

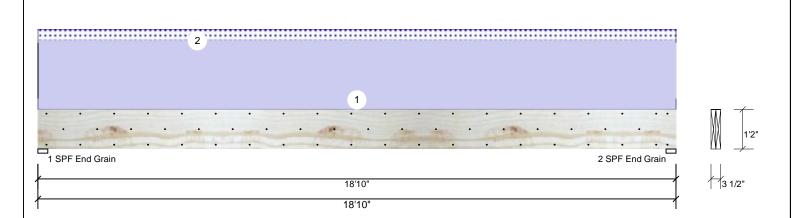
Client: Weaver Development Project: Sinclair (190320B) Address: Sinclair (190320B) Date: 5/25/2020 Input by:

Christine Shivy Job Name: GDH

Project #:

### 1.750" X 14.000" **Kerto-S LVL** 2-Ply - PASSED **GDH**

Level: Level



Member Inf	formation						Reaction	ns UNPAT	TERNED	) lb (Uplift)	1		
Type:	Girder		Applicat	ion: F	loor		Brg	Live	Dead	Snow	Win	d	Const
Plies:	2		Design	Method: A	ASD		1	0	2598	377		0	0
Moisture Cond	dition: Dry		Building	Code: II	BC 2012		2	0	2598	377		0	0
Deflection LL:	480		Load Sh	naring: N	No								
Deflection TL:	360		Deck:	١	Not Checked								
Importance:	Normal												
Temperature:	Temp <= '	100°F											
							Bearing	S					
							Bearing	Length	Cap. F	React D/L lb	Total Ld.	. Case	Ld. Comb.
							1 - SPF	3.500"	28%	2598 / 377	2975 L		D+S
							End						
Analysis Re	sults						Grain						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2 - SPF	3.500"	28%	2598 / 377	2975 L		D+S
Moment	11644 ft-lb	9'5"	24299 ft-lb	0.479 (48%	6) D	Uniform	End Grain						
Unbraced	13332 ft-lb	9'5"	13339 ft-lb	0.999 (100%)	D+S	L							

Uniform

1

# **Design Notes**

Shear

1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".

9'5 1/16" 0.459 (L/480) 0.150 (15%) S

9'5 1/16" 0.612 (L/360) 0.880 (88%) D+S

1'4 3/4" 9408 lb

0.235 (24%) D

- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 7'8 5/8" o.c.
- 6 Bottom braced at bearings.

2213 lb

LL Defl inch 0.068 (L/3239)

TL Defl inch 0.538 (L/410)

7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	225 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Siding / Plywood
2	Uniform			Тор	40 PLF	0 PLF	40 PLF	0 PLF	0 PLF	2'0" Roof Load
	Self Weight				11 PLF					

## Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Handling & Installation

  1. UVI beams must not be out or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 1/8/2023

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



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Client: Project: Address: Sinclair (190320B)

Weaver Development Sinclair (190320B)

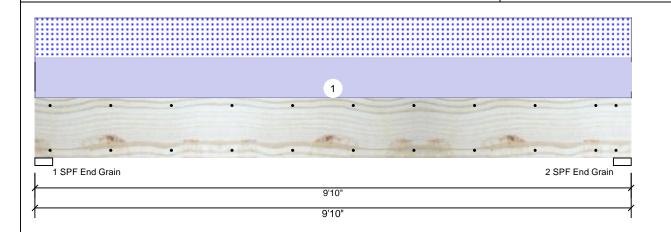
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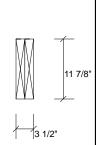
Christine Shivy Job Name: GDH-3

Project #:

1.750" X 11.875" GDH-3 **Kerto-S LVL** 2-Ply - PASSED

Level: Level





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Member Inform	nation
Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
Temperature:	Temp <= 100°F

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked

Reaction	ons UNPAT	TERNED I	(Uplift)		
Brg	Live	Dead	Snow	Wind	Const
1	0	1422	1377	0	0
2	0	1422	1377	0	0

### Analysis Results Analysis Actual Location Allowed Capacity Comb. Case 0.273 (27%) D+S Moment 6254 ft-lb 4'11" 22897 ft-lb L Unbraced 6254 ft-lb 4'11" 9857 ft-lb 0.634 (63%) D+S L 2105 lb 10197 lb 0.206 (21%) D+S Shear 1'2 5/8" ī LL Defl inch 0.058 (L/1928) 4'11" 0.234 (L/480) 0.250 (25%) S TL Defl inch 0.119 (L/948) 4'11" 0.312 (L/360) 0.380 (38

### Bearings Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1422 / 1377 1 - SPF 3.500" 2799 L D+S End Grain 1422 / 1377 2799 L D+S 2 - SPF 3.500"

## **Design Notes**

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. M to exceed 6".
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- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

38%) D+S	L
Maximum end o	distance not
d loads.	

ID Trib Width Side Dead 0.9 Load Type Location Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Top 280 PLF 0 PLF 280 PLF 0 PLF 0 PLF Self Weight 9 PLF

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regarding installation requirements, multi-ply
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6. For flat roofs provide proper drainage to prevent ponding

End

Grain

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