

Trenco 818 Soundside Rd Edenton, NC 27932

Re: Ash\_RF Lamco Custom Homes

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource (Albermarle,NC).

Pages or sheets covered by this seal: E12977389 thru E12977410

My license renewal date for the state of North Carolina is December 31, 2019.

North Carolina COA: C-0844



April 30,2019

Gilbert, Eric

**IMPORTANT NOTE:** Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.



0-	-Q-8	29-6-0						
0-	-d-8	29-5-8					1	
Plate Offsets (X,Y) [10:0-3-0	,Edge], [25:0-0-0,0-1-12], [25:0-3-0,0-1-	4], [26:0-1-12,0-0-0]						
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	<b>CSI.</b> TC 0.14 BC 0.10 WB 0.16 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.01 18-20 -0.01 18-20 0.01 18 0.01 18-20	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 201 lb	<b>GRIP</b> 244/190 FT = 20%
		BR		Structural wood	choothin	a directly apr	aliad or 6.0.0 oc purling	

TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc		
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.		
OTHERS	2x4 SP No.3	WEBS	1 Row at midpt	9-27, 11-26	

REACTIONS. All bearings 29-5-0.

(lb) - Max Horz 2=-176(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 27, 28, 29, 30, 31, 32, 24, 23, 22, 21, 20

Max Grav All reactions 250 lb or less at joint(s) 2, 2, 27, 28, 29, 30, 31, 26, 24, 23, 22, 21, 18 except 32=299(LC 26), 20=292(LC 31)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 14-9-0, Corner(3) 14-9-0 to 17-9-0, Exterior(2) 17-9-0 to 30-4-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 27, 28, 29, 30, 31, 32, 24, 23, 22, 21, 20.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





VIIIIII IIIIIIIII MULTINI, 036322 GI 111111111 April 30,2019

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SEAL 036322 April 30,2019

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0-0	Q-8	29-6-0						
0-0-0	d-8	29-5-8						
Plate Offsets (X,Y) [10:0-3-0]	,Edge], [24:0-0-0,0-1-12], [24:0-3-0,0-1-	4], [25:0-1-12,0-0-0]						
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.16 BC 0.11 WB 0.16 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.01 18-19 -0.01 18-19 0.01 18 0.01 18-19	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 200 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-		BR	ACING-					
TOP CHORD 2x4 SP No.2		10	P CHORD S	structural wood	sneathing	g directly appl	lied or 6-0-0 oc purlins	i.

BOT CHORD

WEBS

Rigid ceiling directly applied or 10-0-0 oc bracing.

9-26, 11-25

1 Row at midpt

# OTHERS 2x4 SP No.3

**REACTIONS.** All bearings 29-5-8.

2x4 SP No.2

(lb) - Max Horz 2=173(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 26, 27, 28, 29, 30, 31, 23, 22, 21, 20, 19

Max Grav All reactions 250 lb or less at joint(s) 2, 2, 26, 27, 28, 29, 30, 25, 23, 22, 21, 20, 18 except 31=299(LC 26), 19=305(LC 31)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

BOT CHORD

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 14-9-0, Corner(3) 14-9-0 to 17-9-0, Exterior(2) 17-9-0 to 29-6-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 26, 27, 28, 29, 30, 31, 23, 22, 21, 20, 19.



818 Soundside Road Edenton, NC 27932

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	000	000	000	
Plate Offsets (X,Y) [4:0-2-0,E	dge]			
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI.         DEFL.           TC         0.05         Vert(LL)           BC         0.03         Vert(CT)           WB         0.02         Horz(CT)           Matrix-P         Wind(LL)	in (loc) l/defl L/d -0.00 8 >999 360 -0.00 8 >999 240 0.00 6 n/a n/a 0.00 8 ***** 240	PLATES         GRIP           MT20         244/190           Weight: 25 lb         FT = 20%
LUMBER-		BRACING-		

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD 2x4 SP No.3 OTHERS

REACTIONS. All bearings 5-6-0.

(lb) - Max Horz 2=41(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 9, 8

Max Grav All reactions 250 lb or less at joint(s) 2, 2, 6, 9, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 2-9-8, Corner(3) 2-9-8 to 5-7-0, Exterior(2) 5-7-0 to 6-5-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOI = 1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1

4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 8.



Structural wood sheathing directly applied or 5-7-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only design parameters and READ NOTES ON TIPS ON MICLODED MITER REFERENCE PAGE mit-14/3 at building component, not besign valid for use only with MITeK exconnectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road

Edenton, NC 27932



April 30,2019 MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. teters shown, and is for an individual building component, not arameters and property incorporate this design into the overall nord members only. Additional temporary and permanent bracing

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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 10-0-0, Exterior(2) 10-0-0 to 13-0-0, Interior(1) 13-0-0 to 20-10-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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## Continued on page 2

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A MITek Affiliate B18 Soundside Road Edenton, NC 27932

Job		Truss	Truss Type	Qty	Ply	Lamco Custom Homes	
							E12977396
Ash_RF		C3	COMMON GIRDER	1	2		
					<b>)</b>	Job Reference (optional)	
Builders Fir	stSource, A	lbemarle, NC 28001		8	220 s Nov	16 2018 MiTek Industries, Inc. Mon Apr 29 13:29:31 2019	Page 2
			ID:GgN1	vbXHPg6CC	W?w_rAV	/msz1jWL-FCURNs4o5Xbc8bYNX4Kjdial?1yZck8UGCYv	?VzLrNo

## LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 5=-108 7=-1114(B) 8=-1114(B) 14=-1114(B) 15=-1114(B) 16=-1114(B) 17=-1114(B) 18=-1114(B) 19=-1114(B) 20=-1114(B)

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REACTIONS. All bearings 3-7-8.

(lb) - Max Horz 2=49(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 5, 6

Max Grav All reactions 250 lb or less at joint(s) 5, 2, 2, 6

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 3-6-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6.



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	l		3-7-8				Į.		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.17 BC 0.12 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.01 0.00	(loc) 4-7 4-7 2	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 0.0 *	Code IRC2015/TPI2014	Matrix-MP	Wind(LL)	0.01	4-7	>999	240	Weight: 16 lb	FT = 20%
LUMBER-		BR	ACING-						

TOP CHORD

BOT CHORD

#### LUMBER-TOP CHORD

2x4 SP No.2 BOT CHORD 2x4 SP No.2

2x4 SP No.3 WEBS

REACTIONS. (lb/size) 4=119/Mechanical, 2=175/0-3-8 Max Horz 2=48(LC 16) Max Uplift 4=-9(LC 16) Max Grav 4=134(LC 2), 2=200(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-6-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.



Structural wood sheathing directly applied or 3-8-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON MICLODED MITER REFERENCE PAGE mit-14/3 at building component, not besign valid for use only with MITeK exconnectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





- 4) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-6-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 6) Unbalanced snow loads have been considered for this design. 7) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs
- non-concurrent with other live loads
- 8) Provide adequate drainage to prevent water ponding.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Refer to girder(s) for truss to truss connections.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s). The design/selection of such connection device(s) is the responsibility of others

# LOAD CASE(S) Standard

## Continued on page 2

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign valid for use only with with every connectors. This design is based only upon parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



SEAL

CARO

ORTH



Job	Truss	Truss Type	Qty	Ply	Lamco Custom Homes	
						E12977399
Ash_RF	D3	ROOF SPECIAL	1	2		
				<b>_</b>	Job Reference (optional)	
Builders FirstSource, A	lbemarle, NC 28001		8.2	220 s Nov	16 2018 MiTek Industries, Inc. Mon Apr 29 13:29:34 2019	Page 2

8.220 S NOV 16 2018 MITEK industries, inc. Mon Apr 29 13:29:34 2019 Page 2 ID:GgN1wbXHPg6COW?w\_rAWmsz1jWL-gn9Z?u7gOSzB?2HxCCtQFKCqeE3YpDGwy9nZcqzLrNI

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-51, 6-8=-20, 4-5=-61

Concentrated Loads (lb) Vert: 4=-600

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BOLL	0.0	Codo IPC2015/TPI2014	Motrix MD	Wind(LL)	0.01	7	~000	240	Woight: 17 lb	ET - 200
BCDL	10.0	Code IRC2013/1F12014	IVIAULX-IVIF	VVIIIU(LL)	0.01	'	>999	240	Weight. 17 lb	FT = 20
LUMBER-			BF	RACING-						
TOP CHORD	2x4 SP No.2		TC	P CHORD	Structural	wood	sheathir	ng directly app	blied or 2-8-0 oc purlins,	
BOT CHORD	2x4 SP No.2				except end	d vert	icals, and	d 2-0-0 oc pur	lins: 4-7, 4-5. Except:	
WEBS	2x4 SP No.3				10-0-0 oc	bracir	ng: 3-4	-		
			BC	DT CHORD	Rigid ceilir	ng dir	ectly app	lied or 10-0-0	oc bracing.	
REACTIONS.	(lb/size) 2=34	16/0-3-8, 6=557/Mechanical								
	Max Horz 2=37	7(LC 16)								

Max Grav 2=429(LC 36), 6=560(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-420/115

BOT CHORD 2-7=-123/335, 6-7=-199/539

WEBS 4-6=-717/264

## NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-6-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s). The design/selection of such connection device(s) is the responsibility of others.

### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-51, 6-8=-20, 4-5=-61 Concentrated Loads (lb)

Vert: 4=-600



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

BOT CHORD

LUMBER-		
TOP CHORD	2x4 SP No.2	

BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

**REACTIONS.** All bearings 3-7-8.

(lb) - Max Horz 2=38(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 7, 2, 6 Max Grav All reactions 250 lb or less at joint(s) 7, 2, 2, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 3-6-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 2, 6.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 2-8-0 oc purlins,

except end verticals, and 2-0-0 oc purlins: 4-7, 4-5. Except:

Rigid ceiling directly applied or 10-0-0 oc bracing.

6-0-0 oc bracing: 3-4

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



			1-11-8			1		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.05 BC 0.03 WB 0.00	DEFL. i Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) -0.00	n (loc) ) 7 ) 7 ) 3	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MP	Wind(LL) 0.00	0 7	>999	240	Weight: 8 lb	FT = 20%

BRACING-

TOP CHORD

```
LUMBER-
```

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=38/Mechanical, 2=125/0-3-0, 4=22/Mechanical Max Horz 2=26(LC 16) Max Uplift 3=-8(LC 16), 2=-22(LC 12), 4=-5(LC 13) Max Grav 3=45(LC 2), 2=144(LC 2), 4=34(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.



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		2-8-7	
LOADING (psf)         SPACING-         2-0-0           TCLL (roof)         20.0         Plate Grip DOL         1.15           Snow (Pf/Pg)         15.4/20.0         Lumber DOL         1.15           TCDL         10.0         Rep Stress Incr         YES           BCDL         10.0         Code IRC2015/TPI2014         100	CSI. TC 0.11 BC 0.07 WB 0.00 Matrix-MP	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.00         7         >999         360           Vert(CT)         -0.00         4-7         >999         240           Horz(CT)         -0.00         3         n/a         n/a           Wind(LL)         0.01         4-7         >999         240	PLATES         GRIP           MT20         244/190           Weight: 10 lb         FT = 20%

BRACING-

TOP CHORD

BOT CHORD

```
LUMBER-
```

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (lb/size) 3=50/Mechanical, 2=171/0-8-1, 4=28/Mechanical Max Horz 2=30(LC 12) Max Uplift 3=-11(LC 12), 2=-50(LC 12), 4=-6(LC 13) Max Grav 3=59(LC 2), 2=197(LC 2), 4=44(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) zone; cantilever left and right exposed ; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat
- roof snow: Lumber DoL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.



Structural wood sheathing directly applied or 2-8-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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17

NAILED

	0-Q <sub>1</sub> 8	2-0-0	1		7-0-0					8-11-8	9-Q <sub>1</sub> 0	
	0-0-8	1-11-8	1		5-0-0					1-11-8	0-0-8	
Plate Offsets (X	(,Y) [3:0-1-4,0	)-2-8]										
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 2 TCDL	) 20.0 20.4/20.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 NO	CSI. TC BC WB	0.60 0.34 0.06	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.03 -0.05 0.01	(loc) 7-8 7-8 5	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	<b>GRIP</b> 244/190
DOLL	0.0	Code IRC2015/TP	12014	Matri	x-MS	Wind(LL)	0.02	7-8	>999	240	Weight: 39 lb	FT = 20%

18

NAILED

4x4 =

Special

2x4 =

DODL	10.0			
LUMBER-		BF	RACING-	
TOP CHORD	2x4 SP No.2	тс	OP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except
BOT CHORD	2x4 SP No.2			2-0-0 oc purlins (5-3-11 max.): 3-4.
WEBS	2x4 SP No.3	BC	OT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 2=400/0-3-0, 5=401/0-3-0 Max Horz 2=10(LC 58)

Max Uplift 2=-72(LC 8), 5=-72(LC 9)

2x4 =

Max Grav 2=428(LC 2), 5=431(LC 35)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-847/135, 3-4=-803/131, 4-5=-850/135

BOT CHORD 2-8=-105/791, 7-8=-107/800, 5-7=-109/794

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

8

Special

1.5x4 ||

- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0 Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 11) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 33 lb down and 32 lb up at 2-0-0, and 33 lb down and 32 lb up at 7-0-0 on top chord, and 16 lb down and 36 lb up at 2-0-0, and 16 lb down and 36 lb up at 6-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-51, 3-4=-61, 4-6=-51, 9-12=-20

#### Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	Lamco Custom Homes	
						E12977404
Ash_RF	P3	HIP GIRDER	1	1		
					Job Reference (optional)	
Builders FirstSource, A	lbemarle, NC 28001		8.2	220 s Nov	16 2018 MiTek Industries, Inc. Mon Apr 29 13:29:40 2019	Page 2

8.220 s Nov 16 2018 Mi Lek Industries, Inc. Mon Apr 29 13:29:40 2019 Page 2 ID:GgN1wbXHPg6COW?w\_rAWmsz1jWL-VxWqGxBRzIkKjzk5YT\_rUbSnif2UDxxpK5EtpUzLrNf

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 8=-9(B) 7=-9(B) 17=-4(B) 18=-4(B)

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	0-0-8 0-0-8	<u>3-10-0</u> <u>3-9-8</u>	5-2	-0		8- 3	11-8 -9-8		9-0-0 0-0-8	
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	*	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	<b>CSI.</b> TC 0.18 BC 0.23 WB 0.04 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.01 -0.03 0.01 0.02	(loc) 8-11 8-11 5 8-11	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 35 lb	<b>GRIP</b> 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied, except

2-0-0 oc purlins (6-0-0 max.): 3-4.

Rigid ceiling directly applied.

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 2=370/0-3-0, 5=370/0-3-0 Max Horz 2=-16(LC 21) Max Uplift 2=-57(LC 12), 5=-57(LC 13) Max Grav 2=506(LC 39), 5=506(LC 39)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-676/439, 3-4=-577/416, 4-5=-676/439

BOT CHORD 2-8=-346/583, 7-8=-339/577, 5-7=-347/583

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-10-0, Exterior(2) 3-10-0 to 9-4-15, Interior(1) 9-4-15 to 9-10-8 zone; cantilever left and right exposed ; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
   Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
  10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WEATH CAS C Vermannen SEAL 036322 GI minum April 30,2019

818 Soundside Road Edenton, NC 27932

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Plate Offsets (X, r) [2:0-0-14,0-0-5]									
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.32 BC 0.25 WB 0.00 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc -0.02 4- -0.05 4- 0.00 2 0.05 4-	) l/defl 7 >999 7 >999 2 n/a 7 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 20 lb	<b>GRIP</b> 244/190 FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

**REACTIONS.** (lb/size) 2=220/0-3-0, 4=168/0-1-8

Max Horz 2=53(LC 16) Max Uplift 2=-28(LC 12), 4=-29(LC 13)

Max Grav 2=251(LC 2), 4=189(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 4-10-4 zone; cantilever left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs
- non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

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- 15-6-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.



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#### NOTES-

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 6-0-0, Exterior(2) 6-0-0 to 9-0-0, Interior(1) 9-0-0 to 11-6-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

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<sup>1)</sup> Unbalanced roof live loads have been considered for this design.

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Max Uplift 1=-1(LC 14), 3=-5(LC 15)

Max Grav 1=139(LC 2), 3=139(LC 2), 4=286(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;

- MWFRS (envelope) and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-0-0, Exterior(2) 4-0-0 to 7-0-0, Interior(1) 7-0-0 to 7-6-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD

REACTIONS. (lb/size) 1=107/3-11-4, 3=107/3-11-4 Max Horz 1=-17(LC 12) Max Grav 1=121(LC 2), 3=121(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.





