- IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY THAT ALL DIMENSIONS, ROOF
  PITCHES, AND SQUARE FOOTAGE ARE CORRECT PRIOR TO CONSTRUCTION. K&A HOME
  DESIGNS, INC. IS NOT RESPONSIBLE FOR ANY DIMENSIONING, ROOF PITCH, OR SQUARE
  FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- 2. ALL WALLS SHOWN ON THE FLOOR PLANS ARE DRAWN AT 4" UNLESS NOTED OTHERWISE.
- 3. ALL ANGLED WALLSHOWN ON THE PLANS ARE 45 DEGREES UNLESS NOTED OTHERWISE.
- 4. STUD WALL DESIGN SHALL CONFORM TO ALL NORTH CAROLINA STATE BUILDING CODE REQUIREMENTS.
- DO NOT SCALE PLANS. DRAWING SCALE MAY BE DISTORTED DUE TO COPIER IMPERFECTIONS.
- 6. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH NORTH CAROLINA RESIDENTIAL STATE BUILDING CODE, 2018 EDITION.

# **SQUARE FOOTAGE**

<u>HEATED SQUARE I</u>	<u>FOOTAGE</u>	<u>UNHEATED SQUARE F</u>	<u>OOTAGI</u>
FIRST FLOOR=	1663	GARAGE=	N/A
SECOND FLOOR=	N/A	FRONT PORCH=	218
THIRD FLOOR=	N/A	SCREEN PORCH=	296
BASEMENT=	N/A	SIDE PORCH=	60
		STORAGE=	N/A

TOTAL HEATED= 1663 TOTAL UNHEATED= 574

## CRAWL SPACE VENTILATION CALCULATIONS

-VENT LOCATIONS MAY VARY FROM THOSE SHOWN ON THE PLAN BUT SHOULD BE PLACED TO PROVIDE ADEQUATE VENTILATION AT ALL POINTS TO PREVENT DEAD AIR POCKETS.

-100% VAPOR BARRIER MUST BE PROVIDED WITH 12" MIN. LAP JOINTS.

-THE TOTAL AREA OF VENTILATION OPENINGS MAY BE REDUCED TO 1/1500 AS LONG AS REQUIRED OPENINGS ARE PLACED SO AS TO PROVIDE CROSS-VENTILATION OF THE SPACE. THE INSTALLATION OF OPERABLE LOUVERS SHALL NOT BE PROHIBITED. (COMPLY WITH NC CODE MIN. WITH REGARD TO VENT PLACEMENT FROM CORNERS)

N/A SQ. FT. OF CRAWL SPACE/1500

N/A SQ. FT. OF REQUIRED VENTILATION

PROVIDED BY: N/A

VENTS AT 0.45 SQ. FT. NET FREE

VENTILATION EACH= N/A SQ. FT. OF VENTILATION

\*\*FOUNDATION DRAINAGE- WATERPROOFING PER SECTIONS 405 & 406.

# ATTIC VENTILATION CALCULATIONS

- CALCULATIONS SHOWN BELOW ARE BASED ON VENTILATORS USED AT LEAST 3 FT. ABOVE THE CORNICE VENTS WITH THE BALANCE OF VENTIALTION PROVIDED BE EAVE VENTS.

- CATHEDRAL CEILINGS SHALL HAVE A MIN. 1" CLEARANCE BETWEEN THE BOTTOM OF THE ROOF DECK AND THE INSULATION.

2250 SQ. FT. OF ATTIC/300= 7.5

EACH OF INLET AND OUTLET REQUIRED.

## \*WALL AND ROOF CLADDING DESIGN VALUES

- WALL CLADDING IS DESIGNED FOR A 24.1 SQ. FT. OR GREATER POSITIVE AND NEGATIVE PRESSURE.

- ROOF VALUES BOTH POSITVE AND NEGATIVE SHALL BE AS FOLLOWS:

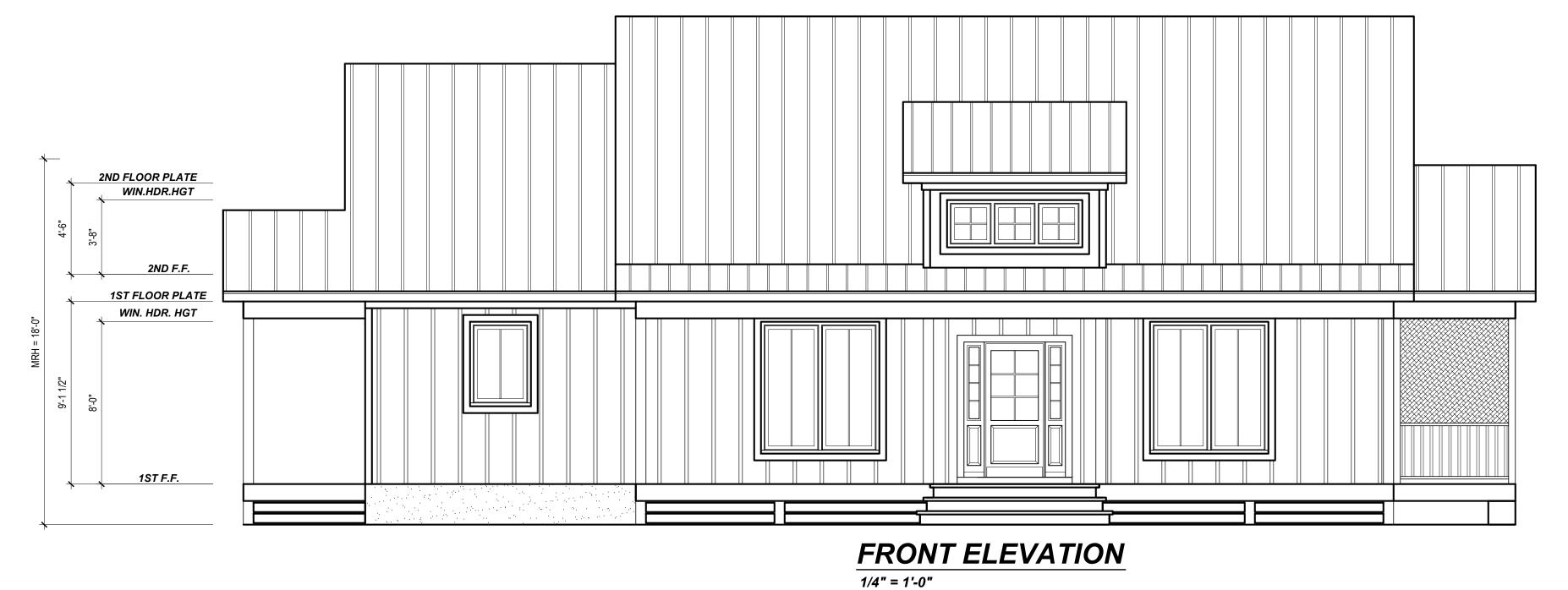
45.5 LBS. PER SQ. FT. FOR ROOF PITCHES OF 0/12 TO 2.25/12

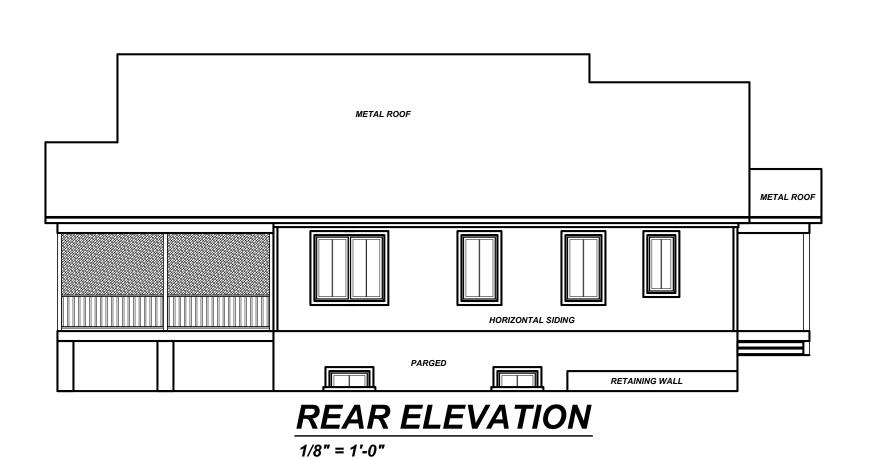
34.8 LBS. PER SQ. FT. FOR ROOF PITCHES OF 2.25/12 TO 7/12

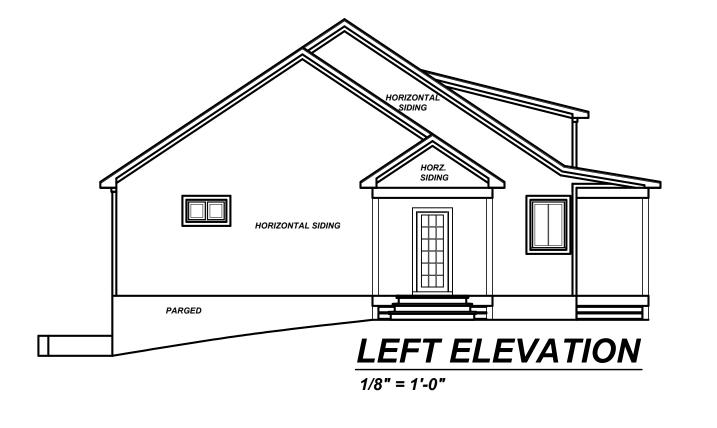
21 LBS. PER SQ. FT. FOR ROOF PITCHES OF 7/12 TO 12/12

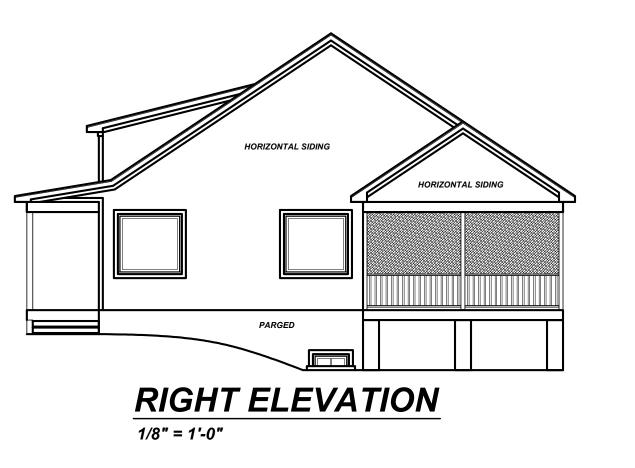
\*\* MEAN ROOF HEIGHT 30' OR LESS











Project #:
20-359

Date:
2-26-21

Drawn/Design By:
KBB

Scale:
REFER TO ELEV.

REVISIONS

No. Date: Remarks

1
2
3
4

101 Ten-Ten Rd. aleigh, NC 27603 ffice: (919) 302-0693



ewicz

e Lane Res

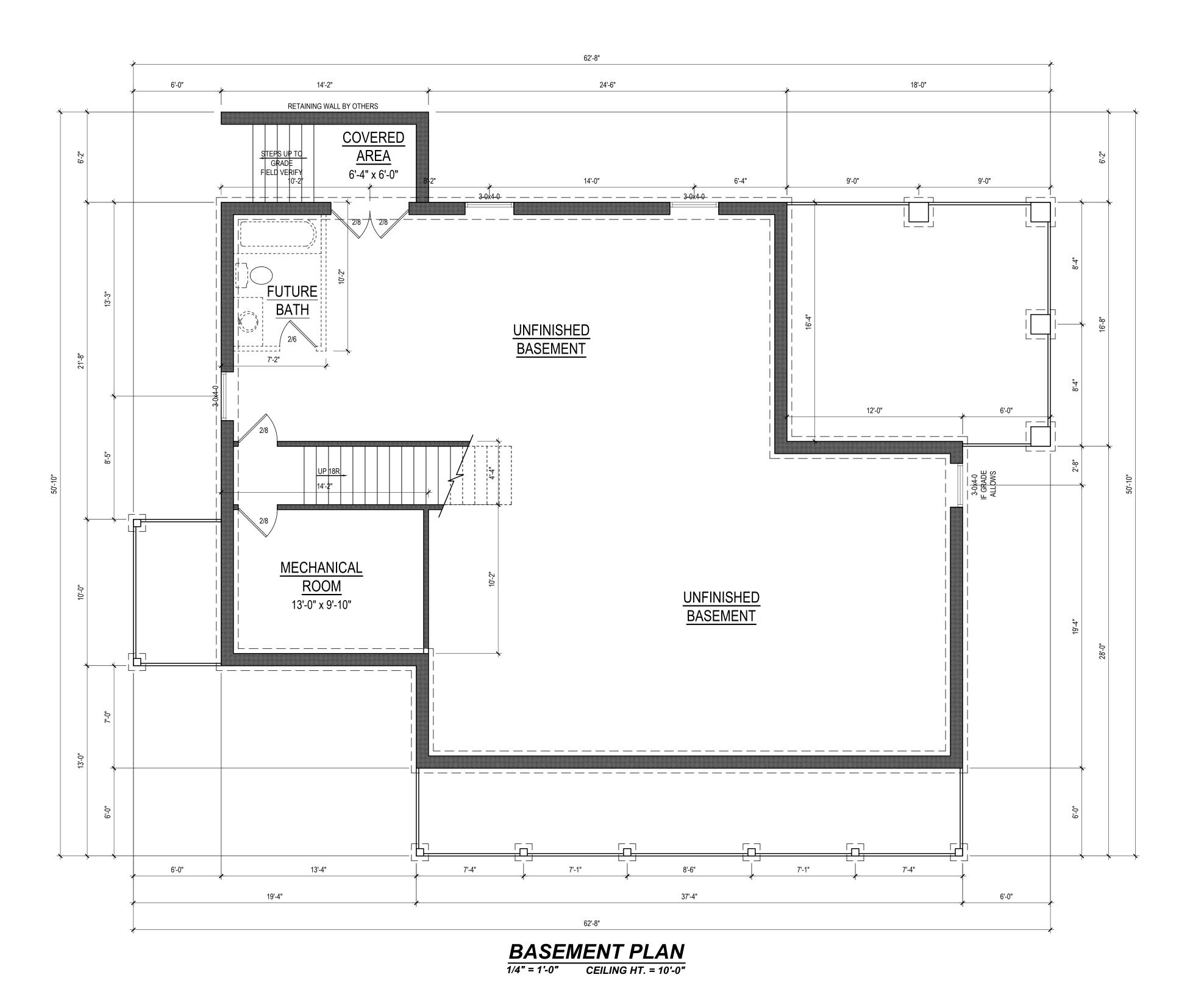
Kimberly Misklewid 5807 Turner Store La Raleigh, NC 27603

**ELEVATIONS** 

sneet Number

Kimberly Misklewicz 5807 Turner Store Lane Raleigh, NC 27603

BASEMENT



2) DESIGN LOADS:

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION (DL & LL)
ALL FLOORS	40	10	L/360
ATTIC (pull down access)	20	10	L/240
ATTIC (no access)	10	5	L/240
EXTERNAL BALCONY	60	10	L/360
ROOF	20	10	L/180
ROOF TRUSS	20	20	L/240
WIND LOAD	[BASE	D ON 115 MPH	(3-second gusts)]

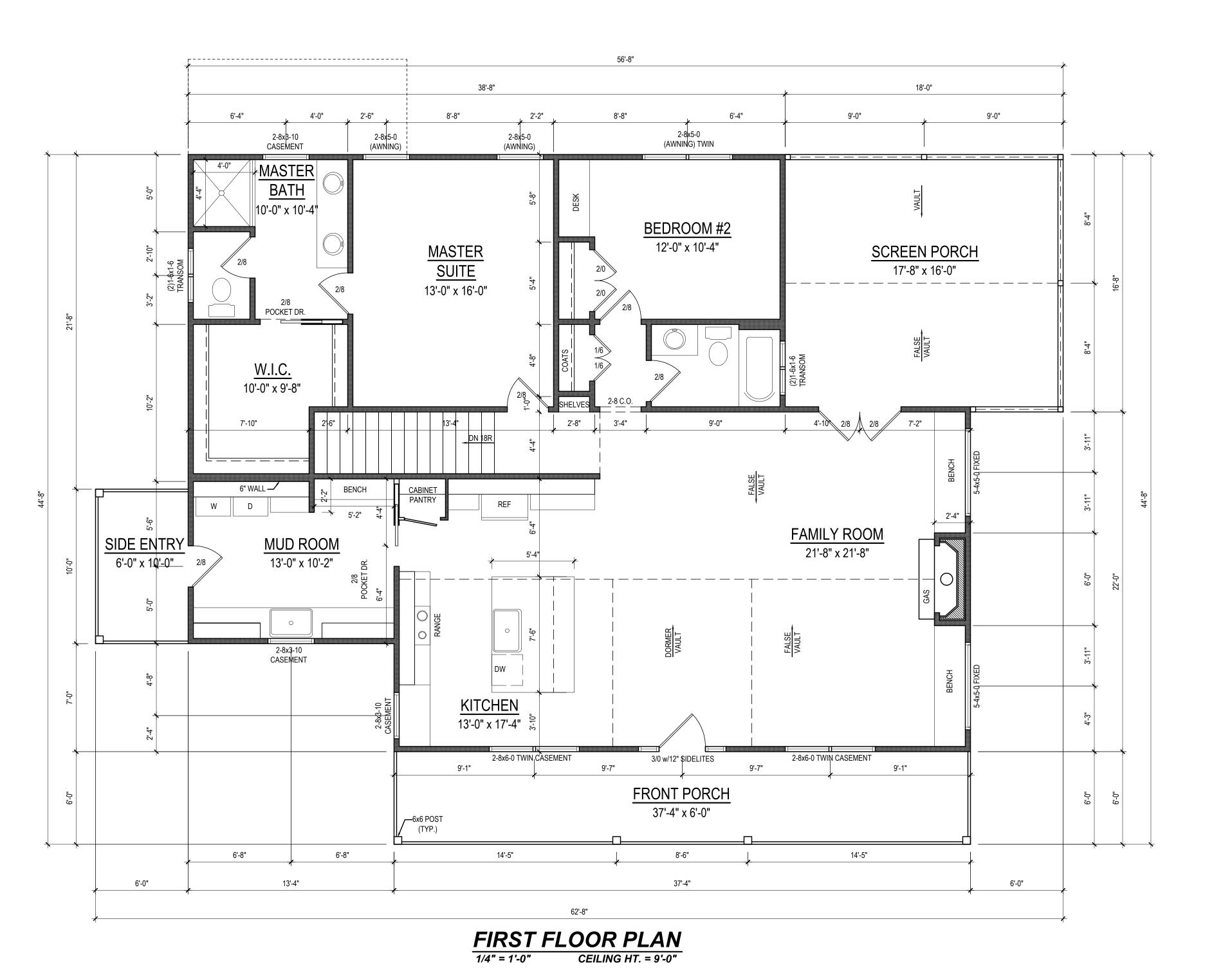
#### 3) MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF

- 4) CONCRETE SHALL HAVE A MINIMUM 28 DAY STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE (UNO).
- 5) MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2018 NC RESIDENTIAL BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT
- 6) ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI) UNO.
- ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL.
- 7) ALL LOAD BEARING HEADERS SHALL BE (2)2x10 (UNO). ALL WINDOW AND DOOR HEADERS SHALL BE SUPPORTED BY (1) JACK STUD AND (1) KING STUD AT EACH END UNLESS NOTED. ALL OTHER BEAMS SHALL BE SUPPORTED BY 2 STUDS OR THE AMOUNT OF STUDS REQUIRED FOR FULL BEARING AT EACH END UNLESS NOTED. POINT LOADS (STIFF KNEES, ETC.) SHALL CONSIST OF 2 STUDS UNLESS NOTED. ALL SUPPORTS OF 2 STUDS OR MORE SHALL BE TRANSFERRED THROUGH EACH FLOOR TO THE FOUNDATION.
- 8) ALL EXTERIOR WALLS TO BE SHEATHED WITH MIN. 7/16" WOOD STRUCTURAL PANELS FASTNED WITH 8D NAILS 6" O.C. AT EDGES AND 12" O.C. AT INT. SUPPORTS. BLOCKING SHALL BE INSTALLED IF LESS THAN 50 PERCENT OF THE WALL LENGTH IS SHEATHED. WHERE BLOCKING IS REQ'D, ALL PANELS SHALL BE FASTENED AT 3" O.C AT EDGES AND 6" O.C. AT INT. SUPPORTS.
- 9) ALL STRUCTURAL STEEL SHALL ASTM A-36. STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1\2" INCHES AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO LAG SCREWS (1/2 DIAMETER AND 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDING THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- 10) ANCHOR BOLT PLACEMENT PER SECTION R403.1.6. 1/2" DIAMETER ANCHOR BOLTS SPACED AT 6'-0" O/C AND PLACED 12" FROM THE END OF EACH PLATE SECTION
- 11) FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF 2018 NC RESIDENTIAL BUILDING CODE
- 12) WALL AND BOOF CLADDING VALUES:
- WALL CLADDING SHALL BE DESIGNED FOR A 24.1 SQ.FT. OR GREATER POSITIVE AND NEGATIVE PRESSURE
- ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:
- 45.5 LBS/SQFT FOR ROOF PITCHES OF 0/12 TO 2.25/12
- 34.8 LBS/SQFT FOR ROOF PITCHES OF 2.25/12 TO 7/12 21.0 LBS/SQFT FOR ROOF PITCHES OF 7/12 TO 12/12
- \*\* MEAN ROOF HEIGHT 30' OR LESS
- 13) FOR ROOF SLOPES FROM 2:12 THROUGH 4:12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER
- 14) IT IS THE CONTRACTOR'S RESPONSIBLITY TO VERIFY ALL DIMENSIONS AND SQ. FTG. ARE CORRECT PRIOR TO CONSTRUCTION.

  DESIGNER IS NOT RESPONSIBLE FOR DIMENSIONING OR SQ. FTG. ERRORS ONCE CONSTRUCTION BEGINS

# TABLE N1102.1 INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT

	MAXIMUM			MINIMUN	M INSULATION F	R-VALUE	
CLIMATE ZONE	GLAZING U-FACTOR	CEILINGS	EILINGS WALLS	FLOORS	BASEMENT WALLS	SLAB PERIMETER	CRAWL SPACE WALLS
3	.35	R-38 or R-30	R-15	R-19	R-5/13	R-0	R-5/13
4	.35	R-38 or R-30	R-15	R-19	R-10/15	R-10	R-10/15



Project #:
20-359

Date:
2-26-21

Drawn/Design By:
KBB

Scale:
1/4" = 1'-0"

	RI	EVISIONS
No.	<u>Date:</u>	<u>Remarks</u>
1		
2		
3		
4		

aleigh, NC 27603 flice: (919) 302-0693

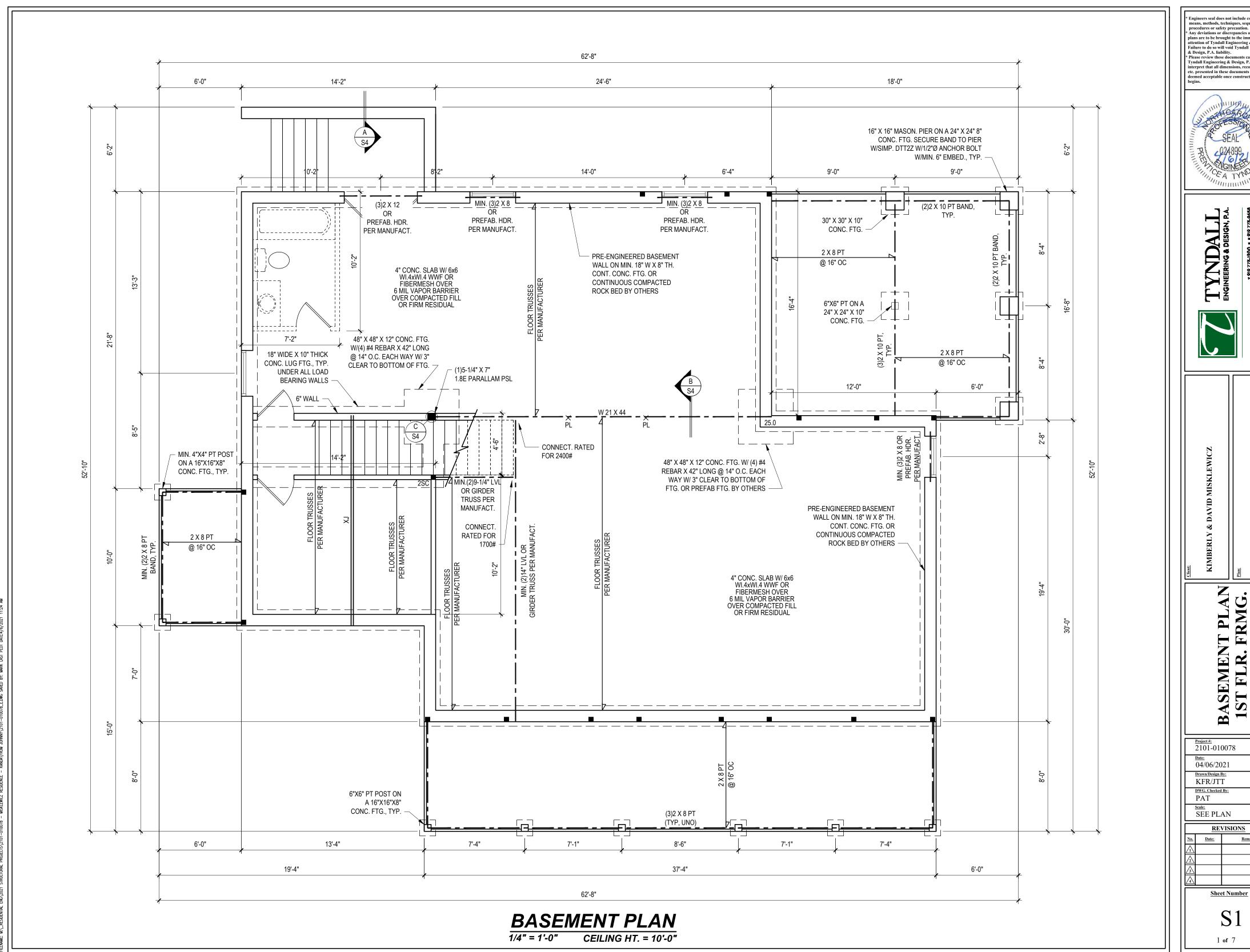


Misklewicz Residence

Kimberly Misklewicz 5807 Turner Store Lane Raleigh, NC 27603

FIRST FLOOR

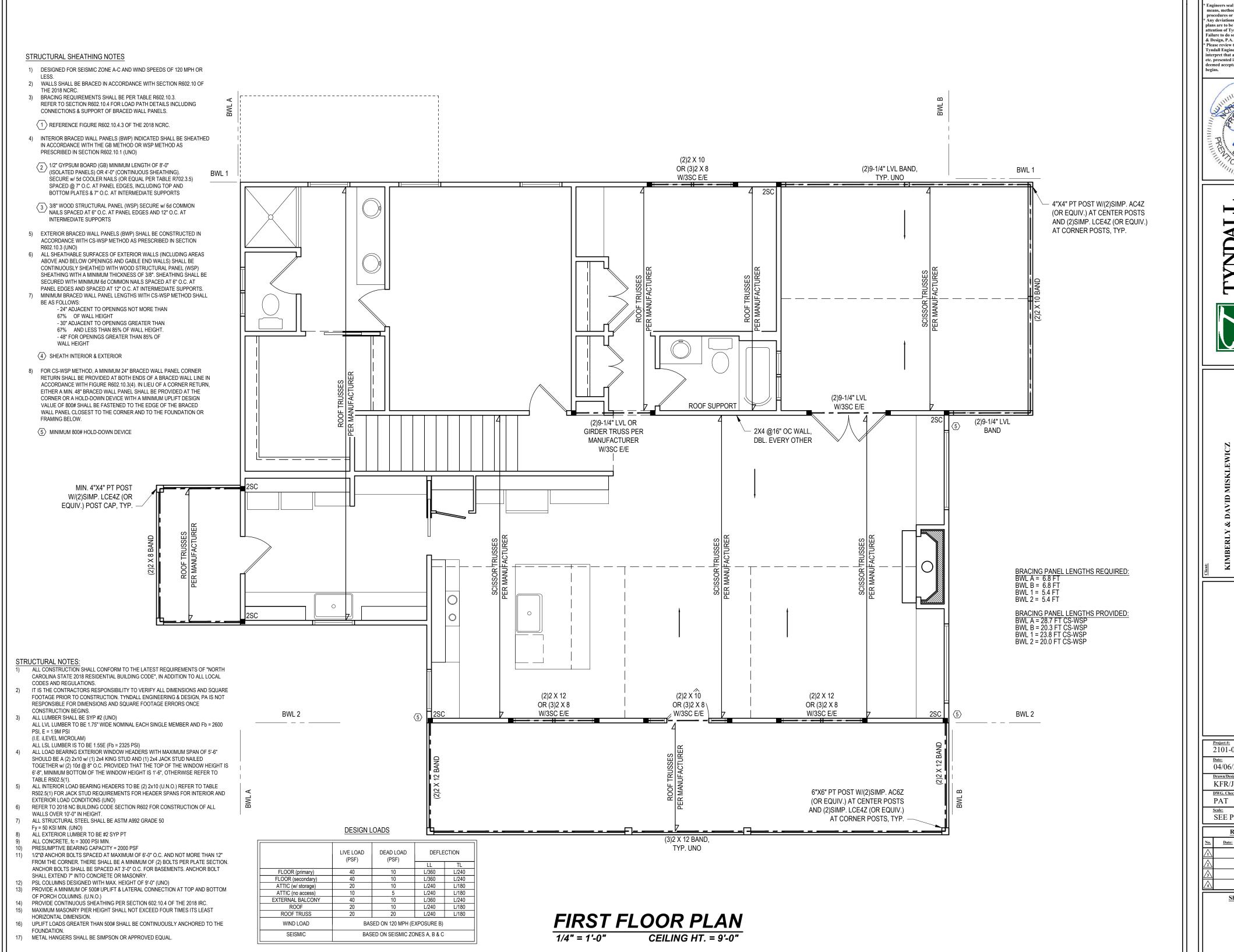
Sheet Numb



\* Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.
\* Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.
\* Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.



MISKLEWICZ RESIDENCE



means, methods, techniques, sequen-procedures or safety precaution. Any deviations or discrepancies on \* Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. \* Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed seconstruction.



TYNDALL
ENGINEERING & DESIGN, P.A

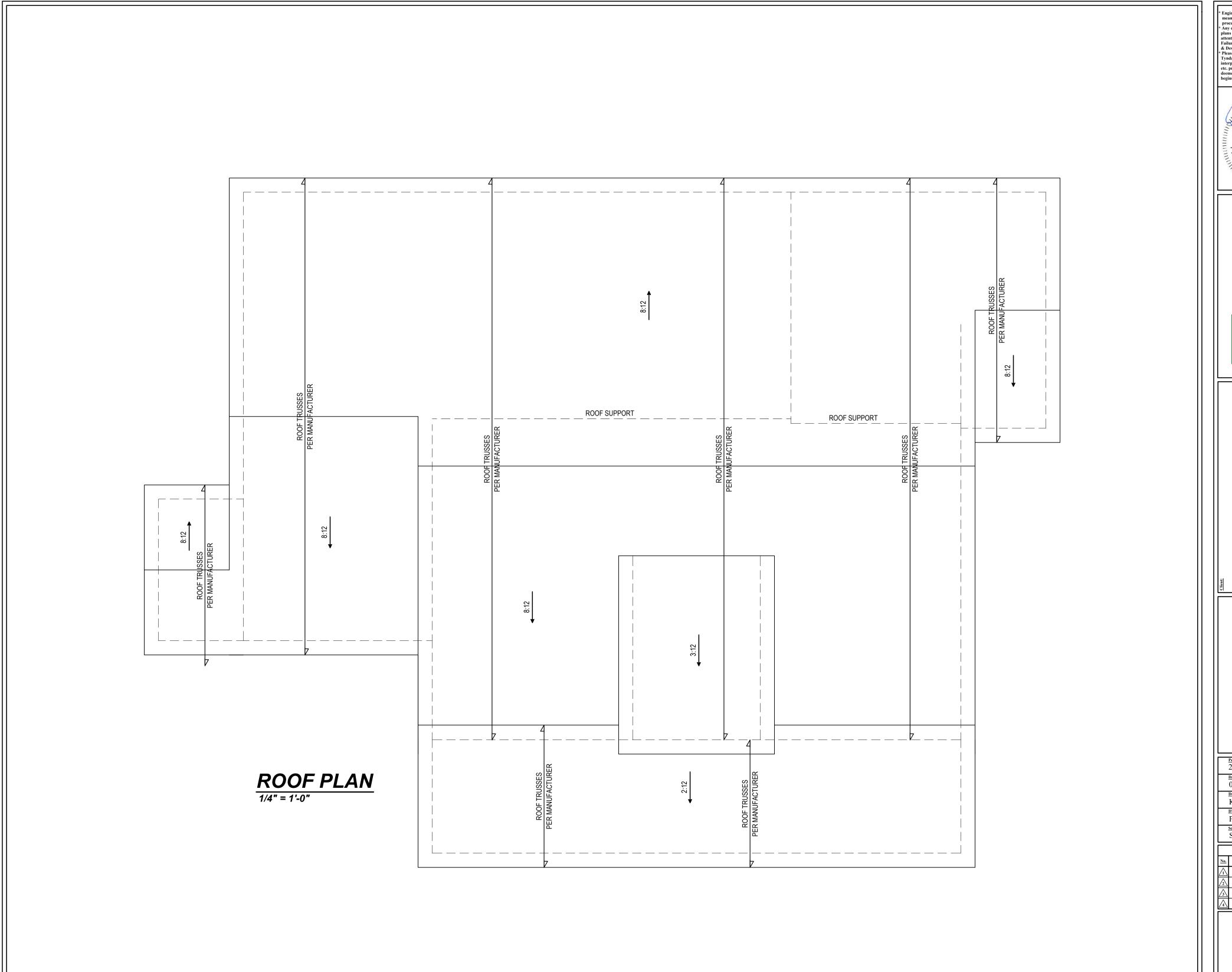
ST 2N

2101-010078 04/06/2021 Drawn/Design By: KFR/JTT DWG. Checked By:

PAT SEE PLAN

REVISIONS Date:

**Sheet Number** 



\* Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.
\* Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.
\* Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.



ENGINEERING & DESIGN, P.A.



KIMBERLY & DAVID MISKLEWICZ

ROOF PLAN

Project #: 2101-010078
Date: 04/06/2021
Drawn/Design By: KFR/JTT
DWG. Checked By:

PAT	
Scale: SEE PLAN	

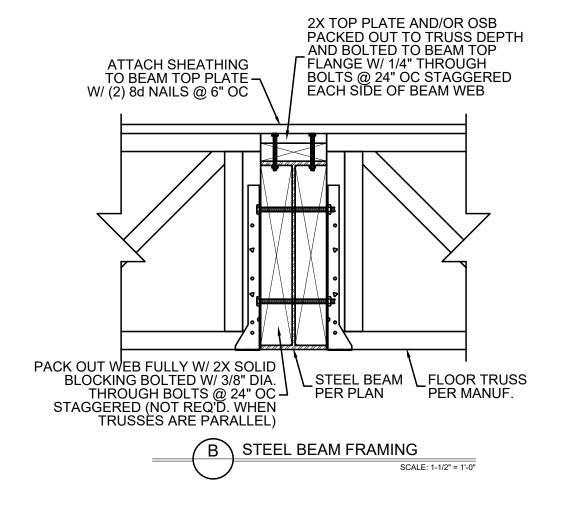
	RE	<u>VISIONS</u>
No.	Date:	Remarks
$\sqrt{1}$		
<u>^2</u> \		
<u></u>		
$\sim$		

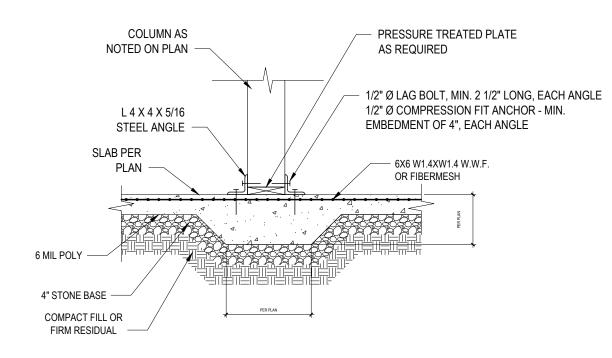
**Sheet Number** 

S3

# **RETAINING WALL DESIGN TABLE**

WALL HEIGHT	TOE	FOOTING WIDTH	KEY DEPTH	FTG. DOWEL HEIGHT	FOOTING DOWEL REINFORCEMENT	WALL VERTICAL REINFORCEMENT	WALL HORIZ. REINFORCEMENT	FOOTING HEEL REINFORCEMENT	FOOTING LONGITUDINAL REINFORCEMENT
12'-0" TO 10'-0"	1'-6"	8'-0"	2'-0"	4'-0"	#5 @ 8" OC	#5 @ 12" OC	#5 @ 12" OC	#5 @ 8" OC	#5 @ 14" OC
10'-0" TO 8'-0"	1'-3"	7'-6"	1'-8"	3'-0"	#4 @ 7" OC	#5 @ 14" OC	#4 @ 12" OC	#5 @ 8" OC	#5 @ 14" OC
8'-0" TO 6'-0"	1'-0"	5'-0"	1'-6"	3'-0"	#5 @ 14" OC	#4 @ 14" OC	#4 @ 12" OC	#4 @ 8" OC	#5 @ 14" OC
6'-0" TO 4'-0"	9"	3'-9"	1'-0"	3'-0"	#4 @ 18" OC	#4 @ 18" OC	#4 @ 16" OC	#4 @ 8" OC	(5) #4 @ 9" OC
4'-0" TO 2'-0"	6"	2'-6"	8"	2'-0"	#4 @ 18" OC	#4 @ 18" OC	#4 @ 16" OC	#4 @ 8" OC	(4) #4
2'-0" OR LESS	4"	2'-4"	N/A	FULL HEIGHT	#4 @ 18" OC	#4 @ 18" OC	#4 @ 16" OC	#4 @ 12" OC	(3) #4 OR (2) #5





BASEMENT WOOD COLUMN SLAB ATTACHMENT

\* Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.

\* Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. hiability.

\* Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.



ENGINEERING & DESIGN, P.A.

1 919 775-1200 - 1919 775-9658

1 919 775-1200 - 1919 775-9658



DETAILS

Project#:
2101-010078

Date:
04/06/2021

Drawn/Design By:
KFR/JTT

DWG, Checked By:
PAT

NOT TO SCALE

REVISIONS

No. Date: Remarks

Date: Remarks

CI

**Sheet Number** 

4 of 7

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION
	( /	( /	LL	TL
ALL FLOORS	40	10	L/360	L/240
ATTIC (w/ walk up stairs)	30	10	L/360	L/240
ATTIC (pull down access)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD		BASED ON 120 MP	H (EXPOSURE B)	
SEISMIC		SEISMIC ZON	IES A, B & C	

- 3) MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- 4) CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES
- BRACING. REFER TO SECTION R404 OF 2018 NC BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.
- ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI, BASED ON 2x10) UNO.
- ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL.
  ALL LUV LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (U.N.O.)
  ALL LISL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2325 PSI, E = 1.6M PSI (U.N.O.)
  ALL PSI LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10. (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD
- ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50.
   ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36.
- ALL STEEL PIPE SHALL BE ASTM A53 GRADE B. 9) STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1/12" A.\*\* LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- 10) PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2"Ø ANCHOR BOLTS SPACED AT 6"-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3"-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.

REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS.

- 11) FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF NC BUILDING CODE.
- 12) WALL AND ROOF CLADDING VALUES:
  WALL CLADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE.
  ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:
  39.0 LBS/SQFT FOR ROOF PITCHES 0/12 TO 1.5/12
  36.0 LBS/SQFT FOR ROOF PITCHES 1.5/12 TO 6/12
  18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12
  "MEAN ROOF HEIGHT 30'-0" OR LESS
- 13) FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- 14) REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT. 15) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.
- 18) PSL COLUMNS DESIGNED WITH MAXIMUM HEIGHT OF 9'-0" (U.N.O.)
- 19) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 20) MAXIMUM MASONRY PEIR HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 21) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

		U-FACTOR	SHGC <sup>b,<u>k</u></sup>	CEILING <sup>m</sup> R-VALUE	FRAMED WALL R-VALUE	WALL R-VALUE <sup>i</sup>	FLOOR R-VALUE	BASEMENT <sup>c,2</sup> WALL R-VALUE	R-VALUE AND DEPTH	CRAWL SPACE C WALL R-VALUE
3	0.35	0.55	0.30	38 or 30 cont	1 <u>5</u> or 13 + <u>2.5</u> h	5/13 or 5/10 cont	19	<u>5/13</u> f	0	5/13
4	0.35	0.55	0.30	38 or 30 cont <sup>j</sup>	15 or 13 + <u>2.5</u> h	5/13 or 5/10 cont	19	<u>10/15</u>	10	10/15
5	0.35	0.55	NR	38 or 30 cont	<sup>n</sup> 19, or 13 + 5 or 15 + 3	13/17 <u>or</u> 13/12.5 cont	30 <sup>g</sup>	10/15	10	10/19

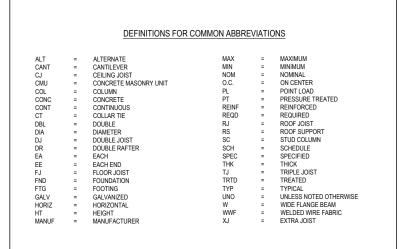
- b. THE FENESTRATION U-FACTOR COLUMN EXCLUDED SKYLIGHTS. THE SOLAR HEAT GAIN COEFFICIENT (SHGC) COLUMN APPLIES TO ALL GLAZED FENESTRATION.
- C. "1915" MEANS R-10 CONTINUOUS INSULATED SHEATHING ON THE INTERIOR OR EXTERIOR OF THE HOME OR R-15 CANTY INSULATION AT THE INTERIOR OF THE BASSMENT WALL OR CRAW, SPACE WALL FOR MONOUTHER USAS INSULATION SHALL BE APPULED FROM THE INSPECTION AND DOWNWAND TO THE OF THE FOOTING OR ANAMISM OF AY BELLOW GROSE HIND-FLYER IS LESS FOR FLOATING SLASS, INSUL SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WALL OR AY WINDERSHIPS LESS. AS SHALL BE ADDED TO THE REQUIRED SLAS BODG A PARLIES FOR HEATED SLASS.
- e. <u>DELETED</u>
  f. BASEMENT WALL INSULATION IS NOT REQUIRED IN WARM-HUMID LOCATIONS AS DEFINED BY <u>FIGURE N1101.7</u> AND <u>TABLE N1101.7</u>. g. OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY. R-19 MINIMUM.
- . THE FIRST VALUE IS CAVITY INSULATION, THE SECOND VALUE IS CONTINUOUS INSULATION, SO 193-9 MEANS R-13 CAVITY INSULATION, PLUS R-5 INSULATED SHEATHING, 1-5 THE SAME R-15 CAVITY INSULATION PLUS R-3 INSULATED SHEATHING, IS TRUCTURAL SHEATHING COVERS 29% OR LESS OF THE EXTERIOR. INSULATING SHEATHING COVERS 29% OR LESS OF THE EXTERIOR. SHALL BE SUPPLEMENTED WITH INSULATED SHEATHING OF AT LEAST R-2, '13 + 25' MEANS R-13 CAVITY INSULATION IS R-7 S SHEATHING THAN 25 FERCIAL THE SHEATHING COVERS OF THE STREAM OF THE STEED R-3 SHEATHING THAN 25 FERCIAL THAN 25
- FOR MASS WALLS, THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WAL
- LIN ADDITION TO THE EXEMPTION IN SECTION IN 192.33. AMAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSENBLES HAVING A LIFACTOR NO GREATER THAN 0.55 SHALL BE PERMITTED TO BE SUBSTITUTED FOR INNIMMAL DODGE COMPLIANT FENESTRATION PRODUCT ASSEMBLES WITHOUT FENELY.

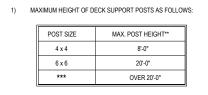
  IN ADDITION TO THE EXEMPTION IN SECTION IN 1102.33. A MAXIMUM OF TWO GLAZED PENESTRATION PRODUCT ASSEMBLES HAVING A SHICK NO GREATER THAN 0.70 SHALL BE PERMITTED TO BE SUBSTITUTED FOR INNIMMAN CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLES WITHOUT PENALTY.
- R.30 SHALL BE DEEMED TO SATISFY THE CELING INSILATION REQUIREMENT WHEREVER THE FULL HEGHT OF MOVEMENSES R.30 INSULATION EXTENDS OVER THE WALL TO PLATE AT THE EVIS. OTHERWISE AS INSULATION IS REQUIRED WHERE ADEQUATE CLEARWISE DAYS TO RESILATION MUST EXTEND TO ETHER THE INSULATION BAFFLE OR WITHIN TRICH OF THE ATTER ONE DEEM.

S WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT

2469 SQ. FT. OF ATTIC / 300 = 8.23 SQ. FT. INLETS/OUTLETS REQUIRED







- THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS IHIS TABLE IS BASED ON NO. THEAT ED SOUT DHENR PINE POSTS.

  MAXIMUM TRIBUTARY AREA IS BASED ON 128 TOTAL SQUARE FEET
  WHICH MAY BE LOCATED AT DIFFERENT LEVELS.
  FROM TOP OF FOOTING TO BOTTOM OF GIRDER
  DECKS WITH POST HEIGHTS OVER 20-0" SHALL BE DESIGNED AND
  SEALED BY A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT.
- DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THESE METHODS:

- THE DECK FLOOR HEIGHT IS LESS THAN 4"-0" AND THE DECK IS
  ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION (4)
  ABOVE. LATERAL BRACING IS NOT REQUIRED.
  4 x 4 WOOD NIKE BRACES MAY BE PROVIDED ON EACH COLUMN IN
  BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST
  AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE
  TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN
  45" AND 60" FROM THE HORIZONTAL KNEE BRACES SHALL BE BOLTED
  TO THE POST AND GIRDFER WITH DNE 56/6" HOT DIPPED (AU ANNIZED O THE POST AND GIRDER WITH ONE 5/8"Ø HOT DIPPED GALVANIZED
- BOLT AT EACH END OF THE BRACE.

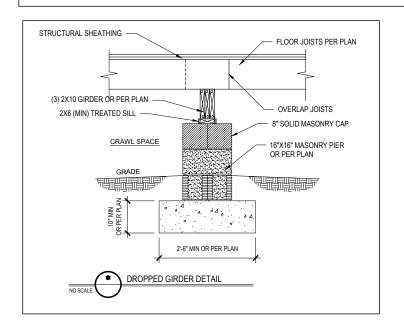
  FOR FREESTANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL

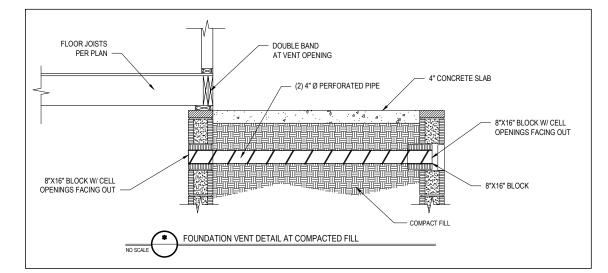
  BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN ACCORDANCE WITH THE FOLLOWING:

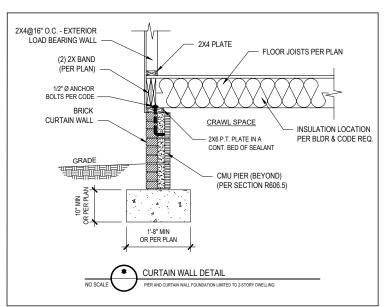
POST SIZE	MAX. TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER
4 x 4	48 SQ. FT.	4'-0"	2'-6"	1'-0"
6 x 6	120 SQ. FT.	6'-0"	3'-6"	1'-8"

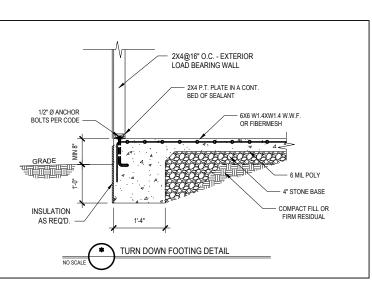
(2) PERPENDICULAR DIRECTIONS FOR FREESTANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE 2 x 6s SHALL BE ATTACHED TO THE POSTS WITH ONE 5/8"0 HOT DIPPED GALVANIZED BOLT AT EACH END OF EACH BRACING MEMBER. E. FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.

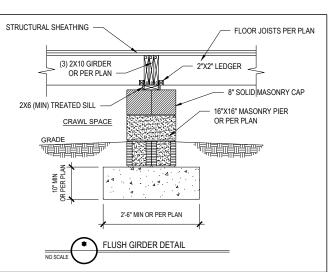
D. 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO

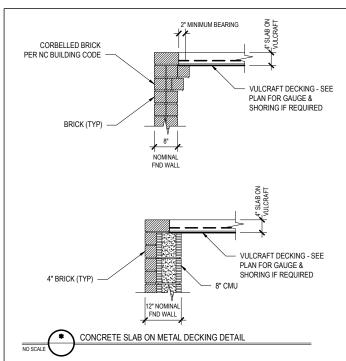


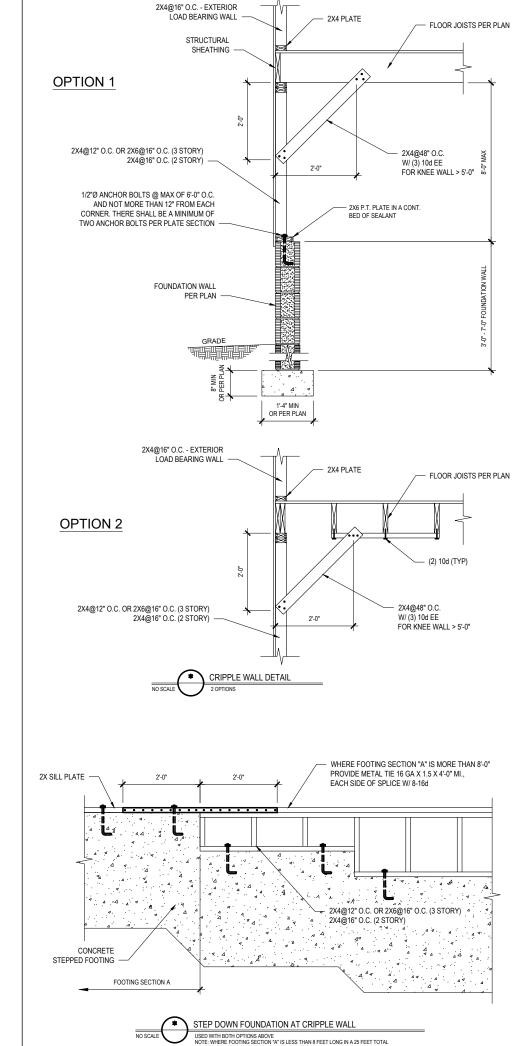


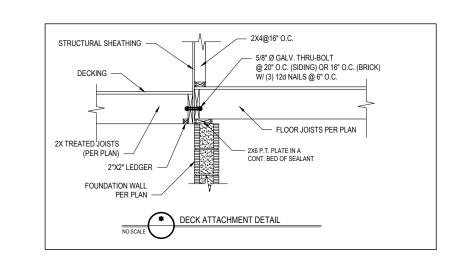












eans, methods, techniques, sequences, rocedures or safety precaution. Any deviations or discrepancies on \* Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability \* Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed seconstruction. emed acceptable once construction



TYNDALL GINEERING & DESIGN, P.A.

RESIDENC

DAVID ]

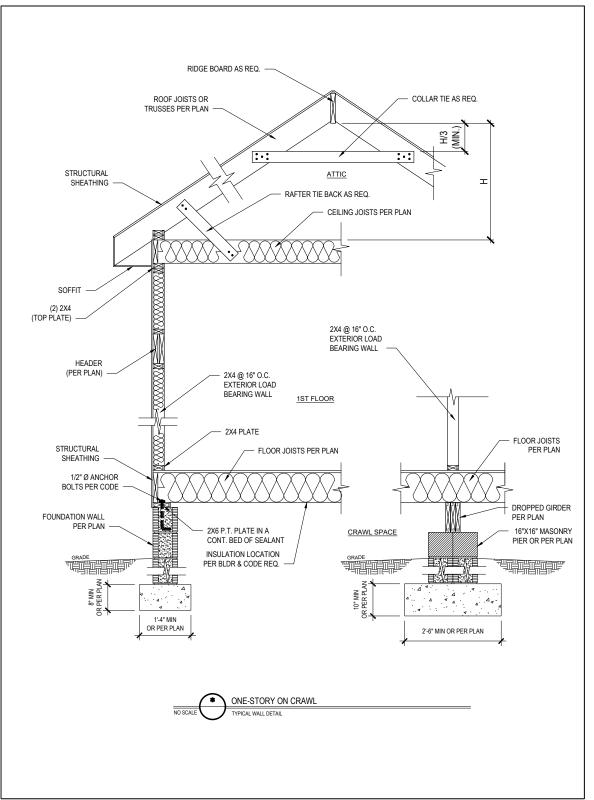
R STA

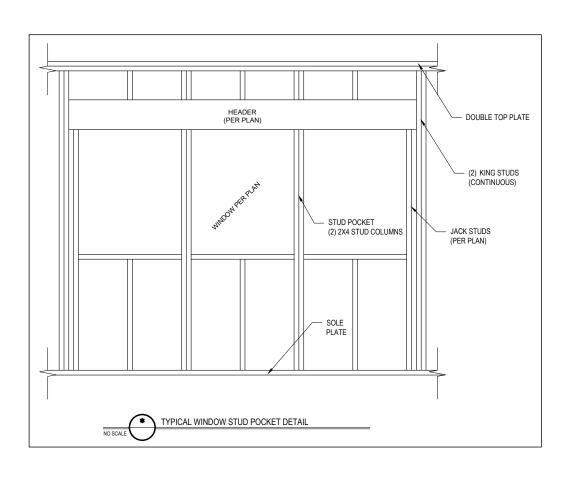
2101-010078 04/06/2021 Drawn/Design By: KFR/JTT DWG. Checked By: PAT

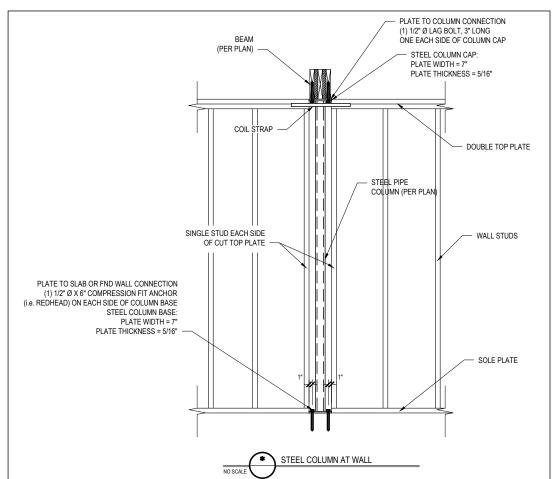
NOT TO SCALE REVISIONS Remarks Date:

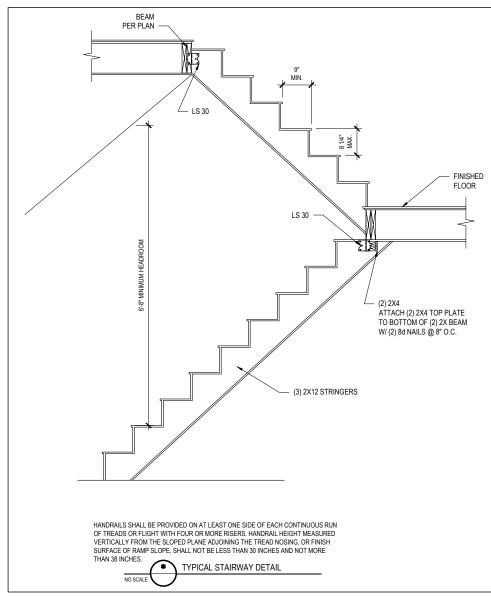
**Sheet Number** 

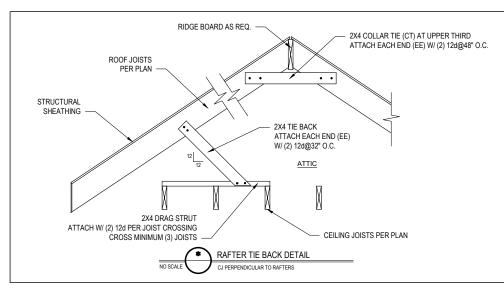
5 of 7

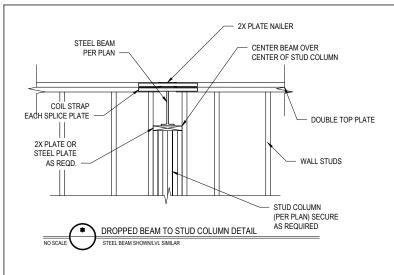


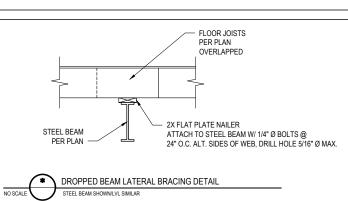


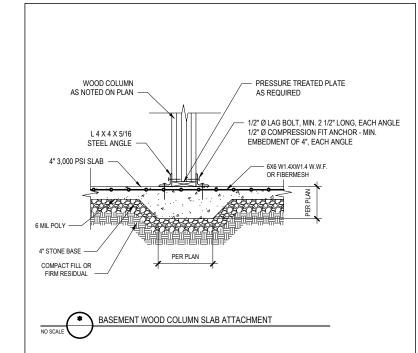


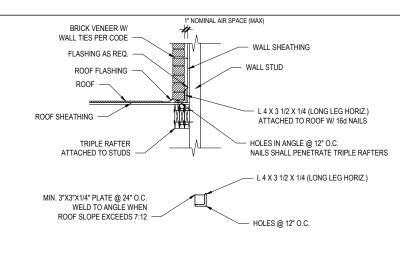












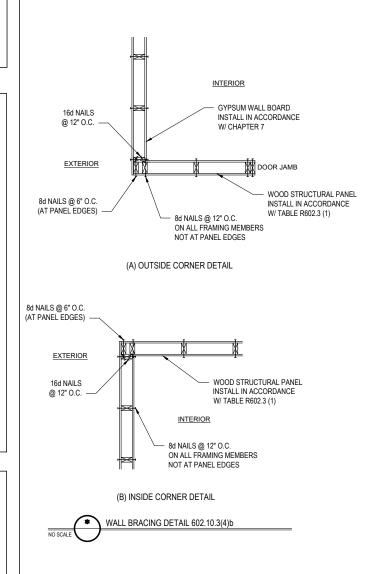
SIZE OF ANGLE (1,3)	NO STORY ABOVE (5)	1 STORY ABOVE (5)	2 STORIES ABOVE (5)	# OF ½" (OR EQUIV.) REINFORCING BARS IN REINFORCED LINTEL (2,4,5)
L3x3x1/4	6'-0"	4'-6"	3'-0"	1
L4x3x1/4	8'-0"	6'-0"	4'-6"	1
L5x3½x5/16	10'-0"	8'-0"	6'-0"	2
L6x3½x5/16	14'-0"	9'-6"	7'-0"	2
2L 5 x 3 ½ x 5/16	20'-0"	12'-0"	9'-6"	4

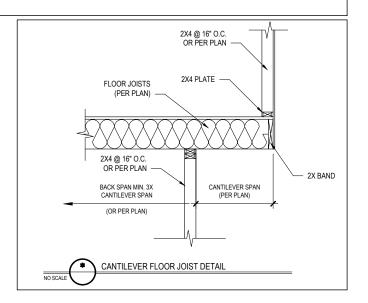
- 1. LONG LEG OF THE ANGLE SHALL BE PLACED IN A VERTICAL POSITION.
  2. DEPTH OF REINFORCED LINTELS SHALL NOT BE LESS THAN 8" AND ALL CELLS OF HOLLOW MASONRY LINTELS SHALL BE GROUTED. REINFORCING BARS SHALL EXTEND NOT LESS THAN 8" INTO THE SUPPORT
  3. STEEL MEMBERS INDICATED ARE ADEQUATE TYPICAL EXAMPLES; OTHER STEEL MEMBERS MEETING STRUCTURAL DESIGN REQUIREMENTS SHALL BE PERMITTED TO BE USED.
  4. EITHER STEEL ANGLE OR REINFORCED LINTEL SHALL SPAN OPENING.
  5. SPANS OVER 4"-0" SHALL BE SHORED UP UNTIL CURED.

ANS OVER 4-0 STALE BE SHOKED OF SINTE COKED.							
	(*)	MASONRY VENEER SUPPORT FIG 703.8.3.1					
NO SCALE							

ABE PAE CBSQ CBSQ CCQ KCCQ CMSTC16 CS RS H1 RT15 H12.5A RT7A H10 RT16 HDQ8-SDS3 UPHD8 HDU2-SDS2.5 PHD2 HDU5-SDS2.5 PHD5 HETA HTA HGAM10KTA HGAM HHDQ14-SDS2.5 UPHD14 HTT HTT HUS HTT HUS HUS LTA1 LPTA LTHA26 HJC26 LTP4 MP4F LUS JUS MAS FA3 MSTAM MSTAM PC PCM PHD-SDS3 PHD CSSQ CSQ CSQ CSQ CSQ CSQ CSQ CSQ CSQ CSQ		CROSS-REFERENCE CHART
A35 MPA1  ABE PAE  CBSQ CBSQ  CCQ KCCQ  CMSTC16 CMSTC16  CS RS  H1 RT15  H1 RT15  H10 RT16  HDQ8-SDS3 UPHD8  HDU2-SDS2.5 PHD2  HDU5-SDS2.5 PHD5  HETA HTA  HGAM10KTA HGAM  HHDQ14-SDS2.5 UPHD14  HTT HTT  HUS HUS  LTA1 LPTA  LTHJA26 HJC26  LTP4 MP4F  LUS JUS  MAS FA3  MSTAM MSTAM  PC PCM  PHD-SDS3 PAC  CCQ  CMSTC16  CMS	SIMPSON STRONG-TIE	USP STRUCTURAL CONNECTORS
ABE PAE CBSQ CBSQ CCQ KCCQ CMSTC16 CS RS H1 RT15 H12.5A RT7A H10 RT16 HDQ8-SDS3 UPHD8 HDU2-SDS2.5 PHD2 HDU5-SDS2.5 PHD5 HETA HTA HGAM10KTA HGAM HHDQ14-SDS2.5 UPHD14 HTT HTT HUS HTT HUS HUS LTA1 LPTA LTHA26 HJC26 LTP4 MP4F LUS JUS MAS FA3 MSTAM MSTAM PC PCM PHD-SDS3 PHD CSSQ CSQ CSQ CSQ CSQ CSQ CSQ CSQ CSQ CSQ	PRODUCT NUMBER	PRODUCT NUMBER
CBSQ         CBSQ           CCQ         KCCQ           CMSTC16         CMSTC16           CS         RS           H1         RT15           H2.5A         RT7A           H10         RT16           H0Q8-SDS3         UPHDB           HDU2-SDS2.5         PHD2           HDU5-SDS2.5         PHD6           HETA         HTA           HGAM10KTA         HGAM           HHDQ14-SDS2.5         UPHD14           HTS         HTW           HTT         HTT           HUS         HUS           LTA1         LPTA           LTHA26         HJC26           LTP4         MP4F           LUS         JUS           MAS         FA3           MSTAM         MSTAM           PC         PCM           PHD-SDS3         PHD           SSP         RSP16	A35	MPA1
CCQ         KCCQ           CMSTC16         CMSTC16           CS         RS           H1         RT15           H2.5A         RT7A           H10         RT16           HDQ8-SDS3         UPHD8           HDU2-SDS2.5         PHD2           HDU5-SDS2.5         PHD5           HETA         HTA           HGAM         HHD014-SDS2.5           UPHD14         HTS           HTTS         HTW           HTT         HTT           HUS         HUS           LTA1         LPTA           LTHA26         HJC26           LTP4         MP4F           LUS         JUS           MAS         FA3           MSTAM         MSTAM           PC         PCM           PHD-SDS3         PHD	ABE	PAE
CMSTC16  CMSTC16  CS  RS  H1  RT15  H2.5A  RT7A  H10  RT16  HDQ8-SDS3  UPHD8  HDU2-SDS2.5  PHD2  HDU5-SDS2.5  PHD5  HETA  HTA  HGAM  HHDQ14-SDS2.5  UPHD14  HTS  HTW  HTT  HTT  HUS  HUS  LTA1  LETA  LTHJA26  LTP4  MAS  FA3  MSTAM  MSTAM  PC  PCM  PHD-SDS3  PHD  RT16  RT1	CBSQ	CBSQ
CS RS H1 RT15 H2.5A RT7A H10 RT16 HDQ8-SDS3 UPHD8 HDU2-SDS2.5 PHD2 HDU5-SDS2.5 PHD5 HETA HTA HGAMIOKTA HGAM HHDQ14-SDS2.5 UPHD14 HTS HTW HTT HTT HTT HUS HUS LTA1 LPTA LTHJA26 HJC26 LTP4 MP4F LUS JUS MAS FA3 MSTAM MSTAM PC PCM PHD-SDS3 PHD STAF	CCQ	KCCQ
H1 RT15 H2.5A RT7A H10 RT16 HD08-SDS3 UPHD8 HDU2-SDS2.5 PHD2 HDU5-SDS2.5 PHD5 HETA HTA HGAMIOKTA HGAM HHD014-SDS2.5 UPHD14 HTS HTW HTT HTT HUS HUS LTA1 LPTA LTHJA26 HJC26 LTP4 MP4F LUS JUS MAS FA3 MSTAM MSTAM PC PCM PHD-SDS3 PHD SSP RSP16	CMSTC16	CMSTC16
H2.5A RT7A H10 RT16 HDQ8-SDS3 UPHD8 HDU2-SDS2.5 PHD2 HDU5-SDS2.5 PHD5 HETA HTA HGAM10KTA HGAM HHDQ14-SDS2.5 UPHD14 HTS HTW HTT HTT HUS HUS LTA1 LPTA LTHJA26 HJC26 LTP4 MP4F LUS JUS MAS FA3 MSTAM MSTAM PC PCM PHD-SDS3 PHD SSP RSP16	CS	RS
H10 RT16 HDQ8-SDS3 UPHD8 HDU2-SDS2.5 PHD2 HDU5-SDS2.5 PHD5 HETA HTA HGAM10KTA HGAM HHDQ14-SDS2.5 UPHD14 HTS HTW HTT HTT HUS HUS LTA1 LPTA LTHJA26 HJC26 LTP4 MP4F LUS JUS MAS FA3 MSTAM MSTAM PC PCM PHD-SDS3 PHD SSP RSP16	H1	RT15
HDQ8-SD33 UPHD8 HDU2-SDS2.5 PHD2 HDU5-SDS2.5 PHD5 HETA HTA HGAM10KTA HGAM HHDQ14-SDS2.5 UPHD14 HTS HTW HTT HTT HUS HUS LTA1 LPTA LTHLA26 HLC26 LTP4 MP4F LUS JUS MAS FA3 MASTAM MSTAM PC PCM PHD-SDS3 PHD SSP RSP16	H2.5A	RT7A
HDU2-SDS2.5 PHD2 HDU5-SDS2.5 PHD5 HETA HTA HGAM10KTA HGAM HHDQ14-SDS2.5 UPHD14 HTS HTW HTT HTT HUS HUS LTA1 LPTA LTHJA26 HJC26 LTP4 MP4F LUS JUS MAS FA3 MSTAM MSTAM PC PCM PHD-SDS3 PHD SSP RSPT6	H10	RT16
HDU5-SDS2.5 PHD5 HETA HTA HGAM HGAM10KTA HGAM HHDQ14-SDS2.5 UPHD14 HTS HTW HTT HTT HUS HUS LTA1 LPTA LTHJA26 HJC26 LTP4 MP4F LUS JUS MAS FA3 MSTAM MSTAM PC PCM PHD-SDS3 PHD SSP RSPT6	HDQ8-SDS3	UPHD8
HETA HTA  HGAM10KTA HGAM  HHDQ14-SDS2.5 UPHD14  HTS HTW  HTT HTT  HUS HUS  LTA1 LPTA  LTHJA26 HJC26  LTP4 MP4F  LUS JUS  MAS FA3  MSTAM MSTAM  PC PCM  PHD-SDS3 PHD  SSP RSPT6	HDU2-SDS2.5	PHD2
HGAM/OKTA         HGAM           HHDQ14-SDS2.5         UPHD14           HTS         HTW           HTT         HTT           HUS         HUS           LTA1         LPTA           LTHJA26         HJC26           LTP4         MP4F           LUS         JUS           MAS         FA3           MSTAM         MSTAM           PC         PCM           PHD-SDS3         PHD           SSP         RSP16	HDU5-SDS2.5	PHD5
HHD014-SDS2.5 UPHD14 HTS HTW HTT HTT HUS HUS LTA1 LPTA LTHJA26 HJC26 LTP4 MP4F LUS JUS MAS FA3 MSTAM MSTAM PC PCM PHD-SDS3 PHD SSP RSP16	HETA	HTA
HTS HTW HTT HIT HUS HUS LTA1 LPTA LTHJA26 HLC26 LTP4 MP4F LUS JUS MAS FA3 MSTAM MSTAM PC PCM PHD-SDS3 PHD SSP RSP16	HGAM10KTA	HGAM
HTT HUS HUS LTA1 LPTA LTHLA26 HLC26 LTP4 MP4F LUS JUS MAS FA3 MASTAM MSTAM PC PCM PHD-SDS3 PHD SSP RSPT6	HHDQ14-SDS2.5	UPHD14
HUS HUS  LTA1 LPTA  LTHJA26 HJC26  LTP4 MP4F  LUS JUS  MAS FA3  MSTAM MSTAM  PC PCM  PHD-SDS3 PHD  SSP RSPT6	HTS	HTW
LTA1         LPTA           LTHA26         HJC26           LTP4         MP4F           LUS         JUS           MAS         FA3           MSTAM         MSTAM           PC         PCM           PHD-SDS3         PHD           SSP         RSPT6	нтт	HTT
LTHJA26         HJC26           LTP4         MP4F           LUS         JUS           MAS         FA3           MSTAM         MSTAM           PC         PCM           PHD-SDS3         PHD           SSP         RSPT6	HUS	HUS
LTP4         MP4F           LUS         JUS           MAS         FA3           MSTAM         MSTAM           PC         PCM           PHD-SDS3         PHD           SSP         RSPT6	LTA1	LPTA
LUS         JUS           MAS         FA3           MSTAM         MSTAM           PC         PCM           PHD-SDS3         PHD           SSP         RSPT6	LTHJA26	HJC26
MAS         FA3           MSTAM         MSTAM           PC         PCM           PHD-SDS3         PHD           SSP         RSPT6	LTP4	MP4F
MSTAM         MSTAM           PC         PCM           PHD-SDS3         PHD           SSP         RSPT6	LUS	JUS
PC         PCM           PHD-SDS3         PHD           SSP         RSPT6	MAS	FA3
PHD-SDS3         PHD           SSP         RSPT6	MSTAM	MSTAM
SSP RSPT6	PC	PCM
	PHD-SDS3	PHD
STC TR1	SSP	RSPT6
	STC	TR1

STAD





ngineers seal does not include construction \* Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution.

\* Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.

\* Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction.



TYNDALL ENGINEERING & DESIGN, P.A.

RESIDENCE

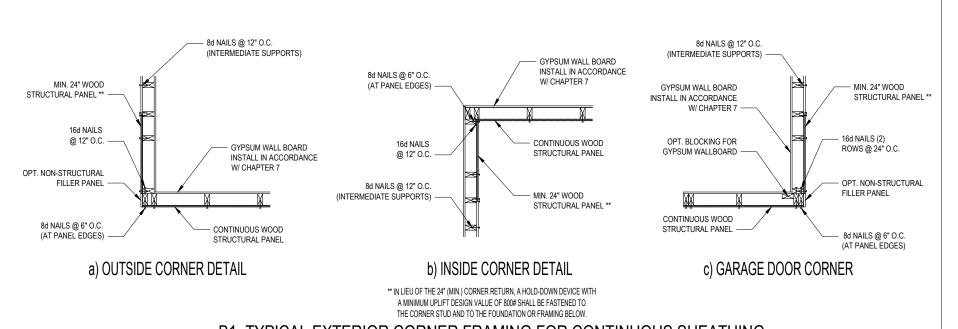
DAVID MISKLEWIC

ARD TAIL S STA

2101-010078 04/06/2021 KFR/JTT DWG. Checked By: PAT

NOT TO SCALE REVISIONS Date: Remarks

**Sheet Number** 



### B1: TYPICAL EXTERIOR CORNER FRAMING FOR CONTINUOUS SHEATHING NO SCALE

#### STRUCTURAL SHEATHING NOTES

- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
   WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC
   BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.

## The reference figure R602.10.4.3 OF THE 2018 NCRC.

- INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 2) 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING
- (3) 3/8° WOOD STRUCTURAL PANEL (WSP) SECURE W/ 6d COMMON NAILS SPACED AT 6° O.C. AT PANEL EDGES AND 12° O.C. AT INTERMEDIATE SUPPORTS
- 5. EXTERIOR BRACED WALL PANELS (BWP) SHALL BE
- CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD
  AS PRESCRIBED IN SECTION R602.10.3 (UNO)

  6. ALL SHEATHABLE SURFACES OF EXTERIOR WALLS ALL SHEATHABLE SUNFALES OF EATERION WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE
- A MINIMUM THICKNESS OF 30°S. SHEATHING SHALL BE SECURED WITH MINIMUM 64 COMMON NAILS SPACED AT 6°O.C. AT PANEL EDGES AND SPACED AT 12°O.C. AT INTERNIEDIATE SUPPORTS.

  MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS:
   24°ADJACENT TO OPENINGS NOT MORE THAN 67% OF WALL HEIGHT
   30°ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 65% OF WALL HEIGHT
   48° FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT

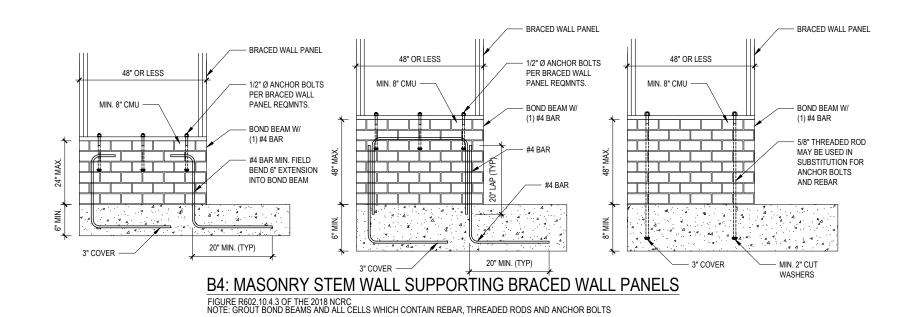
## 4 SHEATH INTERIOR AND EXTERIOR

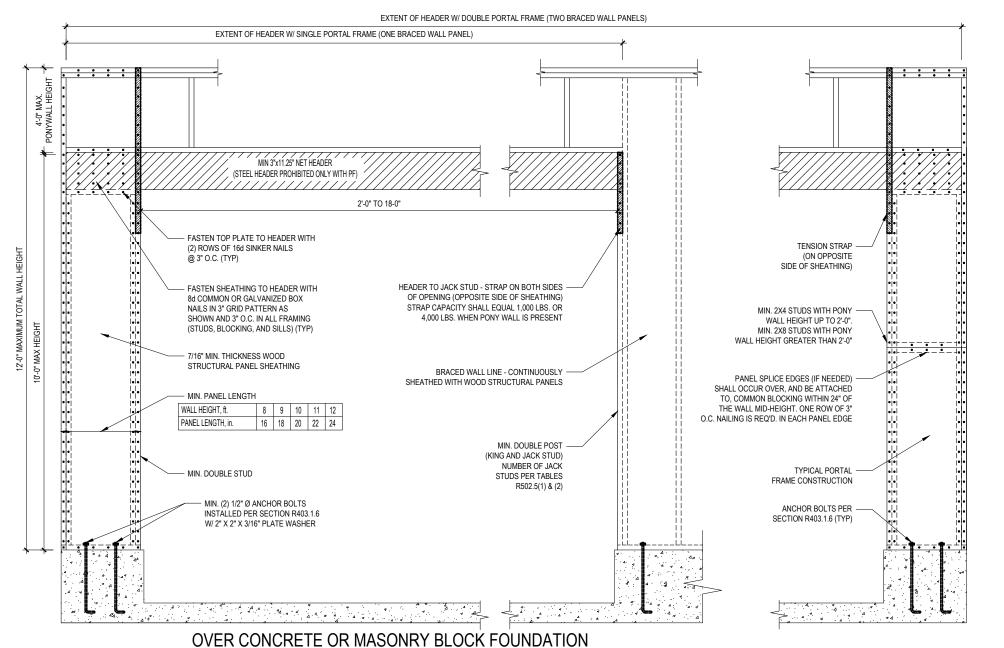
8. FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602-10.3 (4). IN LIEU OF A CORNER RETURN, EITHER A MINIMUM 49" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW

5 MINIMUM 800# HOLD-DOWN DEVICE

REQUIRED BRACED WALL PANEL CONNECTIONS								
			REQUIRED CONNECTION					
METHOD	MATERIAL	MIN. THICKNESS	@ PANEL EDGES	@ INTERMEDIATE SUPPORTS				
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.				
GB	GYPSUM BOARD	1/2"	5d COOLER NAIL** @ 7" O.C.	5d COOLER NAIL** @ 7" O.C.				
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.				

\*\*OR EQUIVALENT PER TABLE R702.3.5 **B3: BRACE WALL PANEL CONNECTIONS** NO SCALE





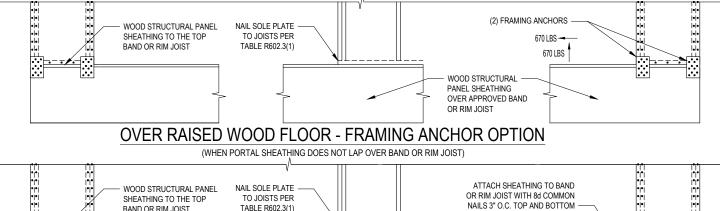
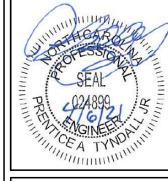


TABLE R602.3(1) BAND OR RIM JOIST WOOD STRUCTURAL OVER APPROVED BAND OR RIM JOIST OVER RAISED WOOD FLOOR - OVERLAP OPTION

B2: METHOD CS-PF: CONTINUOUSLY SHEATHED PORTAL FRAME

(WHEN PORTAL SHEATHING LAPS OVER BAND OR RIM JOIST)

eans, methods, techniques, sequences, rocedures or safety precaution. Any deviations or discrepancies on \* Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. \* Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed seconstruction.



TYNDALL ENGINEERING & DESIGN, P.A



RESIDENC

DAVID ]

2101-010078 04/06/2021 KFR/JTT DWG. Checked By

NOT TO SCALE REVISIONS

PAT

**Sheet Number**