

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 20090082 22 Mitchell MAnor - Hampton El C

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carter Components (Sanford, NC)).

Pages or sheets covered by this seal: E14880759 thru E14880786

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



September 18,2020

# Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	A01	Hip Girder	1	2	Job Reference (optional)	E14880759

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:12 ID:TbwUYn0TWMEt8CRb?hJNUEzsBTZ-Mock Me



Scale = 1:65.1

#### Plate Offsets (X, Y): [4:0-5-12,0-2-8], [9:0-5-12,0-2-8]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 NO IRC2015/TPI2014	CSI TC BC WB Matrix-S	0.46 0.70 0.20	<b>DEFL</b> Vert(LL) Vert(CT) Horz(CT)	in 0.26 -0.44 0.12	(loc) 17 17-19 11	l/defl >999 >959 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 367 lb	<b>GRIP</b> 244/190 • FT = 20	) )%	
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD REACTIONS FORCES TOP CHORD	2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 *Except 19-4,19-6,15-6,15-9:: Structural wood shea 5-11-1 oc purlins, ex 2-0-0 oc purlins (4-10 Rigid ceiling directly bracing. (size) 2=0-5-8, 1 Max Uplift 2=-888 (L0 Max Grav 2=2235 (L (lb) - Maximum Comp Tension 1-2=0/14, 2-22=-451 22-23=-4470/1544, 2 3-24=-4444/1543, 3- 4-25=-4330/1496, 4- 26-27=-5802/1989, 28	* 2x4 SP No.2 athing directly applie cept applied or 9-1-10 oc 1=0-5-8 C 6), 11=-887 (LC 6) C 15), 11=2234 (LC pression/Maximum 3/1544, 3-24=-4453/1544, 25=-4353/1492, 26=-5803/1989, i-29=-5802/1989,	BOT CHORD d or WEBS 15) NOTES 1) 2-ply truss i (0.131"x3") Top chords oc. Bottom cho	2-39=-1318/3977, 2 21-40=-1318/3977, 2 21-40=-1318/3977, 2 0-41=-1245/3978, 1 9-42=-1245/3978, 1 9-42=-1245/3978, 1 9-43=-1986/6331, 1 7-46=-1986/6331, 1 7-46=-1986/6331, 1 5-48=-1986/6331, 1 9-50=-1244/3976, 1 4-52=-1317/3975, 3 2-1=-1/195, 3-20=- 4-19=-66/198, 6-11 8-15=-564/336, 9-12 10-14=-75/153, 10-7 o be connected toge nails as follows: connected as follow:	1-39=-1 20-40= 41-42= 19-43= 18-44= 17-45= 46-47= 16-48= 15-49= 11-52= 76/155 19=-565 7=0/299 5=-661/ 13=-1/1 ether wither wither s: 2x4 - lows: 2)	318/3977, -1318/3977, -1245/3978, -1986/6331, -1986/6331, -1986/6331, -1986/6331, -1244/3976, -1244/3976, -1244/3976, -1317/3975, -1317/3975, -4-20=0/294, 5/336, 0, 6-15=-669/15 2065, 9-14=0/2 95 h 10d 1 row at 0-9-0 c4 - 1 row at	99, ( 294, -	<ul> <li>4) Win Vas II; E Inte</li> <li>11-4</li> <li>24-(</li> <li>(2):</li> <li>end</li> <li>DOI</li> <li>5) TCL</li> <li>DOI</li> <li>sno</li> <li>Plati</li> <li>Ct=</li> <li>6) This</li> <li>exc:</li> <li>7) This</li> <li>loaa</li> <li>ove</li> <li>8) Prov</li> <li>9) This</li> </ul>	d: ASCE d=103m xp C; Er for (1) 2 I-13, Inte -3 to 33 33-10-0 t vertical ==1.00 L: ASCE =1.25 P w); Pf=2t e DOL= <sup>-1</sup> t.10, Lu= truss ha of 12.0 changs n vide ade- truss ha of 12.0 changs n	7-10; ph; TC closee -1-0 tc rior (1 -0-0, 1 right e 7-10 (1 -0-0, 1 right e 7-10 (1 -0-0, 1 right e 7-10 (1 -0-0, 1 right e -0-0, 0 (1) -0-0, 0 (	Vult=130mph ( DL=0.0psf; BC 2; C-C Exterior ( ) 2-11-0, Exterior ) 2-11-0, Exterior ) 2-11-0, Exterior ) 11-4-13 to 24- nterior (1) 33-0- 10-0; cantilever 1 xposed; Lumbe ; Pr=20.0 psf (rc OL=1.00); Pg=2 f (flat roof snow Category II; Exp n checked for u n designed for g 2.00 times flat i ncurrent with ot drainage to pre- n designed for a n concurrent with	3-second g DL=0.0psf; (2) -0-11-0 (2) -0-11-0 (2) -0-11-0 (2) -0-11-0 (2) -0-11-0 (3) -0-11-0 (4) -0-10 (5) -0-10-0 (5) -0-0 (5) -0-0	yust) ; h=0ft; Cat. to 2-1-0, 0 to ior (2) -0, Exterior ht exposed ; 0 plate grip d: Lumber round DOL=1.15 ly Exp.; w load only, min roof live f 15.4 psf on uds. ponding. bottom r live loads.	-
	29-30=-5802/1989, 6 6-31=-5801/1989, 31 7-32=-5801/1989, 7- 8-33=-5800/1988, 33 9-34=-5801/1988, 9- 10-35=-4350/1491, 1 36-37=-4451/1543, 3 11-38=-4511/1543, 1	-30=-5802/1989, -32=-5801/1989, 8=-5801/1989, I-34=-5800/1988, 35=-4327/1495, 0-36=-4442/1542, 7-38=-4468/1543, 1-12=0/14	0-9-0 oc. Web conne 2) All loads ar except if no CASE(S) se provided to unless othe 3) This truss F only, excep	cted as follows: 2x4 e considered equally ted as front (F) or ba ection. Ply to ply coni distribute only loads rwise indicated. as been checked for as neted.	- 1 row applied lock (B) f nections noted a	at 0-9-0 oc. d to all plies, ace in the LOA s have been as (F) or (B), n roof live load	AD		Winnin	The second second	OR TEST SE/ 0363	AL 322	A	

Continued on page 2 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-1473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANS/TP/1 Quality Criteria</u>, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	A01	Hip Girder	1	2	Job Reference (optional)	E14880759

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- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 888 lb uplift at joint 2 and 887 lb uplift at joint 11.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

### LOAD CASE(S) Standard

Dead + Snow (balanced): Lumber Increase=1.15, Plate 1) Increase=1.00

Uniform Loads (lb/ft)

Vert: 1-4=-51, 4-9=-61, 9-12=-51, 2-11=-20

Concentrated Loads (lb)

- Vert: 4=-27 (B), 7=-22 (B), 9=-27 (B), 20=-15 (B), 14=-15 (B), 24=-16 (B), 26=-22 (B), 27=-22 (B),



Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	A02	Нір	1	1	Job Reference (optional)	E14880760

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:14 ID:18YA8Ik6OooMLQ07WIORwuzsBSd-Mock Me



#### Scale = 1:66

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 YES IRC201	5/TPI2014	CSI TC BC WB Matrix-S	0.94 0.81 0.24	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.21 -0.45 0.16	(loc) 13 11-13 8	l/defl >999 >944 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 179 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 *Excep 15-3,15-5,11-5,11-7: Structural wood shea 3-0-12 oc purlins, ex	t* 2x4 SP No.2 athing directly appliec cept	2) I or	Wind: ASCE Vasd=103mp II; Exp C; Ent Interior (1) 2- Interior (1) 15 29-4-13, Inter 33-10-0 to 36 end vertical r DOI = 1.00	7-10; Vult=130mp h; TCDL=0.0psf; E closed; C-C Exteri 1-0 to 6-6-3, Exter i-0-0 to 20-11-0, E ior (1) 29-4-13 to i-10-0; cantilever lu ight exposed; Lum	h (3-sec BCDL=0 or (2) -0- rior (2) 6 Exterior (2) 33-10-0, eft and ri aber DOL	ond gust) 0psf; h=0ft; C 11-0 to 2-1-0 6-3 to 15-0-0 2) 20-11-0 to Exterior (2) ght exposed .=1.60 plate g	cat. , ; jrip					
BOT CHORD WEBS REACTIONS	2-0-0 oc purlins (2-2 Rigid ceiling directly bracing. 1 Row at midpt (size) 2=0-5-8, 8 Max Uplift 2=-895 (LI Max Gray, 2=1508 (I	-0 max.): 4-6. applied or 4-10-11 or 5-15, 5-11 3=0-5-8 C 6), 8=-895 (LC 6) C 11) &=1508 (I C 1	3) (1)	TCLL: ASCE DOL=1.25 PI snow); Pf=20 Plate DOL=1 Ct=1.10, Lu= This truss ha	7-10; Pr=20.0 psf ate DOL=1.00); Pg .4 psf (flat roof snd .00); Category II; E 0-0-0 s been checked fo	(roof live g=20.0 p ow: Lum Exp C; P or uniforn	e load: Lumbe sf (ground ber DOL=1.1 artially Exp.; n snow load o	ər 5 ınly,					
FORCES	(lb) - Maximum Com	pression/Maximum	5)	This truss ha	s been designed fo	or greate	er of min roof	live					
TOP CHORD	1-2=0/14, 2-17=-300 3-17=-2949/1607, 3- 4-18=-2567/1469, 4- 5-19=-2376/1404, 5- 6-20=-2374/1404, 6- 7-21=-2618/1469, 7- 8-22=-3000/1607, 8-	0/1607, 18=-2618/1469, 19=-2374/1404, 20=-2376/1404, 21=-2567/1469, 22=-2949/1607, 9=0/14	6) 7) 8)	oad of 12.0 p overhangs no Provide adeq This truss ha chord live loa * This truss h on the bottom	ost of 2.00 times fl on-concurrent with uate drainage to p s been designed f d nonconcurrent v as been designed n chord in all areas v 2.00 0 wide wit	at roof lo other liv prevent v or a 10.0 vith any for a live s where a	ad of 15.4 ps e loads. vater ponding psf bottom other live load e load of 20.0 a rectangle	r on Is. psf			22	WH CA	ROUN
BOT CHORD	2-16=-1384/2650, 15 14-15=-1486/2872, 1 12-13=-1486/2872, 1 10-11=-1384/2650, 8	5-16=-1384/2650, 13-14=-1486/2872, 11-12=-1486/2872, 3-10=-1384/2650	9)	3-06-00 tail b chord and an Provide mech bearing plate	y 2-00-00 wide will y other members. nanical connection capable of withsta	i (by othe anding 8	een the botto ers) of truss to 95 lb uplift at	m )		4	in the	ORFESS	N. N.
WEBS	3-16=0/201, 3-15=-3 5-15=-663/356, 5-13 6-11=-286/635, 7-11	52/213, 4-15=-286/63 =0/307, 5-11=-663/35 =-352/213, 7-10=0/20	35, 10 56, 01	) Graphical put or the orienta bottom chord	tion of the purlin a	does no long the	t depict the si top and/or	ze		11111		SEA 0363	L 22
NOTES			LC	DAD CASE(S)	Standard						- 0	•	1 E

 This truss has been checked for uniform roof live load only, except as noted.



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Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	A03	Нір	1	1	Job Reference (optional)	E14880761

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:14 ID:mITGrE7AseqpBLTGO0zyDezsBS7-Mock Me

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A. A. City G September 18,2020

818 Soundside Road Edenton, NC 27932



#### Scale = 1:66.2

		-											
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 YES IRC2015	5/TPI2014	CSI TC BC WB Matrix-S	0.96 0.83 0.67	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.18 -0.40 0.14	(loc) 13-14 13-14 9	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 176 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD	2x4 SP No.2 *Except 2x4 SP No.2 2x4 SP No.2 *Except Structural wood shea 2-2-0 oc purlins, exce 2-0-0 oc purlins (2-2- Rigid ceiling directly	* 5-6:2x4 SP No.1 * 3-16,8-11:2x4 SP athing directly applie ept -0 max.): 5-6. applied or 5-0-0 oc	2) No.3 ed or 3)	Wind: ASCE Vasd=103mp II; Exp C; End Interior (1) 2- 25-9-10, Inter 33-10-0 to 36 end vertical ri DOL=1.00 TCLL: ASCE	7-10; Vult=130mph h; TCDL=0.0psf; Bc closed; C-C Exterior 1-0 to 10-1-6, Exter rior (1) 25-9-10 to 3: -10-0; cantilever lef ight exposed; Lumb 7-10; Pr=20.0 psf (	(3-sec CDL=0 r (2) -0- ior (2) 3-10-0, it and r per DOI	ond gust) .0psf; h=0ft; ( .11-0 to 2-1-0 10-1-6 to Exterior (2) ght exposed .=1.60 plate	Cat. ), ; grip er					
WEBS REACTIONS	I Row at midpt 1 Row at midpt (size) 2=0-5-8, 9 Max Uplift 2=-877 (LC Max Grav 2=1564 (L (b) - Maximum Com	5-13 =0-5-8 C 6), 9=-877 (LC 6) C 11), 9=1553 (LC pression/Maximum	4) 11) 5)	DOL=1.25 Plasnow); Pf=20 Plate DOL=1 Ct=1.10, Lu= This truss has except as not This truss has	ate DOL=1.00); Pg= .4 psf (flat roof snov .00); Category II; E: 0-0-0 s been checked for ted. s been designed for	=20.0 p w: Lum kp C; P uniforn	sf (ground ber DOL=1.1 artially Exp.; n snow load o	5 only, live					
TOP CHORD	Tension 1-2=0/14, 2-17=-310: 3-17=-3039/1554, 3 4-18=-2405/1307, 5- 5-6=-2189/1273, 6-19 7-19=-2375/1307, 7-1 8-20=-3014/1554, 9-5 9-10=0/14	3/1554, 4=-2460/1308, 18=-2384/1307, 9=-2354/1307, 8=-2431/1308, 20=-3078/1554,	6) 7) 8)	load of 12.0 p overhangs no Provide adeq This truss has chord live loa * This truss h on the bottom 3.06-00 tall b	s for 2.00 times flat on-concurrent with c juate drainage to pri s been designed for d nonconcurrent wi as been designed for n chord in all areas y 2.00-00 wide will	t roof lo other liv event v r a 10.0 th any or a liv where fit betw	ad of 15.4 ps e loads. vater ponding psf bottom other live loa e load of 20.0 a rectangle	g. ds. Dpsf					1111).
BOT CHORD	2-16=-1334/2743, 15 14-15=-1334/2743, 1 13-21=-995/2134, 12 11-12=-1334/2720, 9	5-16=-1334/2743, 4-21=-995/2134, 2-13=-1334/2720, 9-11=-1334/2720	9)	chord and an Provide mech bearing plate joint 2 and 87	y other members, w nanical connection ( capable of withstar 77 lb uplift at joint 9.	vith BC (by othe nding 8	DL = 10.0psf ers) of truss t 77 lb uplift at	0		4	A	OR FESS	ROUN
WEBS	3-16=-6/300, 3-14=-6 5-13=-47/1, 6-13=-15 8-11=-6/299	659/370, 5-14=-157/ 57/480, 8-13=-664/3	/499, 10 370,	Graphical pur or the orienta bottom chord	rlin representation d tion of the purlin ald	loes no ong the	t depict the s top and/or	size				SEA	
NOTES 1) This truss only, exce	has been checked for t pt as noted.	uniform roof live loa	LC d	OAD CASE(S)	Standard					111VV	A A A A A A A A A A A A A A A A A A A		EER. KIN



Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	A04	Common	2	1	Job Reference (optional)	E14880762

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:15 ID:JSY0Uy8s4RPvGFuPJfUedMzsBQp-Mock Me



		3, 2 , -										-	
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 YES IRC201	5/TPI2014	<b>CSI</b> TC BC WB Matrix-S	0.54 0.86 0.45	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.18 -0.52 0.09	(loc) 10-13 14 10	l/defl >999 >825 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 222 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD WEBS REACTIONS	2x4 SP No.2 2x6 SP No.2 2x4 SP No.2 *Excep SP No.3 Structural wood she 2-11-1 oc purlins. Rigid ceiling directly bracing. 1 Row at midpt (size) 2=0-5-8, 1 Max Uplift 2=-718 (L Max Gray 2=1628 (I	t* 15-5,13-7,19-14:2: athing directly applie applied or 7-2-4 oc 17-18 I0=0-5-8 C 6), 10=-718 (LC 6) C 11). 10=1628 (LC 6)	2) x4 d or 3) 11)	Wind: ASCE Vasd=103mp II; Exp C; En Interior (1) 2- 20-11-8, Inte 33-10-0 to 33 end vertical n DOL=1.00 TCLL: ASCE DOL=1.25 P snow); Pf=15 Plate DOL=1 Ct=1.10 This truss ha	7-10; Vult=130m ch; TCDL=0.0psf; closed; C-C Exter 1-0 to 14-11-8, tc 5-10-0; cantilever right exposed; Luu 7-10; Pr=20.0 ps late DOL=1.00); F 5.4 psf (flat roof sr .00); Category II; as been checked f	ph (3-sec BCDL=0 rior (2) -0 xterior (2) 33-10-0, left and r mber DOI of (roof liv/ Pg=20.0 p now: Lum Exp C; P	ond gust) .0psf; h=0ft; ( .11-0 to 2-1-( .14-11-8 to Exterior (2) ght exposed =.1.60 plate ( e load: Lumb sf (ground ber DOL=1.1 artially Exp.; n snow load of	Cat. ), grip er 5 pnly,					
FORCES	(lb) - Maximum Com Tension 1-2=0/20, 2-20=-326 3-20=-3214/1186, 3- 4-5=-2785/990, 5-21 6-21=-2871/1100, 6- 7-22=-2932/1100, 7- 8-9=-2891/990, 9-23 10-23=-3267/1186.	57/1186, 4=-2891/990, =-2932/1100, 22=-2871/1100, 8=-2785/990, i=-3214/1186, 10-11=0/20	5) 6) 7) 8)	except as no This truss ha load of 12.0 overhangs n 200.0lb AC u 17-11-8 from apart. All plates are This truss ha chord live loa	ted. Is been designed psf or 2.00 times on-concurrent wit init load placed or left end, support 2x4 MT20 unles is been designed ad opproprurent	for greate flat roof lo h other liv n the botti ed at two s otherwis for a 10.0	er of min roof pad of 15.4 ps re loads. om chord, points, 5-0-0 se indicated. I psf bottom	live sf on					RO
BOT CHORD	2-16=-1003/2911, 19 15-24=-507/1853, 11 14-25=-507/1853, 11 12-13=-1003/2911, 19 5-15=-413/286, 15-1 6-17=-366/1179, 3-1 6-18=-366/1179, 13 7-13=-413/286, 9-13 18-19=-14/0 14-19=	5-16=-1003/2911, 5-16=-1003/2911, 4-24=-507/1853, 3-25=-507/1853, 10-12=-1003/2911 7=-380/1160, 5=-376/286, 18=-380/1160, i=-376/286, 17-19=-1 0/36	9) 10 14/0, LC	<ul> <li>chord live loa</li> <li>* This truss f</li> <li>on the bottor</li> <li>3-06-00 tall b</li> <li>chord and ar</li> <li>provide mec</li> <li>bearing plate</li> <li>joint 10 and 2</li> <li>DAD CASE(S)</li> </ul>	ad nonconcurrent has been designe n chord in all area by 2-00-00 wide w y other members hanical connectio e capable of withs 718 lb uplift at joir Standard	with any d for a live as where vill fit betw s. on (by othe tanding 7 ot 2.	other live loa e load of 20.0 a rectangle een the botto ers) of truss to 18 lb uplift at	ds. )psf om o		A CONTRACTOR OF CONTRACTOR	A L	SEA 0363	L 22
NOTES 1) This truss only, exce	has been checked for ept as noted.	uniform roof live load	d							S		A. G.	EER. KINN



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Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	A05	Common	5	1	Job Reference (optional)	E14880763

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:15 ID:UI8v4HfnV9MaS6qjzFmG?wzsBRR-Mock Me



Scale = 1:64.4

#### Plate Offsets (X, Y): [2:0-0-13,0-1-12], [10:0-0-13,0-1-12]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL	(psf) 20.0 15.4/20.0 10.0 0.0*	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 YES IRC2015	5/TPI2014	CSI TC BC WB Matrix-S	0.67 0.90 0.50	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.20 -0.52 0.09	(loc) 10-12 13 10	l/defl >999 >815 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL	10.0											Weight: 220 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD WEBS REACTIONS	2x4 SP No.2 2x6 SP No.2 2x4 SP No.2 *Except SP No.3 Structural wood shea 2-5-6 oc purlins. Rigid ceiling directly bracing. 1 Row at midpt (size) 2=0-5-8, 1	* 14-5,12-7,18-13:2x athing directly applied applied or 6-10-11 oc 16-17 0= Mechanical	2) 4 or 3)	Wind: ASCE Vasd=103mp II; Exp C; En Interior (1) 2- 20-11-8, Inte 32-10-4 to 35 end vertical r DOL=1.00 TCLL: ASCE DOL=1.25 PI snow); Pf=15 Plate DOL=1 Ct=1.10	7-10; Vult=130m h; TCDL=0.0psf closed; C-C Exte 1-0 to 14-11-8, E frior (1) 20-11-8 t 5-10-4; cantilever ight exposed; Lu 7-10; Pr=20.0 p ate DOL=1.00); i.4 psf (flat roof s .00); Category II	nph (3-sect ; BCDL=0. prior (2) -0- Exterior (2) o 32-10-4, r left and ri umber DOL sf (roof live Pg=20.0 p g=20.0 p ; Exp C; P	ond gust) 0psf; h=Oft; 11-0 to 2-1-( 14-11-8 to Exterior (2) ght exposed = 1.60 plate = load: Lumb sf (ground ber DOL=1.1 artially Exp.;	Cat. ), grip per					
	Max Horiz 2=13 (LC 0 Max Uplift 2=-724 (L0 Max Grav 2=1636 (L	6) C 6), 10=-641 (LC 6) C 11), 10=1586 (LC 1	4) 11) 5)	This truss ha except as no This truss ha	s been checked ted. s been designed	for uniform I for greate	n snow load r of min roof	only, live					
FORCES	(lb) - Maximum Comp	pression/Maximum		load of 12.0 p	osf or 2.00 times	flat roof lo	ad of 15.4 p	sf on					
TOP CHORD	1-2=0/20, 2-19=-328 3-19=-3234/1200, 3- 4-5=-2805/1004, 5-21 6-20=-2891/1114, 6- 7-21=-2973/1128, 7- 8-9=-2933/1020, 9-2: 10-22=-3356/1252	7/1200, 4=-2911/1004, 0=-2953/1114, 21=-2911/1128, 8=-2827/1020, 2=-3295/1252,	6) 7) 8) 9)	200.0lb AC u 17-11-8 from apart. All plates are This truss ha chord live loa * This truss h	nit load placed o left end, suppor 2x4 MT20 unles s been designed id nonconcurrent as been designed	the botto ted at two ss otherwis for a 10.0 t with any o ad for a live	e loads. om chord, points, 5-0-0 e indicated. psf bottom other live loa	) Ids.			A.L.	NITH CA	ROUT
BOT CHORD	2-15=-1029/2927, 14 14-23=-534/1870, 13 13-24=-534/1870, 12 11-12=-1088/3006. 1	-15=-1029/2927, -23=-534/1870, -24=-534/1870, 0-11=-1088/3006	10	on the botton 3-06-00 tall b chord and an	n chord in all are by 2-00-00 wide w by other members ar(s) for truss to t	as where a will fit betw s.	a rectangle een the botto	om		A	ip	SEA	No.
WEBS NOTES 1) This truss only, exce	5-14=-413/285, 14-11 6-16=-365/1178, 3-1- 6-17=-387/1209, 12- 7-12=-409/279, 9-12- 17-18=-14/0, 13-18= has been checked for the pot as noted.	6=-379/1159, 4=-375/286, 17=-401/1189, =-441/336, 16-18=-14 0/35	<sup>10</sup> , 11)	) Provide mecl bearing plate joint 2 and 64	capable of withs 11 lb uplift at join Standard	t 10.	ers) of truss t 24 lb uplift at	to t		1111AA.	K. K.		EER KUU

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	A06	Нір	1	1	Job Reference (optional)	E14880764

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818 Soundside Road Edenton, NC 27932



Scale = 1:66

#### Plate Offsets (X, Y): [4:0-3-0,Edge], [5:0-3-0,0-2-4], [9:0-3-0,0-2-4]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 YES IRC2015	/TPI2014	CSI TC BC WB Matrix-S	0.80 0.84 0.96	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.16 -0.33 0.10	(loc) 13-16 13-16 11	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 209 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD	2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 *Except Structural wood shea 2-2-0 oc purlins, exc 2-0-0 oc purlins (3-2-	* 3-17:2x4 SP No.3 athing directly applied tept end verticals, and 14 max ): 5-9	2) or 3)	Wind: ASCE Vasd=103mp II; Exp C; Enc Interior (1) 2- Interior (1) 18 35-9-4; cantile right exposed TCLL: ASCE	7-10; Vult=130mph h; TCDL=0.0psf; B( losed; C-C Exterior 1-0 to 10-1-6, Exter -7-3 to 30-5-4, Exter ever left and right e: ; Lumber DOL=1.6( 7-10; Pr=20.0 psf (i	(3-sec CDL=0 r (2) -0 ior (2) erior (2 xposec 0 plate roof liv	ond gust) .0psf; h=0ft; .11-0 to 2-1-( 10-1-6 to 18- ) 30-5-4 to I; end vertica grip DOL=1. e load: Lumb	Cat. ), 7-3, al 00 er					
BOT CHORD WEBS REACTIONS	2-0-0 oc purins (3-2- Rigid ceiling directly bracing. 1 Row at midpt (3) (size) 2=0-5-8, 1 Max Horiz 2=90 (LC Max Uplift 2=-859 (LC Max Grav 2=1589 (L	14 max.): 5-9. applied or 4-11-14 oc 9-12 1= Mechanical 7) C 6), 11=-788 (LC 6) C 11). 11=1607 (LC 1	4) 1)	DOL=1.25 Pia snow); Pf=20 Plate DOL=1. Ct=1.10, Lu=1 This truss has except as not This truss has	the DOL=1.00); Pg= 4 psf (flat roof snov 00); Category II; Ex 0-0-0 s been checked for ed. s been designed for of as 2 00 times flot	=20.0 p w: Lum (p C; P uniforr	er of min roof	5 only,					
FORCES	(lb) - Maximum Com	pression/Maximum		overhangs no	n-concurrent with c	ther liv	ve loads.	51 011					
TOP CHORD	1-ension 1-2=0/14, 2-18=-316i 3-18=-3107/1516, 3- 4-19=-2422/1252, 4- 5-20=-2284/1221, 6- 6-7=-2284/1221, 7-8: 8-9=-1690/907, 9-10:	9/1516, 19=-2498/1252, 5=-2416/1252, 20=-2284/1221, =-1690/907, =-136/134, 10-11=-88	6) 7) 8) /75	Provide adeq This truss has chord live loa * This truss h on the bottom 3-06-00 tall b	uate drainage to pro- s been designed for d nonconcurrent wi as been designed for o chord in all areas y 2-00-00 wide will i	event v a 10.0 th any or a liv where fit betw with BC	vater ponding ) psf bottom other live loa e load of 20.0 a rectangle veen the botto DL = 10.0pc	g. Ids. Opsf om				TH CA	ROWY
BOT CHORD	2-17=-1344/2737, 16 15-16=-981/2091, 15 14-21=-981/2091, 13 13-22=-670/1540, 12 12-23=-27/254, 23-2- 11-24=-27/254	-17=-1344/2737, -21=-981/2091, -14=-981/2091, -22=-670/1540, 4=-27/254,	9) 10) 11)	Refer to girde Provide mech bearing plate joint 2 and 78 Graphical pur or the orienta	r(s) for truss to trus anical connection ( capable of withstar 8 lb uplift at joint 11 lin representation d tion of the purlin alc	by other by other ding 8 l. loes no	ections. ers) of truss t 59 lb uplift at t depict the s top and/or	size		A state	AN AN	SEA	
WEBS	3-17=-5/302, 3-16=-7 5-13=-4/101, 6-13=-4 7-12=-975/587, 9-12: 9-11=-1456/729	701/397, 5-16=-170/50 130/289, 7-13=-423/80 =-875/1824,	06, <sup>DO,</sup> LO	bottom chord AD CASE(S)	Standard					1111		0303	CR. K
NOTES 1) This truss only, exce	has been checked for opt as noted.	uniform roof live load									111	A. G	LEFRIN

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	A07	Half Hip	1	1	Job Reference (optional)	E14880765

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#### Scale = 1:66.6

Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.00		тс	0.61	Vert(LL)	-0.19	13-15	>999	240	MT20	244/190	
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.25		BC	0.91	Vert(CT)	-0.45	13-15	>940	180			
TCDL	10.0	Rep Stress Incr	YES		WB	0.77	Horz(CT)	0.14	10	n/a	n/a			
BCLL	0.0*	Code	IRC2015	5/TPI2014	Matrix-S									
BCDL	10.0											Weight: 188 lb	FT = 20%	
				N/ 1 4005	-				-			•		
			2)	Wind: ASCE	7-10; Vult=130	mph (3-sec	ond gust)	Cat						
TOP CHORD	2x4 SP No.2			Vasu=103mp	n, TCDL=0.0ps	si, BCDL=0	.0psi, n=0it,	oal.						
BOICHORD	2X4 SP No.2			Interior (1) 2-	1-0 to 6-6-3 Ex	(2) = 0	-6-3 to 15-0-	0,						
WEB2	2X4 SP No.3 "Except			Interior (1) 15	5-0-0 to 32-9-4	Exterior (2)	32-9-4 to	0,						
	15-5,5-15,6-10,15-6,	11-0.2X4 SP NU.2		35-9-4 cantil	ever left and ric	tht exposed	end vertic	al						
BRACING	o			right exposed	: Lumber DOL:	=1.60 plate	arip DOL=1.	.00						
TOP CHORD	Structural wood shea	athing directly applie	ed or 3)	TCLL: ASCE	7-10: Pr=20.0	psf (roof live	e load: Lumb	ber						
	3-0-14 oc punins, ex	(cept end verticals, a	and -/	DOL=1.25 Pl	ate DOL=1.00)	; Pg=20.0 p	sf (ground							
	2-0-0 00 putitits (3-1-	applied or 4 11 10 /	20	snow); Pf=20	.4 psf (flat roof	snow: Lum	ber DOL=1.	15						
BOT CHORD	hracing		50	Plate DOL=1	.00); Category	II; Exp C; P	artially Exp.;	;						
WEBS	1 Row at midnt	8-10		Ct=1.10, Lu=	0-0-0									
PEACTIONS	(size) = 2 - 0.5 - 8 - 1	0- Mechanical	4)	This truss ha	s been checked	d for uniforn	n snow load	only,						
REACTIONS	(Size) 2=0-5-0, 1 Max Horiz 2=72 (LC)	0= Mechanicai 7)		except as not	ted.									
	Max   Inlift 2876 (I (	7) C 6) 10–-763 (I C 6	5)	This truss ha	s been designe	d for greate	er of min root	flive						
	Max Grav 2=1511 (I	C 11) 10=1434 (I C	2 11)	load of 12.0 p	ost or 2.00 time	s flat roof lo	ad of 15.4 p	ost on						
FORCES	(lb) - Maximum Com	pression/Maximum	) ) )	overnangs no	on-concurrent w		e loads.	~						
FORCES		pression/maximum	6) 7)	Provide adeq	uate drainage i	to prevent v	vater ponding	g.						
	1-2=0/14 2-17=-300	7/1562	()	chord live loa	d nonconcurre	nt with any	other live los	ade						
	3-17=-2957/1562. 3-	18=-2628/1419.	8)	* This trues h	as been design	ned for a live	a load of 20	noef						
	4-18=-2575/1419, 4-	19=-2381/1357,	0)	on the bottom	as been design	eas where	a rectandle	opsi						
	5-19=-2382/1357, 5-	6=-2856/1560,		3-06-00 tall b	v 2-00-00 wide	will fit betw	een the bott	om						
	6-7=-2090/1135, 7-8	=-2090/1135,		chord and an	y other membe	rs.						11111 00	1111	
	8-20=-96/98, 9-20=-9	96/98, 9-10=-171/13	31 9)	Refer to girde	er(s) for truss to	truss conn	ections.					IN TH UA	ROUL	
BOT CHORD	2-16=-1394/2603, 15	5-16=-1394/2603,	10	) Provide mech	nanical connect	tion (by othe	ers) of truss	to			×	A	· Anti-	
	14-15=-1441/2799, 1	3-14=-1441/2799,		bearing plate	capable of with	nstanding 7	63 lb uplift a	t			62	EE SS		1
	12-13=-1290/2560, 1	1-12=-1290/2560,		joint 10 and 8	376 lb uplift at jo	pint 2.					12		12.2	-
	10-11=-803/1658		. 11	) Graphical pui	rlin representati	ion does no	t depict the	size			- I	ie i	N	-
WEBS	4-15=-286/668, 3-15	=-352/221, 3-16=0/	189,	or the orienta	tion of the purli	n along the	top and/or			-		SEA	L 🚯	Ξ
	5-15=-727/315, 8-10	=-2032/1080,		bottom chord						Ξ.		0000	20	Ξ.
	6-11-817/479 8-11	-140/333, 380/860	LC	AD CASE(S)	Standard					1		0363	22 :	-
NOTES	0 11-01/14/0, 0-11	- 003/000									0			-
1) This trues	has been checked for	uniform roof live loa	d								-	·	air	3
only, exce	nas been checked for		u .								15	A MGINI	EENAN	5
o,, oxoo											11	710	DEN	
												IL A G	ILDIN	



A. A. GILD September 18,2020

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	A08	Half Hip Girder	1	2	Job Reference (optional)	E14880766

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# Plate Offsets (X, Y): [4:0-5-12,0-2-8]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 NO IRC2015/TPI20	14 N	<b>CSI</b> TC BC WB Matrix-S	0.39 0.68 0.34	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.25 -0.43 0.11	(loc) 16-17 16-17 12	l/defl >999 >981 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 390 lb	<b>GRIP</b> 244/190 FT = 20%	
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD REACTIONS FORCES TOP CHORD	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 *Except 19-4,19-6,16-6,16-9, Structural wood shea 5-8-13 oc purlins, ex 2-0-0 oc purlins (4-9- Rigid ceiling directly b bracing. (size) 2=0-5-8, 1 Max Horiz 2=54 (LC Max Uplift 2=-826 (LC Max Grav 2=2367 (L (Ib) - Maximum Comp Tension 1-2=0/14, 2-22=-479 22-23=-4754/1420, 2 3-24=-4724/1419, 3- 4-25=-4570/1429, 4- 3-24=-4724/1419, 3- 4-25=-4570/1429, 4- 3-23=-6453/1995, 7- 8-33=-6453/1995, 8- 3-34=-6453/1995, 9- 35-36=-3169/1006, 1 10-37=-3169/1006, 1 11-12=-2310/760	13-9,13-11:2x4 SP N athing directly applied (cept end verticals, a -13 max.): 4-11. applied or 9-2-9 oc 2= Mechanical 13) C 6), 12=-728 (LC 6) C 11), 12=2376 (LC pression/Maximum 7/1420, 25=-4593/1425, 26=-5967/1872, -27=-5966/1872, -29=-5967/1873, 30=-6453/1995, 33=-6453/1995, 33=-6453/1995, 33=-6453/1995, 33=-6453/1995, 33=-6453/1995, 33=-3169/1006, 6-37=-3169/1006, 1-39=-3169/1006,	BOT CHO lo.2 d or ind WEBS 11) VWEBS 11) 2-ply (0.13 Top c oc. Botto 0-9-0 Web 2) All lo excer CASI provi unles 3) This only,	RD         2-4           21-         20-           19-         44-           17-         46-           15-         14-           51-         13-           54-         3-2           4-1         6-1           7-1         9-1           11-         11-           truss to be         ">">">"           ""X3") nail           hords cor         connected           adds are cod         cods are cod           if noted         cit odists           s otherwise         sotherwise           russ has be         except as	40=-1261/4191, 21 - $41=-1261/4191, 21$ - $41=-1261/4191, 2$ - $42=-1236/4160, 4$ - $43=-1236/4160, 1$ - $43=-1236/4160, 1$ - $43=-1953/6611, 1$ - $45=-1953/6611, 1$ - $47=-1953/6611, 1$ - $50=-1520/5191, 1$ - $50=-1520/5191, 1$ - $52=-1520/5191, 1$ - $52=-21/78, 12-55$ - $21=0/206, 3-20=-4$ 19=-618/2063, 5-12 19=-618/2063, 5-12 10=-618/2063, 5-126, 5-1263, 5-1263, 5-1263, 5-1263, 5-1263,	-40=-1 -40=-1 -2-43=- 9-44=- 9-44=- 9-44=- 9-44=- 8-451=- -21/7 4-51=- -21/2	261/4191, -1261/4191, -1261/4191, -1953/6611, -1953/6611, -1953/6611, -1953/6611, -1520/5191, -1520/5191, -1520/5191, -1520/5191, -21/78, 8 4-20=0/280, 5/297, -, 6-16=-288/93 1427, 9-14=0/2 56/296, h 10d 1 row at 0-9-0 cc. ace in the LOA s have been as (F) or (B), n roof live load	3, 255, AD	<ul> <li>4) Wi Va Va II; Int 32 Ve DC 5) TCC 5) TCC 5) TCC 5) TC Ct 6) Th ex 8) Pr 9) Th ch</li> </ul>	nd: ASCE sd=103m Exp C; Er erior (1) 2 4-13, Intu- 9-4 to 35 tical right JL=1.00 LL: ASCI JL=1.25 F pow); Pf=2 the DOL= =1.10, Lu is truss h: d of 12.0 erhangs r pyide ade is truss h: ord live lo	E 7-10; ph; TC closece 2-1-0 tc erior (1 i-9-4; c e expose E 7-10; l'ate D 0.4 psi 1.00); l'ate D 0.4 psi 1.00); as bee poted. as bee psf or non-col as bee ad nor	Vult=130mph (3 CDL=0.0psf; BCD d; C-C Exterior (2 2-11-0, Exterior) antilever left and sed; Lumber DOL ; Pr=20.0 psf (roc OL=1.00); Pg=20 f (flat roof snow: Category II; Exp n checked for un n designed for gg 2.00 times flat rc ncurrent with oth drainage to previous n designed for a nconcurrent with NCAL SEA 0363	second gust L=0.0psf; h=0 ) -0-11-0 to 2 (2) 2-11-0 to	) ) Dft; Cat. :-1-0, 2) d; end grip Jmber id =1.15 xp.; vad only, roof live .4 psf on iding. Diads.

Continued on page 2 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. WARNING - Verify design parameters and KEAD NOTES ON THIS AND INCLODED MITER KETEKENCE PAGE MIT-14's rev. 5/ 32/20 DEFVICE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	A08	Half Hip Girder	1	2	Job Reference (optional)	E14880766

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Carter Components (Sanford), Sanford, NC - 27332,

- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 11) Refer to girder(s) for truss to truss connections.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 728 lb uplift at joint 12 and 826 lb uplift at joint 2.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

### LOAD CASE(S) Standard

Dead + Snow (balanced): Lumber Increase=1.15, Plate 1) Increase=1.00

Uniform Loads (lb/ft)

Vert: 1-4=-51, 4-11=-61, 2-12=-20

Concentrated Loads (lb)

- Vert: 4=-27 (F), 20=-15 (F), 24=-16 (F), 26=-22 (F),
- 27-22 (F), 28-22 (F), 29-22 (F), 30-22 (F), 31-22 (F), 32-22 (F), 33-22 (F), 34-22 (F), 35-22 (F), 36-22 (F), 37-22 (F), 38-22 (F),

- 39=-22 (F), 40=-61 (F), 41=-75 (F), 50=-22 (F), 39=-22 (F), 40=-61 (F), 41=-75 (F), 45=-15 (F), 43=-15 (F), 44=-15 (F), 45=-15 (F), 46=-15 (F), 47=-15 (F), 48=-15 (F), 49=-15 (F), 50=-15 (F),
- 51=-15 (F), 52=-15 (F), 53=-15 (F), 54=-15 (F),
- 55=-15 (F)



Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	B01	Hip Supported Gable	1	1	Job Reference (optional)	E14880767

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:18 ID:5bGPLk\_owD?GFM\_Zens?8\_zsCiL-Mock Me



17-6-8

Scale = 1:39.3		
Plate Offsets (X, Y):	[6:0-3-0,0-2-0], [9:0-3-0,	,0-2-0]

Loading		(psf)	Spacing	2-0-0		csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)		20.0	Plate Grip DOL	1.00		TC	0.07	Vert(LL)	n/a	-	n/a	999	MT20	244/190	
Snow (Pf/Pg)	2	20.4/20.0	Lumber DOL	1.25		BC	0.03	Vert(CT)	n/a	-	n/a	999			
TCDL		10.0	Rep Stress Incr	YES		WB	0.06	Horz(CT)	0.00	13	n/a	n/a			
BCLL		0.0*	Code	IRC20	15/TPI2014	Matrix-S		, í							
BCDL		10.0											Weight: 109 lb	FT = 20%	
LUMBER				V	VEBS 9	-18=-95/6. 8-19=-12	27/96.	7-20=-127/96	j.	13) Pro	vide med	chanica	al connection (bv	others) of truss	to
TOP CHORD	2x4 SP N	lo.2			e	-21=-95/6, 5-22=-20	)0/148	, 4-23=-188/1	39,	bea	ring plat	e capa	ble of withstandi	ng 71 lb uplift at i	joint
BOT CHORD	2x4 SP N	0.2			1	0-17=-200/148, 11-	15=-18	38/139	,	2,6	b uplift	at join	t 18, 96 lb uplift a	t joint 19, 96 lb (	, uplift
OTHERS	2x4 SP N	0.3								at je	oint 20, 6	i lb upl	ift at joint 21, 147	Ib uplift at joint :	22,
SLIDER	Left 2x6 S	SP No.2 1	-8-1. Right 2x6 SP N	No.2 1	) This trues ha	s been checked for	unifor	n roof live loa	Ч	141	lb uplift	at join	t 23, 147 lb uplift	at joint 17, 141 l	b
02.02.1	1-8-1			10.2		s been checked for	uniion		u	upli	ft at joint	15 an	d 71 lb uplift at jo	int 13.	
				-		$7 10. Vult_120mph$	12 000	ond quet)		14) Gra	phical p	urlin re	presentation doe	s not depict the	size
	Structura		athing directly applie	⊿ dor	Vasd-103mr	h: TCDI –0 0psf: BC		Ond gust) Onst: h-Oft: (	Cat	ort	he orient	ation o	of the purlin along	the top and/or	
	6-0-0 00	nurlins ever	ent	u ui	II: Exp C: En	closed: C-C Corner	(3) -0-	11-0 to 2-1-0	<i>J</i> at.	bot	tom chor	d.			
	2-0-0 00	nurlins (6-0-	-0 max ): 6-9		Exterior (2) 2	-1-0 to 2-9-4 Corne	r (3) 2	-9-4 to 14-9-4	L		CASE(S)	Star	ndard		
	Rigid ceil	ling directly	applied or 10-0-0 oc		Exterior (2) 1	4-9-4 to 15-5-8. Cor	ner (3	) 15-5-8 to	,		(-)				
	bracing	ing directly			18-5-8: cantil	ever left and right ex	kposed	d : end vertica	ıl						
DEACTIONS	(cizo)	2-1769	12-1769 15-176	. 0	right exposed	: Lumber DOL=1.60	) plate	grip DOL=1.0	00						
REACTIONS	(3120)	2=17-0-0,	18-17-6-8 10-17-	.e.a 3	) Truss desian	ed for wind loads in	the pla	ane of the trus	ss						
		20=17-6-8	, 10=17-0-0, 13=17- 21=17-6-8 22=17-	-6-8	only. For stu	ds exposed to wind	(norm	al to the face)	),						
		23=17-6-8	, 21-11 0 0, 22-11	00,	see Standard	Industry Gable End	Deta	ils as applicat	ole,						
	Max Uplift	2=-71 (LC	6), 13=-71 (LC 6),		or consult qu	alified building desig	gner as	s per ANSI/TP	ข 1.						
		15=-141 (l	LC 6). 17=-147 (LC 6	6). <sup>4</sup>	) TCLL: ASCE	7-10; Pr=20.0 psf (r	roof liv	e load: Lumbe	er						
		18=-6 (LÒ	6), 19=-96 (LC 6),	,,	DOL=1.25 PI	ate DOL=1.00); Pg=	=20.0 p	osf (ground							
		20=-96 (L0	C 6), 21=-6 (LC 6),		snow); Pf=20	.4 psf (flat roof snov	v: Lum	ber DOL=1.1	5						
		22=-147 (l	LC 6), 23=-141 (LC 6	6)	Plate DOL=1	.00); Category II; Ex	αp C; F	artially Exp.;							
	Max Grav	2=147 (LC	2), 13=147 (LC 2),	_	Ct=1.10, Lu=	0-0-0							11111 01		
		15=231 (L	C 11), 17=239 (LC 1	11), 5	) This truss ha	s been checked for	uniforr	n snow load c	only,				TH UA	Rain	
		18=135 (L	C 2), 19=167 (LC 1)	,	except as no	ied.						X	A	6.91A.	
		20=167 (L	C 1), 21=135 (LC 2)	, t	<li>I his truss hat the second second</li>	s been designed for	great	er of min roof	live		/	53	·inff	Of the	20
		22=239 (L	.C 11), 23=231 (LC 1	11)		osi or 2.00 times hat	1001 10	bad of 15.4 ps	aron		L			1 all	2
FORCES	(lb) - Max	kimum Com	pression/Maximum	-	overnangs no	on-concurrent with o		/e loads.			-		. Q		-
	Tension			1	) Provide aded	uate drainage to pre	event v	valer ponding			-		SEA	L 🚯	=
TOP CHORD	1-2=0/15	, 2-3=-70/45	5, 3-4=-36/50,	c c	) All plates are	2X4 IVI 1 20 UTILESS 0	n chor	d booring			Ξ		02/		Ξ.
	4-5=-73/4	47, 5-6=-133	3/136, 6-7=-118/140		0) Gable require	s continuous botton		u bearing.					0363	22	3
	7-8=-118	/140, 8-9=-1	118/140, 9-10=-133/	136, 1	1) This trues ha	spaceu al 2-0-0 0c.	a 10 (	) nef hottom			-	- B	<b>1</b>		5
	10-11=-7	3/47, 11-12	=-36/50, 12-13=-70/	45, '	chord live los	d nonconcurrent wit	h anv	other live load	de			1	N	- · ·	2
	13-14=0/	10 00 00 00	EE/00 01 00 EE/0	o 1	2) * This truss h	as been designed for	oraliv	e load of 20 0	nsf			2.0	NO.	EHIX	
BUICHURD	2-23=-55	100, 22-23= 5/00 10 20	-00/00, 21-22=-55/8 - 55/99 19 10. 55/	o, ' oo	on the botton	chord in all areas	where	a rectangle				1	SI GIN		
	20-21=-0	5/88 16-17	55/88 15-1655/	00, 88	3-06-00 tall h	v 2-00-00 wide will f	fit betv	een the botto	m			1	A C	IL BEIN	
	13-155	5/88	- 55,00, 15-10=-55/	00,	chord and an	y other members.			•				1111.0	in in it	
	10 10=-0	0,00				•								1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	

September 18,2020

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Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	B02	Hip Girder	1	2	Job Reference (optional)	E14880768

Scale = 1:48

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Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDI	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 NO IRC2015	5/TPI2014	<b>CSI</b> TC BC WB Matrix-S	0.76 0.45 0.85	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.07 -0.15 0.03	(loc) 10-11 10-11 7	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 274 lb	<b>GRIP</b> 244/190 187/143	
LUMBER TOP CHORD BOT CHORD WEBS WEDGE BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.2 *Except 2x8 SP 2400F 2.0E 2x4 SP No.3 *Except Left: 2x8 SP 2400F 2 Right: 2x10 SP 2400F Structural wood shea 3-10-4 oc purlins, exi 2-0-0 oc purlins, exi 2-0-0 oc purlins (4-3- Rigid ceiling directly bracing. (size) 2=0-5-8, 7	* 4-5:2x4 SP No.3 * 10-4:2x4 SP No.2 .0E F 2.0E athing directly applied cept 8 max.): 4-5. applied or 10-0-0 oc =0-5-8 40	1) d or 2) 3)	2-ply truss to (0.131"x3") n Top chords c oc. Bottom chord staggered at Web connect All loads are except if note CASE(S) sec provided to d unless otherw This truss ha only, except a	be connected toge ails as follows: onnected as follow s connected as follow 0-4-0 oc. ed as follows: 2x4 considered equally d as front (F) or ba tion. Ply to ply con istribute only loads vise indicated. s been checked for as noted.	ether wit s: 2x4 - lows: 2x - 1 row applied ack (B) f nections noted a r uniform	h 10d 1 row at 0-9- (8 - 2 rows at 0-9-0 oc. t to all plies, ace in the LC s have been as (F) or (B), n roof live loa	-0 DAD	12) * Th on t 3-00 cho 13) Prov bea joint 14) Gra or th bott 15) Use 8-16 left cho	his truss he botto 3-00 tall rd and a vide med ring plat t 2 and 1 phical prine or chor USP TH 3d nails c USP TH 3d nails c rd.	has be m chor by 2-00 ny othe chanica e capa 1858 lb urlin re tation c rd. HDH26 into Tru onnect	en designed for a d in all areas wh 0-00 wide will fit th er members. al connection (by ble of withstandir uplift at joint 7. presentation doe f the purlin along -2 (With 22-16d i uss) or equivalen truss(es) to back	a live load of 20.0psf are a rectangle between the bottom others) of truss to ug 1247 lb uplift at s not depict the size the top and/or nails into Girder & t at 3-8-5 from the s face of bottom	_
FORCES TOP CHORD BOT CHORD	Max Uplift 2=-1247 (L Max Uplift 2=-1247 (L Max Grav 2=6587 (L (lb) - Maximum Comp Tension 1-2=0/27, 2-13=-905: 13-14=-8996/1519, 3 3-4=-7743/1324, 4-5: 5-6=-8107/1771, 6-7; 2-12=-1059/6627, 12 11-15=-1059/6627, 12 11-15=-1059/6627, 13 16-17=-863/5918, 17 10-18=-863/5918, 9-	12) C 6), 7=-1858 (LC 6 C 11), 7=7695 (LC 1 pression/Maximum 9/1519, 1-14=-8982/1519, 6344/1440, =-9377/2176 1-55-1059/6627, 1-16=-863/5918, 1-16=-863/5918, 10=-1553/6878,	6) 4) 11) 5) 6)	Wind: ASCE Vasd=103mp II; Exp C; Enn Interior (1) 2- cantilever leff exposed; Lur TCLL: ASCE DOL=1.25 PI snow); Pf=20 Plate DOL=1 Ct=1.10, Lu= This truss ha except as not	7-10; Vult=130mpł h; TCDL=0.0psf; E closed; C-C Exterio 1-0 to 2-6-0, Exterio and right exposed her DOL=1.60 pla 7-10; Pr=20.0 psf ate DOL=1.00); Pg .4 psf (flat roof snc 00); Category II; E 0-0-0 s been checked for ed.	n (3-sec CDL=0 or (2) -0- or (2) 2 ; end v ate grip 1 (roof live =20.0 p w: Lum xp C; P r uniform	ond gust) .0psf; h=0ft; ( .11-0 to 2-1-0: 6-0 to 17-3-1 ertical right DOL=1.00 e load: Lumb sf (ground ber DOL=1.1 artially Exp.; n snow load o	Cat. ), 12; er 5 only,	16) Use 12- <sup>-</sup> 2-0- 16-3 cho 17) Fill : LOAD C	USP TH 10d x 1- <sup>-</sup> 0 oc ma 3-7 to co rd. all nail h CASE(S)	HD26 ( 1/2 nail ax. start onnect t oles wi ) Star	With 18-16d nails s into Truss) or e ing at 5-7-9 from rruss(es) to back here hanger is in indard	into Girder & quivalent spaced at the left end to face of bottom contact with lumber.	
WEBS NOTES	9-19=-1553/6878, 8- 8-20=-1553/6878, 7- 3-12=-255/1729, 3-1 4-11=-198/4033, 4-1 5-10=-903/4600, 6-1 6-8=-547/1730	19=-1553/6878, 20=-1553/6878 1=-825/262, 0=-550/579, 0=-767/451,	7) 8) 9) 10] 11]	This truss ha load of 12.0 p overhangs nd Provide adeq All plates are The Fabricati This truss ha chord live loa	s been designed for sef or 2.00 times fla n-concurrent with uate drainage to p MT20 plates unles on Tolerance at joi s been designed for d nonconcurrent w	or greate at roof lo other liv revent v ss other int $7 = 1$ or a 10.0 vith any	er of min roof pad of 15.4 ps re loads. vater ponding wise indicate 2% 0 psf bottom other live loa	live sf on g. d. ds.		ATTITUM.		SEA 0363	L 22 LBERTIN	

Continued on page 2 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-1473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANS/TP/1 Quality Criteria</u>, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

# 818 Soundside Road Edenton, NC 27932

September 18,2020

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	B02	Hip Girder	1	2	Job Reference (optional)	E14880768

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1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.00 Uniform Loads (lb/ft)

Vert: 1-4=-51, 4-5=-61, 5-7=-51, 2-7=-20

Concentrated Loads (lb)

Vert: 12=-1751 (B), 8=-1340 (B), 15=-1399 (B), 16=-1376 (B), 17=-1340 (B), 18=-1340 (B), 19=-1340 (B), 20=-1340 (B)



Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	C01	Common	4	1	Job Reference (optional)	E14880769

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:18



Scale = 1:40.7

4-10-3

# Plate Offsets (X, Y): [8:0-4-0,0-3-4]

Loading FCLL (roof) Snow (Pf/Pg) FCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 YES IRC2015	5/TPI2014	CSI TC BC WB Matrix-S	0.29 0.74 0.21	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.17 -0.36 0.03	(loc) 2-8 2-8 6	l/defl >999 >648 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 87 lb	<b>GRIP</b> 244/190 FT = 20%	
JUMBER FOP CHORD 30T CHORD WEBS <b>3RACING</b> FOP CHORD 30T CHORD 30T CHORD <b>REACTIONS</b> ( FORCES TOP CHORD WEBS NOTES 1) This truss f only, excep 2) Wind: ASC Vasd=103n II; Exp C; E Interior (1) 12-11-8, Inil 17-10-0 to end vertical DOL=1.25 snow); Pf=: Plate DOL= Ct=1.10	2x4 SP No.2 2x4 SP No.1 2x4 SP No.1 2x4 SP No.2 *Except Structural wood shea 4-7-8 oc purlins. Rigid ceiling directly a bracing. (size) 2=0-5-8, 6 Max Uplift 2=-524 (LC Max Grav 2=860 (LC (lb) - Maximum Comp Tension 1-2=0/17, 2-9=-1470/ 3-10=-1125/590, 4-10 4-11=-1072/590, 5-11 5-12=-1440/770, 6-12 2-8=-631/1292, 6-8=- 4-8=-221/547, 5-8=-3 has been checked for ut as noted. E 7-10; Vult=130mph nph; TCDL=0.0psf; BC inclosed; C-C Exterior 2-1-0 to 6-11-8, Exteri terior (1) 12-11-8 to 17 20-10-0; cantilever left I right exposed; Lumbo E 7-10; Pr=20.0 psf (r Plate DOL=1.00); Pg= 15.4 psf (flat roof snow =1.00); Category II; Ex	* 8-4:2x4 SP No.3 athing directly applied applied or 7-11-8 oc =0-5-8 2 6), 6=-524 (LC 6) 11), 6=860 (LC 11) pression/Maximum /770, 3-9=-1440/770, 0=-1072/590, 1=-1125/590, 2=-1470/770, 6-7=0/ 631/1292 880/269, 3-8=-380/26 uniform roof live load (3-second gust) DL=0.0psf; h=0ft; C. (2) -0-11-0 to 2-1-0, or (2) 6-11-8 to '-10-0, Exterior (2) : and right exposed ; er DOL=1.60 plate gr oof live load: Lumbe 20.0 psf (ground <i>r</i> : Lumber DOL=1.15 p C; Partially Exp.;	4) 5) d or 6) 7) 8) , LC 17 59 1 at. rip r	This truss has except as not This truss has load of 12.0 p overhangs not chord live loa * This truss has chord live loa * This truss h on the bottom 3-06-00 tall b chord and an Provide mech bearing plate joint 2 and 52 DAD CASE(S)	s been checked for ied. s been designed for osf or 2.00 times flat on-concurrent with c s been designed for d nonconcurrent wi as been designed fin chord in all areas i y 2-00-00 wide will y other members. nanical connection ( capable of withstar 14 lb uplift at joint 6. Standard	uniform r greate t roof lc other liv r a 10.0 where a fit betw 'by othe adding 5.	n snow load of er of min roof rad of 15.4 p: e loads. psf bottom other live loa e load of 20.0 a rectangle een the botto ers) of truss t 24 lb uplift at	only, f live sf on dds. 0psf om to t		Minimu.		SEA 0363	ROUL 22 E.R. AL	

Septemb September 18,2020

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Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	C02	Hip Girder	1	1	Job Reference (optional)	E14880770
Carter Components (Sanford), Sa	nford, NC - 27332,	Run: 8.42 S Aug 2	2020 Print: 8.4	420 S Aug 25	2020 MiTek Industries, Inc. Fri Sep 18 08:30:19	Page: 1

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	-0-1	11-0 3-4-	12	6-5-10	I	9-11-8	1	13-5-6	1	16-6-	4	1	19-11-0	20-10-0
	0-1	1-0 3-4-	12	3-0-14		3-5-14		3-5-14		3-0-1	4	I	3-4-12	0-11-0
			12 5 F	NA	ILED N/	AILED N	IAILED	NAILED	NAILE	D				
3-4-12		2 4x5=	3 3 15 29	NAILED 3x5 = 17 16 16 5 22 2 44 II NAILED NA	2314 3x5 =	18 18 18 18 18 18 18 18 18 18	5 13 3x8 = MAILED	19 19 25 NAILED	6 20 6 20 1226 3x5 = NAILE		NAILI 2 11 3x NAILI	ED 3x5 ≈ 17 10 5= 2x4 ⊪ ED	10.11.0	8 9 4x5=
		3-4-	12	2-11-2		3-7-10		3-7-10		2-11-	-2		3-4-12	
Scale = $1:41.5$ Plate Offsets (	X Y)· [4·0-3-0 0-2-4]	[6:0-3-0 0-2-4]												
	, , , , . [ <del>+</del> .0 <sup>.</sup> 0 <sup>.</sup> 0,0 <sup>.</sup> 2 <sup>.</sup> 4],	0					D		(1-)	1.1.7	1	DI 4		
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Inc. Code	1-11-2 1.00 1.25 r NO IRC20	4 015/TPI2014	CSI TC BC WB Matrix-S	0.25 0.49 0.10	DEFL Vert(LL) Vert(CT) Horz(CT	in 0.07 -0.13 ) 0.05	(loc)   13 > 13 > 8	//defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight:	<b>6 G</b> 2 101 lb F	<b>RIP</b> 44/190 T = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Structural wood shea 3-11-1 oc purlins, ex 2-0-0 oc purlins (3-1	athing directly ap cept 1-15 max.): 4-6.	plied or	<ol> <li>Wind: ASCE Vasd=103m II; Exp C; Er right expose DOL=1.60 p</li> <li>TCLL: ASCE DOL=1.25 P snow); Pf=20</li> </ol>	7-10; Vult=1 ph; TCDL=0. closed; C-C d; end vertic late grip DOL 7-10; Pr=20 late DOL=1.0 0.4 psf (flat ro	30mph (3-se 0psf; BCDL= Exterior (2); a cal right exposi- 1.00 0.0 psf (roof li 00); Pg=20.0 bof snow: Lur	cond gust) 0.0psf; h=0 cantilever le sed; Lumbe ve load: Lu psf (ground nber DOL=	fft; Cat. eft and er mber d 1. 1.15	Conc Ve 18 23	centrate ert: 11= =-13 (E =-77 (E	ed Loa -47 (B 3), 19= 3), 24=	ds (lb) ), 13=-12 =-13 (B), =-12 (B),	2 (B), 5=-1; 21=-26 (B) 25=-12 (B)	8 (B), 16=-26 (B), , 22=-47 (B), , 26=-77 (B)
BOT CHORD	Rigid ceiling directly bracing.	applied or 7-5-12	2 oc	Plate DOL=1 Ct=1.10, Lu=	1.00); Catego =0-0-0	ory II; Exp C;	Partially Ex	p.;						
REACTIONS	(size) 2=0-5-8, 8 Max Uplift 2=-524 (Lo Max Grav 2=1140 (L	8=0-5-8 C 6), 8=-524 (LC .C 11), 8=1140 (I	6) _C 11)	except as no 5) This truss ha	oted. as been desig	gned for grea	ter of min r	oof live						
FORCES	(lb) - Maximum Com Tension	pression/Maximu	ım	overhangs n 6) Provide ade	on-concurrer	nt with other I	ive loads.	lina						
TOP CHORD	1-2=0/16, 2-3=-2183 16-17=-1924/706, 4- 4-18=-2030/768, 5-1 5-19=-2030/768, 6-1 6-20=-1902/707, 20- 7-21=-1958/699, 7-8 2-15=-627/1912, 15 22-23=-627/1912, 14 14-24=-519/1733, 13 13-25=-519/1734, 12 12-26=-627/1912, 11 10-11=-627/1912, 11	/767, 3-16=-195 17=-1901/707, 8=-2030/768, 9=-2030/768, 21=-1925/706, =-2183/767, 8-9: 22=-627/1912, 1-23=-627/1912, 3-24=-519/1733, 2-25=-519/1734, 1-26=-627/1912	7/699, =0/16	<ul> <li>7) This truss hat chord live loc</li> <li>8) * This truss live loc</li> <li>8) * This truss live loc</li> <li>3-06-00 tall live loc</li> <li>3-06-00 tall live loc</li> <li>9) Provide mechord and al</li> <li>9) Provide mechora loc</li> <li>9) bearing plate joint 2 and 5</li> <li>10) Graphical puor the orient</li> </ul>	as been desig as been desig ad nonconcu has been des m chord in all by 2-00-00 wi y other mem shanical conn e capable of 1 24 lb uplift at urlin represen ation of the p	gned for a 10 rrent with any signed for a li I areas where ide will fit bet hbers. lection (by otl withstanding joint 8. tation does r urlin along th	of point point of point point of other live we load of 2 a a rectangl ween the b hers) of trus 524 lb uplif ot depict the e top and/o	m bads. 20.0psf e ottom ss to t at ne size or		4	the state of the s	ORTH	CAR	
WEBS	3-15=0/138, 3-14=-1 4-13=-114/337, 5-13 6-13=-114/337, 6-12 7-12=-185/121, 7-10	86/121, 4-14=-46 =-361/213, =-46/299, =0/137	6/298,	bottom chord 11) "NAILED" in (0.148"x3.25 12) In the LOAD of the truss a	d. dicates 3-10c i") toe-nails p CASE(S) se are noted as t	d (0.148"x3") ber NDS guid ection, loads a front (F) or ba	or 3-12d ines. applied to th ack (B).	ne face		111111		C	SEAL 36322	2
NOTES 1) This truss only, exce	TES         This truss has been checked for uniform roof live load only, except as noted.         1)       Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.00         Uniform Loads (lb/ft)         Vert: 1-4=-49, 4-6=-59, 6-9=-49, 2-8=-19						BERTINIA 8,2020							



Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	D01	Monopitch	6	1	Job Reference (optional)	E14880771

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:19 Page: 1 ID:ai72NcUF55RpiWEchgo6bKycavL-Mock Me -0-11-0 8-0-0 0-11-0 8-0-0 2x4 u 12 3 Г 3 ð 6 2-3-14 2-6-6 5 2 4 3x5 = 2x4 🛛 8-0-0 Plate Offsets (X, Y): [2:0-3-4,Edge] Spacing 2-0-0 CSI DEFL in l/defl L/d PLATES GRIP (psf) (loc) 20.0 Plate Grip DOL 1.00 тс 0.88 Vert(LL) -0.21 2-4 >437 240 MT20 244/190 15.4/20.0 Lumber DOL 1.25 BC 0.80 Vert(CT) -0.42 2-4 >218 180 10.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 4 n/a n/a 0.0 Code IRC2015/TPI2014 Matrix-P

BCDL 10.0 LUMBER 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on TOP CHORD 2x4 SP No 1 overhangs non-concurrent with other live loads. BOT CHORD 2x4 SP No 2 WEBS 2x4 SP No.3 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. BRACING \* This truss has been designed for a live load of 20.0psf 7) TOP CHORD Structural wood sheathing directly applied or on the bottom chord in all areas where a rectangle 2-2-0 oc purlins, except end verticals. 3-06-00 tall by 2-00-00 wide will fit between the bottom BOT CHORD Rigid ceiling directly applied or 10-0-0 oc chord and any other members. bracing. Refer to girder(s) for truss to truss connections. REACTIONS (size) 2=0-3-8, 4= Mechanical Provide mechanical connection (by others) of truss to Max Horiz 2=41 (LC 7) bearing plate capable of withstanding 214 lb uplift at Max Uplift 2=-299 (LC 6), 4=-214 (LC 6) joint 4 and 299 lb uplift at joint 2. Max Grav 2=383 (LC 11), 4=330 (LC 11) LOAD CASE(S) Standard FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/16, 2-5=-54/0, 5-6=-20/0, 3-6=-15/45, 3-4=-253/214 BOT CHORD 2-4=-42/46

#### NOTES

Scale = 1:25.1

Loading

TCDL

BCLL

TCLL (roof)

Snow (Pf/Pg)

1) This truss has been checked for uniform roof live load only, except as noted.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=0.0psf; BCDL=0.0psf; h=0ft; Cat. II; Exp C; Enclosed; C-C Exterior (2) -0-11-0 to 2-1-0, Interior (1) 2-1-0 to 4-10-4, Exterior (2) 4-10-4 to 7-10-4; cantilever left and right exposed ; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.00
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber 3) DOL=1.25 Plate DOL=1.00); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.00); Category II; Exp C; Partially Exp.; Ct=1.10
- 4) This truss has been checked for uniform snow load only, except as noted.



Weight: 28 lb

FT = 20%

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	D02	Monopitch Supported Gable	2	1	Job Reference (optional)	E14880772

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			1									1	
		-0-1	11-0			8-	0-0 0-0					-	
	2-6-6 0-3-14		2	¢	9	3	12 2x4 <b>II</b> 10		2x4 II		2	< <sup>4</sup> II 6	2.3-14
				3x5 =			8 2x4 II		7 2x4 ш		2:	×4 II	
Scale = 1:26			-			8-	0-0					-	
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	1-11-4 1.00 1.25 YES IRC2015/	TPI2014	<b>CSI</b> TC BC WB Matrix-P	0.18 0.12 0.08	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 6	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 31 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS OTHERS BRACING TOP CHORD BOT CHORD REACTIONS FORCES TOP CHORD BOT CHORD WEBS NOTES	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 2x4 SP No.3 2x4 SP No.3 Structural wood shea 6-0-0 oc purlins, exc Rigid ceiling directly bracing. (size) 2=8-0-0, 6 Max Horiz 2=83 (LC Max Uplift 2=-253 (L 7=-152 (LI Max Grav 2=188 (LC (LC 11), 8 (Ib) - Maximum Com Tension 1-2=0/16, 2-9=-72/0, 4-10=-33/6, 4-5=-41/ 2-8=-40/53, 7-8=-40/ 4-7=-89/152, 3-8=-20	athing directly applied cept end verticals. applied or 10-0-0 oc 5=8-0-0, 7=8-0-0, 8=8- 6) C 6), 6=-70 (LC 6), C 6), 8=-284 (LC 6) C 2), 6=67 (LC 2), 7=1 =340 (LC 11) apression/Maximum , 3-9=-72/32, 3-10=-41 /34, 5-6=-50/70 /53, 6-7=-40/53 61/284	4) 5) or 6) 0-0 7) 8) 9) 13 10) /0, 11) LOA	TCLL: ASCE DOL=1.25 Plas snow); Pf=15 Plate DOL=1. Ct=1.10 This truss have coverhangs not Gable require Gable studs s This truss have chord live loa * This truss have chord live loa * This truss have chord live loa extension the bottom 3-06-00 tall b chord and an Provide mect bearing plate 6, 253 lb uplif uplift at joint & AD CASE(S)	7-10; Pr=20.0 p ate DOL=1.00); .4 psf (flat roof s .00); Category II s been checked ted. s been designee osf or 2.00 times on-concurrent wi se continuous be spaced at 2-0-0 s been designee d nonconcurren as been designen chord in all are y 2-00-00 wide y other member nanical connecti capable of with t at joint 2, 152 3. Standard	esf (roof live Pg=20.0 p snow: Lum I; Exp C; P for uniforn d for greate flat roof lc ith other live totom chore oc. d for a 10.0 t with any ed for a live as where will fit betw 's. on (by othe standing 7 lb uplift at	e load: Lumb sf (ground ber DOL=1.1 artially Exp.; n snow load of er of min roof vad of 15.4 ps e load of 15.4 ps e load of 15.4 ps e load of 20.0 a rectangle een the botto ers) of truss t 0 lb uplift at j joint 7 and 28	er 5 only, live sf on ds. opsf om o int 34 lb				11111111111111111111111111111111111111	ARO.
<ol> <li>This truss only, exce</li> <li>Wind: ASt Vasd=103 II; Exp C; Exterior (2 cantilever exposed;</li> <li>Truss des only. For see Stanc or consult</li> </ol>	thas been checked for apt as noted. CE 7-10; Vult=130mph 3mph; TCDL=0.0psf; B0 Enclosed; C-C Corner 2) 2-1-0 to 4-10-4, Corner 2) 4-10-4,	uniform roof live load (3-second gust) CDL=0.0psf; h=0ft; Ca (3) -0-11-0 to 2-1-0, her (3) 4-10-4 to 7-10-4; ; end vertical right te grip DOL=1.00 the plane of the truss (normal to the face), d Details as applicable gner as per ANSI/TPI	t. 4; 1.							M. HILLINS		SEA 0363	AL B22 EEER-RATION BILBERTING BIL



Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J01	Jack-Open	2	1	Job Reference (optional)	E14880773

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-0-11-0		
	1-9-1	3-2-4
0-11-0	1-9-1	1-5-3

Special



# Special <u>1-10-13 3-2-4</u> 1-10-13 1-3-7

Scale = 1:32.9

Plate Offsets (X, Y): [3:0-3-0,0-2-0]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0*	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 NO IRC2015	/TPI2014	<b>CSI</b> TC BC WB Matrix-P	0.13 0.18 0.02	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.01 -0.02 0.02	(loc) 6-7 6-7 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL	10.0											weight: 15 b	F1 = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD REACTIONS FORCES TOP CHORD	2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 *Except Structural wood shea 3-2-4 oc purlins; a-4 Rigid ceiling directly bracing. (size) 4= Mecha 7=0-5-8 Max Horiz 7=124 (LC Max Uplift 4=-51 (LC (LC 6) Max Grav 4=86 (LC (LC 2) (lb) - Maximum Com Tension 2-7=-158/112, 1-2=0	<ul> <li>* 3-6:2x4 SP No.3</li> <li>athing directly applied end verticals, and applied or 6-0-0 oc</li> <li>nical, 5= Mechanical,</li> <li>6), 5=-14 (LC 6), 7=-</li> <li>11), 5=93 (LC 11), 7=</li> <li>pression/Maximum</li> <li>/52, 2-3=-69/20, 3-4=</li> </ul>	4) 5) d 7) 8) -111 <sup>9</sup> ) -222 11) -222 11)	This truss has except as not This truss has load of 12.0 p overhangs no Provide adeq This truss has chord live loa * This truss has chord live loa * This truss has chord live loa * This truss has chord all b chord and an Refer to girde Provide mect bearing plate 7, 51 lb uplift Graphical put or the orienta bottom chord Hanger(s) or provided suffi	s been checked for ed. s been designed for sf or 2.00 times flat on-concurrent with o uate drainage to pre- s been designed for d nonconcurrent wit as been designed for d nonconcurrent wit d nonconcurent wit d nonconcurrent wit d nonconcurent wit d nonco	uniform greate roof lo ther liv event v a 10.0 h any vor a liv where it betw where it betw ss com by othe ding 1 uplift a coes no ng the evice(s scentra	n snow load of er of min roof rad of 15.4 p: re loads. vater ponding psf bottom other live load e load of 20.0 a rectangle een the botto nections. ers) of truss t t louplift at t joint 5. t depict the s top and/or e shall be ted load(s) 2	only, live sf on g. ds. )psf co ; joint size 64					
BOT CHORD WEBS NOTES 1) This truss only, exce 2) Wind: AS Vasd=100 II; Exp C; right expc DOL=1.61 SDOL=1.21 Snow); Pf Plate DOI Ct=1.10, I	6-7=-7/6, 5-6=0/0 3-6=-78/83 s has been checked for ept as noted. CE 7-10; Vult=130mph 3mph; TCDL=0.0psf; BC Enclosed; C-C Exterior osed ; end vertical right 0 plate grip DOL=1.00 SCE 7-10; Pr=20.0 psf (r 5 Plate DOL=1.00); Pg= =20.4 psf (flat roof snov L=1.00); Category II; Ex Lu=0-0-0	uniform roof live load (3-second gust) DL=0.0psf; h=0ft; Ca (2); cantilever left an exposed; Lumber roof live load: Lumber -20.0 psf (ground v: Lumber DOL=1.15 p C; Partially Exp.;	13) <b>LO</b> at. 1) id	Ib down at 1- on bottom ch connection dd In the LOAD of the truss a <b>AD CASE(S)</b> Dead + Sno Increase=1. Uniform Loa Vert: 1-2= Concentrate Vert: 3=-2	9-1 on top chord, ar ord. The design/sel avice(s) is the respo CASE(S) section, lo re noted as front (F) Standard w (balanced): Lumb 00 ds (lb/ft) 51, 2-3=-51, 3-4=- d Loads (lb) 26 (B), 6=-26 (B)	nd 69 l ection nsibilit ads ap or bac er Inci 61, 5-7	b down at 1- of such y of others. pplied to the f k (B). ease=1.15, F	9-1 face Plate		M. HILLING		SEA O363	L 22 EEER.K

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



September 18,2020

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J02	Jack-Open	2	1	Job Reference (optional)	E14880774

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3-10-12

Scale =	1:23.4
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00010 = 1.23.4	r											
Loading	(psf)	Spacing	2-0-0	CSI	0.64		in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (root)	20.0	Plate Grip DOL	1.00		0.21	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
Snow (PT/Pg)	15.4/20.0	Lumber DOL Bon Stroop Inor	1.25 VES	BC	0.15		-0.02	2-4	>999	180		
BCU	10.0	Code	IEC2015/TDI201/	Matrix-P	0.00		0.00	3	n/a	n/a		
BCDL	10.0	Code	IKC2015/1112012								Weight: 14 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD BRACING TOP CHORD	<ul> <li>2x4 SP No.2</li> <li>2x4 SP No.2</li> <li>Structural wood she 3-10-12 oc purlins.</li> </ul>	eathing directly appli	7) * This tr on the b 3-06-00 chord a ed or 8) Refer to 9) Provide	uss has been design ottom chord in all ar tall by 2-00-00 wide nd any other membe girder(s) for truss t mechanical connec	ned for a liv reas where e will fit betw ers. to truss con tion (by oth	e load of 20. a rectangle yeen the bott nections. ers) of truss	0psf com to					
BOT CHORD	<ul> <li>Rigid ceiling directly bracing.</li> </ul>	/ applied or 10-0-0 o	c joint 3 a	nd 206 lb uplift at joi	int 2.	42 ID UPIIIT A	t					
REACTIONS	(size) 2=0-5-8, Mechanie Max Horiz 2=142 (L Max Uplift 2=-206 (I Max Gray 2=224 (I	3= Mechanical, 4= cal C 6) _C 6), 3=-142 (LC 6) C 2), 3=126 (LC 11)	LOAD CAS	E(S) Standard								
	(LC 5)	0 2), 0=120 (20 11)	,									
FORCES	(lb) - Maximum Cor Tension	npression/Maximum										
TOP CHORD	1-2=0/17, 2-3=-62/4	18										
BOT CHORD	2-4=0/0											
NOTES												
1) This truss	s has been checked for	r uniform roof live loa	ad									
only, exce	ept as noted.	(0,										
2) Wind: AS Vasd=10: II; Exp C; right expo	CE 7-10; Vult=130mpl 3mph; TCDL=0.0psf; E Enclosed; C-C Exterio psed ; end vertical righ	G3-second gust) CDL=0.0psf; h=0ft; or (2); cantilever left a t exposed; Lumber	Cat. and							The	OPTH CA	ROUT
3) TCLL: AS DOL=1.2 snow); Pf Plate DO Ct=1 10	SCE 7-10; Pr=20.0 psf 5 Plate DOL=1.00); Pg i=15.4 psf (flat roof snc L=1.00); Category II; E	(roof live load: Lumb =20.0 psf (ground w: Lumber DOL=1.1 xp C; Partially Exp.;	ber 15						<b>U</b> THIN		SEA 0363	L 22
<ol> <li>This truss</li> </ol>	s has been checked for	uniform snow load	only,						1			1 3
5) This truss	s has been designed fo	or greater of min roof	live							1.0	N. ENOIN	FER. X
load of 12 overhang	2.0 psf or 2.00 times fla Is non-concurrent with	at roof load of 15.4 pa other live loads.	sf on							14	ALC AIN	BERIN
6) This truss	s has been designed for	or a 10.0 psf bottom	ids.								A. C	all'unit
											Septembe	er 18,2020



Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J03	Jack-Open	3	1	Job Reference (optional)	E14880775

1-11-7

0-9-14

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1-9-6

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-1-10-10	2-7-8
1-10-10	2-7-8



12 4.37 □

3x8 II



Scale = 1:24.3

Plate Offsets (X, Y): [5:0-2-11,0-1-0]

														_
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL	(psf) 20.0 15.4/20.0 10.0 0.0*	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 YES IRC2015	5/TPI2014	<b>CSI</b> TC BC WB Matrix-R	0.57 0.20 0.00	<b>DEFL</b> Vert(LL) Vert(CT) Horz(CT)	in 0.01 0.01 -0.01	(loc) 4-5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 244/190	
BCDL	10.0											Weight: 12 lb	FI = 20%	_
LUMBER TOP CHORD 30T CHORD WEBS BRACING TOP CHORD 30T CHORD REACTIONS	$\begin{array}{l} 2x4 \ \text{SP No.2} \\ 2x4 \ \text{SP No.2} \\ 2x6 \ \text{SP No.2} \\ 2x6 \ \text{SP No.2} \\ \hline \end{array}$	athing directly applie cept end verticals. applied or 10-0-0 oc nical, 4= Mechanica C 6) 6), 4=-13 (LC 8), 5= 11), 4=35 (LC 5), 5=	5) 6) d or 7) : I, 9) :-499 LC :298	This truss ha load of 12.0 p overhangs no This truss ha chord live loa * This truss h on the botton 3-06-00 tall b chord and an Refer to gird@ Provide mech bearing plate joint 5, 66 lb DAD CASE(S)	s been designed fo osf or 2.00 times fla on-concurrent with of s been designed fo d nonconcurrent wi as been designed fo chord in all areas y 2-00-00 wide will y other members. r(s) for truss to tru- nanical connection capable of withstau uplift at joint 3 and Standard	r greate t roof lc other liv r a 10.0 tih any for a livu where fit betw ss conr (by othe nding 4 13 lb up	er of min roof ad of 15.4 p re loads. p psf bottom other live loa e load of 20.1 a rectangle een the bott nections. ers) of truss t 99 lb uplift at blift at joint 4.	live sf on dds. Dpsf om to						
FORCES	(lb) - Maximum Com	pression/Maximum												
	l ension 2-5238/470 1-2-0	/59 2-336/17												
BOT CHORD	4-5=0/0	100, 2 0= 00/11												
NOTES												minin	UIL.	
1) This truss	has been checked for	uniform roof live load	b									IN'TH CA	ROUL	
only, exce	pt as noted.	(0									N	R	LIN	
<ol> <li>vinici ASI Vasd=103</li> <li>Lis Exp C; right expo DOL=1.60</li> <li>TCLL: ASI DOL=1.25</li> <li>snow); Pfr Plate DOL Ct=1.10</li> <li>This truss except asi</li> </ol>	LE /-10; Vult=130mpn imph; TCDL=0.0psf; BC Enclosed; C-C Corner sed ; end vertical right ) plate grip DOL=1.00 CE 7-10; Pr=20.0 psf (i ; Plate DOL=1.00); Pg= =15.4 psf (flat roof snov =1.00); Category II; Ex has been checked for noted.	(3-second gust) CDL=0.0psf; h=0ft; C (3); cantilever left an exposed; Lumber roof live load: Lumbe =20.0 psf (ground v: Lumber DOL=1.1t p C; Partially Exp.; uniform snow load o	Cat. Id 5 nly,							Manutan		SEA 0363	L 22 EER.H ILBERTIT	
												Septembe	r 18,2020	

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J04	Jack-Open	6	1	Job Reference (optional)	E14880776

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:20 ID:0yKyOZueH5kUBZi?uVht1qzsBXb-Mock Me

3-1-14

3-1-14

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GRIP

244/190

FT = 20%

		2.0.3	2	1	2 Jacobian J	5=		3			1-10-0	_
Scale = 1:22.9					-	3	-1-14		-			
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 YES IRC2015	j/TPI2014	CSI TC BC WB Matrix-P	0.12 0.09 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 -0.01 0.00	(loc) 2-4 2-4 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 11 lb
LUMBER TOP CHORD BOT CHORD BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.2 2x4 SP No.2 Structural wood shea 3-1-14 oc purlins. Rigid ceiling directly bracing. (size) 2=0-5-8, 3 Mechanica Max Horiz 2=120 (LC Max Uplift 2=-187 (L1 Max Grav 2=197 (LC (LC 5)	athing directly applied applied or 10-0-0 oc 3= Mechanical, 4= al C 6) C 6), 3=-109 (LC 6) C 2), 3=98 (LC 11), 4=	7) t or 8) 9) <b>LO</b> =57	* This truss I on the bottor 3-06-00 tall I chord and ar Refer to gird Provide mec bearing plate joint 3 and 1 AD CASE(S)	nas been desi m chord in all by 2-00-00 wi yy other mem er(s) for truss hanical conne e capable of v 87 lb uplift at Standard	igned for a live areas where a de will fit betw bers. s to truss coni ection (by oth vithstanding 1 joint 2.	e load of 20.0 a rectangle een the botto nections. ers) of truss t 09 lb uplift at	Opsf om to				
FORCES TOP CHORD BOT CHORD NOTES 1) This truss i only, excep 2) Wind: ASC Vasd=1031 II; Exp C; right expos DOL=1.60 3) TCLL: ASC DOL=1.25 snow); Pf= Plate DOL	(lb) - Maximum Com Tension 1-2=0/14, 2-3=-51/30 2-4=0/0 has been checked for ot as noted. E 7-10; Vult=130mph mph; TCDL=0.0psf; BC Enclosed; C-C Exterior plate grip DOL=1.00 CE 7-10; Pr=20.0 psf (I Plate DOL=1.00); Pg= (15.4 psf (flat roof snow =1.00); Category II; E>	pression/Maximum a uniform roof live load (3-second gust) CDL=0.0psf; h=0ft; C; (2); cantilever left ar exposed; Lumber roof live load: Lumber =20.0 psf (ground w: Lumber DDL=1.15 cp C; Partially Exp.;	at. Id							Ch.		ORTH C ORTEES SE

- Ct=1.10 4) This truss has been checked for uniform snow load only, except as noted.
- This truss has been designed for greater of min roof live 5) load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

annunun a SEAL annun III. 036322 A. GILB September 18,2020

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





-0-11-0

0-11-0

Scale	= 1:22.9	

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J05	Jack-Open Girder	3	1	Job Reference (optional)	E14880777

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:20 ID:zcz8N26Ypx7nzTffV?XKIrzsBXI-Mock Me

Page: 1

	0-11-0			
		1-4-3	3-6-13	
		1-4-3	2-2-11	
1	0-11-0			



## Special



### Special



Scale = 1:37.4

Plate Offsets (X, Y): [2:1-0-12,0-2-0], [3:0-3-0,0-1-8]

(psf 20.( 20.4/20.( 10.( 0.( 10.0) 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Left 2x6 SP No.2 Structural wood s 3-6-13 oc purlins 2-0-0 oc purlins:	<ul> <li>Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code</li> <li> 1-9-7</li> <li>heathing directly applie except</li> </ul>	2-0-0 1.00 1.25 NO IRC2015/T 4) 1 6) F 6) F d or 7) 7	TPI2014 This truss has except as note This truss has load of 12.0 p overhangs no Provide adequ	CSI TC BC WB Matrix-P s been checked for ed. been designed fo sf or 2.00 times fla n-concurrent with o	0.10 0.39 0.03	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.04 -0.06 0.06 only,	(loc) 5-6 5-6 4	l/defl >999 >715 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 19 lb	<b>GRIP</b> 244/190 FT = 20%
20.( 20.4/20.( 10.( 0.( 10.( 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Left 2x6 SP No.2 Structural wood s 3-6-13 oc purlins 2-0-0 oc purlins:	Plate Grip DOL Lumber DOL Rep Stress Incr * Code 1-9-7 heathing directly applie except	1.00 1.25 NO IRC2015/T 4) 7 6 5) 7 6) F d or 7) 7	TPI2014 This truss has except as not This truss has oad of 12.0 p overhangs no Provide adequ	TC BC WB Matrix-P been checked for ed. been designed fo sf or 2.00 times fla n-concurrent with o	0.10 0.39 0.03	Vert(LL) Vert(CT) Horz(CT)	-0.04 -0.06 0.06	5-6 5-6 4	>999 >715 n/a	240 180 n/a	MT20 Weight: 19 lb	244/190 FT = 20%
20.4/20. 10.0 0.0 10.0 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Left 2x6 SP No.2 Structural wood s 3-6-13 oc purlins 2-0-0 oc purlins:	Lumber DOL Rep Stress Incr Code	1.25 NO IRC2015/T 4) 1 5) 1 6) F d or 7) 7	TPI2014 This truss has except as not This truss has load of 12.0 p overhangs no Provide adequ	BC WB Matrix-P s been checked for ed. s been designed fo sf or 2.00 times fla n-concurrent with o	0.39 0.03	Vert(CT) Horz(CT)	-0.06 0.06 only,	5-6 4	>715 n/a	180 n/a	Weight: 19 lb	FT = 20%
10.0 0.0 10.0 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Left 2x6 SP No.2 Structural wood s 3-6-13 oc purlins 2-0-0 oc purlins:	* Rep Stress Incr Code 1-9-7 heathing directly applie except	NO IRC2015/T 4) 1 5) 1 1 6) F 6) F d or 7) 1	TPI2014 This truss has except as note This truss has load of 12.0 p poverhangs no Provide adequ	WB Matrix-P s been checked for ed. s been designed fo sf or 2.00 times fla n-concurrent with of	0.03	Horz(CT)	0.06 only,	4	n/a	n/a	Weight: 19 lb	FT = 20%
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2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Left 2x6 SP No.2 Structural wood s 3-6-13 oc purlins 2-0-0 oc purlins:	1-9-7 heathing directly applie except	4) 1 6 5) 1 6 6) F d or 7) 1	This truss has except as not This truss has load of 12.0 p overhangs no Provide adequ	been checked for ed. been designed fo sf or 2.00 times fla n-concurrent with o	uniforn	n snow load	only,					
Rigid ceiling direct bracing. size) 2=0-5- Mecha Max Horiz 2=94 ( Max Uplift 2=-108 (LC 6)	5-4. tty applied or 6-0-0 oc 3, 4= Mechanical, 5= nical .C 6) (LC 6), 4=-72 (LC 6), 5	8) * 9) F 10) F 5=-2	This truss has chord live load * This truss ha on the bottom 3-06-00 tall by chord and any Refer to girde Provide mech bearing plate 4, 108 lb upliff	uate drainage to pr been designed fo d nonconcurrent wi as been designed f chord in all areas / 2-00-00 wide will / other members. r(s) for trus to tru anical connection capable of withstat t at joint 2 and 2 lb	at roof lo other liv revent v or a 10.0 ith any for a live where a fit betw uss conn (by othe nding 7 uplift a	ad of 15.4 p e loads. vater ponding psf bottom other live loa e load of 20.0 a rectangle een the botto nections. ers) of truss t 2 lb uplift at j i joint 5.	f live sf on g. ads. 0psf om to joint					
/lax Grav 2=217 (LC 13	(LC 2), 4=78 (LC 11), 5 )	=116	or the orientat	tion of the purlin al	ong the	top and/or	5126					
(lb) - Maximum C Tension	ompression/Maximum	12) "	12) "NAILED" indicates 3-10d (0.148"x3") or 3-12d									
Iension         DP CHORD       1-2=0/15, 2-3=-66/40, 3-7=0/0, 4-7=0/0         DT CHORD       2-6=-8/15, 6-8=0/0, 5-8=0/0         PEBS       3-6=-164/90         OTES         This truss has been checked for uniform roof live load only, except as noted.         Wind: ASCE 7-10; Vult=130mph (3-second gust)         Vasd=103mph; TCDL=0.0psf; BCDL=0.0psf; h=0ft; Cat.         II; Exp C; Enclosed; C-C Exterior (2); cantilever left and right exposed; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.00         TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.25 Plate DOL=1.00); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.00); Category II; Exp C; Partially Exp.; Ct=1.10, Lu=0-0-0				<ul> <li>(0.148"x3.25") toe-nails per NDS guidlines.</li> <li>3) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 238 lb down and 10 lb up at 1-4-3 on top chord, and 66 lb down and 1 lb up at 1-4-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.</li> <li>4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).</li> <li><b>OAD CASE(S)</b> Standard</li> <li>Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.00 Uniform Loads (lb/ft) Vert: 1-3=-51, 3-4=-61, 2-5=-20 Concentrated Loads (lb) Vert: 3=-5 (F), 6=-1 (F), 7=-4 (F), 8=-9 (F)</li> </ul>					Mannan		SEA 0363	EER. HUMIN
	(LC 6) lax Grav 2=217 (LC 13) (lb) - Maximum Cr Tension 1-2=0/15, 2-3=-66 2-6=-8/15, 6-8=0/ 3-6=-164/90 as been checked f as noted. 57-10; Vult=130m ph; TCDL=0.0psf; hclosed; C-C Exte d; end vertical rig late grip DOL=1.00; F 14te DOL=1.00); F 14te DOL=1.00); C 14te pol, 100); Category II; =0-0-0	(LC 6) flax Grav 2=217 (LC 2), 4=78 (LC 11), 5 (LC 13) (lb) - Maximum Compression/Maximum Tension 1-2e//15, 2-3=-66/40, 3-7=0/0, 4-7=0/0 2-6=-8/15, 6-8=0/0, 5-8=0/0 3-6=-164/90 as been checked for uniform roof live load as noted. 7-710; Vult=130mph (3-second gust) ph; TCDL=0.0psf; BCDL=0.0psf; h=0ft; C hclosed; C-C Exterior (2); cantilever left a d; end vertical right exposed; Lumber late grip DOL=1.00 Pr-20.0 psf (roof live load: Lumber late DDL=1.00); Pg=20.0 psf (ground 0.4 psf (flat roof snow: Lumber DOL=1.1ft 1.00); Category II; Exp C; Partially Exp.; =0-0	(LC 6)(LC 6)(LC 1)Itax Grav $2=217$ (LC 2), $4=78$ (LC 11), $5=116$ (LC 13)(lb) - Maximum Compression/Maximum12)Tension12 $1-2=0/15$ , $2-3=-66/40$ , $3-7=0/0$ , $4-7=0/0$ 13) $2-6=-8/15$ , $6-8=0/0$ , $5-8=0/0$ 13) $3-6=-164/90$ 14)as been checked for uniform roof live load as noted.14) $7-10$ ; Vult=130mph (3-second gust)14) $ph$ ; TCDL=0.0psf; BCDL=0.0psf; h=0ft; Cat. nclosed; C-C Exterior (2); cantilever left and late grip DOL=1.00;10 $7-10$ ; Pr=20.0 psf (roof live load: Lumber late going DOL=1.00;10 $7-10$ ; Pr=20.0 psf (roof live load: Lumber late pol_e1.00; Pg=20.0 psf (ground 0.4 psf (flat roof snow: Lumber DOL=1.15 1.00); Category II; Exp C; Partially Exp.; =0-0-0	<ul> <li>(LC 6)</li> <li>(LC 6)</li> <li>(LC 7)</li> <li>(LC 13)</li> <li>(LC 14)</li> <li>(LC 13)</li> <li>(LC 14)</li> <li>(LC 13)</li> <li>(LC 14)</li> <li></li></ul>	<ul> <li>(LC 6)</li> <li>(LC 13)</li> <li>(Ib) - Maximum Compression/Maximum Tension</li> <li>(L2 13)</li> <li>(Ib) - Maximum Compression/Maximum Tension</li> <li>(Ib) -</li></ul>	<ul> <li>(LC 6)</li> <li>(LC 6)</li> <li>(LC 13)</li> <li>(Ib) - Maximum Compression/Maximum Tension</li> <li>(12) (NALED" indicates 3-10d (0.148"x3") o or the orientation of the purlin along the bottom chord.</li> <li>(12) "NAILED" indicates 3-10d (0.148"x3") o (0.148"x3.25") toe-nails per NDS guidlin</li> <li>(13) Hanger(s) or other connection device(s) provided sufficient to support concentra Ib down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on bottom ch design/selection of such connection device(s) provided sufficient to support concentra Ib down and 10 lb up at 1-4-3 on bottom ch design/selection of such connection device(s) provided sufficient to support concentra Ib down and 10 lb up at 1-4-3 on bottom ch design/selection of such connection device(s) provided sufficient to support concentra Ib down and 10 lb up at 1-4-3 on bottom ch design/selection of such connection device(s) provided sufficient to support concentra Ib down and 10 lb up at 1-4-3 on bottom ch design/selection of such connection device(s) provided sufficient to support concentra Ib down and 10 lb up at 1-4-3 on bottom ch design/selection of such connection device(s) provided sufficient to support concentrated Ib down and 10 lb up at 1-4-3 on bottom ch design/selection of such connection device(s) provided sufficient to support concentrated Ib down and 10 lb up at 1-4-3 on bottom ch design/selection of such connection device(s) provided sufficient to support concentrated Ib down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch down and 10 lb up at 1-4-3 on top ch dow</li></ul>	<ul> <li>(LC 6)</li> <li>(LC 6)</li> <li>(LC 7)</li> <li>(LC 13)</li> <li>(LC 14)</li> <li>(LA 14<sup>1</sup>x3.25<sup>1</sup>) toe-nails per NDS guidlines.</li> <li>(L) Al<sup>2</sup>x3.25<sup>1</sup>) toe-nails per NDS guidlines.</li> <li>(L) Ha<sup>2</sup>x3.25<sup>1</sup>) toe-nails per NDS guidlines.<td><ul> <li>(LC 6) (LC 2), (4=78 (LC 11), 5=116 (LC 13)</li> <li>(Ib) - Maximum Compression/Maximum Tension</li> <li>(12=0/15, 2-3=-66/40, 3-7=0/0, 4-7=0/0</li> <li>2-6=-8/15, 6-8=0/0, 5-8=0/0</li> <li>3-6=-164/90</li> <li>(LT 2) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.</li> <li>(LT 48"x3.25") toe-nails</li></ul></td><td><ul> <li>(LC 6) (LC 13)</li> <li>(H) - Maximum Compression/Maximum Tension</li> <li>(L2 13)</li> <li>(H) - Maximum Compression/Maximum Tension</li> <li>(H) - Second State Contentiated I puritary and 2 and 2 b uplit at joint 2. And 2 b u</li></ul></td><td><ul> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2.</li> <li>4. 108 Ib uplit at joint 2.</li></ul></td><td><ul> <li>4, 108 lb uplift at joint 2 and 2 lb uplift at joint 2.</li> <li>4, 108 lb uplift at joint 2.</li> <li>10 Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.</li> <li>12 "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.</li> <li>13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 238 lb down and 10 lb up at 1-4-3 on top chord, and 66 lb down and 10 lb up at 1-4-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.</li> <li>14) In the LOAD CASE(S) Section, loads applied to the face of the truss are noted as front (F) or back (B).</li> <li>LOAD CASE(S) Standard</li> <li>10 Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.00</li> <li>Uniform Loads (lb/ft)</li> <li>Vert: 13=-51 (F), 6=-1 (F), 7=-4 (F), 8=-9 (F)</li> </ul></td><td><ul> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib up at 1-4-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.</li> <li>5, 7-10; 7, Pr=20.0 psf (roof live load: Lumber lincrease=1.00</li> <li>4, 108 Ib uplift at joint 2, and 5 Ib/It</li> <li>4, 108 Ib uplift at joint 2, and 5 Ib/It</li> <li>4, 108 Ib uplift a</li></ul></td></li></ul>	<ul> <li>(LC 6) (LC 2), (4=78 (LC 11), 5=116 (LC 13)</li> <li>(Ib) - Maximum Compression/Maximum Tension</li> <li>(12=0/15, 2-3=-66/40, 3-7=0/0, 4-7=0/0</li> <li>2-6=-8/15, 6-8=0/0, 5-8=0/0</li> <li>3-6=-164/90</li> <li>(LT 2) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.</li> <li>(LT 48"x3.25") toe-nails</li></ul>	<ul> <li>(LC 6) (LC 13)</li> <li>(H) - Maximum Compression/Maximum Tension</li> <li>(L2 13)</li> <li>(H) - Maximum Compression/Maximum Tension</li> <li>(H) - Second State Contentiated I puritary and 2 and 2 b uplit at joint 2. And 2 b u</li></ul>	<ul> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2 and 2 lb uplit at joint 5.</li> <li>4. 108 Ib uplit at joint 2.</li> <li>4. 108 Ib uplit at joint 2.</li></ul>	<ul> <li>4, 108 lb uplift at joint 2 and 2 lb uplift at joint 2.</li> <li>4, 108 lb uplift at joint 2.</li> <li>10 Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.</li> <li>12 "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.</li> <li>13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 238 lb down and 10 lb up at 1-4-3 on top chord, and 66 lb down and 10 lb up at 1-4-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.</li> <li>14) In the LOAD CASE(S) Section, loads applied to the face of the truss are noted as front (F) or back (B).</li> <li>LOAD CASE(S) Standard</li> <li>10 Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.00</li> <li>Uniform Loads (lb/ft)</li> <li>Vert: 13=-51 (F), 6=-1 (F), 7=-4 (F), 8=-9 (F)</li> </ul>	<ul> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib uplift at joint 2, and 2 Ib uplift at joint 5.</li> <li>4, 108 Ib up at 1-4-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.</li> <li>5, 7-10; 7, Pr=20.0 psf (roof live load: Lumber lincrease=1.00</li> <li>4, 108 Ib uplift at joint 2, and 5 Ib/It</li> <li>4, 108 Ib uplift at joint 2, and 5 Ib/It</li> <li>4, 108 Ib uplift a</li></ul>



Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J06	Jack-Open	3	1	Job Reference (optional)	E14880778

2-5-8

-0-11-0

Carter Components (Sanford), Sanford, NC - 27332,

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3-6-13

Page: 1





Scale = 1:29.4

Plate Offsets (X, Y): [4:0-3-0,0-2-0]

Loading	(psf)	Spacing	2-0-0		csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.00		тс	0.16	Vert(LL)	0.01	2-7	>999	240	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.25		BC	0.22	Vert(CT)	-0.02	2-7	>999	180		
TCDL	10.0	Rep Stress Incr	YES		WB	0.04	Horz(CT)	0.02	5	n/a	n/a		
BCLL	0.0*	Code	IRC20	15/TPI2014	Matrix-P							-	
BCDL	10.0											Weight: 20 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS SLIDER BRACING TOP CHORD	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Left 2x6 SP No.2 1 Structural wood shea 3-6-13 oc purlins, ex 2-0-0 oc purlins: 4-5 Disid oc silica di octione states	I-7-10 athing directly applie ccept	4 5 ed or 7 8	<ul> <li>This truss ha except as no</li> <li>This truss ha load of 12.0 j overhangs no</li> <li>Provide adec</li> <li>This truss ha chord live loa</li> <li>* This truss h</li> </ul>	s been checked fo ted. s been designed fo osf or 2.00 times fla on-concurrent with juate drainage to p s been designed fo d nonconcurrent w as been designed n chord in all areas	r uniforr or greate at roof lo other liv revent v or a 10.0 vith any for a liv	n snow load of er of min roof bad of 15.4 ps re loads. vater ponding ) psf bottom other live loa e load of 20.0 a rectangle	only, live sf on g. ds. Dpsf					
BUICHORD	bracing.	applied or 10-0-0 oc	5	3-06-00 tall b	y 2-00-00 wide will	l fit betw	een the botto	om					
REACTIONS	(size) 2=0-5-8, 5 Mechanic Max Horiz 2=138 (LC Max Uplift 2=-130 (L 6=-104 (L Max Grav 2=202 (LC 11) (LC 11)	5= Mechanical, 6= al C 6) C 6), 5=-39 (LC 6), C 6) C 2), 5=34 (LC 10), 6	9 1 6=149	<ul> <li>Refer to girde</li> <li>Provide mech</li> <li>bearing plate</li> <li>5, 130 lb upli</li> <li>Graphical pu</li> <li>or the orienta</li> <li>bottom chorce</li> </ul>	volter members. er(s) for truss to tru- nanical connection capable of withsta ft at joint 2 and 104 rlin representation ition of the purlin al	uss con (by oth anding 3 1 Ib uplif does no long the	nections. ers) of truss t 9 lb uplift at j t at joint 6. ot depict the s top and/or	o oint size					
1011020	Tension		L	OAD CASE(S)	Standard								
TOP CHORD BOT CHORD WEBS	1-2=0/15, 2-3=-72/48 2-7=-9/9, 6-7=0/0 4-7=-154/141	8, 3-4=-56/60, 4-5=-	1/0									TH CA	ROUT
NOTES											N.	O'.FESS	dia Vin
<ol> <li>This truss only, exce</li> <li>Wind: ASG Vasd=103 II; Exp C; right expo DOL=1.60</li> <li>TCLL: AS DOL=1.25 snow); Pf= Plate DOL Ct=1.10, L</li> </ol>	has been checked for pt as noted. CE 7-10; Vult=130mph Bmph; TCDL=0.0psf; B( Enclosed; C-C Exterior sed; end vertical right ) plate grip DOL=1.00 CE 7-10; Pr=20.0 psf ( 5 Plate DOL=1.00); Pg= =20.4 psf (flat roof snov _=1.00); Category II; Ex- _u=0-0-0	uniform roof live load (3-second gust) CDL=0.0psf; h=0ft; C r (2); cantilever left a exposed; Lumber roof live load: Lumber 20.0 psf (ground w: Lumber DOL=1.1! cp C; Partially Exp.;	d Cat. and er 5									SEA 0363	EER. Human



Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J07	Jack-Open	27	1	Job Reference (optional)	E14880779

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Page: 1



3-6-13

#### Scale = 1:27.3

Loading TCLL (root Snow (Pf/F TCDL	(psf) f) 20.0 Pg) 15.4/20.0 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.00 1.25 YES	CSI TC BC WB	0.32 0.14 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.02 0.01	(loc) 2-5 2-5 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCLL BCDL	0.0* 10.0	Code	IRC2015/1PI2014	Matrix-P							Weight: 19 lb	FT = 20%
LUMBER TOP CHO BOT CHO SLIDER BRACING TOP CHO BOT CHO REACTIO	<ul> <li>RD 2x4 SP No.2</li> <li>RD 2x4 SP No.2</li> <li>Left 2x6 SP No.2 2</li> <li>RD Structural wood sheat 3-6-13 oc purlins.</li> <li>RD Rigid ceiling directly bracing.</li> <li>NS (size) 2=0-5-8, 4</li> <li>Mechanic Max Horiz 2=176 (LC</li> </ul>	2-4-0 athing directly applie applied or 10-0-0 or l= Mechanical, 5= al 2 6)	6) This trus chord live 7) * This tru on the bo 3-06-00 t chord an 8) Refer to 9) Provide r bearing p joint 4 ar LOAD CASE	e has been designed e load nonconcurrent e load nonconcurrent ss has been designer ttom chord in all area all by 2-00-00 wide w d any other members girder(s) for truss to t nechanical connectio late capable of withs d 97 lb uplift at joint 2 (S) Standard	for a 10.0 with any d for a liv as where vill fit betw truss con rruss con n (by oth tanding 1 2.	) psf bottom other live loa e load of 20.0 a rectangle veen the botto nections. ers) of truss t 53 lb uplift at	ids. Dpsf om					
	Max Horiz 2=176 (LC 6) Max Uplift 2=-97 (LC 6), 4=-153 (LC 6) Max Grav 2=202 (LC 2), 4=180 (LC 11), 5=70 (LC 5)											
FORCES	(lb) - Maximum Com Tension	pression/Maximum										
ТОР СНО ВОТ СНО	RD 1-2=0/15, 2-3=-92/78 RD 2-5=0/0	3, 3-4=-92/97										
NOTES												
1) This tr	uss has been checked for	uniform roof live loa	d									
only, e	except as noted.											11
2) Wind: Vasd= II; Exp right e DOL=	ASCE 7-10; Vult=130mph 103mph; TCDL=0.0psf; B( C; Enclosed; C-C Exterior xposed ; end vertical right 1.60 plate grip DOL=1.00	(3-second gust) CDL=0.0psf; h=0ft; 0 (2); cantilever left a exposed; Lumber	Cat. and						4	I. I.	OR OFES	ROLIN
3) TCLL: DOL= snow); Plate I Ct=1.1	ASCE 7-10; Pr=20.0 psf ( 1.25 Plate DOL=1.00); Pg= ; Pf=15.4 psf (flat roof snov DOL=1.00); Category II; Ex 0	roof live load: Lumbe =20.0 psf (ground v: Lumber DOL=1.1 xp C; Partially Exp.;	er 5								SEA 0363	L 22
4) This tr	uss has been checked for	uniform snow load o	only,							-	1. A.	- 1 - E -
except 5) This tr load o overha	t as noted. uss has been designed for f 12.0 psf or 2.00 times flat angs non-concurrent with c	greater of min roof roof load of 15.4 ps ther live loads.	live sf on								A. C	



September 18,2020

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J08	Jack-Open	2	1	Job Reference (optional)	E14880780

-1-3-9

1-3-9

Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:21 ID:NszgIMeSrXFMvt9ZD6yWv?zsB5V-Mock Me

3-9-15

3-9-15

12 2.12

3-9-15





1-0-0

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Scale	= 1:20.8	
ocale	- 1.20.0	

# Plate Offsets (X, Y): [2:0-3-13, Edge]

Loading	(psf)	Spacing	2-0-0		csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.00		тс	0.23	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
Snow (Pf/Pg)	15.4/20.0	Lumber DOL	1.25		BC	0.15	Vert(CT)	-0.02	2-4	>999	180		
TCDL	10.0	Rep Stress Incr	YES		WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2018	5/TPI2014	Matrix-P								
BCDL	10.0											Weight: 14 lb	FT = 20%
LUMBER			5)	This truss ha	s been designed fo	or greate	er of min roo	f live					
TOP CHORD	2x4 SP No.2			load of 12.0	osf or 2.00 times fla	at roof lo	ad of 15.4 p	sf on					
BOT CHORD	2x4 SP No.2			overhangs n	on-concurrent with	other liv	e loads.						
BRACING			6)	This truss ha	s been designed fo	or a 10.0	) psf bottom						
TOP CHORD       Structural wood sheathing directly applied or 3-9-15 oc purlins.       chord live load nonconcurrent with any other live loads.         7)       * This truss has been designed for a live load of 20.0psf													
BOT CHORD	Rigid ceiling directly bracing.	applied or 10-0-0 oc	0	on the bottor 3-06-00 tall b	n chord in all areas y 2-00-00 wide wil	s where I fit betw	a rectangle veen the bott	om					
REACTIONS	(size) 2=0-3-14	, 3= Mechanical, 4=	8)	Refer to gird	er(s) for truss to tru	iss conr	ections.						
	Max Horiz 2=95 (LC	(6)	9)	Provide mec	nanical connection	(by oth	ers) of truss	to					
	Max I Inlift 2=-422 (I	(C, 6) 3=-128 (I C, 6)		bearing plate	capable of withsta	anding 1	28 lb uplift a	t					
	Max Grav 2=249 (1	C(2) $3=115$ (I C 11)	4=72	joint 3 and 42	22 lb uplift at joint 2	2.							
	(LC 5)	0 2), 0=110 (20 11),	/2 LC	AD CASE(S)	Standard								
FORCES	(lb) - Maximum Con Tension	npression/Maximum											
TOP CHORD BOT CHORD	1-2=0/16, 2-5=-24/9 2-4=0/0	9, 3-5=-24/18											
NOTES													
1) This truss	has been checked for	uniform roof live loa	d										
only, exce	ept as noted.											minin	11111
2) Wind: AS	CE 7-10; Vult=130mph	n (3-second gust)										IN TH CA	Roll
Vasd=103	Bmph; TCDL=0.0psf; B	CDL=0.0psf; h=0ft; C	Cat.								1	A	
II; Exp C;	Enclosed; C-C Corner	(3) -1-3-9 to 2-11-6,									A.	O'.EES	No.
Exterior (2	2) 2-11-6 to 3-9-3; can	tilever left and right									11	1P	The Sta
exposed ;	; end vertical right expo	osed; Lumber DOL=1	.60										MIL.
plate grip	DOL=1.00									-		0.54	· · · · · · · · · · · · · · · · · · ·
3) TCLL: AS	CE 7-10; Pr=20.0 psf	(roof live load: Lumbe	er									SEA	L : I
DOL=1.25	5 Plate DOL=1.00); Pg	=20.0 psf (ground	_									0363	22 E
snow); Pf	=15.4 pst (flat roof sno	w: Lumber DOL=1.1	5							-		. 0000	: :
Plate DOI	L=1.00); Category II; E	xp C; Partially Exp.;									-	<b>1</b>	1 2
Ct=1.10											- 1	·	Airi
4) I NIS TRUSS	nas been checked for	uniform show load c	miy,								25	S. GIN	EFRANS
except as	noteu.										11	10	BEN
												11, A. C	allenn
												111111	11111

September 18,2020

TERENGINEERING BY AMITEK Affiliate 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J09	Jack-Open	2	1	Job Reference (optional)	E14880781

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NGINEERING

818 Soundside Road Edenton, NC 27932









2-9-9

3x5 =

Scale = 1:20.9

Plate Offsets (X, Y): [2:0-2-12, Edge]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 YES IRC2015	/TPI2014	CSI TC BC WB Matrix-P	0.10 0.08 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 -0.01 0.00	(loc) 2-4 2-4 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 10 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD BRACING TOP CHORD BOT CHORD	2x4 SP No.2 2x4 SP No.2 Structural wood she 2-9-9 oc purlins. Rigid ceiling directly bracing.	athing directly applied applied or 10-0-0 oc	6) 7) d or 8) 9)	This truss has chord live loa * This truss h on the bottom 3-06-00 tall b chord and an Refer to girde Provide mect	s been designed fo d nonconcurrent wi as been designed f o chord in all areas y 2-00-00 wide will y other members. rr(s) for truss to tru nanical connection	r a 10.0 ith any or a live where fit betw ss conr (by othe	psf bottom other live loa e load of 20.0 a rectangle een the botto nections. ers) of truss t	ids. Opsf om					
REACTIONS	(size) 2=0-3-0, 3 Mechanic Max Horiz 2=67 (LC Max Uplifi 2=-189 (L Max Grav 2=179 (LC (LC 5)	3= Mechanical, 4= al 6) C 6), 3=-81 (LC 6) C 2), 3=85 (LC 14), 4	10) =52 <b>LO</b>	Provide mech bearing plate 3 and 189 lb AD CASE(S)	at joint(s) 2. anical connection ( capable of withstar uplift at joint 2. Standard	(by othe nding 8	ers) of truss t 1 lb uplift at j	o oint					
FORCES	(lb) - Maximum Com	pression/Maximum											
TOP CHORD BOT CHORD	1-2=0/16, 2-3=-24/1 2-4=0/0	9											
NOTES	has been checked for	uniform roof live load	4										
<ul> <li>only, exce</li> <li>Wind: ASt</li> <li>Vasd=103</li> <li>II; Exp C;</li> <li>right expo</li> <li>DOL=1.60</li> <li>TCLL: AS</li> <li>DOL=1.25</li> <li>snow); Pf-</li> <li>Plate DOL</li> <li>Ct=1.10</li> <li>This truss</li> <li>except as</li> <li>This truss</li> </ul>	ept as noted. CE 7-10; Vult=130mph mph; TCDL=0.0psf; B Enclosed; C-C Exterio sed ; end vertical right ) plate grip DOL=1.00 CE 7-10; Pr=20.0 psf ( 5 Plate DOL=1.00); Pg: =15.4 psf (flat roof sno _=1.00); Category II; E: has been checked for noted. has been designed fo	(3-second gust) CDL=0.0psf; h=0ft; C r (2); cantilever left ar exposed; Lumber roof live load: Lumbe =20.0 psf (ground w: Lumber DOL=1.15 xp C; Partially Exp.; uniform snow load or r greater of min roof l	at. nd 5 nly, ive									SEA 0363	ROOM L
load of 12 overhangs	0 psf or 2.00 times fla s non-concurrent with o	t roof load of 15.4 pst other live loads.	fon									Septembe	ILD r 18,2020

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J10	Roof Special Girder	2	1	Job Reference (optional)	E14880782

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:22 ID:RINLvUqsJ88ECApSbmj109zsB5G-Mock Me

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Special



Scale = 1:31.7

# Plate Offsets (X, Y): [2:0-2-12,Edge]

Ct=1.10, Lu=0-0-0

except as noted.

This truss has been checked for uniform snow load only,

4)

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 20.4/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 NO IRC2015	5/TPI2014	CSI TC BC WB Matrix-P	0.11 0.13 0.06	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 -0.01 0.00	(loc) 6 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 20 lb	<b>GRIP</b> 244/190 FT = 20%	
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD	2x4 SP No.2 *Except 2x4 SP No.2 2x4 SP No.3 Structural wood shea 4-9-9 oc purlins, exc 2-0-0 oc purlins: 3-4. Rigid ceiling directly	* 3-4:2x4 SP No.3 athing directly applie ept end verticals, ar applied or 10-0-0 oc	5) 6) 7) d or nd 8)	This truss ha load of 12.0 p overhangs no Provide adeq This truss ha chord live loa * This truss h on the bottom 3-06-00 tall b chord and an	s been designed the set of the se	for greate lat roof lo n other liv prevent v for a 10.0 with any d for a live s where a ill fit betw	er of min roof had of 15.4 p. re loads. vater ponding psf bottom other live loa e load of 20.0 a rectangle een the botto	i live sf on g. uds. Opsf om						
REACTIONS	(size) 2=0-3-0, 5 Max Horiz 2=33 (LC Max Uplift 2=-207 (LC Max Grav 2=262 (LC (h) _ Maximum Com	=0-1-8 6) C 6), 5=-91 (LC 6) ; 2), 5=208 (LC 10)	9) 10)	Bearing at joi using ANSI/T designer sho Provide mech bearing plate	int(s) 5 considers PI 1 angle to grai uld verify capacity nanical connection at joint(s) 5.	parallel to in formula / of bearing n (by othe	o grain value a. Building ng surface. ers) of truss t	to						
TOP CHORD BOT CHORD WEBS	Tension 1-2=0/16, 2-3=-366/3 4-5=-59/69 2-6=-23/318, 5-6=-35 3-6=0/129, 3-5=-343	37, 3-4=-16/17, 5/307 /57	11,	<ul> <li>Provide mech</li> <li>bearing plate</li> <li>5 and 207 lb</li> <li>Graphical put</li> <li>or the orienta</li> <li>bottom chord</li> </ul>	nanical connection capable of withst uplift at joint 2. rlin representation tion of the purlin a	n (by othe anding 9 n does no along the	ers) of truss t 1 lb uplift at j t depict the s top and/or	io ioint size						
NOTES 1) This truss only, exce 2) Wind: ASC Vasd=103 II: Exp C:	has been checked for pt as noted. CE 7-10; Vult=130mph mph; TCDL=0.0psf; BC Enclosed; C-C Exterior	uniform roof live load (3-second gust) CDL=0.0psf; h=0ft; C (2): cantilever left a	13) d cat. nd	Hanger(s) or provided suff lb down and down at 2-9- such connect	other connection icient to support c 27 lb up at 2-9-9 9 on bottom chorr tion device(s) is th	device(s) concentra on top ch d. The do ne respor	) shall be ted load(s) 2 oord, and 62 esign/selectionsibility of oth	92 lb on of ners.		4	i i	OR TH CA	ROUT	
right expo DOL=1.60 3) TCLL: ASI DOL=1.25 snow); Pf= Plate DOL	sed ; end vertical right ( plate grip DOL=1.00 CE 7-10; Pr=20.0 psf (r Plate DOL=1.00); Pg= =20.4 psf (flat roof snov .=1.00); Category II; Ex	exposed; Lumber coof live load: Lumbe 20.0 psf (ground v: Lumber DOL=1.15 p C; Partially Exp.;	14) er LO 5	) In the LOAD of the truss a AD CASE(S) Dead + Sno Increase=1. Uniform Loa	CASE(S) section, re noted as front of Standard ww (balanced): Lur 00 ads (lb/ft)	, loads ap (F) or bac mber Incr	oplied to the f ck (B). ease=1.15, I	face Plate		THILLIN'		SEA 0363	L 22	



818 Soundside Road Edenton, NC 27932

September 18,2020

C

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J11	Monopitch	5	1	Job Reference (optional)	E14880783

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:22 ID:CHsMaDwtRb959PQ\_3SsvKrzsB58-Mock Me

0:22 Page: 1

September 18,2020

818 Soundside Road Edenton, NC 27932



Scale = 1:24.8

Plate Offsets (X, Y): [2:0-2-12,Edge]

Loading         (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0*           BCDL         10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 YES IRC2015/TPI2014	CSI TC 0 BC 0 WB 0 Matrix-P	.35 .25 .00	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.03 -0.05 0.00	(loc) 2-4 2-4 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 17 lb	<b>GRIP</b> 244/190 FT = 20%	
LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING TOP CHORD Structural wood she: 4-9-9 oc purlins, exi BOT CHORD Rigid ceiling directly bracing. REACTIONS (size) 2=0-3-0, 4 Max Horiz 2=44 (LC Max Uplift 2=-252 (L Max Grav 2=251 (LC FORCES (lb) - Maximum Com Tension TOP CHORD 1-2=0/16, 2-3=-36/20 BOT CHORD 2-4=-26/28 NOTES 1) This truss has been checked for only, except as noted. 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=0.0psf; B0 II; Exp C; Enclosed; C-C Exterion right exposed ; end vertical right DOL=1.60 plate grip DOL=1.00 3) TCLL: ASCE 7-10; Pr=20.0 psf ( DOL=1.25 Plate DOL=1.00); Pg- snow); Pf=15.4 psf (flat rof snov Plate DOL=1.00); Category II; Ex Ct=1.10 4) This truss has been checked for except as noted. 5) This truss has been checked for load of 12.0 psf or 2.00 times flat overhangs non-concurrent with c	athing directly applied cept end verticals. applied or 10-0-0 oc i=0-1-8 6) C 6), 4=-153 (LC 6) C 6), 4=195 (LC 11) pression/Maximum 5, 3-4=-150/153 uniform roof live load (3-second gust) CDL=0.0psf; h=0ft; Car (2); cantilever left and exposed; Lumber roof live load: Lumber -20.0 psf (ground v: Lumber DOL=1.15 sp C; Partially Exp.; uniform snow load onl	<ul> <li>6) This truss ha chord live loa</li> <li>7) * This truss h on the bottor 3-06-00 tail the chord and ar</li> <li>8) Bearing at jo using ANSI/ designer sho</li> <li>9) Provide mec bearing plate joint 2 and 1:</li> <li>LOAD CASE(S)</li> </ul>	Is been designed for a ad nonconcurrent with has been designed for n chord in all areas wh yy 2-00-00 wide will fit ny other members. int(s) 4 considers para FPI 1 angle to grain for juld verify capacity of th hanical connection (by a at joint(s) 4. hanical connection (by a capable of withstandi 53 lb uplift at joint 4. Standard	1.0 any o a live here a betwee illel to mula. bearin o othe r othe ng 25	psf bottom ther live loads load of 20.0p rectangle een the botton g grain value . Building g surface. rs) of truss to rs) of truss to is of truss to is 2 lb uplift at	s. isf n		Within		SEA 0363	L L B L B L B L B L B L B L M L M L	

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J12	Jack-Open Structural Gable	2	1	Job Reference (optional)	E14880784

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:22 ID:p9e1htNEzeQxIHd3D?fEHeycavT-Mock Me

Page: 1





Scale = 1:26.1

# Plate Offsets (X, Y): [2:0-5-10,Edge]

(psf) 20.0	Spacing Plate Grip DOL	2-0-0 1.00 1.25		CSI TC BC	0.40	DEFL Vert(LL)	in -0.01 -0.01	(loc) 2-5	l/defl >999	L/d 240 180	PLATES MT20	<b>GRIP</b> 244/190
10.0	Rep Stress Incr	YES		WB	0.00	Horz(CT)	0.00	2-5 4	>333 n/a	n/a		
0.0*	Code	IRC201	5/TPI2014	Matrix-P								
10.0											Weight: 22 lb	FT = 20%
2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Left 2x8 SP 2400F 2 Structural wood shea 3-5-10 oc purlins, ea Rigid ceiling directly bracing. (size) 2=0-10-0, Mechanic Max Horiz 2=112 (LC Max Uplift 2=-486 (L Max Grav 2=278 (LC (LC 5) (lb) - Maximum Com Tension 1-2=0/17, 2-3=-71/1- 4-5=0/0 2-5=-37/41 thas been checked for ept as noted. CE 7-10; Vult=130mph Bmph; TCDL=0.0psf; Bd Enclosed; C-C Corner 2) 2-4-4 to 3-3-14; canti end vertical right expo DOL=1.00 signed for wind loads in studs exposed to wind dard Industry Gable Enc	.0E 2-0-12 athing directly applier xcept end verticals. applied or 10-0-0 oc 4= Mechanical, 5= al 2 6) C 6), 4=-89 (LC 6) 2 2), 4=90 (LC 11), 5 pression/Maximum 4, 3-6=-22/8, 4-6=-19 uniform roof live load (3-second gust) CDL=0.0pst; h=0ft; C (3) -1-10 to 2-4-4 lever left and right sed; Lumber DOL=1 the plane of the trus (normal to the face), d Details as applicab gner as per ANSI/TP	4) d or 6) 7) 8) 9) =66 10 11 12 14 LC sat. , .600 s le, 1.1.	TCLL: ASCE DOL=1.25 PI snow); Pf=15 Plate DOL=1 Ct=1.10 This truss ha except as no This truss ha load of 12.0 g overhangs no Gable studs : This truss ha chord live loa * This truss ha chord live loa * This truss ha on the botton 3-06-00 tall b chord and an ) Refer to girdd ) Provide mecl bearing plate 4 and 486 lb ) Gap betweer diagonal or v DAD CASE(S)	7-10; Pr=20.0 ps ate DOL=1.00); P 4 psf (flat roof sn .00); Category II; s been checked fo ted. s been designed f on-concurrent with spaced at 2-0-0 o s been designed f d nonconcurrent viti as been designed in all area y 2-00-00 wide wi y other members. er(s) for truss to tr nanical connection capable of withst uplift at joint 2. n inside of top cho ertical web shall r Standard	f (roof liv, g=20.0 p ow: Lum Exp C; P or uniform for greate lat roof lo o other liv c. for a 10.0 with any f for a liv s where ill fit betw uss conn n (by oth anding 8 rd bearin tot excee	e load: Lumk sf (ground ber DOL=1. artially Exp.; n snow load er of min roor bad of 15.4 p e load of 15.4 p e load of 15.4 p e load of 20. a rectangle een the bott ections. ers) of truss 9 lb uplift at g and first d 0.500in.	ber 15 ; only, f live sof on ads. Opsf tom to joint				SEA 0363	ROL L 22 EER-R
	$\begin{array}{c} (\text{psf}) \\ 20.0 \\ 15.4/20.0 \\ 10.0 \\ 0.0^* \\ 10.0 \\ 0.0^* \\ 10.0 \\ \end{array}$	(psf) 20.0 15.4/20.0 15.4/20.0 10.0 0.0* 10.0 2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Left 2x8 SP 2400F 2.0E 2-0-12 Structural wood sheathing directly applie 3-5-10 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. (size) 2=0-10-0, 4= Mechanical, 5= Mechanical Max Horiz 2=112 (LC 6) Max Uplift 2=-486 (LC 6), 4=-89 (LC 6) Max Grav 2=278 (LC 2), 4=90 (LC 11), 5 (LC 5) (lb) - Maximum Compression/Maximum Tension 1-2=0/17, 2-3=-71/14, 3-6=-22/8, 4-6=-19 4-5=0/0 2-5=-37/41 thas been checked for uniform roof live load apt as noted. CE 7-10; Vult=130mph (3-second gust) 3mph; TCDL=0.0psf; BCDL=0.0psf; h=0ft; C Enclosed; C-C Corner (3) -1-10-10 to 2-4-4 2) 2-4-4 to 3-3-14; cantilever left and right end vertical right exposed; Lumber DOL=1 DOL=1.00 signed for wind loads in the plane of the trus studs exposed to wind (normal to the face), fard Industry Gable End Details as applicab c qualified building designer as per ANSI/TP	$ \begin{array}{c} (\text{psf}) \\ 20.0 \\ 15.4/20.0 \\ 15.4/20.0 \\ 10.0 \\ 0.0^* \\ 10.0 \end{array} \begin{array}{c} \text{Spacing} \\ \text{Plate Grip DOL} \\ 1.00 \\ \text{Lumber DOL} \\ 1.25 \\ \text{Rep Stress Incr} \\ \text{YES} \\ \text{Code} \end{array} \begin{array}{c} \text{IRC2015} \end{array} \right. \\ \begin{array}{c} \text{Algebre} \\ \text{Code} \\ \text{IRC2015} \end{array} \right. \\ \begin{array}{c} \text{Algebre} \\ \text{Algebre} \\ \text{Algebre} \\ \text{Algebre} \\ \text{YES} \\ \text{Code} \\ \text{IRC2015} \end{array} \right. \\ \begin{array}{c} \text{Algebre} \\ Algebr$	$\begin{array}{c c c c c c c c c c c c c c c c c c c $	(psf) 20.0 15.4/20.0 10.0Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code2-0-0 TC BC WB Watrix-P2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Left 2x8 SP 2400F 2.0E 2-0-124)TCLL: ASCE 7-10; Pr=20.0 ps DOL=1.25 Plate DOL=1.00); PStructural wood sheathing directly applied or 3-5-10 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.4)TCLL: ASCE 7-10; Pr=20.0 ps DOL=1.25 Plate DOL=1.00); Category II; Ct=1.10(size) 2=0-10-0, 4= Mechanical, 5= Mechanical6This truss has been checked for except as noted.(size) 2=0-10-0, 4= Mechanical, 5= Mechanical7)Gable studs spaced at 2-0-0 o orerhangs non-concurrent with 7)(size) 2=0-10-0, 4= Mechanical, 5= Mechanical7)This truss has been designed 1 load of 12.0 psf or 2.00 times fo overhangs non-concurrent with 7)(b) - Maximum Compression/Maximum Tension 1-2=0/17, 2-3=-71/14, 3-6=-22/8, 4-6=-19/16, 4-5=0/0* This truss has been designed 1 oda of 12.0 psf or 2.00 wide with st 4 and 486 lb uplift at joint 2.(b) - Maximum Compression/Maximum Tension 1-2=0/17, 2-3=-71/14, 3-6=-22/8, 4-6=-19/16, 4-5=0/0* This truss has been designed 1 chord and any other members.(b) - Maximum Compression/Maximum Tension 1-2=0/17, 2-3=-71/14, 3-6=-22/8, 4-6=-19/16, 4-5=0/0* This trus has been designed 1 chord and any other members.(b) - Maximum Compression/Maximum Tension 1-2=0/17, 2-3=-71/14, 3-6=-22/8, 4-6=-19/16, 4-5=0/0* This trus has been designed 1 chord and any other members.(c) E7 -10; Vult=130mph (3-second gust) 3mph; TCDL=0.0psf;	$ \begin{array}{ c c c c c } \hline (psf) \\ 20.0 \\ 15.4/20.0 \\ 10.0$	(pst)         Spacing         2-0-0         CSI         DEFL           15.4/20.0         1.42 Grip DOL         1.00         TC         0.40           0.00         10.0         Cde         IRC2015/TPI2014         TC         0.40           2x4 SP No.2         Structural wood sheathing directly applied or 3-5-10 oc purlins, except end verticals.         4)         TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Luml DOL=1.25 Plate DOL=1.00); Ps=20.0 psf (ground snow); Pt=15.4 psf (filtat roof snow: Lumber DOL=1.10); Category II; Exp C; Partially Exp. Ct=1.10           Structural wood sheathing directly applied or 3-5-10 oc purlins, except end verticals.         4)         TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Luml DOL=1.25 Plate DOL=1.00); Category II; Exp C; Partially Exp. Ct=1.10           Structural wood sheathing directly applied or 10-0-0 oc bracing.         (size)         2=0-10-0, 4= Mechanical, 5= Mechanical           Max Horiz 2=112 (LC 6)         Max Uplift 2=-486 (LC 6), 4=-89 (LC 6)         This truss has been designed for a 10.0 psf bottom chord live load 12.0 psf or 2.00 uide will fit between the load 0.20, or the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottor chord live load 12.0 psf or 1.00 psf bottom chord live load 12.0 psf or 1.00 psf bottom chord live load 10.0 psf bottom chord live load 12.0 psf or 1.00 psf bottom chord live load 10.0 psf psf bottom schord live load 10.0 psf bottom chord live load 10.0 psf b	(psf)       Spacing       2-0-0       CSi       DEFL       in         15.4/20.0       1.00       1.00       TC       0.40       Vert(LL)       -0.01         0.0*       0.0*       10.0       IRC2015/TPI2014       BC       0.13       Wert(CT)       -0.01         2x4 SP No.2       2x4 SP No.3       IRC2015/TPI2014       Matrix-P       Horz(CT)       0.00         2x4 SP No.3       ILef 2x8 SP 2400F 2.0E 2-0-12       4)       TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15         Structural wood sheathing directly applied or 3-5-10 oc purins, except and verticals.       Filad ceilling directly applied or 10-0-0 oc bracing.       6)       This truss has been checked for uniform snow load only, except an oted.         (size)       2-01-0.0, 4= Mechanical, 5= Mechanical       Max Horiz 2=112 (LC 6)       6)       This truss has been designed for greater of min roof live loads.         (b) - Maximum Compression/Maximum Tension       12-20/T, 2-3=-71/14, 3-6=-22/8, 4-6=-19/r6, (LC 5)       * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-0-00 wide will fit between the bottom chord in all areas where a rectangle 3-06-00 tall by 2-0-00 wide will fit between the bottom chord in all areas where a rectangle 3-06-00 tall by 2-0-00 wide will fit between the bottom chord in all areas where a rectangle 3-06-00 tall by 2-0-00 wide will fit between the bottom chord in all areas where a rectangle 3-06-00 tall by 2-0-00 w	(psf)       Spacing       2-0-0       CSI       T       0.0       DEFL       in       (ioc)         15.4/20.0       Lumber DOL       1.25       TC       0.40       Vert(LL)       -0.01       2-5         0.0°       0.0°       Code       IRC2015/TPI2014       Matrix-P       Matrix-P       Horz(CT)       0.00       4         2x4 SP No.2       2x4 SP No.3       DL=1.25 Plate DOL=1.00); Pg=20.0 psf (roof live load: Lumber DOL=1.25 Plate DOL=1.00); Pg=20.0 psf (ground snow); Pl=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.00); Pg=20.0 psf (ground snow); Pl=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.00); Pg=20.0 psf (ground snow); Pl=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.00); Pg=20.0 psf (ground snow); Pl=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.00); Pg=20.0 psf (ground snow); Pl=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.00); Pg=20.0 psf (ground snow); Pl=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.00); Pg=20.0 psf (ground snow); Pl=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.00); Plate DOL=1.00; Category II; Exp C; Partially Exp.; Ct=1.10       This truss has been designed for a low load of 12.0 psf of on overhangs non-concurrent with any other live loads.         (size)       2=0-10-0, 4= Mechanical, 5= Mechanical       Max Horiz 2=112 (LC 6)       This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where are catagle 3-0-0 oc.       This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where are catagle 3-0-0 othe bottom chord in all areas where are catagle 3-0-0 oth	(psi)       Spacing       2-0-0       CSI       DEFL       in       (toc)       I/deft         15.4/20.0       10.0       Plate Grip DOL       1.25       TC       0.40       Vert(C1)       -0.01       2-5 >999         0.0       0.0       0.0       10.0       Vert(C1)       -0.01       2-5 >999         2x4 SP No.2       Code       IRC2015/TPI2014       Matrix-P       Vert(C1)       -0.01       2-5 >999         2x4 SP No.2       Code       IRC2015/TPI2014       Matrix-P       Vert(C1)       -0.01       2-5 >999         2x4 SP No.2       Code       Infantian       Spacing       Percentian       Spacing       Percentian       Spacing       Percentian       Spacing       Percentian       Spacing       Percentian       Spacing       Spacing       Percentian       Spacing       Spacing </td <td>(psf) 20.0 15.4/20.0 10.0 10.0 0.0°Spacing Plate Grip DOL 1.00 Lumber DOL 0.0°2-0.0 1.00CSI TC TC C BCDEFL TC 0.40 BC(loc) Vert(CT)(loc) 1.2.5 Vert(CT)(loc) 1.2.5 Vert(CT)Use 2.5 -399180 180 2.5 -3992x4 SP No.2 2x4 SP No.2 (C T)4)TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.05; Plate DOL=1.00; Pg=20.0 psf (ground snow); Pl=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.00; Category II: Exp.; C L=1.10Structural wood sheathing directly applied or 3-6-10 oc puritins, except end verticals. Max Horiz 2 =112 (LC 6) Max Upitit 2=-486 (LC 6), 4=-89 (LC 6) (LC 5)4)Max Horiz 2 = 127 (LC 6) Max Upitit 2=-486 (LC 0), 4=-89 (LC 6) (LC 5)5)6)(b) Maximum Tension 1 = 2=0/17, 2:3=-71/14, 3-6=-22/8, 4-6=-19/16, 4=5=007)Gable studs spaced at 2-0-0c. 8)(b) Maximum Compression/Maximum Tension 1 = 2=0/17, 2:3=-71/14, 3-6=-22/8, 4-6=-19/16, 4=5=007)Gable studs spaced at 2-0-00. 9)(b) Maximum Tension 1 = 2=0/17, 2:3=-71/14, 3-6=-22/8, 4-6=-19/16, 1.2=0/17, 2:3=-71/14, 3-6=-22/8, 4-6=-19/16, 1.1(b) Addition device a space and the space an</td> <td>(ps)         Spacing         2-0-0         CSI         0.4         0.4         Vert(C1)         -0.01         2-5         &gt;999         240         MT20           10.4         20.0         Lumber DOL         1.25         BC         0.13         Vert(C1)         -0.01         2-5         &gt;999         160           0.0         0.0°         Code         IRC2015/TPI2014         WB         0.00         Horz(CT)         0.00         4         n/a         n/a           2x4 SP No.2         SP No.3         Exd SP No.2         SP 2400F 2.0E - 2-0-12         4         TCL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15         Plate DOL=1.25 Plate DOL=1.00); Category II; Exp C; Partially Exp;         Clear 2.24 Pkp;         Clear</td>	(psf) 20.0 15.4/20.0 10.0 10.0 0.0°Spacing Plate Grip DOL 1.00 Lumber DOL 0.0°2-0.0 1.00CSI TC TC C BCDEFL TC 0.40 BC(loc) Vert(CT)(loc) 1.2.5 Vert(CT)(loc) 1.2.5 Vert(CT)Use 2.5 -399180 180 2.5 -3992x4 SP No.2 2x4 SP No.2 (C T)4)TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.05; Plate DOL=1.00; Pg=20.0 psf (ground snow); Pl=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.00; Category II: Exp.; C L=1.10Structural wood sheathing directly applied or 3-6-10 oc puritins, except end verticals. Max Horiz 2 =112 (LC 6) Max Upitit 2=-486 (LC 6), 4=-89 (LC 6) (LC 5)4)Max Horiz 2 = 127 (LC 6) Max Upitit 2=-486 (LC 0), 4=-89 (LC 6) (LC 5)5)6)(b) Maximum Tension 1 = 2=0/17, 2:3=-71/14, 3-6=-22/8, 4-6=-19/16, 4=5=007)Gable studs spaced at 2-0-0c. 8)(b) Maximum Compression/Maximum Tension 1 = 2=0/17, 2:3=-71/14, 3-6=-22/8, 4-6=-19/16, 4=5=007)Gable studs spaced at 2-0-00. 9)(b) Maximum Tension 1 = 2=0/17, 2:3=-71/14, 3-6=-22/8, 4-6=-19/16, 1.2=0/17, 2:3=-71/14, 3-6=-22/8, 4-6=-19/16, 1.1(b) Addition device a space and the space an	(ps)         Spacing         2-0-0         CSI         0.4         0.4         Vert(C1)         -0.01         2-5         >999         240         MT20           10.4         20.0         Lumber DOL         1.25         BC         0.13         Vert(C1)         -0.01         2-5         >999         160           0.0         0.0°         Code         IRC2015/TPI2014         WB         0.00         Horz(CT)         0.00         4         n/a         n/a           2x4 SP No.2         SP No.3         Exd SP No.2         SP 2400F 2.0E - 2-0-12         4         TCL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15         Plate DOL=1.25 Plate DOL=1.00); Category II; Exp C; Partially Exp;         Clear 2.24 Pkp;         Clear

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818 Soundside Road

Edenton, NC 27932

GILB A. GIL September 18,2020

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J13	Jack-Open	2	1	Job Reference (optional)	E14880785

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:23 ID:HLCPvDOskxYoNRCFmiATprycavS-Mock Me

Page: 1





Scale = 1:28.6

#### Plate Offsets (X, Y): [2:0-6-0,0-1-8], [3:0-3-4,0-2-5], [6:0-4-0,0-2-4]

3-2-7

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 15.4/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.25 YES IRC201	5/TPI2014	<b>CSI</b> TC BC WB Matrix-P	0.21 0.31 0.05	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.01 -0.01 0.02	(loc) 5-6 5-6 4	l/defl >999 >999 n/a	L/d 240 180 n/a	<b>PLATES</b> MT20 Weight: 25 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS SLIDER BRACING TOP CHORD	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Left 2x8 SP 2400F 2 Structural wood shea 3-2-4 oc purlins, exa 2-0-0 oc purlins: 3-4.	.0E 1-9-3 athing directly applie cept end verticals, ar	4) 5) 6) d or 7) nd 8)	This truss ha except as no This truss ha load of 12.0 overhangs n Provide adee This truss ha chord live loa * This truss h	as been checked ted. Is been designer psf or 2.00 times on-concurrent w quate drainage t as been designer ad nonconcurrer nas been design	I for uniform d for greate s flat roof Ic ith other liv o prevent v d for a 10.0 tt with any ed for a live	n snow load er of min roo bad of 15.4 p re loads. vater pondin 0 psf bottom other live loa e load of 20.	only, f live sf on g. ads. 0psf					
BOT CHORD	Rigid ceiling directly bracing. (size) 2=0-5-8, 4 Mechanica Max Horiz 2=28 (LC Max Uplift 2=-154 (LL (LC 6) Max Grav 2=185 (LC (LC 11)	applied or 6-0-0 oc 4= Mechanical, 5= al 6) C 6), 4=-3 (LC 7), 5= C 2), 4=5 (LC 2), 5=1	9) 10 =-76 42 11	on the bottor 3-06-00 tall b chord and ar Refer to gird ) Provide mec bearing plate 4, 76 lb upliff ) Graphical pu or the orienta	n chord in all are yy 2-00-00 wide yy other membeu er(s) for truss to hanical connecti e capable of with t at joint 5 and 19 Irlin representation at of the purlin	eas where a will fit betw rs. truss conn ion (by othe istanding 3 54 lb uplift on does no n along the	a rectangle reen the bott ections. ers) of truss Ib uplift at jo at joint 2. t depict the top and/or	om to bint size					
FORCES TOP CHORD BOT CHORD	(lb) - Maximum Com Tension 1-2=0/19, 2-3=-107/ <sup>2</sup> 2-6=-160/143, 5-6=-{	pression/Maximum 189, 3-4=-59/63, 4-5 58/63	12 =0/0 LC	bottom choro ) Gap between diagonal or v DAD CASE(S)	a. n inside of top cł vertical web shal Standard	nord bearin I not excee	g and first d 0.500in.					TH CA	RO
NOTES 1) This truss only, exce 2) Wind: AS( Vasd=103 II; Exp C; right expo DOL=1.6( 3) TCLL: AS DOL=1.25 snow); Pfr Plate DOL	thas been checked for ept as noted. CE 7-10; Vult=130mph mph; TCDL=0.0psf; BC Enclosed; C-C Exterior used ; end vertical right 0 plate grip DOL=1.00 CE 7-10; Pr=20.0 psf (1 5 Plate DOL=1.00); Pg= =15.4 psf (flat roof snov L=1.00); Category II; Ex	uniform roof live load (3-second gust) CDL=0.0psf; h=0ff; C r (2); cantilever left a exposed; Lumber roof live load: Lumbe =20.0 psf (ground w: Lumber DOL=1.1% cp C; Partially Exp.;	d Cat. nd er 5							Contraction of the second seco		SEA 0363	L 22 EEER.H.I.

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.25 Plate DOL=1.00); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.00); Category II; Exp C; Partially Exp.; Ct=1.10, Lu=0-0-0

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



September 18,2020

A. GI A. GILLIN

Job	Truss	Truss Type	Qty	Ply	22 Mitchell MAnor - Hampton El C	
20090082	J14	Jack-Open	3	1	Job Reference (optional)	E14880786

3-2-4

3-2-4

3-2-4

-0-11-0

0-11-0

Carter Components (Sanford), Sanford, NC - 27332,

Run: 8.42 S Aug 25 2020 Print: 8.420 S Aug 25 2020 MiTek Industries, Inc. Fri Sep 18 08:30:23 ID:HLCPvDOskxYoNRCFmiATprycavS-Mock Me

Page: 1



Scale = 1:26.6

Plate Offsets (X, Y): [2:0-1-8,0-0-2]

	,, .). <u>L</u> =,.	1													
Loading	a)	sf)	Spacing	1-11-4		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20	0.0	Plate Grip DOL	1.00		тс	0.24	Vert(LL)	-0.01	2-5	>999	240	MT20	244/190	
Snow (Pf/Pa)	15.4/20	0.0	Lumber DOL	1.25		BC	0.11	Vert(CT)	-0.01	2-5	>999	180	-		
TCDL	1(	0.0	Rep Stress Incr	YES		WB	0.00	Horz(CT)	0.00	4	n/a	n/a			
BCLL	(	0.0*	Code	IRC201	5/TPI2014	Matrix-P		- (- )							
BCDL	10	0.0											Weight: 18 lb	FT = 20%	
LUMBER TOP CHORD SOT CHORD SLIDER BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.2 2x4 SP No.2 Left 2x6 SP No Structural wood 3-2-4 oc purlins Rigid ceiling di bracing. (size) 2=0- Mec Max Horiz 2=15 Max Uplift 2=-9 Max Grav 2=16	.2 2 d shea 5. 5-8, 4 hanica 57 (LC 3 (LC 32 (LC	-0-13 thing directly applie applied or 10-0-0 oc = Mechanical, 5= 1 6) 6), 4=-130 (LC 6) 2), 4=155 (LC 11),	5) 6) ed or 7) c 8) 9) 5=61 LC	This truss ha load of 12.0 j overhangs nu This truss ha chord live loa * This truss h on the bottom 3-06-00 tall b chord and ar Refer to girde Provide mecl bearing plate joint 4 and 93 DAD CASE(S)	s been designed osf or 2.00 times f on-concurrent with s been designed ad nonconcurrent ias been designed n chord in all area y 2-00-00 wide w y other members er(s) for truss to tr hanical connectio capable of withs 3 lb uplift at joint 2 Standard	for greate flat roof lc h other liv for a 10.0 with any d d for a live as where a vill fit betw s. russ conn n (by othe tanding 1: 2.	er of min roof pad of 15.4 p re loads. 0 psf bottom other live loa e load of 20.0 a rectangle reen the bott ections. ers) of truss 1 30 lb uplift at	f live sf on dds. Opsf om to						
FORCES	(lb) - Maximum	Comp	pression/Maximum												
TOP CHORD	1-2=0/18, 2-3= 2-5=0/0	-78/67	, 3-4=-78/83												
NOTES	2 0=0/0														
<ol> <li>This truss h only, excep</li> <li>Wind: ASC Vasd=103m II; Exp C; E right expos DOL=1.60  </li> <li>TCLL: ASC DOL=1.25  </li> <li>snow); Pf= Plate DOL= Ct=1.10</li> <li>This truss h except as n</li> </ol>	has been checked of as noted. E 7-10; Vult=130 mph; TCDL=0.0p Enclosed; C-C E ed end vertical plate grip DOL= 2E 7-10; Pr=20.0 Plate DOL=1.00 15.4 psf (flat roo =1.00); Category has been checked hoted.	ed for u Omph ( ssf; BC tterior right e 1.00 psf (ri ); Pg= f snow II; Ex ed for u	uniform roof live load (3-second gust) DL=0.0psf; h=0ft; C (2); cantilever left a exposed; Lumber oof live load: Lumber 20.0 psf (ground r: Lumber DOL=1.1! p C; Partially Exp.; uniform snow load c	d Cat. Ind er 5 only,							Annual Contraction	King and the second sec	SEA 0363	EEP. 11 122 11 11 11 18,2020	A DAMAN DAMAN



