1), 10=1152(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

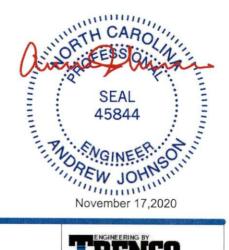
TOP CHORD 2-3=-1949/496, 3-5=-1722/498, 7-9=-1727/500, 9-10=-1954/498

BOT CHORD 2-15=-346/1656, 11-15=-180/1264, 10-11=-348/1661

WEBS 7-11=-99/593, 9-11=-425/249, 5-15=-95/587, 3-15=-420/247, 5-7=-1210/457

NOTES-

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) All plates are 3x4 MT20 unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces
- 7) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



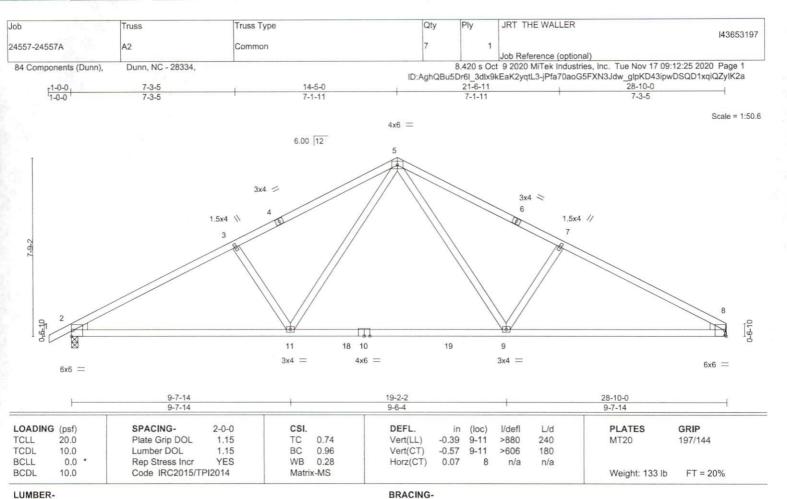
Structural wood sheathing directly applied or 3-5-2 oc purlins.

Rigid ceiling directly applied or 9-9-6 oc bracing.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 **BOT CHORD**

2x4 SP No.3 WEBS

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 8=Mechanical

Max Horz 2=137(LC 16)

Max Uplift 2=-156(LC 12), 8=-135(LC 13) Max Grav 2=1214(LC 1), 8=1152(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

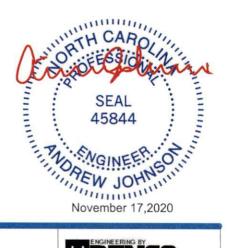
TOP CHORD 2-3=-1970/504, 3-5=-1737/513, 5-7=-1742/515, 7-8=-1974/506

BOT CHORD 2-11=-358/1679, 9-11=-130/1122, 8-9=-360/1684

WEBS 5-9=-145/667, 7-9=-425/282, 5-11=-142/662, 3-11=-422/280

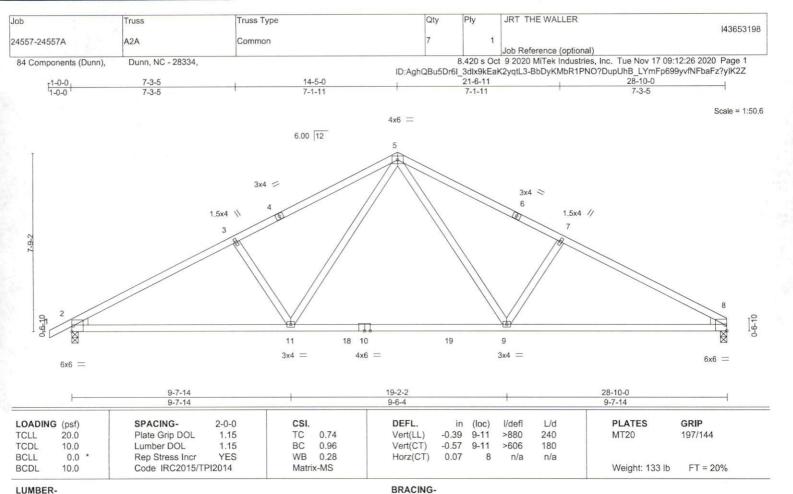
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=135
- 7) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 2-2-0 oc bracing.



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

WEBS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=137(LC 16)

Max Uplift 2=-156(LC 12), 8=-135(LC 13) Max Grav 2=1214(LC 1), 8=1152(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1970/504, 3-5=-1737/513, 5-7=-1742/515, 7-8=-1974/506

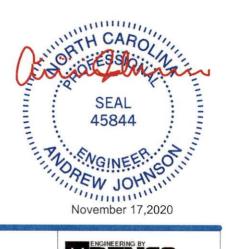
BOT CHORD 2-11=-358/1679, 9-11=-130/1122, 8-9=-360/1684

5-9=-145/667, 7-9=-425/282, 5-11=-142/662, 3-11=-422/280 **WEBS**

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 2-2-0 oc bracing

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Ply JRT THE WALLER Job Qty Truss Truss Type 143653199 24557-24557A A2E Common Supported Gable Job Reference (optional) 84 Components (Dunn), Dunn, NC - 28334 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:28 2020 Page 1 ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-8_Lil2dhZ0d6EX1Cb6ESQzrlov3tQrHgjv3M1uyIK2X 1-0-0 28-10-0 14-5-0 14-5-0

Scale = 1:50.7

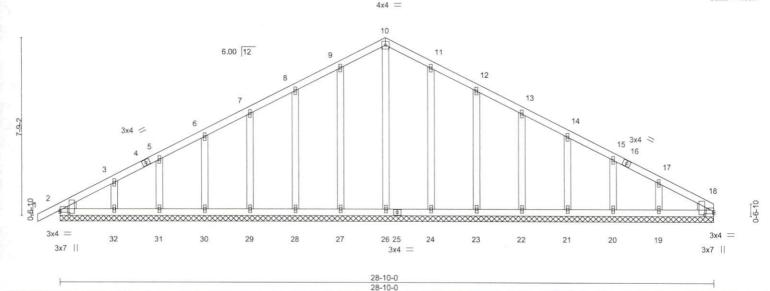


Plate Offsets (X,Y)-[2:0-0-0,0-1-1], [2:0-1-11,0-4-13], [18:Edge,0-1-1], [18:0-1-11,0-4-13] LOADING (psf) SPACING-CSI DEFL in I/defl **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.07 Vert(LL) -0.00 n/r 120 MT20 197/144 TCDL 10.0 Lumber DOL 1.15 BC 0.04 Vert(CT) 0.00 n/r 90 BCLL 0.0 * Rep Stress Incr YES WB 0.14 Horz(CT) 0.01 18 n/a n/a BCDL Code IRC2015/TPI2014 10.0 Matrix-S Weight: 171 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 **BOT CHORD** 2x4 SP No.2 or 2x4 SPF No.2

2x4 SP No.3 OTHERS

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. All bearings 28-10-0.

(lb) - Max Horz 2=133(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 27, 28, 29, 30, 31, 32, 24, 23, 22, 21, 20, 19

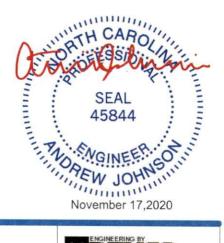
Max Grav All reactions 250 lb or less at joint(s) 2, 26, 27, 28, 29, 30, 31, 32, 24, 23, 22, 21, 20, 19, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE

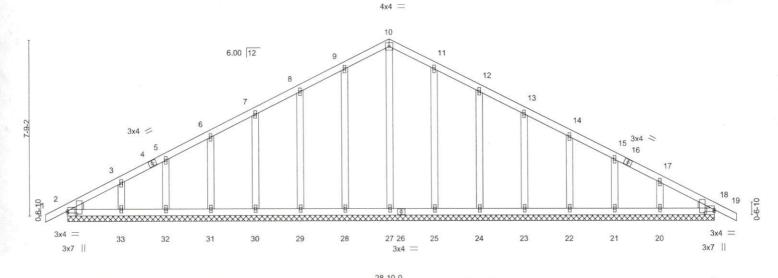
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



JRT THE WALLER Qty Ply Truss Type Job Truss 143653200 AE Common Supported Gable 1 24557-24557A Job Reference (optional) 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:30 2020 Page 1 Dunn, NC - 28334, 84 Components (Dunn), ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-4MSTAkex5etqUqBajXGwWOx5ljlOulkzADYT6mylK2V 28-10-0 1-0-0 14-5-0

Scale = 1:51.3



[2:0-0-0,0-1-1], [2:0-1-11,0-4-13], [18:0-0-0,0-1-1], [18:0-1-11,0-4-13] Plate Offsets (X,Y)-DEFL (loc) **PLATES** GRIP LOADING (psf) SPACINGin I/def 120 197/144 20.0 Plate Grip DOL 1.15 TC 0.07 Vert(LL) -0.00 18 n/r MT20 TCLL TCDL 10.0 Lumber DOL 1.15 BC 0.04 Vert(CT) -0.00 18 n/r 90 Rep Stress Incr WB 0.01 YES 0.14 18 n/a BCLL 0.0 Horz(CT) n/a Code IRC2015/TPI2014 FT = 20% BCDL 10.0 Matrix-S Weight: 173 lb

BRACING-

TOP CHORD

BOT CHORD

28-10-0

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 **BOT CHORD**

2x4 SP No.3 **OTHERS**

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. All bearings 28-10-0. (lb) -Max Horz 2=129(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 2, 28, 29, 30, 31, 32, 33, 25, 24, 23, 22, 21, 20

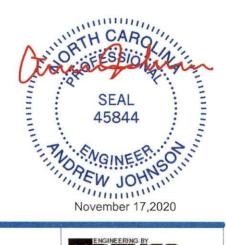
14-5-0

Max Grav All reactions 250 lb or less at joint(s) 2, 27, 28, 29, 30, 31, 32, 33, 25, 24, 23, 22, 21, 20, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) n/a
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 18.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

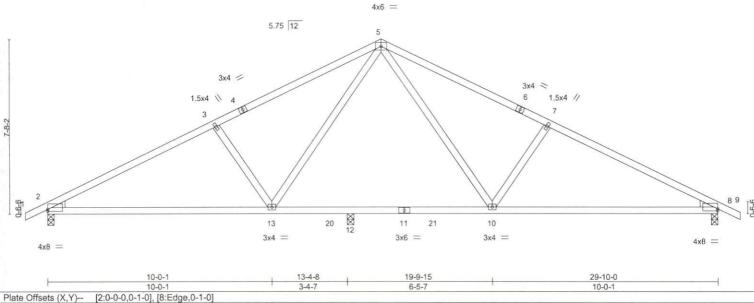
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Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932

JRT THE WALLER Truss Type Qty Job Truss 143653201 24557-24557A Common Job Reference (optional) 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:32 2020 Page 1 84 Components (Dunn) Dunn. NC - 28334. ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-0laDbQgBdF8Yj8LzqyIObp0H1XCQMdIFeX1aAfyIK2T 29-10-0 30-10-0 1-0-0 14-11-0 7-6-9 1-0-0

Scale = 1:51.2



DEFL. LOADING (psf) SPACING-2-0-0 CSI (loc) I/defl **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.75 Vert(LL) -0.22 13-16 >730 240 MT20 197/144 TCDL 10.0 Lumber DOL 1.15 BC 0.90 Vert(CT) -0.49 13-16 >332 180 YES WB 0.27 BCLL 0.0 Rep Stress Incr Horz(CT) 0.06 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Weight: 138 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 **BOT CHORD** 2x4 SP No.2 or 2x4 SPF No.2

WEBS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 8=0-3-8, 12=0-3-8

Max Horz 2=-127(LC 13) Max Uplift 2=-169(LC 12), 8=-169(LC 13)

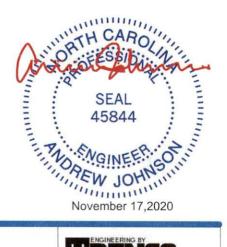
Max Grav 2=1181(LC 1), 8=1193(LC 1), 12=252(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1929/535, 3-5=-1678/532, 5-7=-1711/532, 7-8=-1962/534 2-13=-367/1662, 12-13=-131/1110, 10-12=-131/1110, 8-10=-371/1691 **BOT CHORD** 5-10=-143/643, 7-10=-452/287, 5-13=-143/599, 3-13=-455/287 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=169, 8=169.



Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 9-5-14 oc bracing.

▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



JRT THE WALLER Truss Type Qty Job Truss 143653202 24557-24557A B1E Common Supported Gable ob Reference (optional) 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:33 2020 Page 1 Dunn, NC - 28334, 84 Components (Dunn) ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-Ux8bolhqOYGPLlw9Ofpd70ZcRwmt579PsBm7i5yIK2S 6-10-0 13-8-0 14-8-0 6-10-0 1-0-0 1-0-0 6-10-0 Scale = 1:28.2 4x4 =

5 5.75 12 6 3 13 12 11 3x7 || 3x4 = 3x4 =3x7 ||

13-8-0 [2:0-0-0,0-1-0], [2:0-1-7,0-5-8], [8:Edge,0-1-0], [8:0-1-7,0-5-8] Plate Offsets (X,Y)--CSI. DEFL. **PLATES** GRIP LOADING (psf) SPACING-(loc) I/def 197/144 TCLL 20.0 Plate Grip DOL 1.15 TC 0.07 Vert(LL) 0.00 8 n/r 120 MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.06 Vert(CT) 0.00 9 n/r 90 BCLL Rep Stress Incr YES WB 0.04 Horz(CT) 0.00 8 0.0 n/a n/a Code IRC2015/TPI2014 FT = 20%BCDL 10.0 Matrix-S Weight: 64 lb

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

2x4 SP No.3 **OTHERS**

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. All bearings 13-8-0. (lb) -

Max Horz 2=-63(LC 17)

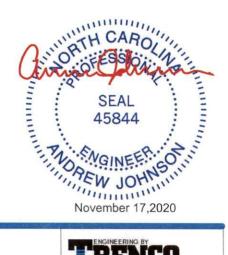
Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 14, 11, 10

Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) n/a
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 8.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collepse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI1 Qu Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



JRT THE WALLER Job Truss Type Qty Ply Truss 143653203 24557-24557A BE GABLE Job Reference (optional) 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:34 2020 Page 1 Dunn, NC - 28334. 84 Components (Dunn), ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-y8iz?5hS9sOGzSVMyNLsgE5dXKuuqWoY5rWgFYyIK2R 30-10-0 29-10-0 14-11-0 1-0-0 1-0-0 7-6-9 1-0-0

Scale = 1:51.2

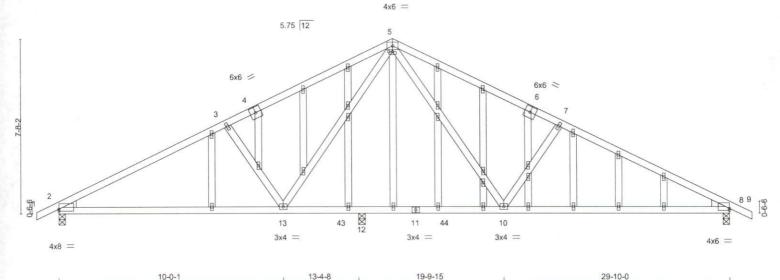


Plate Offsets (X,Y) [2:0-0-0,		10-0-1 [2:0-0-0,0-1-0], [5:0-2-0,0	3-4		3-4-7 6-5-7				'		10-0-1	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (I		l/defl	L/d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.15	TC	0.75	Vert(LL)	-0.22 13	-39	>730	240	MT20	197/144
CDL	10.0	Lumber DOL	1.15	BC	0.90	Vert(CT)	-0.49 13	-39	>332	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.27	Horz(CT)	0.06	8	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matrix	-MS						Weight: 204 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 **BOT CHORD** 2x4 SP No.2 or 2x4 SPF No.2

WEBS 2x4 SP No.3 **OTHERS** 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

(size) 2=0-3-8, 8=0-3-8, 12=0-3-8 REACTIONS.

Max Horz 2=-127(LC 13)

Max Uplift 2=-169(LC 12), 8=-169(LC 13)

Max Grav 2=1181(LC 1), 8=1193(LC 1), 12=252(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1929/535, 3-5=-1678/532, 5-7=-1711/532, 7-8=-1962/534 TOP CHORD **BOT CHORD** 2-13=-367/1662, 12-13=-131/1110, 10-12=-131/1110, 8-10=-371/1691 5-10=-143/643, 7-10=-452/287, 5-13=-143/599, 3-13=-455/287 **WEBS**

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 9-5-14 oc bracing.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



JRT THE WALLER Qty Plv Job Truss Truss Type 24557-24557A BGR Monopitch Girder Job Reference (optional) 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:36 2020 Page 1 84 Components (Dunn), Dunn, NC - 28334, ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-vWpkQnjigTe_Clfk3oNKlfB208h8IGPrY9?nJQyIK2P 9-2-10 5-0-13 4-5-6 Scale = 1:42.1 1.5x4 || 4x8 = 5.75 12 3x7 = 2 9-9-0 \bigotimes 12 13 14 15 16 6 6x6 = 3x6 || 8x8 = 4x4 = 4-5-6 4-1-14 [1:0-3-0,0-3-9], [6:0-3-8,0-4-8], [7:0-4-8,0-1-8] Plate Offsets (X,Y)--SPACING-CSI. DEFL. **PLATES** GRIP LOADING (psf) I/defl 1.15 TCLL 20.0 Plate Grip DOL TC 0.43 Vert(LL) -0.06 7-10 >999 240 197/144 TCDL 10.0 Lumber DOL 1.15 BC 0.47 Vert(CT) -0.12 7-10 >999 180 BCLL Rep Stress Incr NO WB 0.90 0.02 0.0 Horz(CT) 5 n/a n/a Code IRC2015/TPI2014 Weight: 178 lb BCDL 10.0 Matrix-MS FT = 20%BRACING-TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 TOP CHORD Structural wood sheathing directly applied or 4-6-15 oc purlins, 2x6 SP DSS except end verticals. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

REACTIONS.

BOT CHORD

WEBS 2x4 SP No.3

(size) 1=0-3-8, 5=0-3-8

Max Horz 1=250(LC 12)

Max Uplift 1=-457(LC 12), 5=-622(LC 12) Max Grav 1=3786(LC 1), 5=4090(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-6310/799, 2-3=-3400/405

BOT CHORD 1-7=-947/5655, 6-7=-947/5655, 5-6=-497/3021

2-7=-299/2620, 2-6=-3096/528, 3-6=-554/4345, 3-5=-4374/717 **WEBS**

NOTES-

1) 2-ply truss to be connected together with 10d (0.120"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-7-0 oc.

- Webs connected as follows: 2x4 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.
- 7) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1132 lb down and 155 lb up at 2-0-12, 1132 lb down and 155 lb up at 4-0-12, 1132 lb down and 155 lb up at 6-0-12, 1132 lb down and 155 lb up at 8-0-12, and 1132 lb down and 155 lb up at 10-0-12, and 1132 lb down and 155 lb up at 12-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

November 17,20 November 17,2020

Continued on page 2

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



 Job
 Truss
 Truss Type
 Qty
 Ply
 JRT THE WALLER

 24557-24557A
 BGR
 Monopitch Girder
 1
 2
 Job Reference (optional)

84 Components (Dunn),

Dunn, NC - 28334,

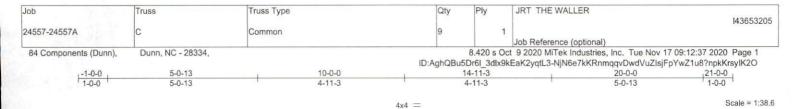
| 2 | Job Reference (optional)

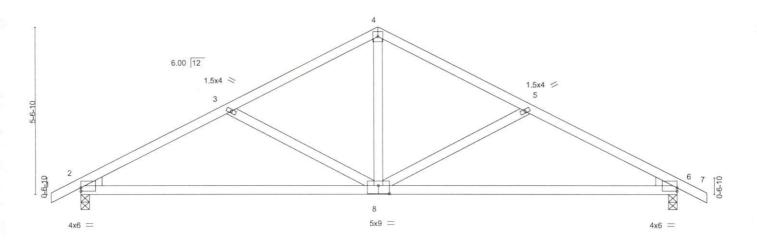
8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:36 2020 Page 2
ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-vWpkQnjigTe_Clfk3oNKlfB208h8IGPrY9?nJQyIK2P

LOAD CASE(S) Standard Uniform Loads (plf)

Vert: 1-4=-60, 5-8=-20 Concentrated Loads (lb)

Vert: 11=-1132(B) 12=-1132(B) 13=-1132(B) 14=-1132(B) 15=-1132(B) 16=-1132(B)





	10-0-0					-	20-0-0							
Plate Off					10-0-0									
riate Oil	sets (A, I)	[2:0-0-0,0-1-5], [6:0-0-0,0	7-1-0], [0.0-4-0,	,0-3-4]							T			
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP		
TCLL	20.0	Plate Grip DOL	1.15	TC	0.30	Vert(LL)	-0.15	8-14	>999	240	MT20	197/144		
TCDL	10.0	Lumber DOL	1.15	BC	0.90	Vert(CT)	-0.31	8-14	>770	180				
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.23	Horz(CT)	0.03	6	n/a	n/a				
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-MS						Weight: 92 lb	FT = 20%		

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

WEBS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 6=0-3-8

Max Horz 2=92(LC 16)

Max Uplift 2=-115(LC 12), 6=-115(LC 13) Max Grav 2=860(LC 1), 6=860(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1294/358, 3-4=-969/260, 4-5=-969/260, 5-6=-1294/358

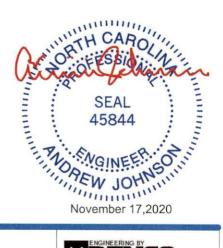
BOT CHORD 2-8=-229/1101, 6-8=-232/1101

WEBS 4-8=-69/561, 5-8=-358/216, 3-8=-358/216

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 5-0-2 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guildance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



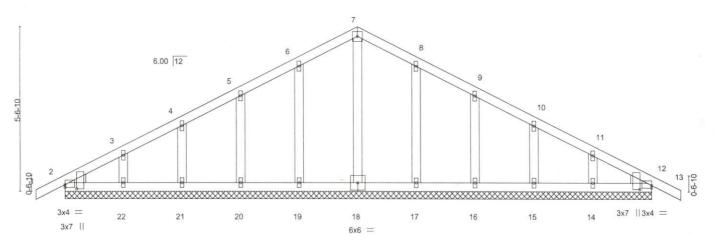
JRT THE WALLER Job Truss Truss Type Qty Ply 143653206 24557-24557A CF Common Supported Gable Job Reference (optional) 84 Components (Dunn), Dunn, NC - 28334 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:38 2020 Page 1 ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-rvxUrTkyC5uhR3o7BDPoq4GTGxTQmO280TUuOJyIK2N 1-0-0 10-0-0 20-0-0 21-0-0 10-0-0 1-0-0

4x4 =

Scale = 1:39.2

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



20-0-0 [2:0-0-0,0-1-1], [2:0-1-11,0-4-13], [12:0-0-0,0-1-1], [12:0-1-11,0-4-13] Plate Offsets (X,Y)--LOADING (psf) SPACING-2-0-0 CSI DEFL (loc) I/def **PLATES** GRIP TCLL 20.0 Plate Grip DOL TC 0.07 Vert(LL) -0.00 120 1.15 13 n/r MT20 197/144 TCDL Lumber DOL BC 0.03 10.0 1.15 Vert(CT) -0.00 13 n/r 90 0.06 BCLL 0.0 Rep Stress Incr YES WB Horz(CT) 0.00 12 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Weight: 106 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 **BOT CHORD**

OTHERS 2x4 SP No.3

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. All bearings 20-0-0

(lb) - Max Horz 2=92(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 20, 21, 22, 17, 16, 15, 14, 12

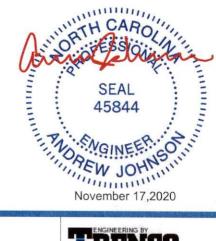
Max Grav All reactions 250 lb or less at joint(s) 2, 18, 19, 20, 21, 22, 17, 16, 15, 14, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) n/a
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 12.



🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qu Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Truss Type JRT THE WALLER Joh Truss Qty Ply 143653207 24557-24557A Valley Job Reference (optional) Dunn, NC - 28334, 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:39 2020 Page 1 84 Components (Dunn). ID:AghQBu5Dr6I_3dlx9kEaK2yqtL3-J5Vs3plbzO0Y3DNJkww1NHoaHLm?VplHF7DRwlylK2M 27-4-13 13-8-6 13-8-6 Scale = 1:44.8 4x4 = 5.75 12 3x4 / 3x4 > 10 12 11 9 8 6x6 = 27-4-13 0-0-8 27-4-4 LOADING (psf) SPACING-2-0-0 CSI DEFL. **PLATES** GRIP I/defl L/d TCLL 20.0 Plate Grip DOL 1.15 TC 0.30 Vert(LL) n/a n/a 999 MT20 197/144 TCDL 10.0 Lumber DOL 1.15 BC 0.20 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.16 Horz(CT) 0.00 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Weight: 111 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2

BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

OTHERS 2x4 SP No.3

REACTIONS. All bearings 27-3-12.

(lb) - Max Horz 1=-104(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 11, 9 except 12=-124(LC 12), 8=-123(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=402(LC 22), 11=325(LC 25), 12=424(LC 1),

9=321(LC 26), 8=425(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-12=-308/203, 6-8=-308/203

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 1.5x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty JRT THE WALLER 143653208 24557-24557A V2 Valley Job Reference (optional) Dunn, NC - 28334, 8.420 s Oct 9 2020 MiTek Industries, Inc. Tue Nov 17 09:12:40 2020 Page 1 84 Components (Dunn),

 $ID: AghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kEaK2yqtL3-nl3FG9mDki8PhNyVldRGwVLmil7dEHnRTnz?SBylK2LAghQBu5Dr6l_3dlx9kQBu5Dr6l_3dlx9kQBu5Dr6lAghQ$

Scale = 1:38.0

23-2-11

4x4 = 5.75 12 10 15 8

_ +	23-2-3 23-2-3												23-2-11 0-0-8
LOADIN	1.0	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL Lumber DOL	1.15 1.15	TC BC	0.20 0.18	Vert(LL) Vert(CT)	n/a	-	n/a	999	MT20	197/144	
BCLL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(CT)	n/a 0.00	7	n/a n/a	999 n/a			
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-S						Weight: 91 lb	FT = 20%	

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 **BOT CHORD** 2x4 SP No.2 or 2x4 SPF No.2

2x4 SP No.3 **OTHERS**

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 23-1-10.

(lb) - Max Horz 1=88(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 13, 8 except 12=-106(LC 12), 9=-106(LC 13)

All reactions 250 lb or less at joint(s) 1, 7 except 10=372(LC 19), 12=344(LC 23), 13=304(LC 1),

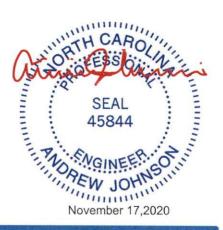
3x4 =

9=344(LC 24), 8=304(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

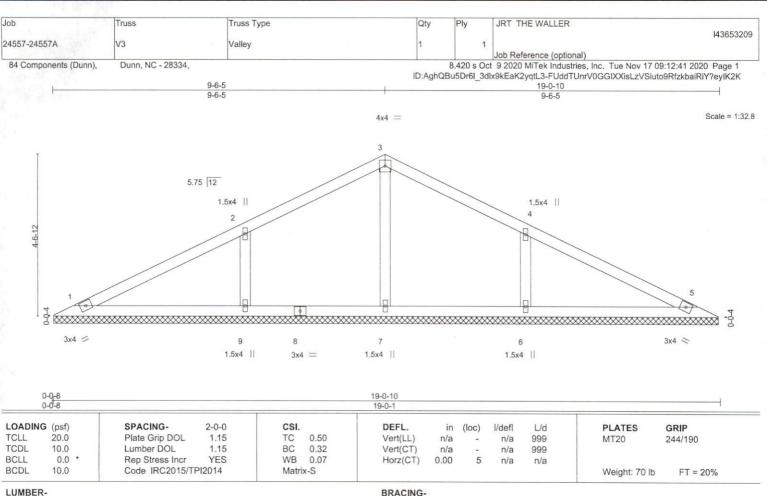
WEBS 3-12=-264/174, 5-9=-264/174

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) All plates are 1.5x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.





Edenton, NC 27932



TOP CHORD 2x4 SP No.3 **BOT CHORD** 2x4 SP No.3

OTHERS 2x4 SP No.3

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 18-11-9.

(lb) - Max Horz 1=71(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=-131(LC 12), 6=-131(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 9=435(LC 23), 6=435(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-9=-320/208, 4-6=-320/208

NOTES-

1) Unbalanced roof live loads have been considered for this design.

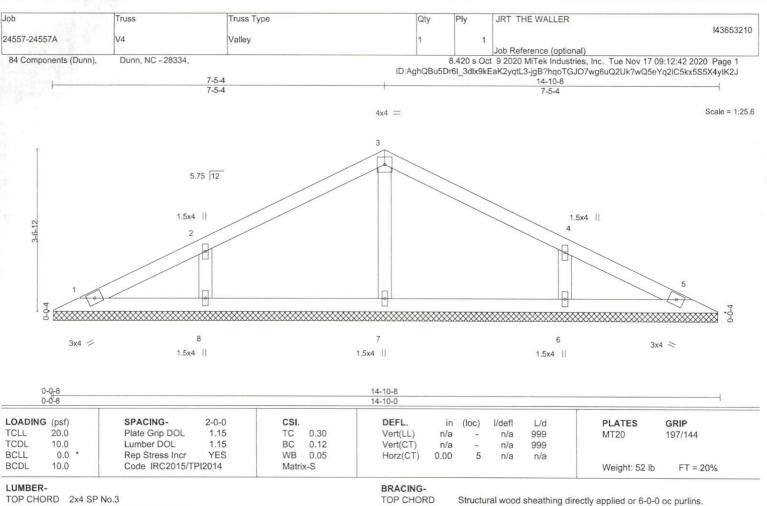
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1 60
- 3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.







BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.3

BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

2x4 SP No.3 **OTHERS**

REACTIONS. All bearings 14-9-8.

(lb) - Max Horz 1=54(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-101(LC 12), 6=-101(LC 13)

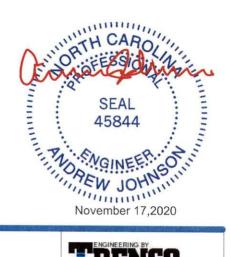
Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=284(LC 1), 8=323(LC 23), 6=323(LC 24)

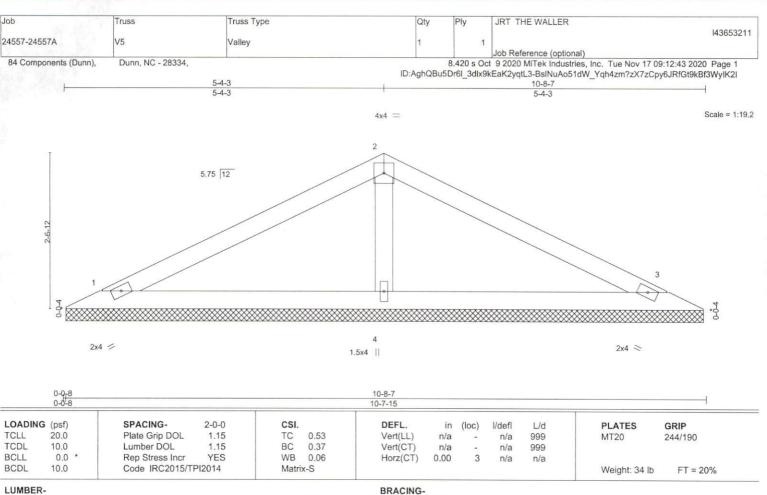
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.





TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3 2x4 SP No.3 **OTHERS**

REACTIONS.

(size) 1=10-7-6, 3=10-7-6, 4=10-7-6

Max Horz 1=-38(LC 17)

Max Uplift 1=-34(LC 12), 3=-41(LC 13), 4=-20(LC 12) Max Grav 1=171(LC 23), 3=171(LC 24), 4=414(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. **WEBS** 2-4=-276/149

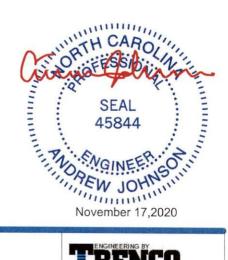
NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



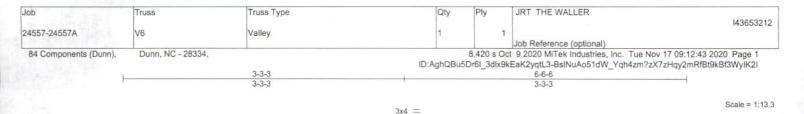
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

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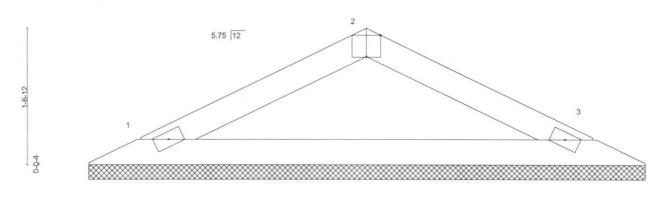


Plate Offsets (X,Y)--[2:0-2-0,Edge] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) I/defl L/d PLATES GRIP TC TCLL 20.0 Plate Grip DOL 1.15 0.21 Vert(LL) n/a n/a 999 MT20 244/190 TCDL 10.0 1.15 BC Lumber DOL 0.60 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-P FT = 20% Weight: 18 lb

LUMBER-

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3 BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

2x4 >

REACTIONS.

(size) 1=6-5-5, 3=6-5-5

Max Horz 1=21(LC 12)

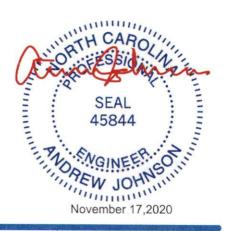
Max Uplift 1=-25(LC 12), 3=-25(LC 13) Max Grav 1=209(LC 1), 3=209(LC 1)

2x4 =

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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Edenton, NC 27932

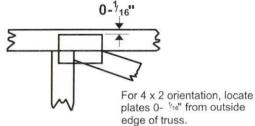
Symbols

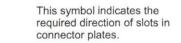
PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.

Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.





* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal

Plate Connected Wood Truss Construction.

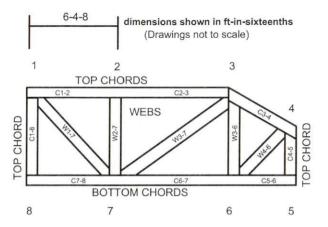
DSB-89: Design Standard for Bracing.

BCSI:

Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate

Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.