

Trenco 818 Soundside Rd Edenton, NC 27932

Re: Jackson_Plan Lamco Custom Homes

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource (Albermarle,NC).

Pages or sheets covered by this seal: E12977059 thru E12977077

My license renewal date for the state of North Carolina is December 31, 2019.

North Carolina COA: C-0844



April 29,2019

Gilbert, Eric

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.



0	r8 32-8-0
0	-8 32-7-8
Plate Offsets (X,Y)	[5:0-1-15,0-0-0], [6:0-2-12,0-2-4], [6:0-0-0,0-1-12], [11:0-3-0,Edge], [16:0-2-12,0-2-4], [16:0-0-0,0-1-12], [17:0-1-15,0-0-0], [20:0-0-0,0-1-5], [20:0-3-1
	Edgel

,=ugoj								
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.13 BC 0.09 WB 0.16 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (lo -0.00 2-3 -0.01 2-3 0.01 2-3 0.01 2-3	c) I/defl 36 >999 36 >999 20 n/a 36 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 202 lb	GRIP 244/190 FT = 20%
LUMBER-TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2OTHERS2x4 SP No.3WEDGE		BR TO BC	ACING- OP CHORD S OT CHORD F	Structural wo Rigid ceiling	ood sheathir directly app	ng directly a lied or 10-0-	pplied or 6-0-0 oc purlins -0 oc bracing.	5.

Right: 2x4 SP No.2

- (lb) Max Horz 2=80(LC 13)
 - Max Uplift All uplift 100 lb or less at joint(s) 30, 31, 33, 34, 35, 36, 27, 26, 24, 23, 22, 21 Max Grav All reactions 250 lb or less at joint(s) 2, 2, 29, 30, 31, 33, 34, 35, 28,
 - 27, 26, 24, 23, 22, 20 except 36=272(LC 34), 21=259(LC 35)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 16-6-0, Corner(3) 16-6-0 to 19-6-0, Exterior(2) 19-6-0 to 32-8-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 30, 31, 33, 34, 35, 36, 27, 26, 24, 23, 22, 21.



818 Soundside Road Edenton, NC 27932

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REACTIONS. All bearings 32-8-0.



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TRENGINEERING BY REENCO A MITek Attiliate 818 Soundside Road

Edenton, NC 27932



	0-0 ₆ 8 2-4-0	8-1-0	13-10	-0	21-10-6	27-	-2-13	32-8-0		
	0-0-8 2-3-8	5-9-0	5-9-0)	8-0-6	5-	-4-6	5-5-3		
Plate Offsets (X	,Y) [2:0-0-0,0	0-0-1], [3:0-2-0,0-1-8], [12	:0-0-3,0-0-7], [12:0-0-7,0-5-8], [17:0)-4-0,0-2-0], [19:0-0-1	13,0-1-9], [19:0-3	3-0,0-0-8], [19:0)-7-15,0-1-5],	[19:0-0-0,0-1-12]	1
LOADING (psf) TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	20.0 15.4/20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.15 1.15 YES 12014	CSI. TC 0.55 BC 0.82 WB 0.80 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.21 18-19 -0.42 18-19 0.19 12 0.12 18-19	l/defl L/d >999 360 >925 240 n/a n/a >999 240	d D D a D	PLATES MT20 MT18HS Weight: 199 lb	GRIP 244/190 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS WEDGE Right: 2x4 SP N	LUMBER- TOP CHORD 2x4 SP No.2 BRACING- TOP CHORD BOT CHORD 2x4 SP No.2 *Except* 3-17: 2x4 SP 2400F 2.0E TOP CHORD Structural wood sheathing directly applied. BOT CHORD Rigid ceiling directly applied. WEBS 1 Row at midpt 4-18 WEBS 2x4 SP No.3 WEBS 1 Row at midpt 4-18 WEDGE Right: 2x4 SP No.2 EACTIONS. (Ib/size) 2=1196/0-3-8. 12=1155/Mechanical									
REACTIONS.	REACTIONS. (Ib/size) 2=1196/0-3-8, 12=1155/Mechanical Max Horz 2=82(LC 13) Max Grav 2=1354(LC 2), 12=1306(LC 2)									
FORCES. (Ib) TOP CHORD BOT CHORD WEBS	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 3-22=-2315/110, 3-4=-4805/181, 4-6=-2762/138, 6-7=-1938/155, 7-8=-1855/208, 8-9=-1983/224, 9-11=-1971/146, 11-12=-2303/138 BOT CHORD 2-20=-69/1951, 19-20=-47/1707, 3-19=-118/3412, 18-19=-147/4281, 17-18=-42/2441, 7-17=-251/105, 15-16=0/254, 13-15=-61/1990, 12-13=-61/1990 WEBS 6-18=0/468, 6-17=-908/73, 15-17=0/1097, 8-17==64/916, 8-15=-80/707, 9-15=-354/129, 11-15=-353/74, 3-20=-1783/59, 4-19=0/999, 4-18=-1851/112									
NOTES- 1) Unbalanced 2) Wind: ASCE MWFRS (em 32-8-0 zone; grip DOL=1.6 3) TCLL: ASCE roof snow: Lu 4) Unbalanced 5) This truss ha non-concurre 6) All plates are 7) This truss ha 8) * This truss ha will fit between	roof live loads ha 7-10; Vult=120m velope) and C-C cantilever left ar 30 7-10; Pr=20.0 p umber DOL=1.15 snow loads have s been designed mt with other live MT20 plates un s been designed tas been designed	ave been considered for the ph (3-second gust) Vasd Exterior(2) -0-10-8 to 2-1 and right exposed ;C-C for a sf (roof live load: Lumber 5 Plate DOL=1.15); Categ been considered for this 1 for greater of min roof live loads. It is so therwise indicated. If or a 10.0 psf bottom choosed of 20.0 psg bottom choosed for a live load of 20.0 psg bottom choosed for a live load of 20.0 psg dand any other member of the solution choosed for a live load of 20.0 psg dand any other member of the solution choosed for a live load of 20.0 psg dand any other member of the solution choosed for a live load of 20.0 psg dand any other member of the solution choosed for a live load of 20.0 psg dand any other member of the solution choosed for a live load of 20.0 psg dand any other member of the solution choosed for the solution choosed for a live load of 20.0 psg dand any other member of the solution choosed for a live load of 20.0 psg dand any other member of the solution choosed for the solution choosed for a live load of 20.0 psg dand and any other member of the solution choosed for the solution choosed for a live load of 20.0 psg dand any other member of the solution choosed for the solution choose	his design. =95mph; TCDL -8, Interior(1) 2 members and f DOL=1.15 Plat ory II; Exp B; P design. e load of 12.0 p ord live load no of on the bottom rs	=6.0psf; BCDL=6.0p -1-8 to 16-6-0, Exter forces & MWFRS for te DOL=1.15); Pg=2(artially Exp.; Ct=1.10 psf or 1.00 times flat nconcurrent with any n chord in all areas w	osf; h=25ft; Cat. II; Ex ior(2) 16-6-0 to 19-6- reactions shown; Lu 0.0 psf (ground snow) roof load of 15.4 psf r other live loads. rhere a rectangle 3-6	φ B; Enclosed; 0, Interior(1) 19- mber DOL=1.60); Pf=15.4 psf (fli on overhangs -0 tall by 2-0-0 w	6-0 to plate at	and a second second	SEAL 036322	

9) Refer to girder(s) for truss to truss connections.

10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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	0-0-8 2-4-0	8-1-0	13-10-0	1		21-10-6		27-2-	13	1	32-11-8	33- Q -0	
	0-0-8 2-3-8	5-9-0	5-9-0	1		8-0-6		5-4	-6		5-8-11	0-0-8	
Plate Offsets (X	(,Y) [2:0-0-0,0	0-0-1], [3:0-1-12,0-1-12], [12:0-0-0,0-0-1],	[18:0-2-12	2,0-2-8], [20	:0-0-0,0-2-12],	20:0-1-4,0	-2-7], [20):0-3-0,0-	0-8], [20:0	0-7-10,0-1	-10]	
LOADING (psf TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL) 20.0 15.4/20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 1.15 1.15 YES I2014	CSI. TC BC WB Matri	0.52 0.77 0.80 ix-AS	DEFL. Vert(L) Vert(C) Horz(C) Wind(I)	ii .) -0.20 Г) -0.4 Т) 0.19 L) 0.1	n (loc)) 19-20 19-20) 12) 12 19-20	l/defl >999 >969 n/a >999	L/d 360 240 n/a 240		PLATES MT20 MT18HS Weight: 201 lb	GRIP 244/190 244/190 FT = 20%
BODIC 10.0 BR/ LUMBER- BR/ TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 *Except* 3-18: 2x4 SP 2400F 2.0E WE WEBS 2x4 SP No.3 *Except* 4-20: 2x6 SP No.2 4-20:					BRACING- TOP CHORD BOT CHORD WEBS	Structu Rigid c 1 Row	ral wooc eiling dir at midpt	l sheathin ectly appl	g directly ied. 4-19	applied.			
REACTIONS.	(Ib/size) 2=12 Max Horz 2=-8 Max Grav 2=13	206/0-3-8, 12=1212/0-3-8 0(LC 14) 366(LC 2), 12=1372(LC 2)											
FORCES. (Ib) TOP CHORD) - Max. Comp./M 3-23=-2345/11 8-9=-2031/220	lax. Ten All forces 250 (2, 3-4=-4766/171, 4-6=-2), 9-11=-2026/142, 11-12=	lb) or less exce 790/136, 6-7=-´ 2461/125	pt when sh 1969/154,	nown. 7-8=-1885/2	203,							
BOT CHORD	2-21=-58/1982	2, 20-21=-38/1692, 3-20=-	93/3286, 19-20	=-119/424	8, 18-19=-17	7/2465,							
WEBS	7-18=-262/105 6-19=0/468, 6- 11-16=-455/64	5, 16-17=0/259, 14-16=-38 -18=-904/71, 16-18=0/111 I, 3-21=-1688/48, 4-20=0/9	9/2137, 12-14=- 9, 8-18=-60/92 972, 4-19=-179	38/2137 0, 8-16=-8 5/102	2/735, 9-16=	=-337/127,							
NOTES													

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 16-6-0, Exterior(2) 16-6-0 to 19-6-0, Interior(1) 19-6-0 to 33-10-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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A MiTek A

April 29,2019



0-0-8		<u>32-11-8</u> 32-11-0									
Plate Offsets (X,Y) [5:0-1-15,0-0-0], [6:0-2-12,0-2-4], [6:0-0-0,0-1-12], [11:0-3-0,Edge], [16:0-2-12,0-2-4], [16:0-0-0,0-1-12], [17:0-1-15,0-0-0]											
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.13 BC 0.09 WB 0.16 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.00 20-22 -0.01 20-22 0.01 20 0.00 2-37	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 203 lb	GRIP 244/190 FT = 20%			
LUMBER- TOP CHORD 2x4 SP No 2		BR TO	ACING-	Structural wood	sheathin	a directly apr	blied or 6-0-0 oc purlins				

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

REACTIONS. All bearings 32-11-0.

(lb) - Max Horz 2=-80(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 31, 32, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22

Max Grav All reactions 250 lb or less at joint(s) 2, 2, 30, 31, 32, 34, 35, 36, 29, 28, 27, 25, 24, 23, 20 except 37=272(LC 34), 22=272(LC 35)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 16-6-0, Corner(3) 16-6-0 to 19-6-0, Exterior(2) 19-6-0 to 33-10-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
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- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 31, 32, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22.



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22-10-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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Job	Truss	Truss Type	Qty	Ply	Lamco Custom Homes	
					E12	2977068
Jackson_Plan	C3G	Common Girder	1	2		
)	Job Reference (optional)	
Builders FirstSource. A	bemarle . NC 28001		8.2	220 s Nov	16 2018 MiTek Industries, Inc. Mon Apr 29 13:12:33 2019 Pa	ade 2

8.220 s Nov 16 2018 MiTek Industries, Inc. Mon Apr 29 13:12:33 2019 Page 2 ID:TzqEIgM?vNsmIViTkhYcdxyrx_7-yTDW?g5ZrgfIXktVk8IgtXQJggVI4JIeApy2GVzLrdi

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-51, 3-5=-51, 10-13=-20

Concentrated Loads (lb)

Vert: 7=-1225(b) 6=-1225(B) 16=-1225(B) 17=-1225(B) 18=-1225(B) 19=-1225(B) 20=-1225(B) 21=-1225(B) 22=-1135(B) 23=-1135(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses safe truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





			5-11-8				\neg	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.19 BC 0.13 WB 0.07 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.01 2-6 -0.02 2-6 0.00 0.02 2-6	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 25 lb	GRIP 244/190 FT = 20%
LUMBER-		BRA	CING-					

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.2
OTHERS	2x4 SP No.3

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD

- REACTIONS. All bearings 5-11-8.
 - (lb) Max Horz 2=52(LC 12)
 - Max Uplift All uplift 100 lb or less at joint(s) 5, 2, 6 Max Grav All reactions 250 lb or less at joint(s) 5, 2, 2 except 6=317(LC 2)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 5-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2, 6.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only design parameters and READ NOTES ON TIPS ON MICLODED MITER REFERENCE PAGE mit-14/3 at 900, 1002/015 BEFORE 052. Design valid for use only with MITeR works connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Snow (Pf/P	g) ′
TCDL	

BCLL

BCDL

LUN	IBER-	

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

2x4 SP No.3 WEBS

REACTIONS. (lb/size) 2=255/0-3-0, 4=204/0-1-8 Max Horz 2=55(LC 12) Max Uplift 2=-54(LC 12), 4=-50(LC 12) Max Grav 2=291(LC 2), 4=230(LC 2)

10.0

10.0

0.0

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Rep Stress Incr

Code IRC2015/TPI2014

NOTES-

1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-10-4 zone; cantilever left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

YES

WB

Matrix-AS

0.00

Horz(CT)

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

0.01

0.10

2

Rigid ceiling directly applied.

4-7

n/a

>681

n/a

240

Structural wood sheathing directly applied, except end verticals.

- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Weight: 22 lb

FT = 20%

818 Soundside Road Edenton, NC 27932

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LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 PCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.10 BC 0.07 WB 0.00	DEFL. Vert(LL) -0.0 Vert(CT) -0.0 Horz(CT) 0.0	in (loc) 00 4-7 01 4-7 00 2	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MP	Wind(LL) 0.0	00 4-7	>999	240	Weight: 12 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 2=152/0-3-8, 4=94/0-1-8 Max Horz 2=32(LC 12) Max Uplift 2=-15(LC 12), 4=-3(LC 16) Max Grav 2=175(LC 2), 4=106(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 2-10-4 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 3-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/ITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

A MiTek Affiliate 818 Soundside Road Edenton, NC 27932



7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 6.



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4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 6.



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REACTIONS. All bearings 13-6-12. (lb) - Max Horz 1=-71(LC 10)

Max Horz 1=-71(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 1, 8, 6

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=265(LC 2), 8=307(LC 25), 6=307(LC 26)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;

MWFRS (envelope) and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 6-9-12, Exterior(2) 6-9-12 to 9-9-12, Interior(1) 9-9-12 to 13-1-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8, 6.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;

- MWFRS (envelope) and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 5-3-12, Exterior(2) 5-3-12 to 8-3-12, Interior(1) 8-3-12 to 10-1-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

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Max Uplift 1=-1(LC 14), 3=-4(LC 15)

Max Grav 1=131(LC 2), 3=131(LC 2), 4=271(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;

- MWFRS (envelope) and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 3-9-12, Exterior(2) 3-9-12 to 6-9-12, Interior(1) 6-9-12 to 7-1-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

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0-0-	-0	4-7-2						
Plate Offsets (X,Y) [2:0-2-0,E	Edge]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.06 BC 0.16 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 13 lb	GRIP 244/190 FT = 20%
LUMBER-		BR	ACING-					
TOP CHORD 2x4 SP No 2		то	P CHORD S	Structural wood	l sheathin	a directly app	lied or 4-7-8 oc purlir	IS.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

REACTIONS. (lb/size) 1=130/4-6-12, 3=130/4-6-12 Max Horz 1=-21(LC 10) Max Grav 1=146(LC 2), 3=146(LC 2)

