

RE: J0820-3845 Weaver / 34 Lilly Farm / Johnston Trenco 818 Soundside Rd Edenton, NC 27932

Site Information: Customer: Project Name: J0820-3845 Lot/Block: Address: City:

Model: Subdivision: State:

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.3 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 26 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	E14496517	A1	9/9/2020	21	E14496537	VC1	9/9/2020
2	E14496518	A1GE	9/9/2020	22	E14496538	VC2	9/9/2020
3	E14496519	A2	9/9/2020	23	E14496539	VC3	9/9/2020
4	E14496520	A3	9/9/2020	24	E14496540	VC4	9/9/2020
5	E14496521	A4	9/9/2020	25	E14496541	VC5	9/9/2020
6	E14496522	A5	9/9/2020	26	E14496542	VC6	9/9/2020
7	E14496523	A6	9/9/2020				
8	E14496524	A7	9/9/2020				
9	E14496525	A7GE	9/9/2020				
10	E14496526	B1	9/9/2020				
11	E14496527	B1GE	9/9/2020				
12	E14496528	B2	9/9/2020				
13	E14496529	B3	9/9/2020				
14	E14496530	B4	9/9/2020				
15	E14496531	C1	9/9/2020				
16	E14496532	C1GE	9/9/2020				
17	E14496533	C2	9/9/2020				
18	E14496534	C3	9/9/2020				
19	E14496535	VB1	9/9/2020				
20	E14496536	VB2	9/9/2020				

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Gilbert, Eric

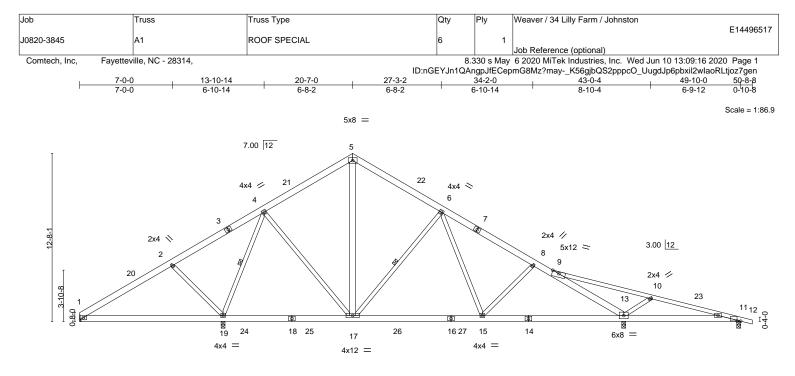
Truss Design Engineer's Name: Gilbert, Enc

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



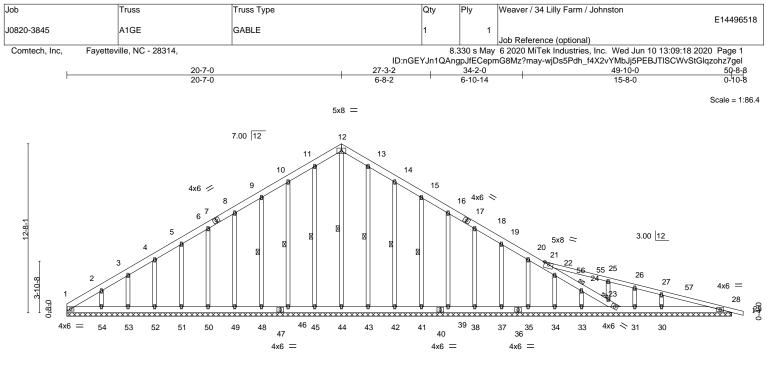


I	10-9-12	20-7-0	30-4-4	41-0-4	49-10-0
	10-9-12	9-9-4	9-9-4	10-8-0	8-9-12
Plate Offsets (X,Y)	[11:0-3-4,0-0-3]				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.86 BC 0.41 WB 0.51 Matrix-S	Vert(LL) -0.11 Vert(CT) -0.17 Horz(CT) 0.02	(loc) l/defl L/d 15-17 >999 360 15-17 >999 240 13 n/a n/a 15-17 >999 240	PLATES         GRIP           MT20         244/190           Weight: 354 lb         FT = 20%
9-12: 2 BOT CHORD 2x6 SP WEBS 2x4 SP	No.1 *Except* x4 SP No.1 No.1 No.2 *Except* x6 SP No.1		BRACING- TOP CHORD BOT CHORD WEBS	Rigid ceiling directly applied o	ectly applied or 6-0-0 oc purlins. or 6-0-0 oc bracing. -19, 6-17
(Ib) - Max H Max U	earings 0-3-8 except (jt=length) 1=Mech orz 1=-297(LC 6) plift All uplift 100 lb or less at joint(s) rav All reactions 250 lb or less at join 22)	, 11 except 13=-189(LC 1		0(LC 17), 11=279(LC	
TOP CHORD         2-4=- 9-13=           BOT CHORD         17-19           WEBS         2-19=	Comp./Max. Ten All forces 250 (lb) c 53/432, 4-5=-879/341, 5-6=-851/342, 6 2019/488, 9-10=-136/537 9=0/367, 15-17=-27/1101, 13-15=-157/ 484/260, 4-19=-1463/351, 4-17=0/65 3=-608/280, 6-15=-23/481	-8=-1514/361, 8-9=-1585/3 1388	359,		
2) Wind: ASCE 7-10; V MWFRS (envelope) 24-11-13 to 46-3-11, DOL=1.60 plate grip	loads have been considered for this d ult=130mph (3-second gust) Vasd=10 and C-C Exterior(2) 0-0-12 to 4-5-9, In Exterior(2) 46-3-11 to 50-8-8 zone;C-1 DOL=1.60	8mph; TCDL=6.0psf; BCDL terior(1) 4-5-9 to 16-2-3, E	xterior(2) 16-2-3 to 24-11	Exp C; Enclosed; -13, Interior(1) shown; Lumber	TH CARO

- 3) All plates are 4x6 MT20 unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11 except (it=lb) 13=189, 19=186.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







	30-4-4 30-4-4				49-10-0 19-5-12					
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15	<b>CSI.</b> TC 0.20 BC 0.16	DEFL. Vert(LL) Vert(CT)	in 0.01 0.01	(loc) 29 29	l/defl n/r n/r	L/d 120 120	PLATES MT20	<b>GRIP</b> 244/190	
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.15 Matrix-S	Horz(CT)	0.01	28	n/a	n/a	Weight: 434 lb	FT = 20%	

LUMBER-		BRACING-		
TOP CHORD 2x6 SP No.1	*Except*	TOP CHORD	Structural wood sheathing	directly applied or 6-0-0 oc purlins.
21-29: 2x4 S	P No.1	BOT CHORD	Rigid ceiling directly applie	d or 6-0-0 oc bracing, Except:
BOT CHORD 2x6 SP No.1			10-0-0 oc bracing: 32-33,3	1-32,30-31,28-30.
OTHERS 2x4 SP No.2		WEBS	1 Row at midpt	12-44, 11-45, 10-46, 9-48, 13-43, 14-42,
				15-41
		JOINTS	1 Brace at Jt(s): 24, 23	

- REACTIONS. All bearings 49-10-0. (lb) - Max Horz 1=-384(LC 6)
  - Max Uplift All uplift 100 lb or less at joint(s) 45, 46, 48, 49, 50, 51, 52, 53, 43, 42, 41, 39, 38, 37, 34, 33 event 1-,106(1,C,8), 28-,126(1,C,7), 54-
  - 42, 41, 39, 38, 37, 34, 33 except 1=-106(LC 8), 28=-126(LC 7), 54=-133(LC 10), 35=-128(LC 11), 30=-183(LC 11) Max Grav All reactions 250 lb or less at joint(s) 32, 1, 45, 46, 48, 49, 50, 51, 52,
    - 53, 54, 43, 42, 41, 39, 38, 37, 35, 34, 33 except 28=281(LC 1), 44=263(LC 20), 31=297(LC 3), 30=354(LC 22)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD
   1-2=-379/319, 2-3=-283/273, 3-4=-223/253, 9-10=-148/262, 10-11=-203/304, 11-12=-233/336, 12-13=-233/336, 13-14=-203/302, 22-24=-273/273, 23-24=-296/343, 23-32=-257/242

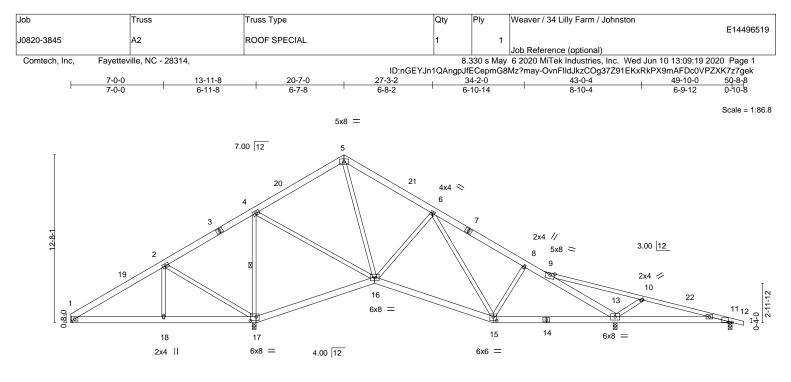
   BOT CHORD
   1-54=-154/288, 53-54=-154/288, 52-53=-154/288, 51-52=-154/288, 50-51=-154/288, 49-50=-154/288, 48-99=-154/288, 46-48=-154/288, 45-46=-154/288, 44-45=-154/288, 43-44=-154/288, 42-43=-154/288, 41-42=-154/288, 39-41=-154/288, 38-39=-154/288
- WEBS 27-30=-275/252

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-0-0 to 4-7-0, Exterior(2) 4-7-0 to 16-2-3, Corner(3) 16-2-3 to 24-11-13, Exterior(2) 24-11-13 to 46-3-11, Corner(3) 46-3-11 to 50-8-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 45, 46, 48, 49, 50, 51, 52, 53, 43, 42, 41, 39, 38, 37, 34, 33 except (jt=lb) 1=106, 28=126, 54=133, 35=128, 30=183.







7-	0-0 <u>13-9-12 13-11-8</u> 0-0 <u>6-9-12</u> 0-1-12	22-10-12	31-10-0	41-0-4	49-10-0
· · · · · ·	<u>-0-0 6-9-12 0-1<sup>11</sup>12</u> [11:0-3-4,0-0-3], [15:0-3-0,0-3-8], [17:0-	8-11-4 5-4 0-3-81	8-11-4	9-2-4	8-9-12
LOADING (psf)           TCLL 20.0           TCDL 10.0           BCLL 0.0 *           BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.56 BC 0.30 WB 0.73 Matrix-S	DEFL. in (loc) Vert(LL) -0.06 15-16 Vert(CT) -0.13 15-16 Horz(CT) 0.03 11 Wind(LL) 0.07 11-13	5 >999 360 5 >999 240 1 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 345 lb         FT = 20%
LUMBER- TOP CHORD 2x6 SP	/ No.1 *Except* x4 SP No.1 ? No.1		BRACING- TOP CHORD Struc BOT CHORD Rigid		ctly applied or 6-0-0 oc purlins. 6-0-0 oc bracing.
(lb) - Max H. Max U Max G FORCES. (lb) - Max. TOP CHORD 1-2=- 8-9=-	earings 0-3-8 except (jt=length) 1=Mecha orz 1=-297(LC 6) plift All uplift 100 lb or less at joint(s) 1 rav All reactions 250 lb or less at joint( 11=289(LC 22) Comp./Max. Ten All forces 250 (lb) or 377/150, 2-4=-27/568, 4-5=-607/237, 5- 1075/265, 9-13=-1609/353, 9-10=-106/	except 17=-170(LC 10), 1 s) except 1=384(LC 21), less except when shown. 6=-662/249, 6-8=-1036/30	17=1990(LC 1), 13=1457(LC 1)		
WEBS 2-18=	120/281, 17-18=-120/281, 16-17=-581, =0/308, 2-17=-671/221, 4-17=-1384/288 =-575/291, 8-15=-253/167, 10-13=-608/2	4-16=-17/969, 5-16=-43/			
<ol> <li>Wind: ASCE 7-10; V MWFRS (envelope) 24-11-13 to 46-3-11, shown; Lumber DOL</li> <li>All plates are 4x6 M<sup>-</sup></li> <li>This truss has been</li> <li>* This truss has been will fit between the b</li> <li>Refer to girder(s) for</li> </ol>	e loads have been considered for this de fult=130mph (3-second gust) Vasd=103r and C-C Exterior(2) 0-0-12 to 4-5-9, Inte , Exterior(2) 46-3-11 to 50-8-8 zone; por =1.60 plate grip DOL=1.60 T20 unless otherwise indicated. designed for a 10.0 psf bottom chord liv n designed for a live load of 30.0psf on t ottom chord and any other members. truss to truss connections. connection (by others) of truss to bearin =150.	nph; TCDL=6.0psf; BCDL rrior(1) 4-5-9 to 16-2-3, E; ch right exposed;C-C for r e load nonconcurrent with he bottom chord in all are	kterior(2) 16-2-3 to 24-11-13, Ir nembers and forces & MWFRS any other live loads. as where a rectangle 3-6-0 tall	by 2-0-0 wide	SEAL 036322

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





Job	Truss	Truss Type	C	Qty	Ply	Weaver / 3	34 Lilly Farm /	Johnston	
J0820-3845	A3	SPECIAL TRUSS	3	3	1				E14496520
Comtech, Inc, Faye	tteville, NC - 28314,			8	.330 s Ma		ence (optional) Tek Industries		3:09:20 2020 Page 1
			ID:nG						Z6G_lbAj3J4sZz7gej
	7-1-12	13-11-8 6-9-12	20-7-0 6-7-8			-3-2 -8-2	31-10-0		
				5x5 =	=				Scale = 1:73.0
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			5			16			
		3x10 =		//	١	$\sim$	3x4 ≷ 6		
		4x6 = 4		/	)		17		
		3			//				
12-8-1	2x4 🔌							4x6 ×	
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	14				×		\	$\langle    \rangle$	<sup>1</sup> ئ
				$\sim$	11				-12
9	1				6x8	=			4-11-5 2-11-12
0-8-0	Ø	13 120.00							1 1
	3x6 =	$13   12_{6x6} = 3x4 =$	= 4.00	12				10 9 <sup>5</sup> 8 2x4	
	10.0.10	42.0.40.42.44.0					1 40 0	6x6 =	
		13-9-12 13-11-8 3-0-0 0-1-12	22-10-12 8-11-4				31-10-0 8-11-4	<u>33-10-0</u> 2-0-0	
Plate Offsets (X,Y) [	10:0-3-0,0-3-8], [12:0-3-0,0-3-8								
LOADING (psf)	SPACING- 2-0-0		DEFL.		n (loc)		L/d	PLATES	GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.1 Lumber DOL 1.1		Vert(LL) Vert(CT)	-0.09 -0.19			360 240	MT20	244/190
BCLL 0.0 *	Rep Stress Incr YES	WB 0.42	Horz(CT)	0.02	2 9		n/a	Mainht 074 lb	FT 200/
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL)	0.02	2 1-13	>999	240	Weight: 271 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SP I			BRACING TOP CHC		Structu	ural wood ch	oothing direct	thy applied or 6.0.0	
BOT CHORD 2x6 SP				κD		end vertical		tly applied or 6-0-0	oc purins,
WEBS 2x4 SP	No.2		BOT CHC WEBS	RD		eiling directl at midpt		6-0-0 oc bracing. ), 4-12	
REACTIONS. (size)	1=Mechanical, 12=0-3-8, 9=	:0-3-8	WEDS		IROW	at mupt	0-10	), 4-12	
	rz 1=287(LC 7) lift 1=-1(LC 10), 12=-193(LC 1)	)) 0 = 56(1 - 11)							
	av 1=486(LC 21), 12=1508(LC								
FORCES (Ib) - May (	Comp /Max Tep - All forces 25	) (Ib) or less except when shown.							
TOP CHORD 1-2=-5	02/61, 4-5=-536/236, 5-6=-578	/249, 6-7=-272/114, 7-9=-725/148							
	167/427, 10-11=-104/506 552/147_4-12=-1368/396_4-1	=-49/580, 7-10=-23/497, 4-13=-1	00/561						
	462/253	+0/000, /-10=-20/49/, 4-13=-1	00/301,						
NOTES-									
1) Unbalanced roof live	oads have been considered fo								
		sd=103mph; TCDL=6.0psf; BCDL 5-9. Interior(1) 4-5-9 to 16-2-3. Ex							

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 16-2-3, Exterior(2) 16-2-3 to 24-11-13, Interior(1) 24-11-13 to 29-1-15, Exterior(2) 29-1-15 to 33-6-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

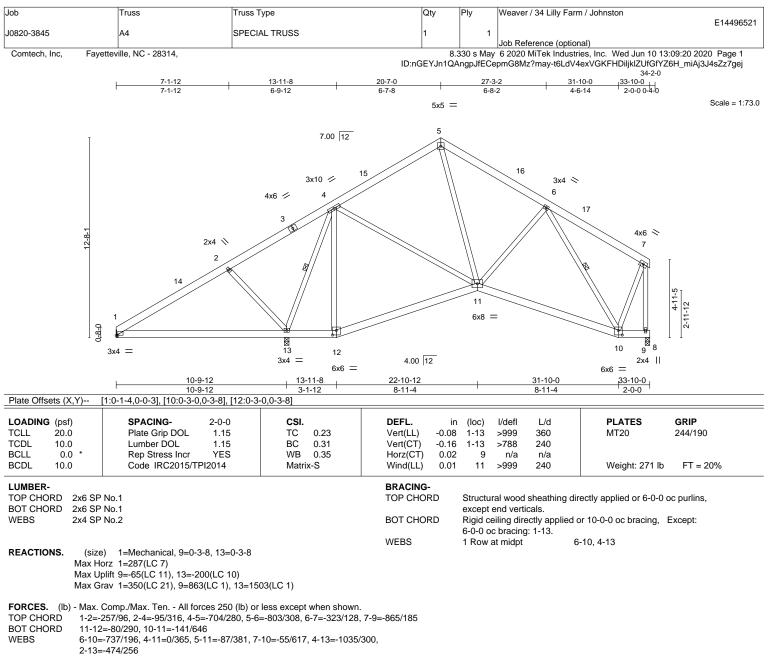
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9 except (jt=lb) 12=193.







#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 16-2-3, Exterior(2) 16-2-3 to 24-11-13, Interior(1) 24-11-13 to 29-1-15, Exterior(2) 29-1-15 to 33-6-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

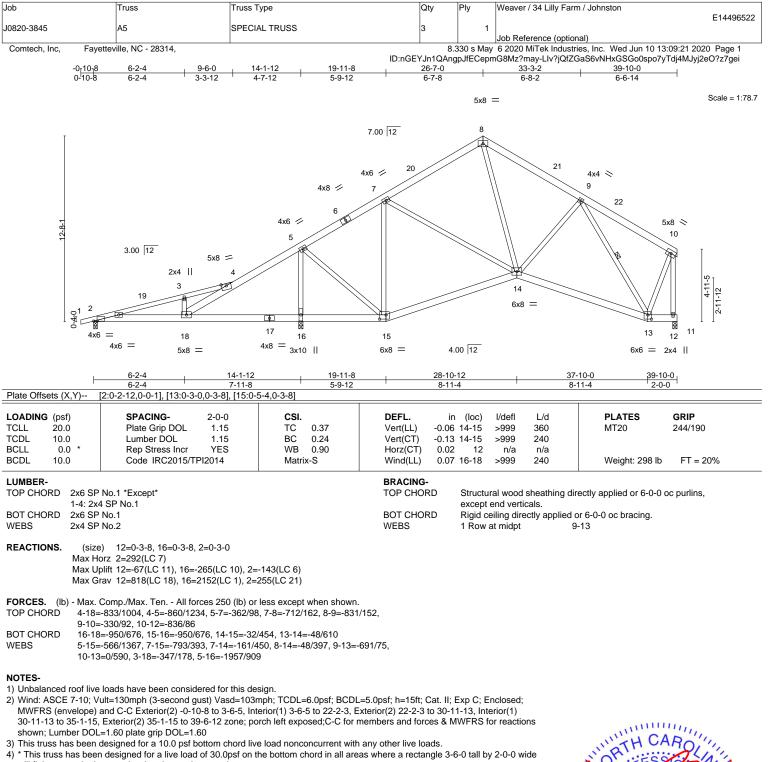
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 13=200.







3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 4) will fit between the bottom chord and any other members.

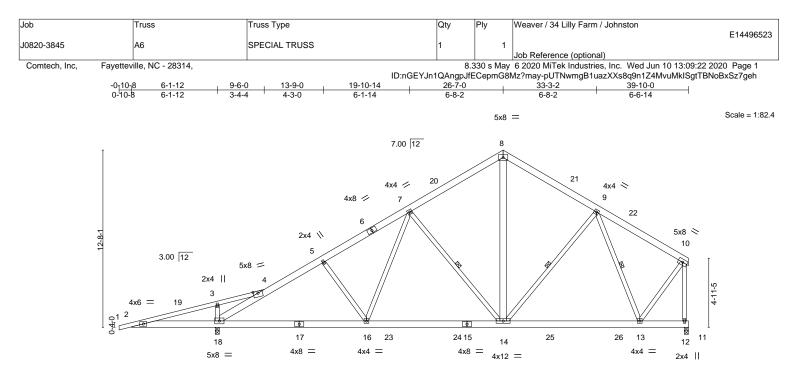
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 16=265, 2=143.

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



818 Soundside Road Edenton, NC 27932

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not Design Valid for Use only with with exe connectors. This design is based only upon parameters shown, and is to an invitude journal of the connectors and property incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP/11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qu** Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



	6-1-12 6-1-12		16-9-12 10-8-0	19-10-14 3-1-2		26-7-0 6-8-2		3-3-2 6-8-2	+ 36-4-4 39-10-0 3-1-2 3-5-12	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC 0.62 BC 0.47 WB 0.37		EFL. ert(LL) ert(CT) orz(CT)	in (loc) -0.13 14-16 -0.20 14-16 0.03 12	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC2015/TF	PI2014	Matrix-S	W	/ind(LL)	-0.05 16-18	>999	240	Weight: 307 lb	FT = 20%

LUMBER-		BRACING-		
TOP CHORD	2x6 SP No.1 *Except* 1-4: 2x4 SP No.1	TOP CHORD	Structural wood sheat except end verticals.	hing directly applied or 5-3-4 oc purlins,
BOT CHORD WEBS	2x6 SP No.1 2x4 SP No.2 *Except*	BOT CHORD	Rigid ceiling directly a 6-0-0 oc bracing: 2-18	pplied or 10-0-0 oc bracing, Except:
	8-14: 2x6 SP No.1	WEBS	1 Row at midpt	7-14, 9-14, 9-13

REACTIONS. (size) 12=0-3-8, 18=0-3-8 Max Horz 18=292(LC 7) Max Uplift 12=-84(LC 11), 18=-199(LC 10) Max Grav 12=1444(LC 17), 18=1936(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-982/991, 3-4=-924/958, 4-18=-2613/1034, 4-5=-2011/179, 5-7=-1879/219, 7-8=-1233/368, 8-9=-1263/366, 9-10=-837/187, 10-12=-1490/264
- BOT CHORD 2-18=-920/1004, 16-18=-261/1814, 14-16=-174/1529, 13-14=-150/935
- WEBS 7-14=-755/209, 8-14=-169/852, 10-13=-124/1195, 7-16=-16/504, 9-13=-698/207, 3-18=-344/175

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 22-2-3, Exterior(2) 22-2-3 to 30-11-13, Interior(1) 30-11-13 to 35-1-15, Exterior(2) 35-1-15 to 39-6-12 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

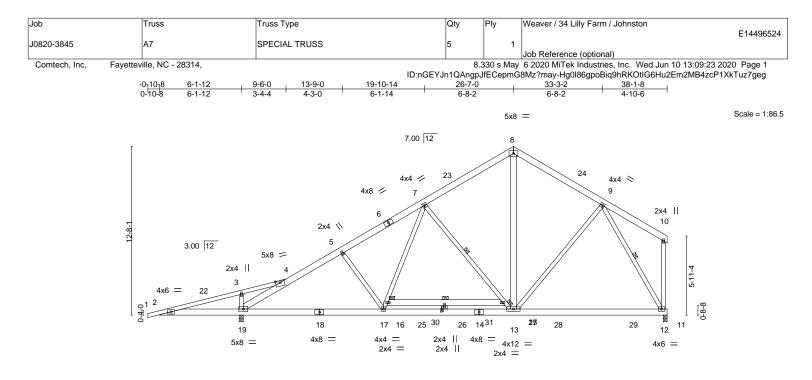
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 18=199.

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







	6-1-12 9-6-0 6-1-12 3-4-4	16-9-12 7-3-12	19-10-14 21-5-0 3-1-2 1-6-2	26-7-0 5-2-0	33-3-2 6-8-2	<u>36-4-4</u> 38-1-8 3-1-2 1-9-4	
Plate Offsets (X,Y)	[20:0-2-0,0-0-4]						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.65 BC 0.61 WB 0.58 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.25 12-13 -0.37 12-13 0.04 12 -0.05 17-19	I/defI L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 314 lb	<b>GRIP</b> 244/190 FT = 20%
1-4: 2x           BOT CHORD         2x6 SP           WEBS         2x4 SP	No.1 *Except* 4 SP No.1 No.1 No.2 *Except* -21: 2x6 SP No.1		BRACING- TOP CHOR BOT CHOR WEBS	except D Rigid o 6-0-0 o 6-0-0 o	t end verticals. ceiling directly applied oc bracing: 2-19. oc bracing: 15-16	lirectly applied or 5-0-4 I or 10-0-0 oc bracing, 7-15. 9-12	•
Max U	e) 12=0-3-8, 19=0-3-8 orz 19=291(LC 7) blift 12=-40(LC 10), 19=-145(LC 10) rav 12=1591(LC 17), 19=1965(LC 2)		WEDS	1 KOW	αι πιαρι	1-10, 5-12	

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-989/988, 3-4=-931/955, 4-19=-2783/922, 4-5=-2188/61, 5-7=-2056/100, 7-8=-1270/284, 8-9=-1297/280
- BOT CHORD
   2-19=-916/1010, 17-19=-196/1959, 13-17=-97/1754, 12-13=-109/739

   WEBS
   7-15=-848/184, 13-15=-1063/135, 8-13=-78/880, 9-13=-9/545, 16-17=-26/430, 7-16=0/661, 9-12=-1462/232, 3-19=-343/175

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

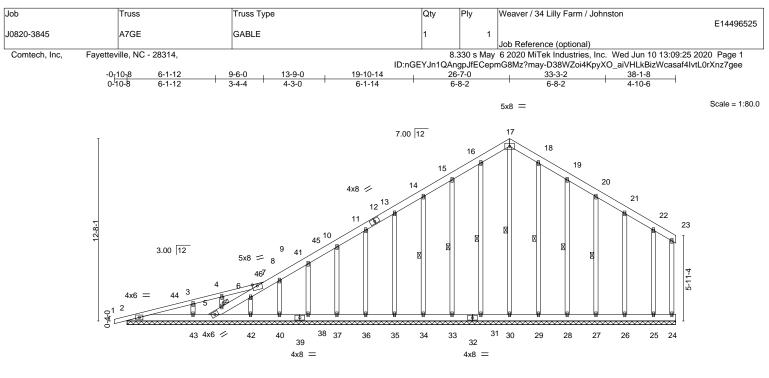
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 22-2-3, Exterior(2) 22-2-3 to 30-11-13, Interior(1) 30-11-13 to 33-3-2, Exterior(2) 33-3-2 to 37-10-4 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 19=145.

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







	6-1-12 6-1-12		16-9-12 10-8-0		+ 19-10-14 3-1-2			33-3-2 13-4-4		36-4-4 38-1- 3-1-2 1-9-	
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.14	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
CDL 10.0	Lumber DOL	1.15	BC	0.11	Vert(CT)	0.00	1	n/r	120		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.14	Horz(CT)	-0.00	24	n/a	n/a		
BCDL 10.0	Code IRC2015/T	PI2014	Matrix	(-S						Weight: 362 lb	FT = 20%

LUMBER-		BRACING-		
TOP CHORD	2x6 SP No.1 *Except*	TOP CHORD	Structural wood sheath	ning directly applied or 6-0-0 oc purlins,
	1-7: 2x4 SP No.1		except end verticals.	
BOT CHORD	2x6 SP No.1	BOT CHORD	Rigid ceiling directly ap	oplied or 6-0-0 oc bracing, Except:
WEBS	2x4 SP No.2		10-0-0 oc bracing: 2-4	3,41-43.
OTHERS	2x4 SP No.2	WEBS	1 Row at midpt	17-30, 16-31, 15-33, 14-34, 18-29, 19-28,
				20-27
		JOINTS	1 Brace at Jt(s): 5	

REACTIONS. All bearings 38-1-8. (lb) - Max Horz 2=406(LC 10)

//ax 11012 2=400(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 24, 30, 31, 33, 34, 35, 36, 37, 38, 40, 43, 29, 28, 27, 26, 25 except 2=-137(LC 6), 42=-113(LC 6)

Max Grav All reactions 250 lb or less at joint(s) 2, 24, 41, 30, 31, 33, 34, 35, 36, 37, 38, 40, 42, 29, 28, 27, 26, 25 except 43=398(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-257/190, 7-8=-327/280, 8-9=-301/283, 9-10=-264/256, 14-15=-182/272,

15-16=-221/313, 16-17=-250/325, 17-18=-250/312, 18-19=-221/274

#### NOTES-

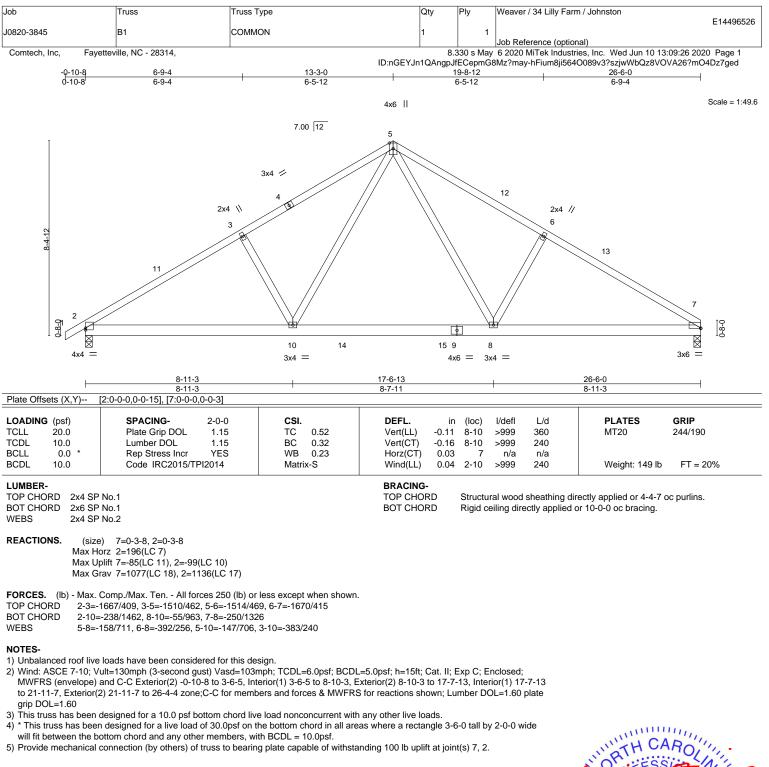
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 22-2-3, Corner(3) 22-2-3 to 30-11-13, Exterior(2) 30-11-13 to 33-5-7, Corner(3) 33-5-7 to 37-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 30, 31, 33, 34, 35, 36, 37, 38, 40, 43, 29, 28, 27, 26, 25 except (jt=lb) 2=137, 42=113.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



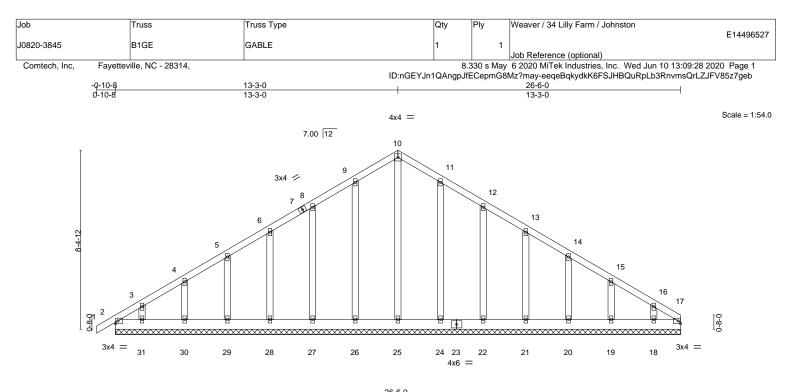
ENGINEERING BY ERENCO A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932









			26-6-0 26-6-0					
OADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. ir	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.04	Vert(LL) -0.00	· · ·	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.02	Vert(CT) -0.00	1	n/r	120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.15	Horz(CT) 0.00	17	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	. ,				Weight: 185 lb	FT = 20%

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 26-6-0.

(lb) - Max Horz 2=244(LC 7)

Max Uplift All uplift 100 lb or less at joint(s) 17, 2, 26, 27, 28, 29, 30, 24, 22, 21, 20, 19 except 31=-108(LC 10), 18=-113(LC 11)

Max Grav All reactions 250 lb or less at joint(s) 17, 2, 25, 26, 27, 28, 29, 30, 31, 24, 22, 21, 20, 19, 18

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## TOP CHORD 2-3=-263/188

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-3-0, Exterior(2) 3-3-0 to 8-10-3, Corner(3) 8-10-3 to 17-7-13, Exterior(2) 17-7-13 to 22-1-3, Corner(3) 22-1-3 to 26-6-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

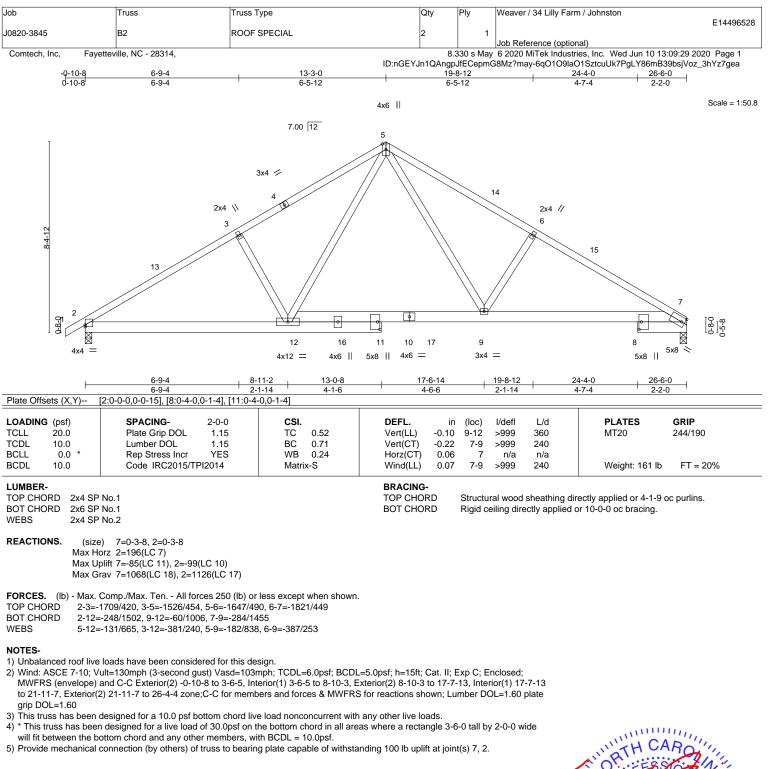
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17, 2, 26, 27, 28, 29, 30, 24, 22, 21, 20, 19 except (jt=lb) 31=108, 18=113.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

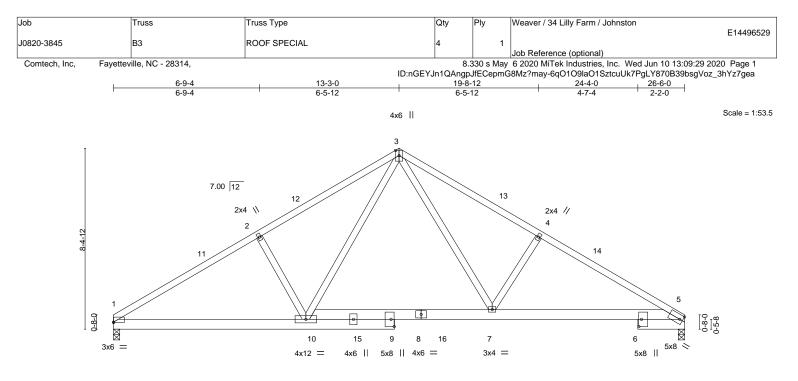






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	6-9-4 6-9-4		8-11-2	<u>13-0-8</u> 4-1-6	17-6			19-8-12 2-1-14	24-4		-
Plate Offsets (X,Y)	[1:0-0-0,0-0-3], [6:0-4-0,0	)-1-8], [9:0-4-	0,0-1-12]								
OADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
ICLL 20.0	Plate Grip DOL	1.15	TC	0.50	Vert(LL)	-0.10	7-1Ó	>999	360	MT20	244/190
CDL 10.0	Lumber DOL	1.15	BC	0.71	Vert(CT)	-0.22	5-7	>999	240		
CLL 0.0 *	Rep Stress Incr	YES	WB	0.24	Horz(CT)	0.06	5	n/a	n/a		
3CDL 10.0	Code IRC2015/TF	PI2014	Matri	x-S	Wind(LL)	0.07	5-7	>999	240	Weight: 160 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-TOP CHORD 2x4 SP No.1

BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 5=0-3-8 Max Horz 1=192(LC 7) Max Uplift 1=-85(LC 10), 5=-85(LC 11) Max Grav 1=1068(LC 17), 5=1069(LC 18)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-1714/432, 2-3=-1531/466, 3-4=-1649/496, 4-5=-1823/455

BOT CHORD 1-10=-266/1511, 7-10=-64/1007, 5-7=-289/1456

WEBS 3-10=-142/670, 2-10=-390/256, 3-7=-184/839, 4-7=-387/254

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed;

MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 8-10-3, Exterior(2) 8-10-3 to 17-7-13, Interior(1) 17-7-13 to 21-11-7, Exterior(2) 21-11-7 to 26-4-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

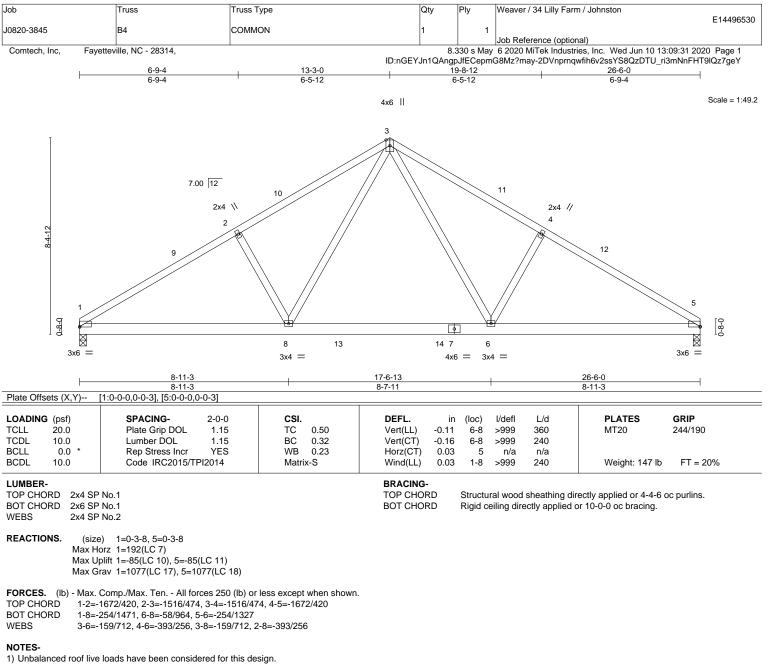
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.



Structural wood sheathing directly applied or 4-1-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed;

MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 8-10-3, Exterior(2) 8-10-3 to 17-7-13, Interior(1) 17-7-13 to 21-11-7, Exterior(2) 21-11-7 to 26-4-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

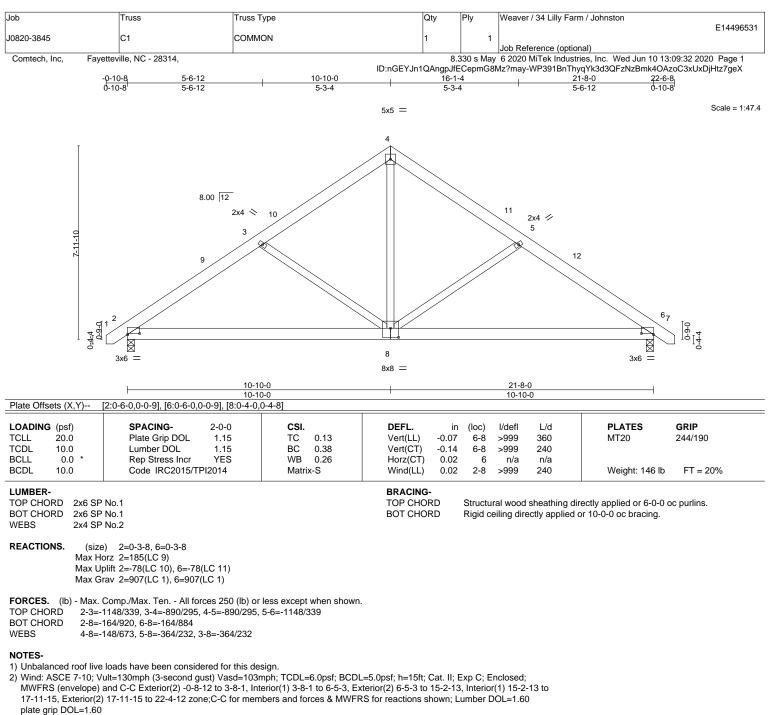
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.







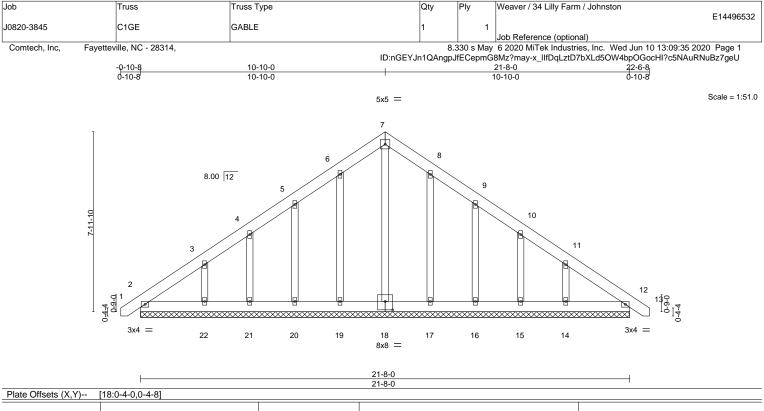
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.







LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.03 BC 0.02 WB 0.12 Matrix-S	DEFL. in Vert(LL) 0.00 Vert(CT) 0.00 Horz(CT) 0.00	12	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 172 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-			BRACING-					

TOP CHORD	2x6 SP No.1	TC
BOT CHORD	2x6 SP No.1	BC
OTHERS	2x4 SP No.2	

OP CHORD
 Structural wood sheathing directly applied or 6-0-0 oc purlins.

 OT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 21-8-0.

(lb) - Max Horz 2=231(LC 9)

- Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 21, 17, 15 except 20=-101(LC 10), 22=-146(LC 10), 16=-103(LC 11), 14=-143(LC 11)
- Max Grav All reactions 250 lb or less at joint(s) 2, 12, 18, 19, 20, 21, 17, 16, 15 except 22=255(LC 17), 14=251(LC 18)

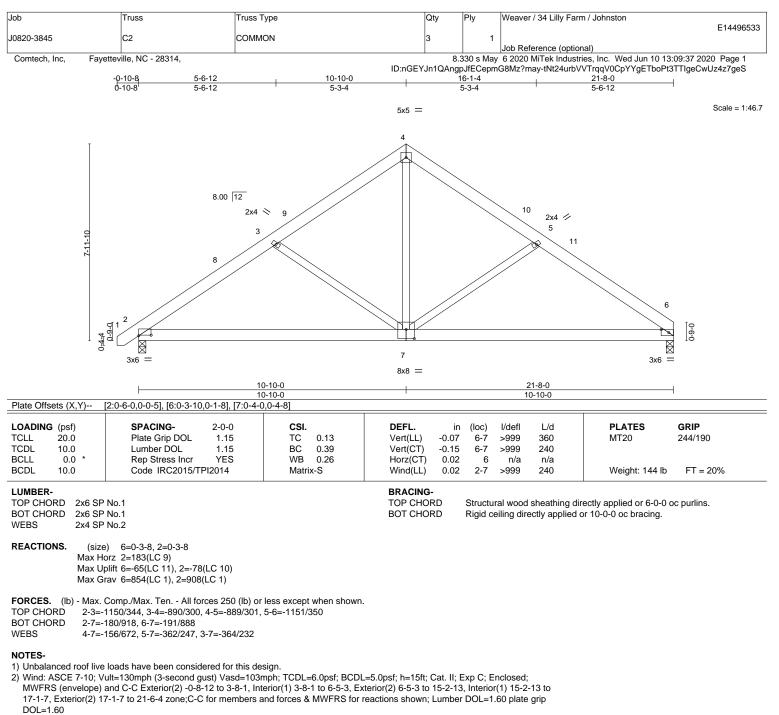
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-12 to 3-8-1, Exterior(2) 3-8-1 to 6-5-3, Corner(3) 6-5-3 to 15-2-13, Exterior(2) 15-2-13 to 17-11-15, Corner(3) 17-11-15 to 22-4-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 19, 21, 17, 15 except (jt=lb) 20=101, 22=146, 16=103, 14=143.







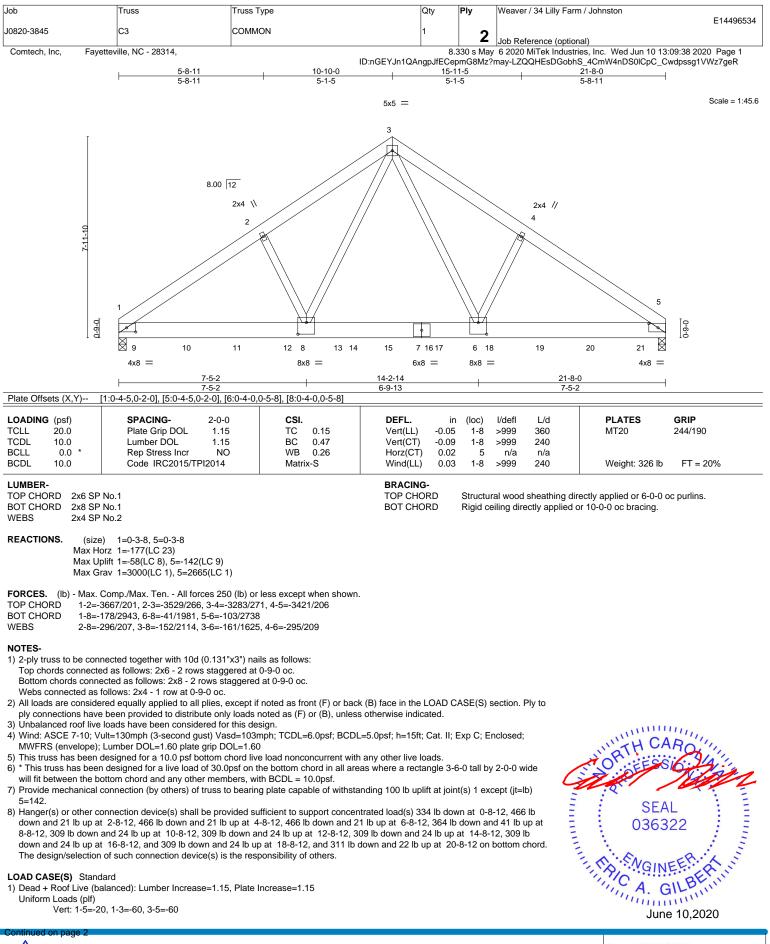
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4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.







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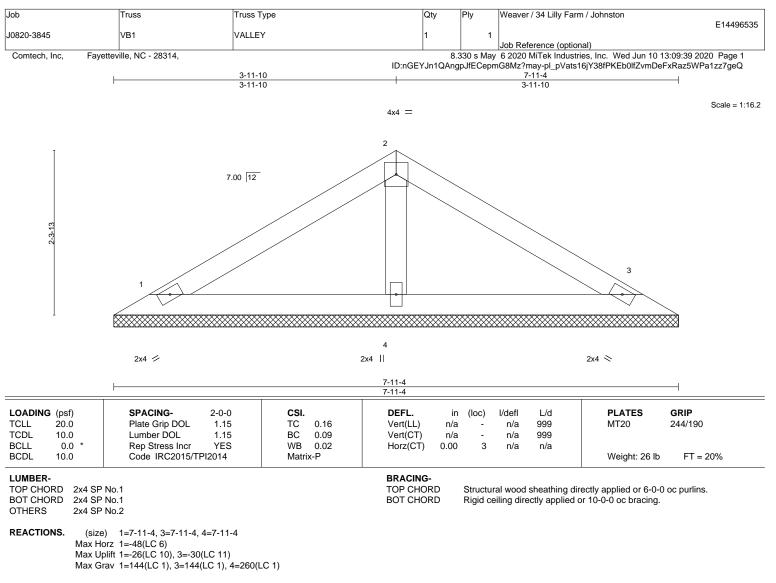
818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Weaver / 34 Lilly Farm / Johnston	
						E14496534
J0820-3845	C3	COMMON	1	2		
				<b>_</b>	Job Reference (optional)	
Comtech, Inc,	Fayetteville, NC - 28314,		8.	330 s May	6 2020 MiTek Industries, Inc. Wed Jun 10 13:09:38 202	20 Page 2
	-		ID:nGEYJn1QAngpJfECe	pmG8Mz?i	may-LZQQHEsDGobhS_4CmW4nDS0lCpC_Cwdpssg1V	/Wz7geR

#### LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 9=-334(F) 10=-466(F) 11=-466(F) 12=-466(F) 13=-364(F) 15=-309(F) 17=-309(F) 18=-309(F) 19=-309(F) 20=-309(F) 21=-311(F)





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

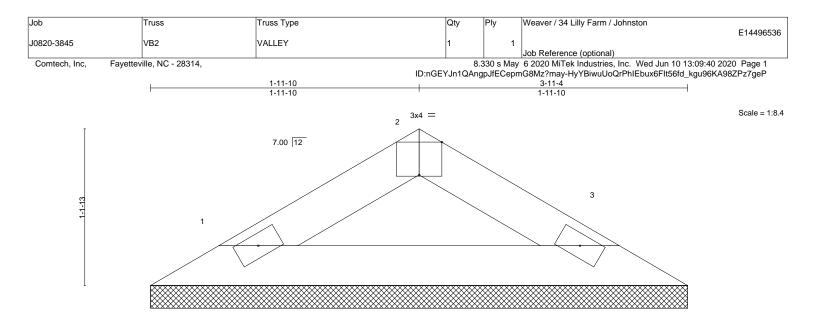
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







2x4 💋

2x4 📎

.OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.02	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.07	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matrix	(-P						Weight: 11 lb	FT = 20%

3-11-/

**REACTIONS.** (size) 1=3-11-4, 3=3-11-4

Max Horz 1=20(LC 7) Max Uplift 1=-9(LC 10), 3=-9(LC 11) Max Grav 1=114(LC 1), 3=114(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

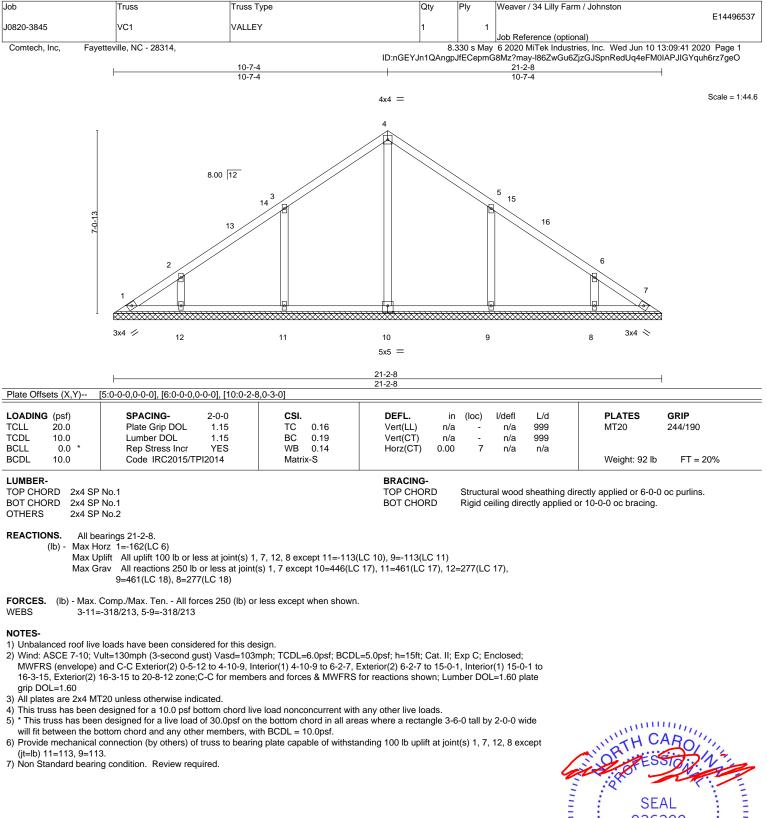
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



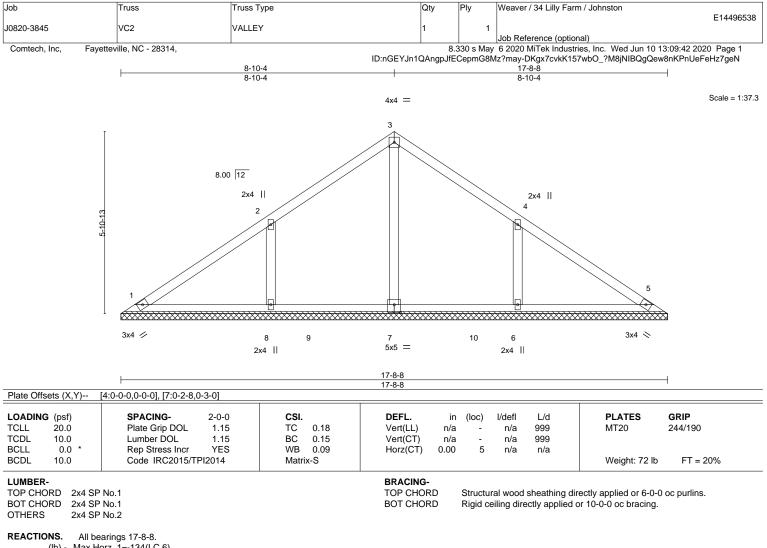






WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



(lb) - Max Horz 1=-134(LC 6)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-130(LC 10), 6=-129(LC 11)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=402(LC 17), 8=462(LC 17), 6=462(LC 18)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES.

WEBS 2-8=-357/249, 4-6=-357/249

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

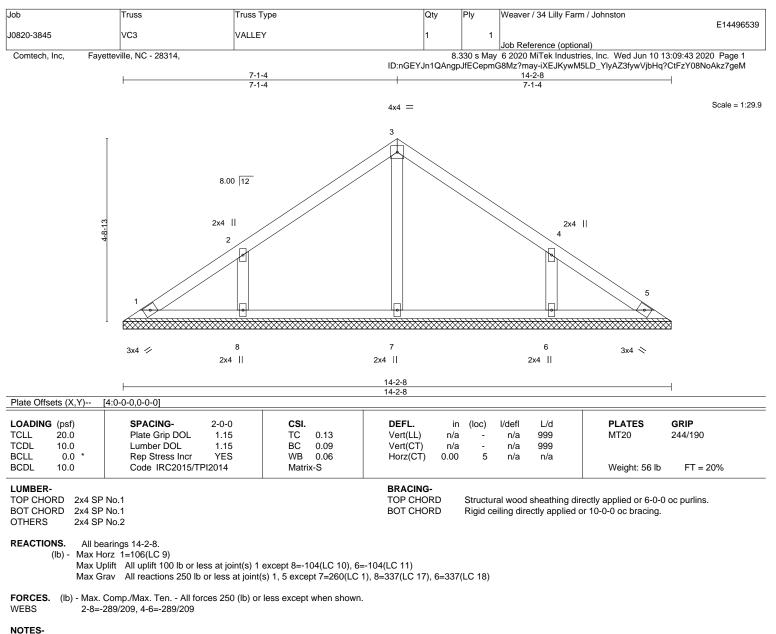
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=130, 6=129.

6) Non Standard bearing condition. Review required.





- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

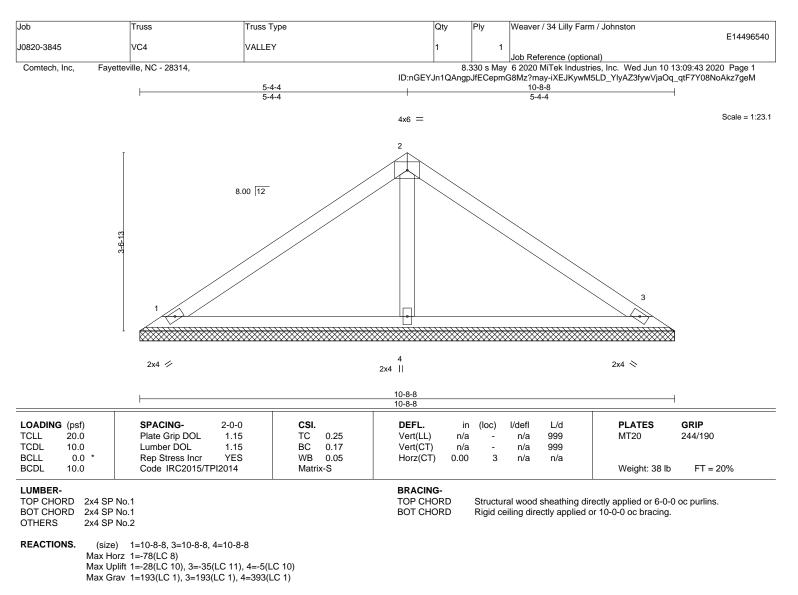
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=104. 6=104.

6) Non Standard bearing condition. Review required.







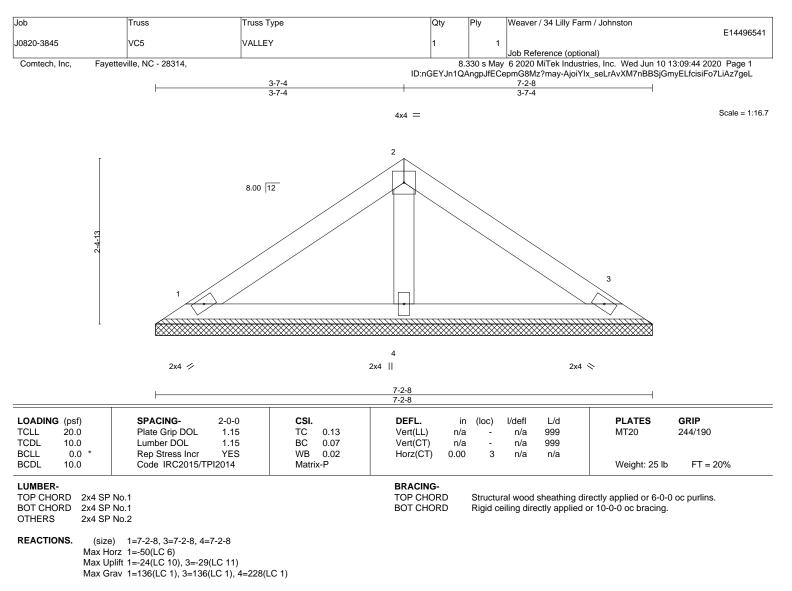
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate
- grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- 4) 1 his truss has been designed for a live load of 30.0pst on the bottom chord in all areas where a rectangle 3-6-0 fall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.
- 6) Non Standard bearing condition. Review required.







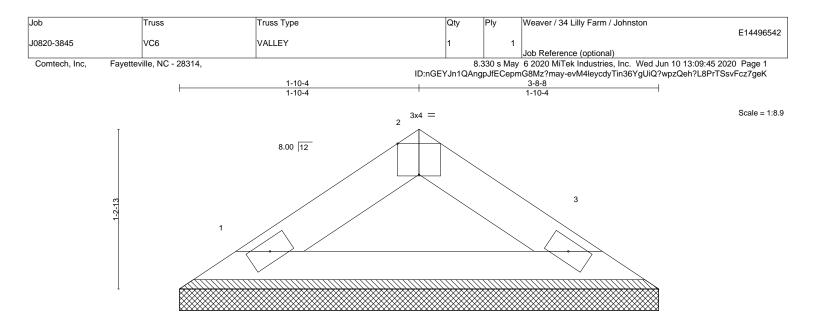
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 6) Non Standard bearing condition. Review required.







2x4 1/

2x4

Rigid ceiling directly applied or 10-0-0 oc bracing.

OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.02	Vert(LL) n/a - n/a 999	MT20 244/190
CDL 10.0	Lumber DOL 1.15	BC 0.07	Vert(CT) n/a - n/a 999	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 3 n/a n/a	
3CDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 11 lb FT = 20%

BOT CHORD

3-8-8

P CHORD 2x4 SP No 1 BOT CHORD 2x4 SP No.1

REACTIONS. (size) 1=3-8-8, 3=3-8-8

Max Horz 1=22(LC 9) Max Uplift 1=-9(LC 10), 3=-9(LC 11)

Max Grav 1=110(LC 1), 3=110(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed;

MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.





