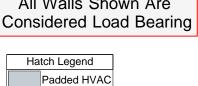
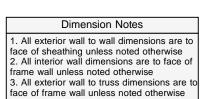


		Products			
PlotID	Length	Product	Plies	Net Qty	
BM1	6' 0"	2x10 SPF No.2	2	2	1
BM2	5' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	
GDH	22' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	
GDH2	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	
					-



Drop Beam



 \bigcirc

HUS26 USP 22

HUS410 USP 1

NA

Varies

16d/3-1/2" 16d/3-1/2"

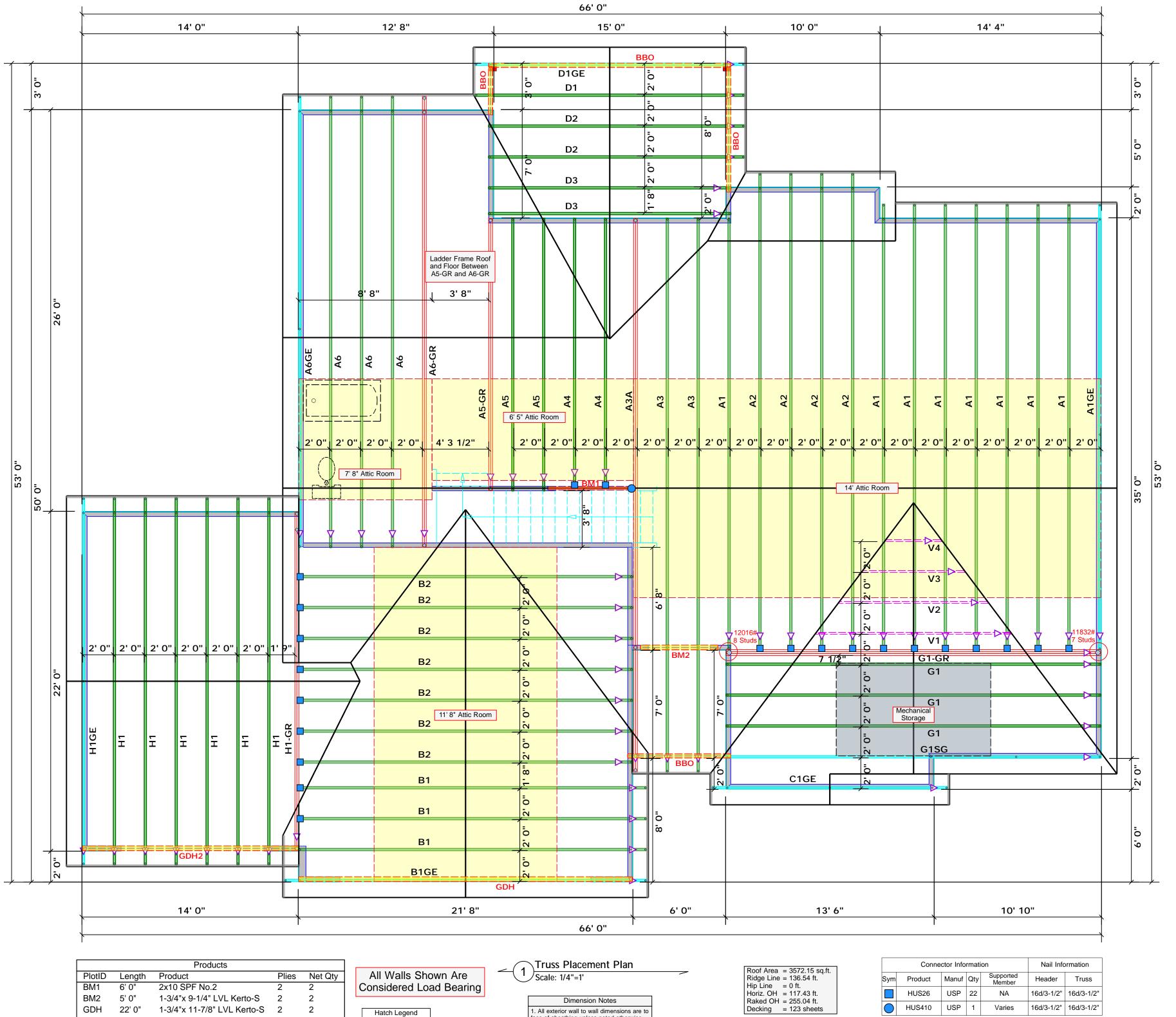
16d/3-1/2" 16d/3-1/2"

Horiz. OH = 117.43 ft.

Raked OH = 255.04 ft.

Decking = 123 sheets

Bearing deeme required but not profess suppor those of registe design exceed s ELOA	COORDING STORES OF THE STORE PROVIDED BY TH										
QP DP QP DP QP DP DP <thdp< th=""> DP DP DP<!--</td--></thdp<>											
Johnston	Lot 5 Patterson	Roof / 3GRF, 4BR	11/04/20	DRAWN BY David Landry	SALESMAN Lenny Norris						
COUNTY	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALESMAN						
Weaver Development Co. I nc.	Lot 5 Patterson	Halifax II	Seal Date	Quote #	J1120-5150						
BUI LDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #						
These the comport design a See indi- identified designer for the of support and colu- designer consult	russes and hents to b at the spec- ividual de d on the r is respon- overall stur- structure umns is t r. For gen BCSI-B1	e designe e incorpo cification sign shee placemen onsible fo ng of the i ructure. T a includin he respon- neral guid and BCS	ENT DIAG d as indiv rated into of the bu- ets for eau t drawing r tempora roof and f he desigr g headers sisibility of ance rega- l-B3 provi- online @	vidual bui o the build uilding descharts of the truss of the truss of the build of the truss of the build arding bra ded with	Iding ling signer. lesign ding em and uss walls, ling acing, the						



2 2 1-3/4"x 11-7/8" LVL Kerto-S 2 2 Hatch Legend GDH2 14' 0" 1-3/4"x 11-7/8" LVL Kerto-S 2 2 Padded HVAC Drop Beam

GDH

22' 0"

Dimension Notes 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

Roof Area = 3572.15 sq.ft.		Conne	ctor Info	rmati	on	Nail Info	ormation
Ridge Line = 136.54 ft. Hip Line = 0 ft.	Sym	Product	Manuf	Qty	Supported Member	Header	Truss
Horiz. OH = 117.43 ft. Raked OH = 255.04 ft.		HUS26	USP	22	NA	16d/3-1/2"	16d/3-1/2"
Decking = 123 sheets	\bigcirc	HUS410	USP	1	Varies	16d/3-1/2"	16d/3-1/2"

	RUS eilly R Fayet Phon	OF & SES load Ir teville e: (910	Te & FL & B odustr , N.C. 1) 864 864-4	OOF EAN 28309 -8787	ΛS ⁺k
deeme require attache Code r founda require but not profess suppor those s registe design exceed	d to comp ments. The equireme tion size d to supp greater t sional sha t system specified red desig	bly with the contra (derived nts) to d and numi port react than 1500 all be reta for any re in the atta n profess	an or equ ne prescri ctor shall from the etermine ber of wo ions grea 0#. A reg ined to de eaction th acched Tak sional sha m for all r	ptive Coc refer to t prescript the minim od studs ter than 3 istered de esign the lat exceed oles. A III be retai	le he ive num 000# esign Is
	_		d Lar		IDS
	(BASED	CN TABL	ES RECEDENT RECOVINED ACTENDER AC	C & (b)) O E A END	50 H3072414(3) 80925012500281 100 100 100 100 100 00 4
COUNTY Johnston	ADDRESS Lot 5 Patterson	MODEL Roof / 3GRF, 4BR	DATE REV. 11/04/20	DRAWN BY David Landry	SALESMAN Lenny Norris
Weaver Development Co. I nc.	Lot 5 Patterson	Halifax II	Seal Date		J1120-5150
BUI LDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #
These to comport design a See indi identifie designe permany for the co support and colu designe consult	russes and ents to b at the spec- ividual de d on the r is respon- ent bracin overall str structure umns is ti r. For ger BCSI-B1	e designe e incorpo cification sign shee placemen onsible fo ucture. T includin he respon- neral guid and BCS	IENT DIAG d as indiv rated into of the bu- ets for eau t drawing r tempora roof and f he desigr g headers sibility of ance rega- neB3 provio online @	vidual bui o the build uilding de ch truss o . The build rry and floor syste o of the tr s, beams, f the build arding bra ded with	Iding signer. lesign Iding em and uss walls, ting acing, the

Tisl	*		Client:	Weaver D	evelopment			ate:	11/4/2020			Pa	age 1 of
	Design		Project: Address:					put by: bb Name:	David Landry Lot 5 Patters				
	-						P	roject #:	J1120-5150				
BM1 S	S-P-F #2	2.0	00" X	10.00	0" 2-F	Ply - P/	ASSE) ^{Le}	vel: Level				
			1										_
•	•	-	•	•	•		•					M	
	- 700			-	atter		-					MÅ	9
							SPF					<u> </u>	<u> </u>
1 SPF			5'6"			2	SPF					,	
ł			5'6"										
ember Inf							1			b (Uplift)			
Type: Plies:	Girder 2		Applica Desigr	ation: Method:	Floor ASD		Brg 1	Live 0	Dead 919	Snow 919	Wind 0	Const 0	
Moisture Cond	-			g Code:	IBC/IRC 2015		2	0	919	919	0	0	
Deflection LL: Deflection TL:	480 360		Load S Deck:	Sharing:	No Not Checked								
mportance:	Normal		Dook										
Temperature:	Temp <= 100°F						Desite						
							Bearing: Bearing		Cap B	eact D/L lb	Total Ld. Ca	se Ld. Co	mb
							1 - SPF	-	41%	919 / 919	1837 L	D+S	JIID.
nalysis Res	sults						2 - SPF	3.500"	41%	919 / 919	1837 L	D+S	
Analysis		cation	Allowed	Capaci	ty Comb.	Case	1						
Moment	2122 ft-lb	2'9"	3946 ft-Ib		4%) D+S	L							
Unbraced Shear	2122 ft-lb 1169 lb	2'9" 1'	3654 ft-lb 2872 lb		8%) D+S 1%) D+S	L							
	0.018 (L/3452)	2'9"	0.126 (L/48			L							
	0.035 (L/1726)		0.168 (L/36	, ,	,	L							
esign Note				, ,			1						
1 Fasten all pl	ies using 2 rows of 10	d Box na	ails (.128x3")	at 12" o.c. l	Maximum end d	istance not	1						
to exceed 6' 2 Refer to last	page of calculations for	or faster	ners required	l for specifie	d loads.								
	designed to be suppor			ige only.									
	ust be supported equa at bearings.	ny by ar	i plies.										
	ed at bearings.	cinglo	nh <i>u</i> width										
6 Bottom brac	Load Type	single		Trib Width	n Side	Dead 0.9	Live	1 Snow	1.15 Wir	nd 1.6 Cons	t. 1.25 Comm	ents	
•					_								
6 Bottom brac 7 Lateral slend	Uniform				Тор	334 PLF	0 PL	F 334	1 PLF	0 PLF	0 PLF A4		

BM1 S-P-F #2	Project: Address: 2.000'' X 10 ,	ver Development .000'' 2-Ply • •	Date Input Job I Proje • • • 2 SPF	t by: David Landry Name: Lot 5 Patterson	Page 2 of 8
BM1 S-P-F #2	Address: 2.000" X 10. • • 5'6"	• •	Job I Proje	Name: Lot 5 Patterson ect #: J1120-5150 Level: Level	9 1/4
BM1 S-P-F #2	2.000" X 10. • •	• •	Proje	ect #: J1120-5150	9 1/4
• •	• • 5'6"	• •	- PASSED	Level: Level	9 1/4
• •	• • 5'6"	• •	•		9 1/4
• •	• • 5'6"	• •	•	 11/2"	9 1/4
• •	• 5'6"		• • 2 SPF	×11/2"	9 1/4
• •	• 5'6"		• 	 11/2"	9 1/4
• •	• 5'6"		• 	→ <mark> </mark> <1 1/2"	9 1/4
	• 5'6"		• 	<u>→</u> <1 1/2"	9 1/4
	• 5'6"		• 	× 11/2"	9 1/4
	• 5'6"		• 	→ <mark>→</mark> <1 1/2"	9 1/4
• •	• 5'6"		• 	→ <mark>→</mark> <1 1/2"	9 1/4
• •	• 5'6"		• 	× 11/2"	9 1/4
• •	• 5'6"		2 SPF	× 11/2"	9 1/4
	5'6"	••••	2 SPF		9 1/4
	5'6"	•••	2 SPF		
	5'6"		2 SPF		
1 SPF			2 SPF	Λ	
					1 1
1			1		
	5'6"		L		1 3"
ł			1		
•			I		
Multi-Ply Analysis					
	- (10 d D 1- (100		·		
Fasten all plies using 2 rows		x3") at 12" o.c Max	imum end distanc	ce not to exceed 6"	
	0.0 %				
load	0.0 PLF				
	157.4 PLF 78.7 lb.				
	IV				
	1 1/2"				
	3"				
Load Combination	•				
	1.00				
				Manufacturer Info	Comtech, Inc.
				-	Comtech, Inc. 1001 S. Rei∎y Road, Suite #639 Fayetteville, NC
					ISA
					28314 910-864-TRUS
		This doois	n is valid until 2/26/2023		соттесн

D-5150 evel RNED Ib (Up Dead S	-156)	Į Į	↓ 3 1/2"
	-156)	×	¥\ _!
	-156)	×	¥\ _!
		-	3 1/2"
		I	3 1/2
	ριιπ()		
	-	Wind C 0 0	Const 0 0
•			Ld. Comb. D+S
28% 1526 / 1	510 3036	L [D+S
Wind 1.6(0 PLF		Comments A3	
	28% 1526 / 1 28% 1526 / 1 28% 1526 / 1 Wind 1.6 0 PLF	28% 1526 / 1510 3036 28% 1526 / 1510 3036 28% 1526 / 1510 3036 Wind 1.6 Const. 1.25 0 PLF 0 PLF 0 PLF 0 PLF	28% 1526 / 1510 3036 L I 28% 1526 / 1510 3036 L I 28% 1526 / 1510 3036 L I Wind 1.6 Const. 1.25 Comments 0 PLF 0 PLF A3 Contech Inc. 1001 S. Relly, Read, Sur 1001 S. Relly, Read, Sur 1001 S. Relly, Read, Sur

	-	Client: We	aver Development	Date:	11/4/2020	Page 4 of 8
1	a Destau	Project:		Input by:	David Landry	
<u>+</u> _	isDesign	Address:		Job Nam Project #	e: Lot 5 Patterson : J1120-5150	
BM2	Korto SIN	/L 1.750" X	0 250" 2 D		Level: Level	
DIVIZ	Nerto-S L		9.250 Z =P	Iy - FASSED		
•	•	•	• •			\overline{M}
				1 1/2"		9 1/
•	•	•	• • –	<u> </u>		
1 SP	PF End Grain	401	2 SPF End Grain			
		4'8"				3 1/2"
1		4'8"		I		
Multi-Ply	-					
Fasten a ll Capacity		of 10d Box nails (.12 0.0 %	3x3") at 12" o.c Ma>	kimum end distance n	ot to exceed 6"	
_oad		0.0 PLF				
∕ield Limit per ∕ield Limit per		163.7 PLF 81.9 lb.				
ield Mode		IV				
Edge Distance Min. End Dista		1 1/2" 3"				
oad Combina		3				
Duration Factor	or	1.00				
structural adequad design criteria a responsibility of th ensure the com application, and to Lumber 1. Dry service cor	red Designs is responsible only of cy of this component based on and loadings shown. It is ne customer and/or the contractor ponent suitability of the interview verify the dimensions and loads. Inditions. unless noted otherwise	the 1. LVL beams must not be cut or d the 2. Refer to manufacturer's 1 to regarding installation required approvals 3. Damaged Beams must not be u 4. Design assumes top edge is lat 5. Provide lateral support at bes	ponding iffed	fs provide proper drainage to prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	Contach Inc. 1001 S. Reily Road, Suite #639 Faynteville, NC US314 910-964-TRUS
	treated with fire retardant or corros	lateral displacement and rotation		gn is valid until 2/26/2023		
√ersion 20.20.04	44 Powered by iStruct™					CSD 🎬

ie lie	Design	Pr	ient: Weav oject: Idress:	er Development				4/2020 <i>v</i> id Landry 5 Pottorson		Page 5 o
	Cargin	AC	uress:					5 Patterson 20-5150		
DH H	Kerto-S LV	1 1.7	750" X 1 [⁄]	1.875" 3	-Plv - P					
							2 2			
		3								
				1						
•		•	• •	and	• •			• • •	•••	M \uparrow
		2 HOLE	114							
1 SPF End	l Grain							2 SPF E	End Grain	, , ,
<i>.</i>				18'10)"				,r	5 1/4"
<u> </u>				18'10)''				r	
ember Inf	formation					Reactions	UNPATT	ERNED lb (Uplift))	
ype:	Girder		Application:	Floor		Brg	Live	Dead Snow	Wind	Const
lies:	3		Design Metho		15	1	0	2720 188	0	0
loisture Cond eflection LL:	480		Building Code Load Sharing:	IBC/IRC 20 ⁻ Yes	15	2	0	2720 188	0	0
eflection TL:			Deck:	Not Checke	d					
nportance:	Normal									
emperature:	Temp <= 100°	F								
						Bearings				
						Bearing L	-	Cap. React D/L lb 18% 2720 / 188	Total Ld. Cas	
						1-SPF 3 End	.500	18% 2720 / 188	2908 L	D+S
alysis Re	sults					Grain				5.0
nalysis		Location Al	-	pacity Comb.	Case	2-SPF 3 End	500"	18% 2720 / 188	2908 L	D+S
loment	12191 ft-lb			36 (44%) D	Uniform	Grain				
Inbraced	13035 ft-Ib	9'5" 13	056 ft-lb 0.99 (10)		L					
hear	2368 lb	1'2 5/8" 11	970 lb 0.19	98 (20%) D	Uniform					
L Defl inch	0.037 (L/6029)	9'5 1/16" 0.4	459 (L/480) 0.08	80 (8%) S	L					
L Defl inch	0.565 (L/390)	9'5 1/16" 0.0	612 (L/360) 0.92	20 (92%) D+S	L					
sign Not										
Fasten all p to exceed 6	lies using 2 rows of '	0d Box nails	(.128x3") at 12"	o.c. Maximum end	distance not					
	t page of calculations									
	designed to be supp nust be supported eq		• •							
	e laterally braced at a									
	ced at bearings.	on cinglo nhu	width							
)	derness ratio based Load Type		ocation Trib V	/idth Side	Dead 0.9	Live 1	Snow 1.1	5 Wind 1.6 Const	. 1.25 Comme	ents
	Uniform			Тор	60 PLF	0 PLF	0 PLI		0 PLF Wall	
	Tie-In	0-0-0 to 1	18-10-0 1-0-0	Тор	20 PSF	0 PSF	20 PSI		0 PSF Roof	
	Uniform			Тор	195 PLF	0 PLF	0 PLI		0 PLF B1GE	
	Self Weight				14 PLF				_	
otes		chemicals		6. For	flat roofs provide p	oper drainage to pr	event Manuf	acturer Info	Comtech, Inc. 1001 S. Reilly Ro	ad Suite #630
culated Structured	Designs is responsible only of of this component based on	the Handling a	& Installation s must not be cut or drille	pon	iding	F	Metsä	Wood erritt 7 Building, 2nd Floor	Fayetteville, NC USA	
sign criteria and sponsibility of the c	loadings shown It is ustomer and/or the contractor	the 2 Refer to to regarding	manufacturer's proc installation requiren	uct information tents, multi-ply			Norwa	k, CT 06851	28314 910-864-TRUS	
plication, and to veri	ent suitability of the intene fy the dimensions and loads.	approvals	details, beam strength v	alues, and code			www.n	022-5850 netsawood.com/us		
	ons, unless noted otherwise	 Design ass Provide la 	Beams must not be used sumes top edge is lateral teral support at bearing	y restrained			ICC-E	S: ESR-3633	Ice	тесн
	ted with fire retardant or corros	avel bearing allow	acement and rotation		is design is va l id					

2			Client: Project:	Weaver Developn	nent	Date	e: ut by:	11/4/2020 David Landry	Page 6 of 8
1	isDesign		Address:			Job	Name:	Lot 5 Patterson	
GDH	Kerto-S	LVL	1.750"	X 11.875"	3-Plv		ject #:	J1120-5150 evel: Level	
<u>[</u>	• • •	• •	• •	• •	• •	• •	•	• • • •	
	· · ·		• •	• •	• •	• •	•		
	End Grain				40140			2 SPF I	
					18'10" 18'10"				1 15 1/4"
Multi-Ply	-								
Fasten a ll 6"	plies using 2	rows of	10d Box nails	: (.128x3") at 12'	" o.c Nail fro	om both sides.	Maxir	num end distance not	to exceed
Capacity Load		0.0 0.0	% PLF						
Yield Limit pe Yield Limit pe			3.7 PLF						
Yield Mode Edge Distand		IV 1 1/	/2"						
Min. End Dist Load Combin	tance	3"							
Duration Fac		1.00	0						
			ala ami I -		6 Ear 4-4	nide seese de la l		Manufacturer Info	Comtech, Inc.
structural adequa	tured Designs is responsible acy of this component be	ased on the	chemicals Handling & Installa 1. LVL beams must not b	e cut or dri ll ed	ponding	ovide proper drainage to pr	even.	Metsä Wood 301 Merritt 7 Building, 2nd Floor	1001 S. Reilly Road, Suite #639 Fayetteville, NC USA
responsibility of ensure the cor	and loadings shown the customer and/or the o mponent suitability of th o verify the dimensions and	It is the contractor to ne intended	 Refer to manufact regarding installatio 	urer's product information n requirements, multi-ply m strength values, and code				Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	28314 910-864-TRUS
Lumber 1. Dry service co	onditions, unless noted othe	erwise	 Damaged Beams mus Design assumes top e Provide lateral suppo 	dge is laterally restrained rt at bearing points to avoid				ICC-ES: ESR-3633	соттесн
	044 Powered by iStru		lateral displacement ar	iu rotation	This design is	valid until 2/26/2023			CSD

	-	C	lient: V	Veaver Dev	velopment		Da	ate:	11/4/2020)			Page 7 of
1	Ph. 4 - 7		roject:					put by:	David La				, age , e.
IS	Design	А	ddress:					b Name:	Lot 5 Pat J1120-51				
GDH2	Kerto-S LV	/1 /	750"	X 11 9	275" 2	-Plv -			evel: Level	50			
ODIIZ		-	1.7.50	A 110	575 2	i iy -							
	<u></u>						<u></u>						
		2											
		2											$\neg \neg$
•	·		•	•	•	•		•	•	•	•		M
-	- This	-			atron			-	Nin I	-			11 7.
•	· · · · ·			•			-	•	•		•		
1 SPF E	ind Grain								2	SPF End Gra	ain I		
ř – – – – – – – – – – – – – – – – – – –					9'10"								3 1/2"
1					9'10"						\rightarrow		
Member Int	formation						Reaction	s UNP	ATTERN	ED lb (Up	lift)		
Type: Plies:	Girder 2		Applicatio		Floor		Brg	Live	De		now	Wind	Const
Moisture Cond			Design M Building (ASD IBC/IRC 2015		1	0 0	16 16		313 313	0 0	0 0
Deflection LL:	480		Load Sha	ring:	No		-				0.0	Ū	0
Deflection TL:	: 360 Normal		Deck:		Not Checked								
Importance: Temperature:	Temp <= 100°F												
	·						Bearings						
							Bearing	-		React D/L		otal Ld. Case	Ld. Comb.
							1 - SPF End	3.500"	28%	5 1653 / 13	13 Z	966 L	D+S
Analysis Re							Grain 2 - SPF	3 500"	28%	5 1653 / 13	13 2	966 L	D+S
Analysis Moment	Actual Lc 6627 ft-lb	ocation A 4'11" 2	llowed 2897 ft-lb	Capacity 0.289 (29		Case L	End	5.500	207	5 1000710	10 2	300 L	DIG
Unbraced	6627 ft-lb		857 ft-lb	0.209 (29		L	Grain						
Shear	2231 lb 8	8'7 3/8" 1	0197 l b	0.219 (22	%) D+S	L							
LL Defl inch			.234 (L/480)	,	,	L							
	0.126 (L/895)	4'11" 0	.312 (L/360)	0.400 (40	%) D+S	L	ļ						
Design Not 1 Fasten all p	tes plies using 2 rows of 100	d Box nails	s (.128x3") at	12" o.c. M	aximum end di	stance not	1						
to exceed 6			. ,										
3 Girders are	e designed to be suppor	ted on the	bottom edge	•	10003.								
4 Top loads n 5 Top braced	nust be supported equa l at bearings.	ally by all p	lies.										
6 Bottom bra	ced at bearings.												
7 Lateral slen	nderness ratio based on Load Type			rib Width	Side	Dead 0.9	Live 1	1 Snow	v 1.15	Wind 1.6 C	Const. 1.	25 Commen	ts
1	Uniform				Тор	60 PLF	0 PLF		0 PLF	0 PLF	0 P		
2	Uniform				Тор	267 PLF	0 PLF	- 26	7 PLF	0 PLF	0 P	LF G1	
	Self Weight					9 PLF							
								<u>.</u>				-	
Notes	Designs is responsible only of the	chemical Handling			6. For fla pondin		roper drainage to	prevent	<mark>Manufacture</mark> Metsä Wood	er Info		Comtech, Inc. 1001 S. Reilly Road Fayetteville, NC	d, Suite #639
structural adequacy of design criteria and	of this component based on the d loadings shown. It is the	1. LVL bean 2. Refer t	ns must not be cut (o manufacturer's	or dri ll ed product inf	ormation			3		Building, 2nd F 06851	loor	USA 28314 910-864-TRUS	
ensure the compon	customer and/or the contractor to nent suitability of the intended rify the dimensions and loads.	regarding fastening approval) installation n details, beam str s	equirements, ength values, a	multi-ply			(800) 622-58 www.metsaw	50		910-864-1RUS	
Lumber 1. Dry service conditi	ions, unless noted otherwise	 Damaged Design a Provide 	, d Beams must not b ssumes top edge is ateral support at	aterally restrain	ned to avoid				CC-ES: ESF			lan	TOOU
2. LVL not to be treat	ated with fire retardant or corrosive	lateral dis	splacement and rota	ation		design is va l id	until 2/26/2023	3				Con	птесн
ersion 20.20.044	Powered by iStruct™											CSD 🗱	

isDesign	Client: Project: Address:	Weaver Development	Job	te: 11/4/2020 ut by: David Landry o Name: Lot 5 Patterson oject #: J1120-5150	Page 8 of 8
GDH2 Kerto-S L	VL 1.750	" X 11.875"	2-Ply - PASSE		
•••	• •	• •	• •	• • •	
SPF End Grain	• •	• •	• •	• • • - 2 SPF End Grain	
		9'10"			3 1/2"
l ł		9'10"			
Multi-Ply Analysis					
Load Yield Limit per Foot Yield Limit per Fastener Yield Mode Edge Distance Min. End Distance Load Combination	of 10d Box nails 0.0 % 0.0 PLF 163.7 PLF 81.9 Jb. IV 1 1/2" 3" 1.00		For fla t roofs provide proper drainage to p	Manufacturar Info	Contech, Inc. 1001 S. Relly Road, Suite #639
Notes Calculated Structured Designs is responsible only of structural adequacy of this component based on design criteria and badings shown. It is	the Handling & Installat	tion ^p cut or drilled	or flat roofs provide proper drainage to p onding	Metsä Wood 301 Merritt 7 Building, 2nd Floor	Comtech, Inc. 1001 S. Relly Road, Suite #639 Fayetteville, NC USA 28314
responsibility of the customer and/or the contractor ensure the component suitability of the intenc application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise	to regarding installation regarding installation fastening details, bean approvals 3. Damaged Beams must 4. Design assumes top ed	irer's product information requirements, multi-ply n strength values, and code not be used ge is laterally restrained : at bearing points to avoid		Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	910-864-TRUS
2. LVL not to be treated with fire retardant or corros Version 20.20.044 Powered by iStruct™	ive lateral displacement and		This design is valid until 2/26/2023		СОП



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J1120-5150 Lot 5 Patterson

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15058952 thru E15058980

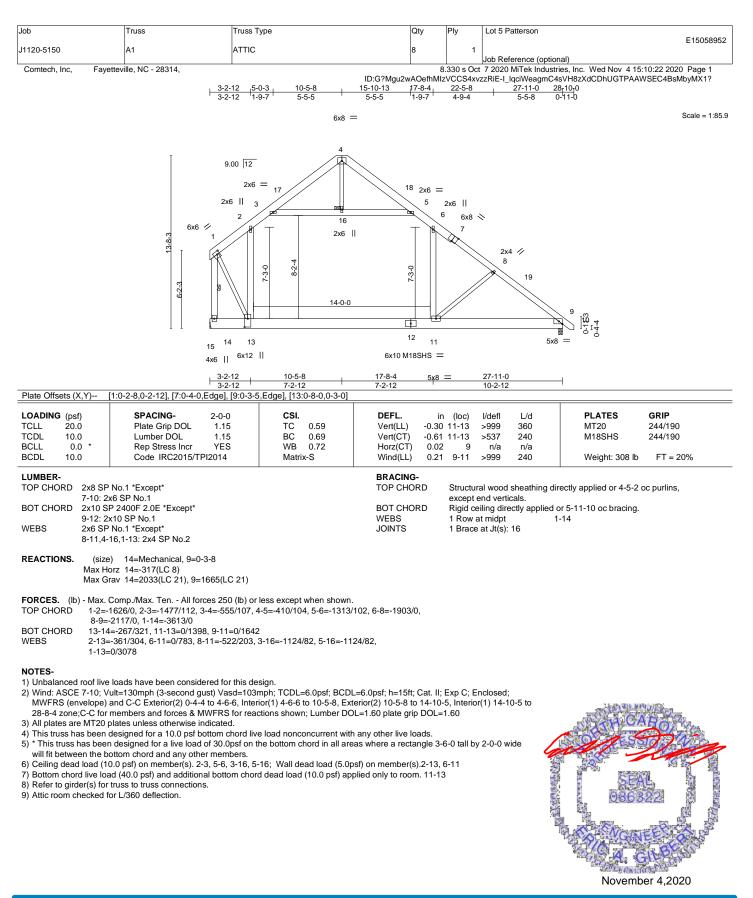
My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



November 4,2020

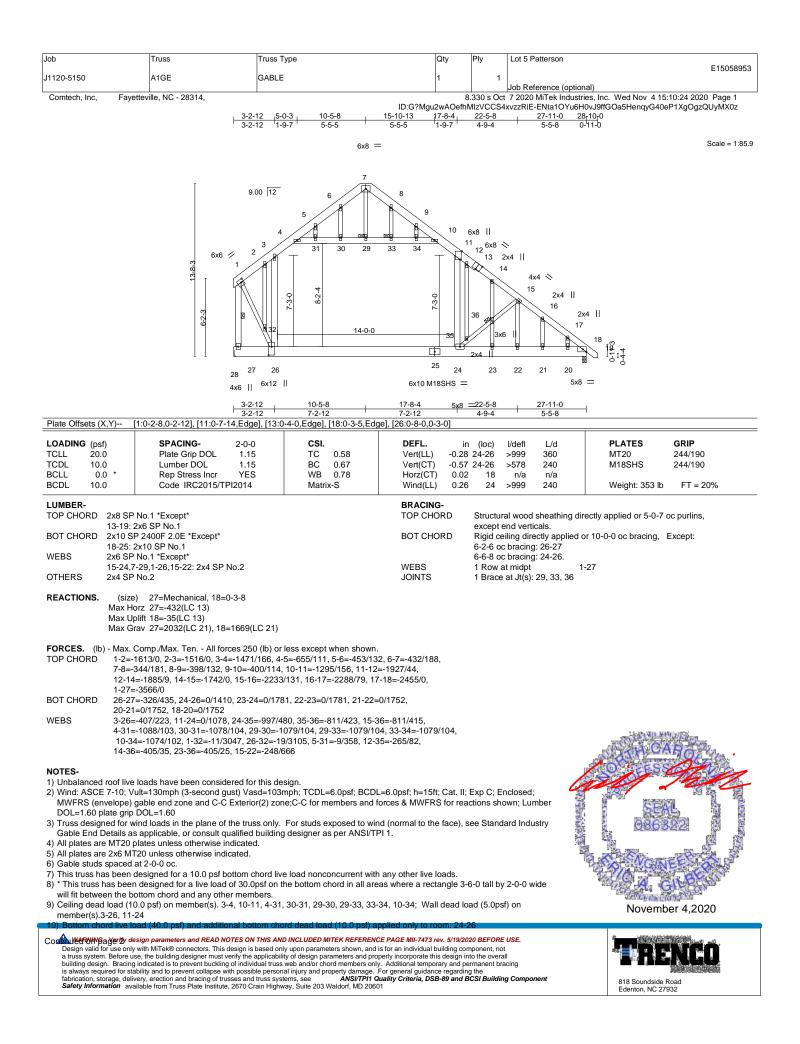
Gilbert, Eric **IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MITek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



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818 Soundside Road Edenton, NC 27932



lob	Truss	Truss Type	Qty	Ply	Lot 5 Patterson		
J1120-5150	A1GE	GABLE	1	1	E15058953		
		-			Job Reference (optional)		
Comtech, Inc, Fayetter	rille, NC - 28314,				7 2020 MiTek Industries, Inc. Wed Nov 4 15:10:24 2020 Page 2		
		ID:G?Mgu2wAOefhMlzVCCS4xvzzRiE-ENta1OYu6H0vJ9ffGOa5HenqyG40eP1XgOgzQUyMX0z					

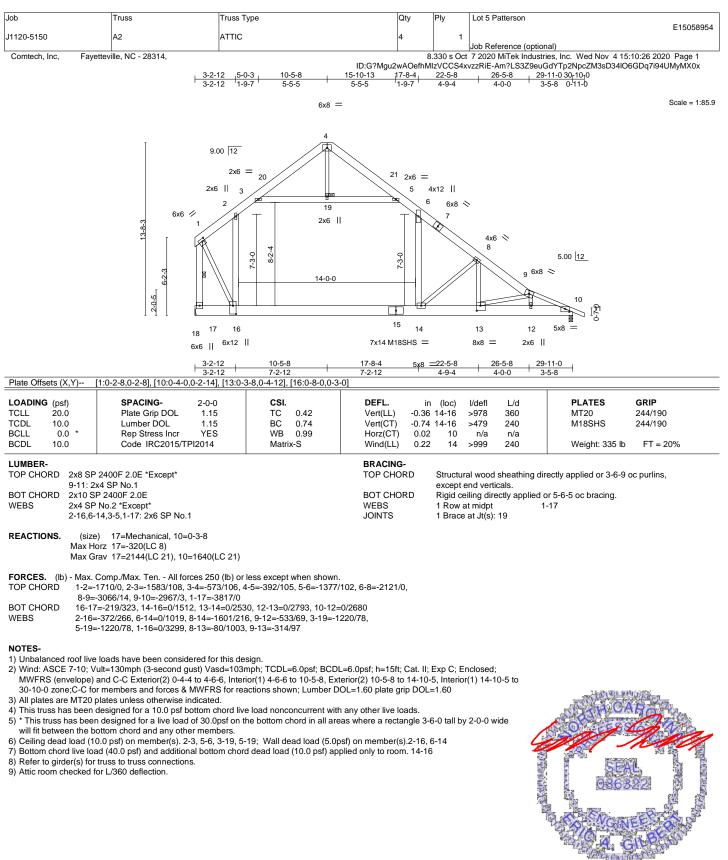
NOTES-

11) Refer to girder(s) for truss to truss connections.
 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18.

13) Attic room checked for L/360 deflection.

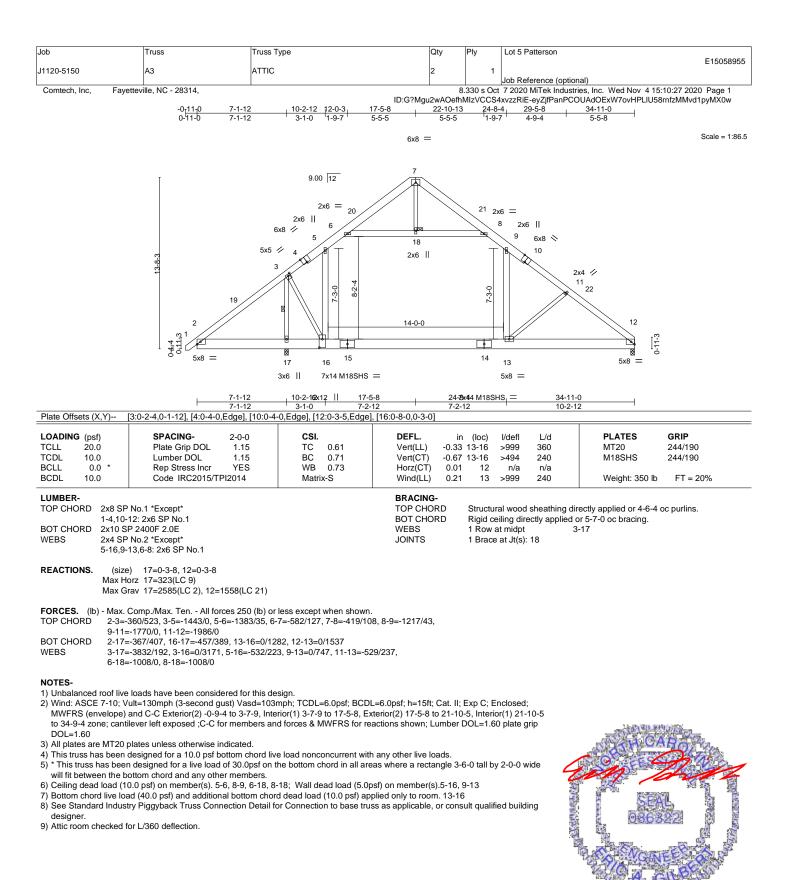
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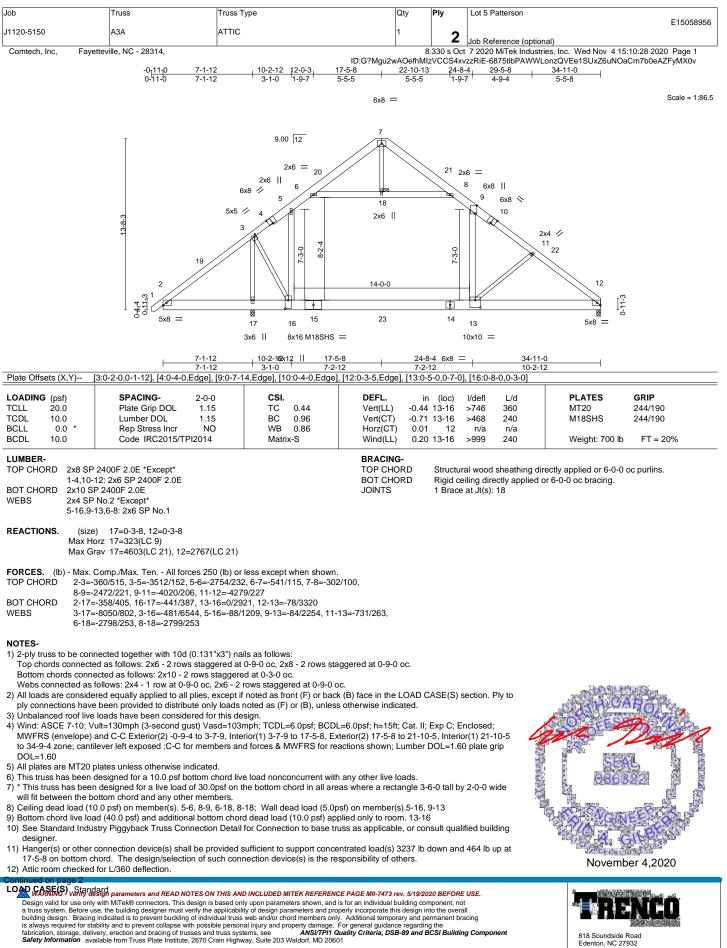
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18 Soundside Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 5 Patterson
J1120-5150	A3A	ATTIC	1	2	E15058956
				Z	Job Reference (optional)
Comtech, Inc, Fayette	rille, NC - 28314,				7 2020 MiTek Industries, Inc. Wed Nov 4 15:10:28 2020 Page 2

ID:G?Mgu2wAOefhMIzVCCS4xvzzRiE-6875tlbPAWWLonzQVEe1SUxZ6uNOaCm7b0eAZFyMX0v

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

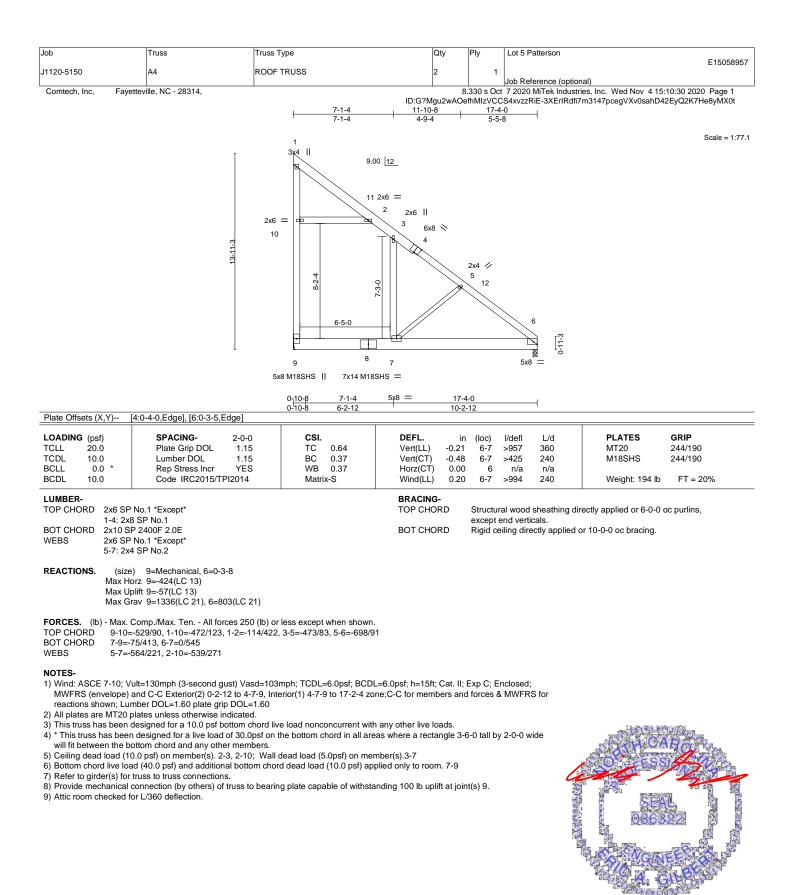
Vert: 1-5=-60, 5-6=-80, 6-7=-60, 7-8=-60, 8-9=-80, 9-12=-60, 2-16=-20, 13-16=-40, 12-13=-20, 6-8=-20

Drag: 5-16=-10, 9-13=-10

Concentrated Loads (lb) Vert: 23=-1837(F)

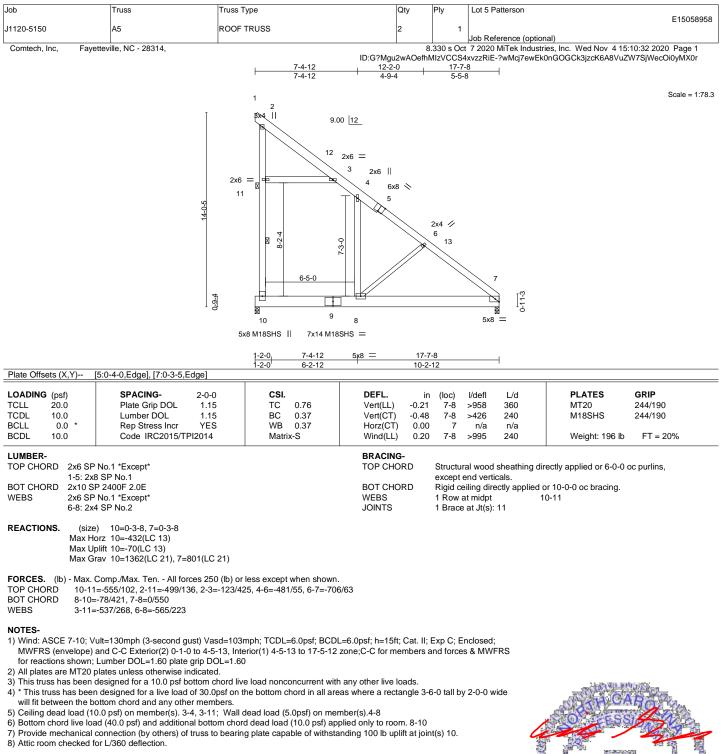
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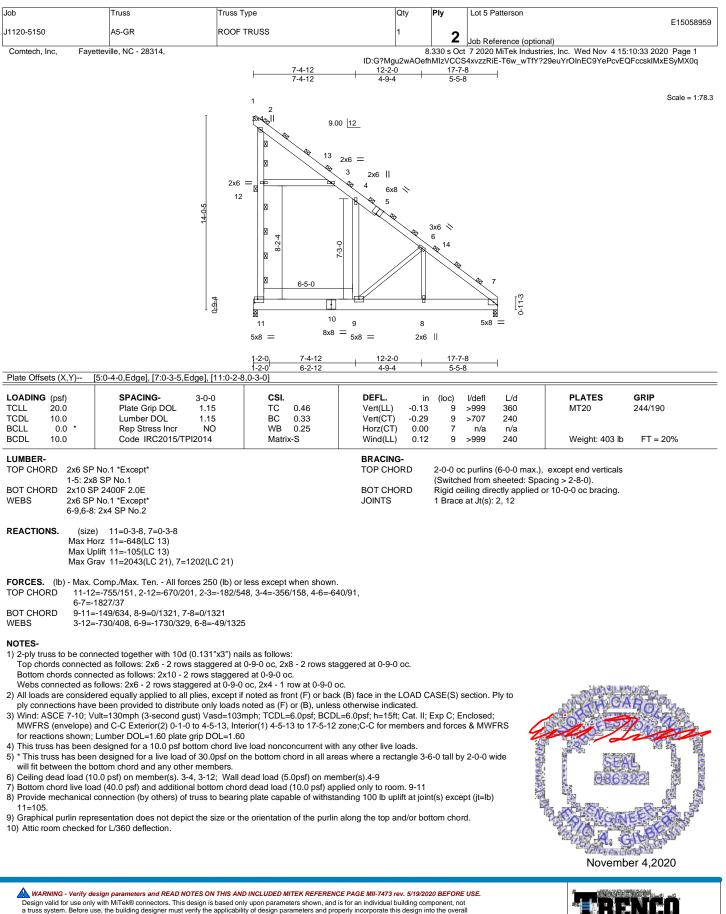






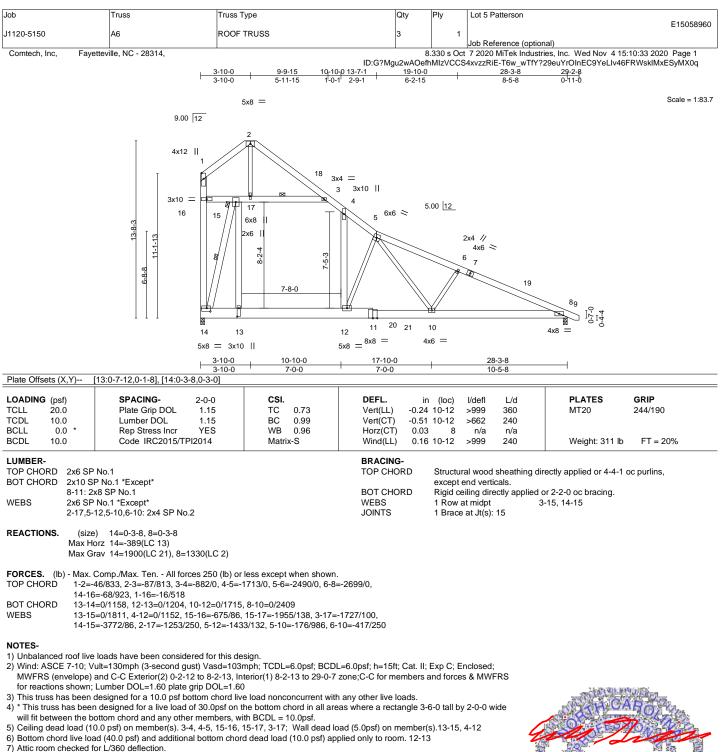
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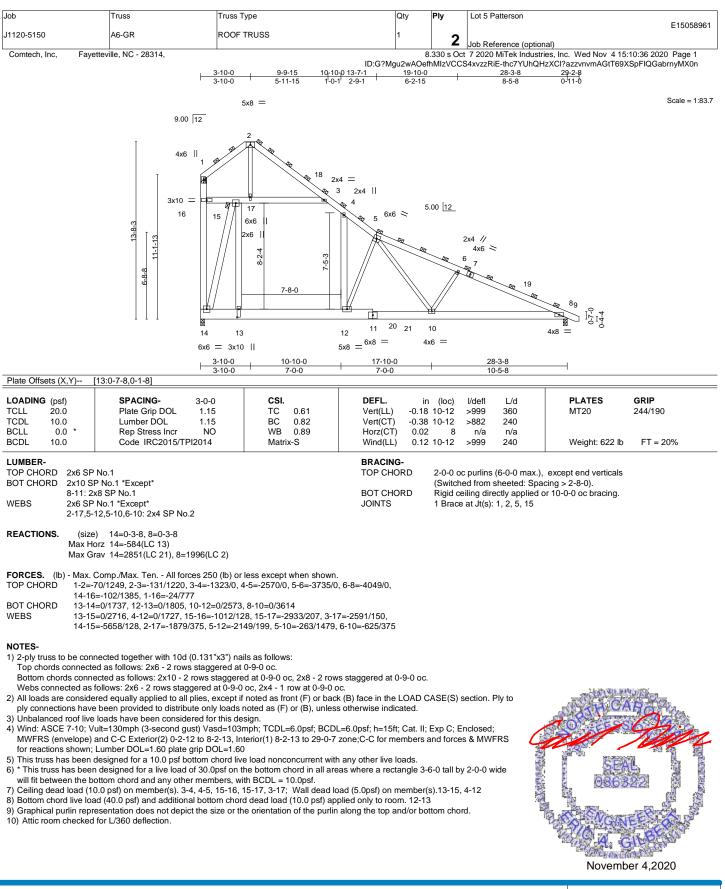
818 Soundside Road Edenton, NC 27932





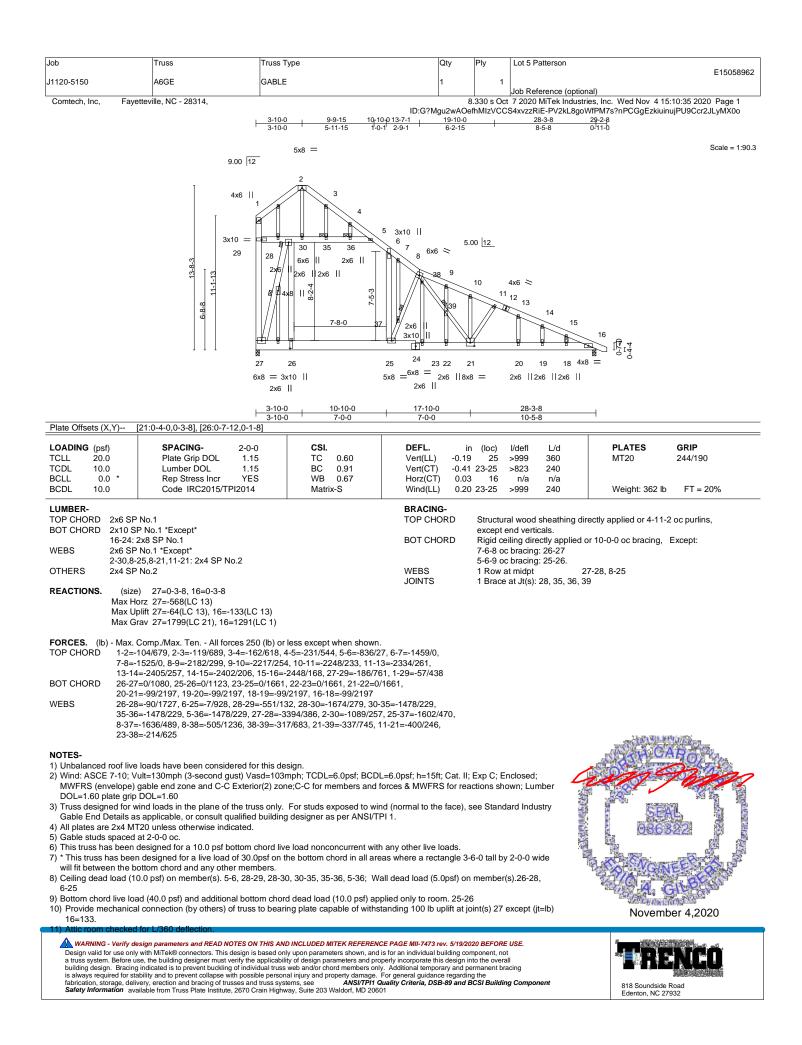
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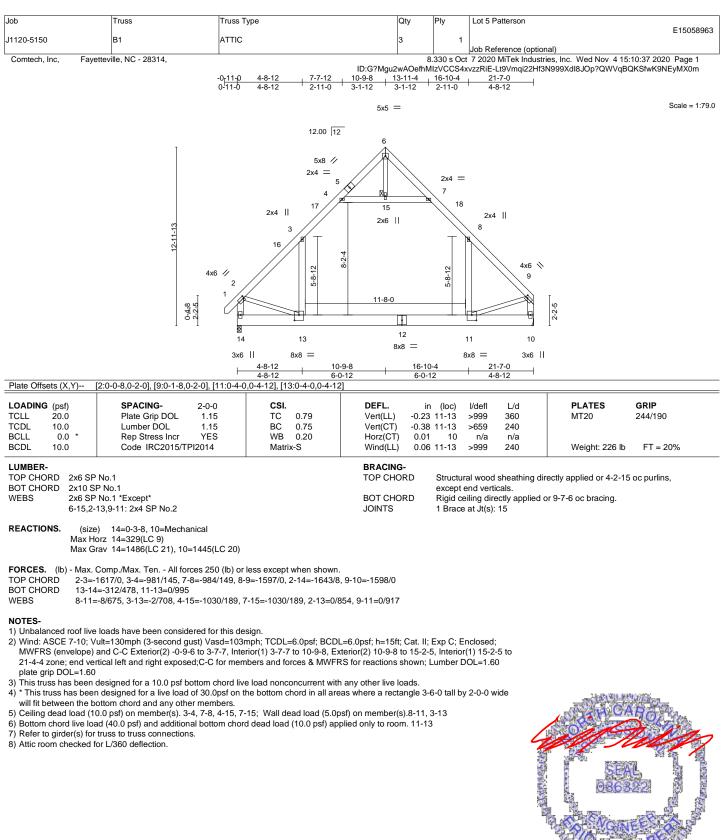




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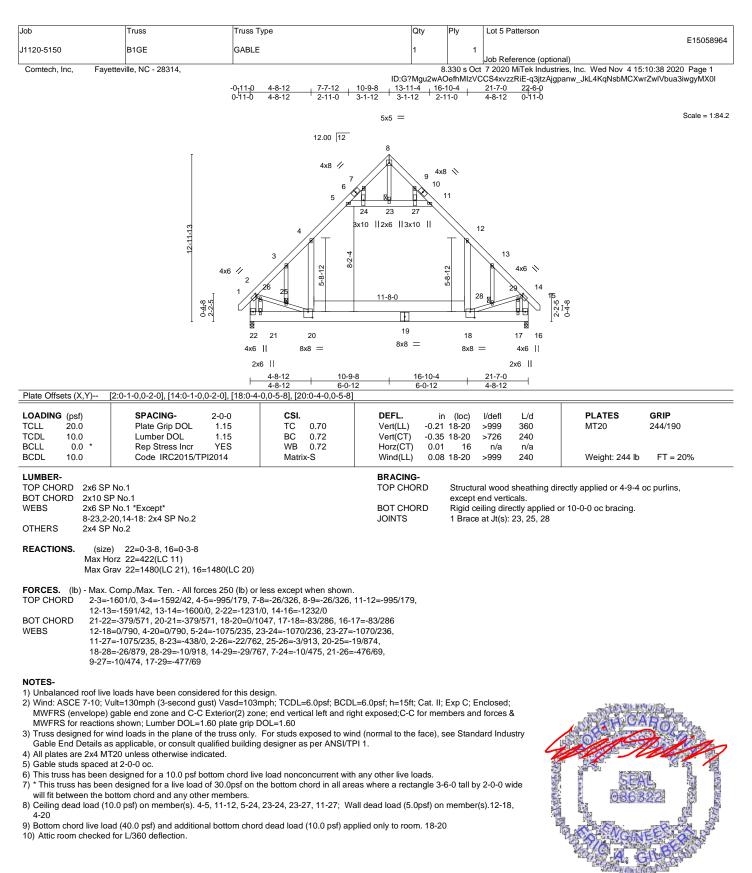


FOND CONT

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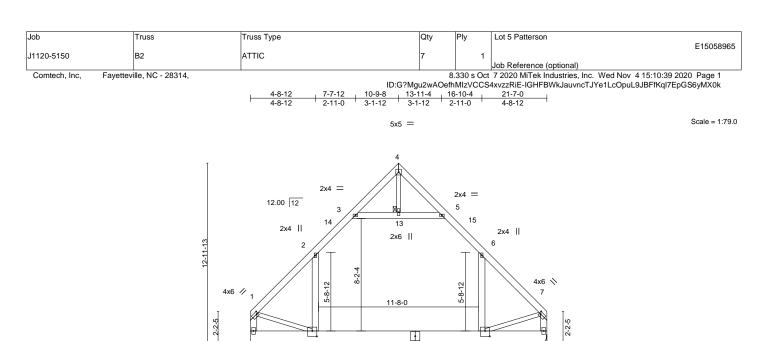
November 4,2020

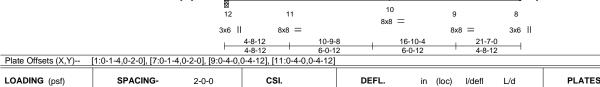




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818 Soundside Road Edenton, NC 27932





LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.78	DEFL. in Vert(LL) -0.23	(loc) l/d 9-11 >9		PLATES MT20	GRIP 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.75	()	9-11 >6			
BCLL 0.0 *	Rep Stress Incr YES	WB 0.20	Horz(CT) 0.01	8 r	n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.07	9-11 >9	99 240	Weight: 223 lb	FT = 20%
LUMBER-			BRACING-				
TOP CHORD 2x6 SP			TOP CHORD	Structural w	ood sheathing di	rectly applied or 4-2-11	l oc purlins,
BOT CHORD 2x10 SF				except end			
	No.1 *Except*		BOT CHORD			or 9-6-8 oc bracing.	
4-13,1-	11,7-9: 2x4 SP No.2		JOINTS	1 Brace at .	Jt(s): 13		
FORCES. (lb) - Max. (TOP CHORD 1-2=-7 BOT CHORD 11-12	av 12=1446(LC 21), 8=1446(LC 20) Comp./Max. Ten All forces 250 (lb) oi 1600/0, 2-3=-984/147, 5-6=-984/147, 6 =-303/406, 9-11=0/997 5/678, 2-11=-7/678, 3-13=-1036/187, 5	-7=-1600/0, 1-12=-1600/0, 7					
0.0-0			, 1 0 0, 0 10				
NOTES-							
,	loads have been considered for this de ult=130mph (3-second gust) Vasd=103	0	6 Opef: b=15ft: Cat II:		eed.		
	and C-C Exterior(2) 0-2-12 to 4-8-12, Ir						
(I /	vertical left and right exposed;C-C for m	()	()	, ,	,		
plate grip DOL=1.60							
3) This truss has been of	designed for a 10.0 psf bottom chord liv	e load nonconcurrent with a	any other live loads.				

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 2-3, 5-6, 3-13, 5-13; Wall dead load (5.0psf) on member(s).6-9, 2-11

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 9-11

7) Refer to girder(s) for truss to truss connections.

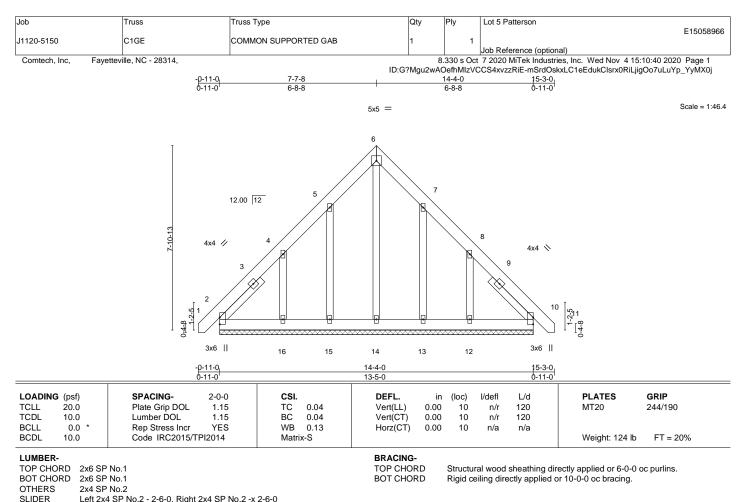
8) Attic room checked for L/360 deflection.



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REACTIONS. All bearings 13-5-0.

Max Horz 2=-224(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 15, 13 except 16=-256(LC 12), 12=-251(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 10, 14, 15, 13 except 16=270(LC 19), 12=265(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 4-16=-280/263, 8-12=-280/260

NOTES-

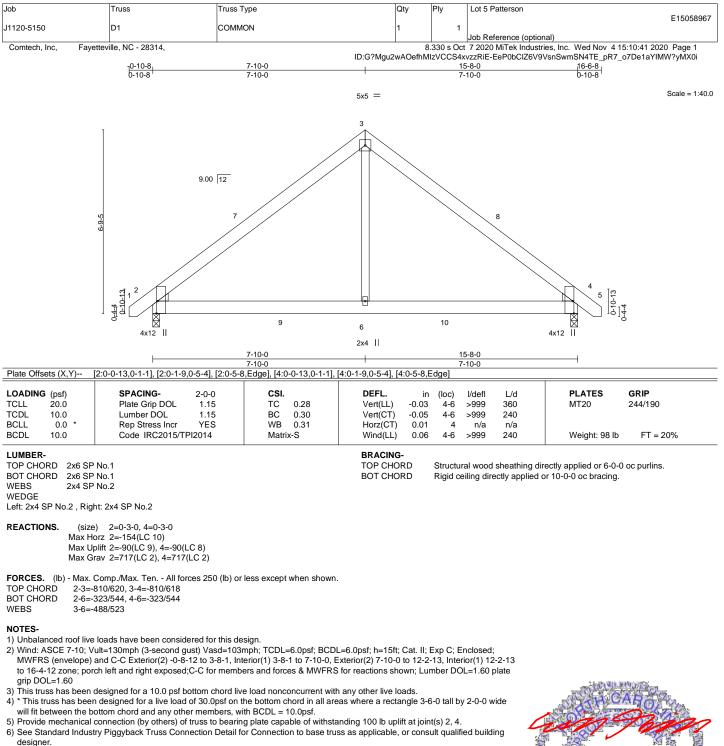
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 15, 13 except (it=lb) 16=256, 12=251.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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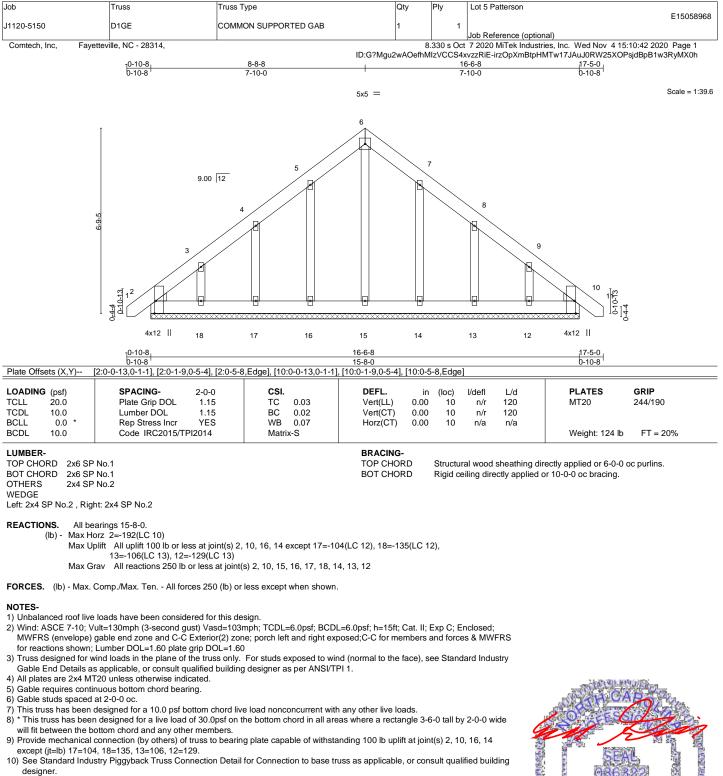




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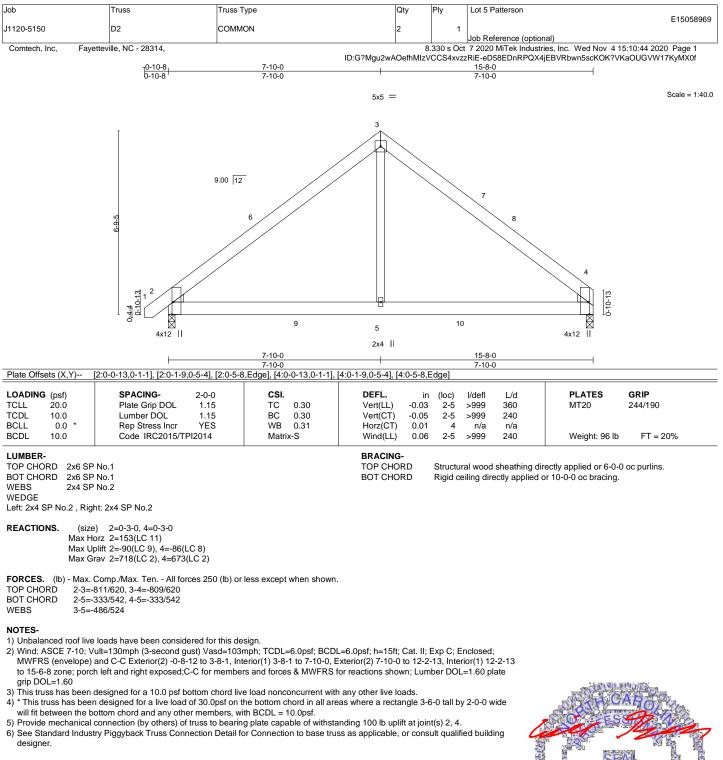
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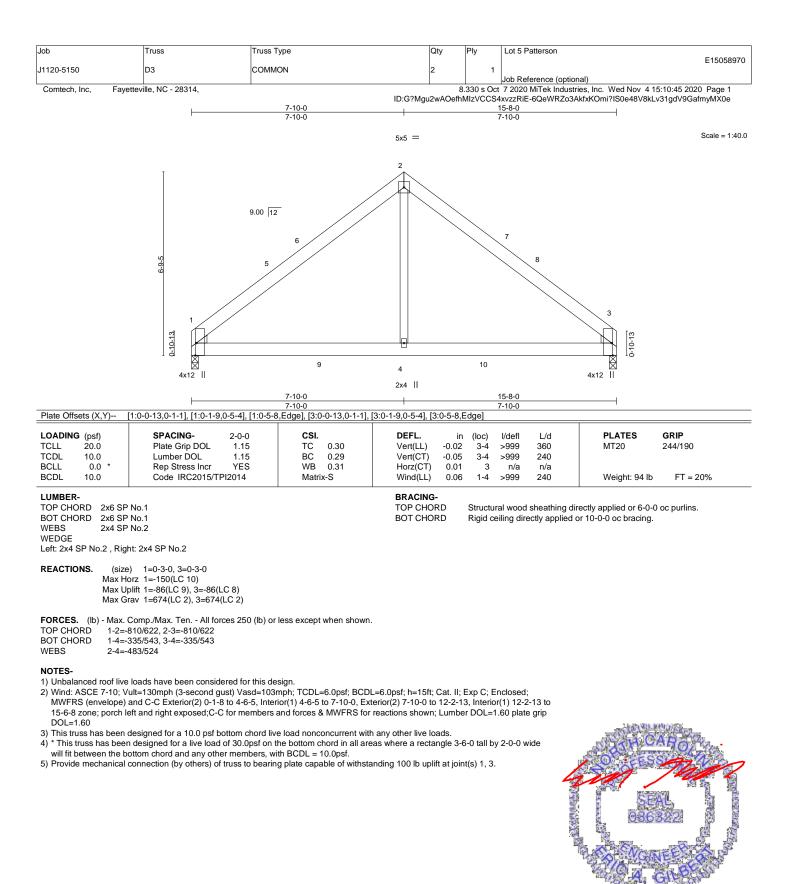




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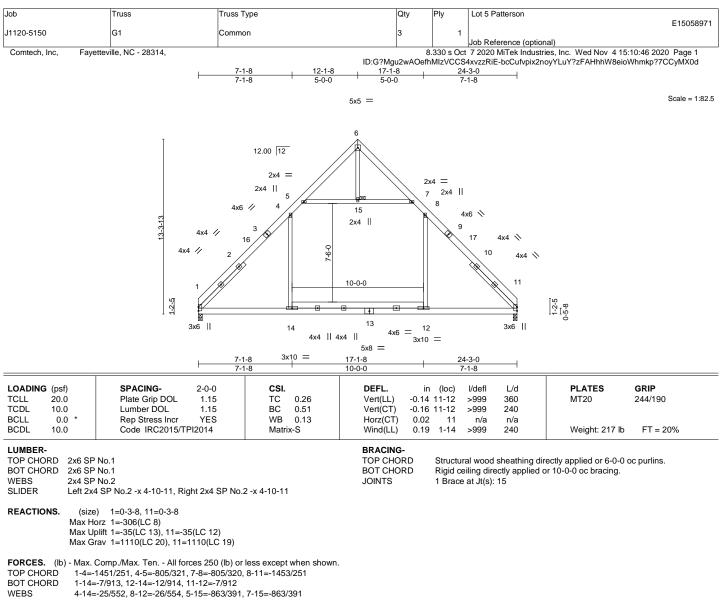


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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0 to 4-4-13, Interior(1) 4-4-13 to 12-1-8, Exterior(2) 12-1-8 to 16-6-7, Interior(1) 16-6-7 to 24-3-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

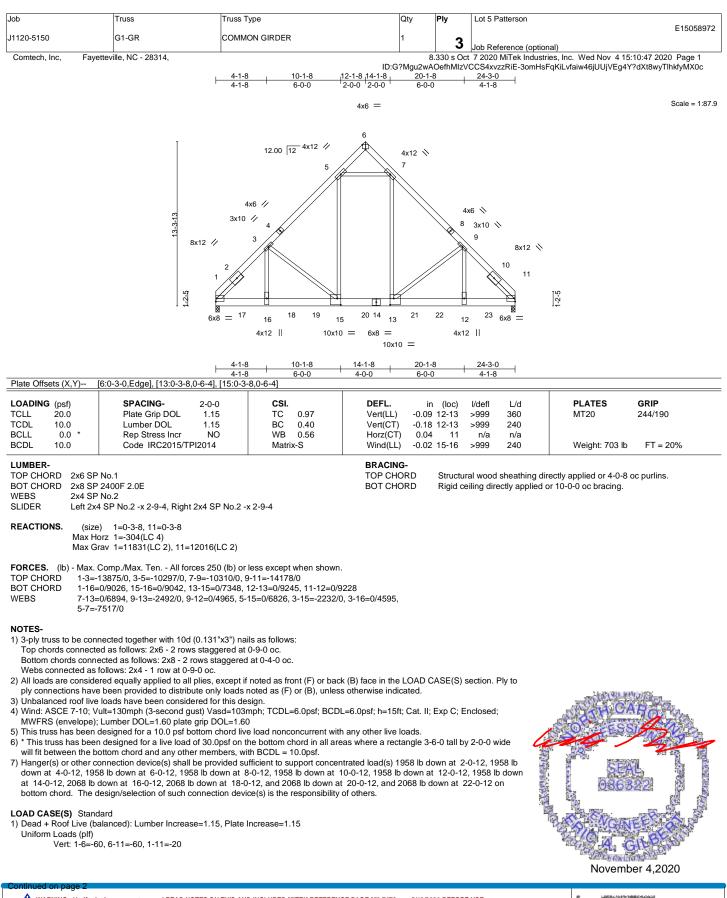
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11.



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lob	Truss	Truss Type	Qty	Ply	Lot 5 Patterson
					E15058972
1120-5150	G1-GR	COMMON GIRDER	1	3	Job Reference (optional)
				· · · ·	JOD Reference (optional)
Comtech, Inc, Fayettevi	lle, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Wed Nov 4 15:10:48 2020 Page 2

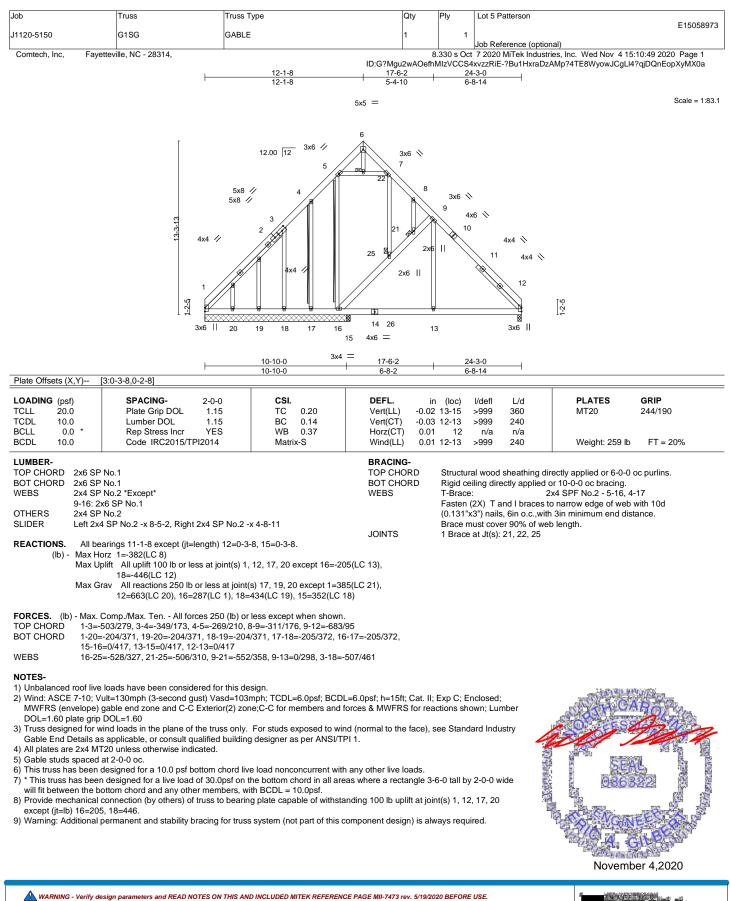
ID:G?Mgu2wAOefhMlzVCCS4xvzzRiE-X?Kf3bryTf1VBrVGgQ?jFimrqyLsGKO3B7UEG5yMX0b

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 13=-1547(B) 12=-1645(B) 15=-1547(B) 16=-1547(B) 17=-1547(B) 18=-1547(B) 19=-1547(B) 20=-1547(B) 21=-1645(B) 22=-1645(B) 23=-1645(B) 23=-1645(B)

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818 Soundside Road Edenton, NC 27932

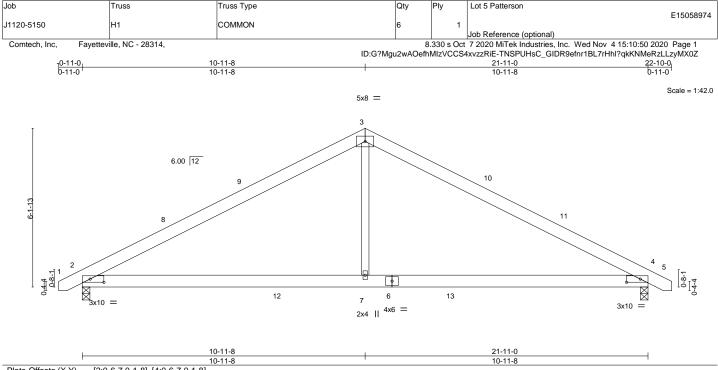


Plate Off	Plate Offsets (X,Y) [2:0-6-7,0-1-8], [4:0-6-7,0-1-8]									
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc	c) l/defl	L/d	PLATES GRIP		
TCLL	20.0	Plate Grip DOL 1.15	TC 0.63	Vert(LL) -	-0.08 4-	7 >999	360	MT20 244/190		
TCDL	10.0	Lumber DOL 1.15	BC 0.50	Vert(CT) -	-0.18 4-	7 >999	240			
BCLL	0.0 *	Rep Stress Incr YES	WB 0.15	Horz(CT)	0.02	4 n/a	n/a			
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL)	0.06 2-	7 >999	240	Weight: 122 lb FT = 20%		

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No 2

REACTIONS. (size) 2=0-3-8, 4=0-3-8 Max Horz 2=76(LC 11) Max Uplift 2=-64(LC 12), 4=-64(LC 13) Max Grav 2=953(LC 2), 4=953(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1379/292, 3-4=-1379/292

BOT CHORD 2-7=-93/1123, 4-7=-93/1123

WEBS 3-7=0/655

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-2 to 3-7-11, Interior(1) 3-7-11 to 10-11-8, Exterior(2) 10-11-8 to 15-4-5, Interior(1) 15-4-5 to 22-8-2 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

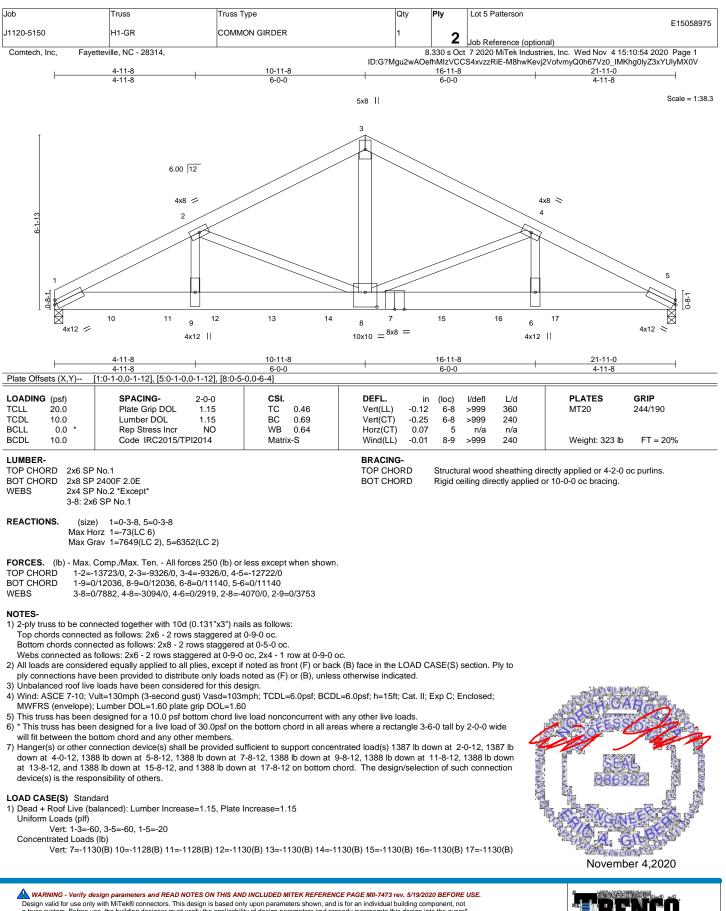


Structural wood sheathing directly applied or 5-6-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

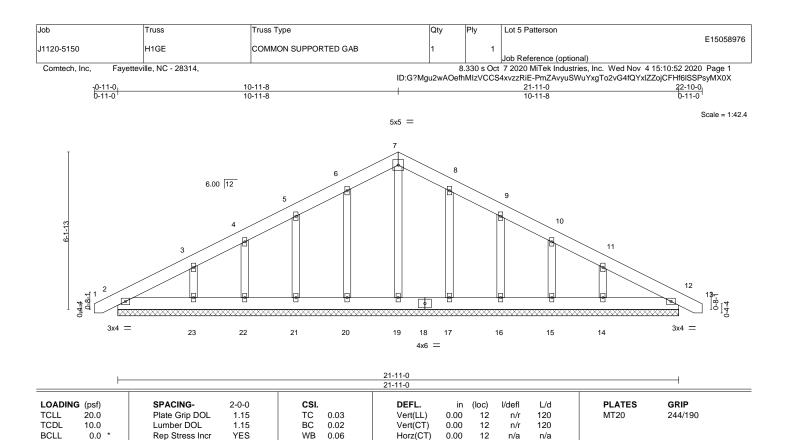
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818 Soundside Road Edenton, NC 27932



BRACING-

TOP CHORD

BOT CHORD

BCDL

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1OTHERS2x4 SP No.2

10.0

REACTIONS. All bearings 21-11-0.

(lb) - Max Horz 2=119(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 20, 21, 22, 17, 16, 15, 12 except 23=-109(LC 12), 14=-106(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 19, 20, 21, 22, 23, 17, 16, 15, 14, 12

Matrix-S

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Code IRC2015/TPI2014

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 21, 22, 17, 16, 15, 12 except (jt=lb) 23=109, 14=106.



Weight: 154 lb

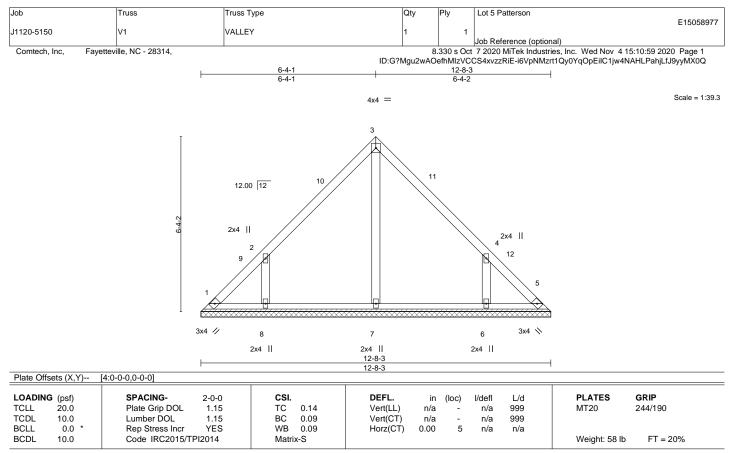
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

FT = 20%

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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 12-8-3.

(lb) - Max Horz 1=-144(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-161(LC 12), 6=-161(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=342(LC 19), 6=342(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-355/291, 4-6=-355/291

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 6-4-1, Exterior(2) 6-4-1 to 10-8-14, Interior(1) 10-8-14 to 12-3-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This trust has been designed for a few particular into any ford into any ford in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
 1) * This trust has been designed for a few particular into any ford in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=161, 6=161.

6) Non Standard bearing condition. Review required.

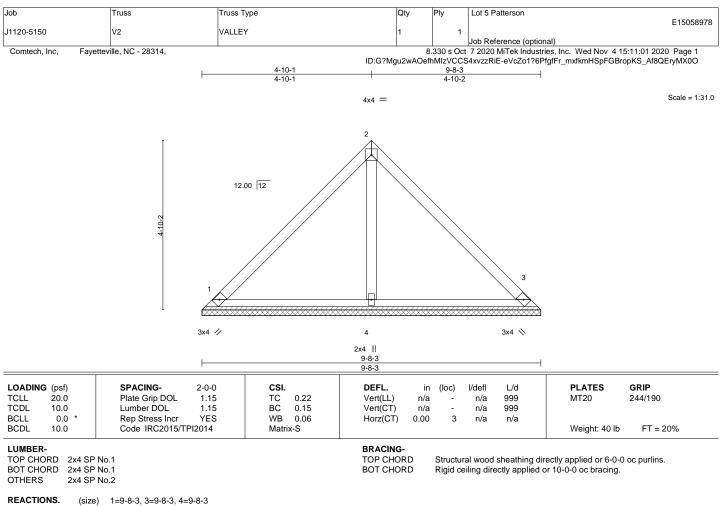


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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Max Horz 1=-108(LC 8)

Max Uplift 1=-27(LC 13), 3=-27(LC 13)

Max Grav 1=204(LC 1), 3=204(LC 1), 4=311(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

- grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

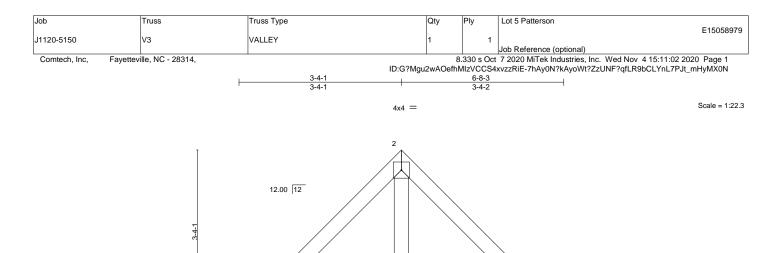
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



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3x4 🚿

З

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

	6-8-3											
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	тс	0.15	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.07	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-P						Weight: 27 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=6-8-3, 3=6-8-3, 4=6-8-3

Max Horz 1=-72(LC 8)

Max Uplift 1=-26(LC 13), 3=-26(LC 13) Max Grav 1=146(LC 1), 3=146(LC 1), 4=187(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

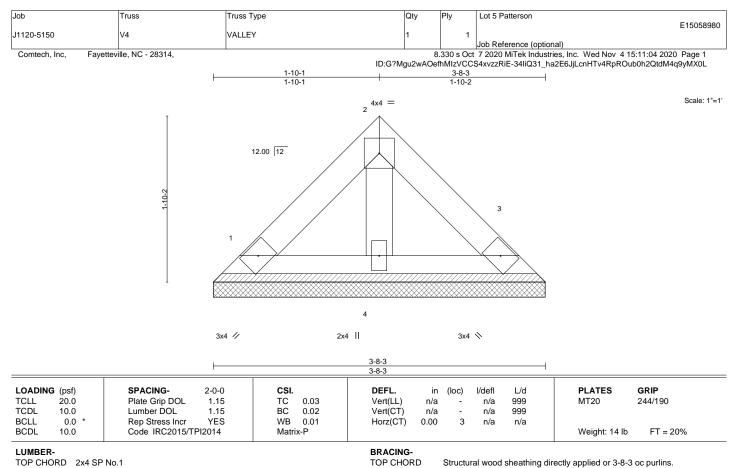
6) Non Standard bearing condition. Review required.





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BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD

2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 OTHERS

REACTIONS. (size) 1=3-8-3, 3=3-8-3, 4=3-8-3 Max Horz 1=-36(LC 8)

Max Uplift 1=-13(LC 13), 3=-13(LC 13)

Max Grav 1=72(LC 1), 3=73(LC 1), 4=93(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

- grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



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