66'-0" 24'-0" Knee Wall - Mın 24" Hıgh Concrete Patio Carport (smooth concrete) 781 square feet Vinyl Ceiling 12'-8" 29'-4" 8" CMU Foundation Wall over 16"w x 10"d deep footing with 2pc of 1/2" rebar 18'-0" Back Porch (broomed finish concrete) 8'-8" 9'-4" Back Porch and Landing to be Same Height Dropped 3-2x10 spruce girder (typ) TJI FLOOR SYSTEM BY ENGINEER ****** 8" CMU Foundation Wall over 16"w x 10"d deep footing with 2pc of 1/2" rebar Front Porch (broomed finish concrete) 12'-8" 8'-6" 12'-8" 14'-8" 9'-4" 33'-6" 66'-0"



Span Table for Joist and Rafters.

-Floors shall be constructed in accordance with the provisions of Chapter 5 of the NC State Building Code, Sect. R502.2 and Sects R319 and R320.

-Spans for floor joist shall be in accordance with Tables R502.3.1(1) and R502.3.1(2). For other grades and species and for other loading conditions, refer to the AF\$PA

-The allowable span of girders fabricated of dimension lumber shall not exceed the values set forth in Tables R502.5(1) and R502.5(2).

-Local soil conditions and/or local practice may necessitate a more stringent footing and foundation wall design. Consult with local building inspector. Soil design bearing pressure is assumed 2000 psf.

Carry all footings to firm undisturbed bearing:

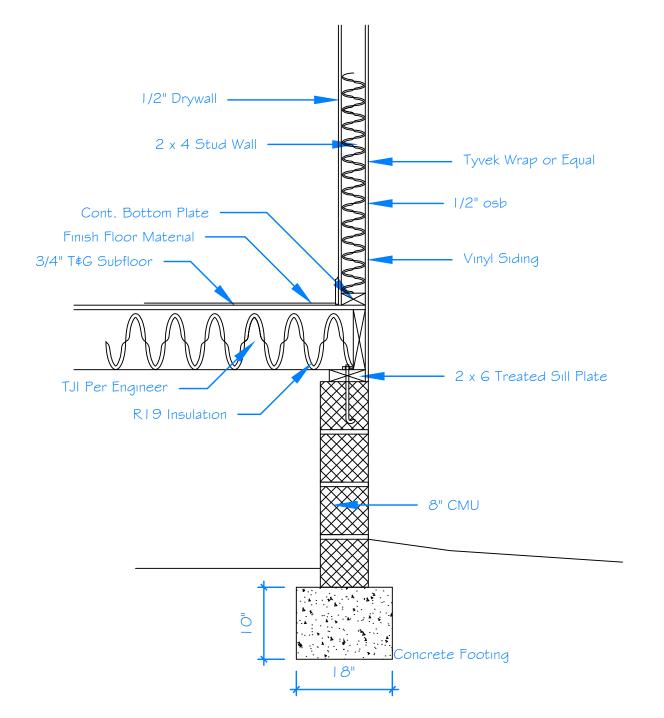
-18" x 10" footing for 8" foundation wall.

-24" x 10" footing for 12" foundation wall.

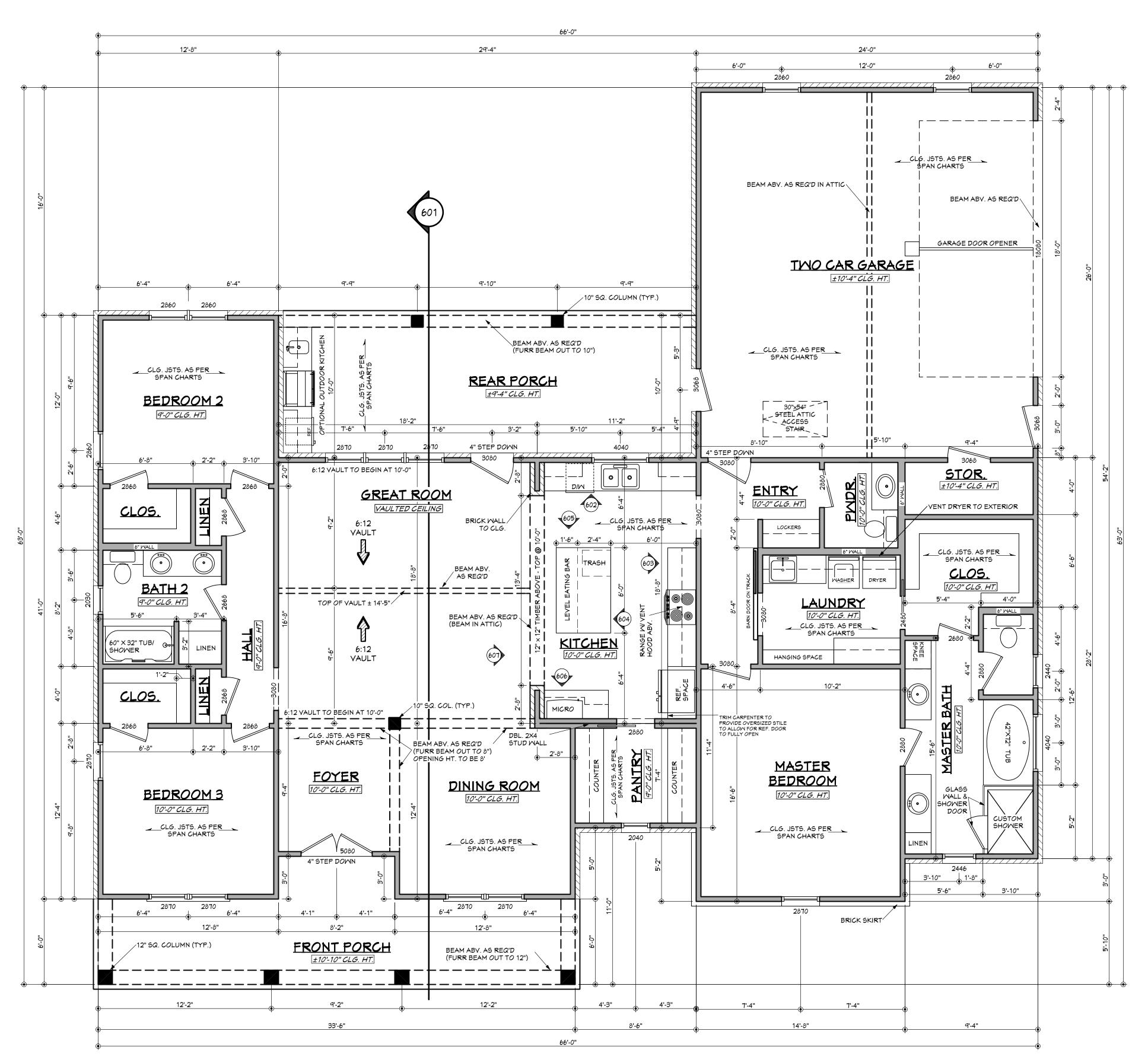
Pier Footings (Typical Unless Otherwise Notes)
-Provide 1'-8" x 2'-4" x 10" deep concrete footing under 8" x 16" masonry piers.

-Provide 2'-0" square x 10" deep concrete footing with under 16" square masonry piers.

-Grout piers solid with 2500psi concrete (typ).



Foundation Section



NOTE: CONTRACTOR TO LOCATE WATER HEATER AND HVAC UNITS AT SITE.

FLOOR PLAN

AREAS:	2044	S.F. HEATED - NOT INCLUDING MASONRY
	624	S.F. UNHEATED - GARAGE
	293	S.F. UNHEATED - REAR PORCH
	37	S.F. UNHEATED - STORAGE
	226	S.F. UNHEATED - FRONT PORCH
	1180	S.F. UNHEATED - TOTAL
	3224	S.F. TOTAL (WITHOUT MASONRY)

NOTES:

1. ALL DIMENSIONS & SITE CONDITIONS TO BE VERIFIED BY CONTRACTOR

PRIOR TO CONSTRUCTION.

2. ALL FINISHES (INTERIOR & EXTERIOR) TO BE VERIFIED WITH OWNER PRIOR TO CONSTRUCTION.

3. VERIFY ALL DOOR AND WINDOW STYLES AND SIZES WITH OWNER PRIOR TO CONSTRUCTION. MANUFACTURER TO SUPPLY ALL ROUGH OPENING SIZES.
4. CONTRACTOR TO VERIFY ALL CLEARANCES OF ALL DOORS, WINDOWS AND OTHER ITEMS THAT ARE CRITICAL, PRIOR TO CONSTRUCTION.

5. CONTRACTOR TO ADAPT PLANS AS REQUIRED TO MEET ALL APPLICABLE CODES AT SITE.

6. ALL BEAMS TO BE SIZED BY A LICENSED STRUCTURAL ENGINEER.
7. PORCHES, BALCONIES OR RAISED FLOOR SURFACES LOCATED MORE THAN 30 INCHES ABOVE THE FLOOR OR GRADE BELOW AT ANY POINT WITHIN 36 INCHES HORIZONTALLY SHALL HAVE GUARDS NOT LESS THAN 36 INCHES IN HEIGHT. OPEN SIDES OF STAIRS WITH A TOTAL RISE OF MORE THAN 30 INCHES ABOVE THE FLOOR OR GRADE BELOW SHALL HAVE GUARDS NOT LESS THAN 34 INCHES IN HEIGHT MEASURED VERTICALLY FROM THE NOSING OF THE TREADS. INSECT SCREENING SHALL NOT BE CONSIDERED AS A GUARD. IRC 2018, R312.1.1 & R312.1.2

8. M1305.1.2 APPLIANCES IN ATTICS. ATTICS CONTAINING APPLIANCES SHALL BE PROVIDED WITH AN OPENING AND A CLEAR AND UNOBSTRUCTED PASSAGEWAY LARGE ENOUGH TO ALLOW REMOVAL OF THE LARGEST APPLIANCE, BUT NOT LESS THAN 30 INCHES HIGH AND 22 INCHES WIDE AND NOT MORE THAN 20 FEET LONG MEASURED ALONG THE CENTERLINE OF THE PASSAGEWAY FROM THE OPENING TO THE APPLIANCE. THE PASSAGEWAY SHALL HAVE CONTINUOUS SOLID FLOORING IN ACCORDANCE WITH CHAPTER 5 NOT LESS THAN 24 INCHES WIDE. A LEVEL SERVICE SPACE AT LEAST 30 INCHES DEEP AND 30 INCHES WIDE SHALL BE PRESENT ALONG ALL SIDES OF THE APPLIANCE WHERE ACCESS IS REQUIRED. THE CLEAR ACCESS OPENING DIMENSIONS SHALL BE A MINIMUM OF 20 INCHES BY 30 INCHES, AND LARGE ENOUGH TO ALLOW REMOVAL OF THE LARGEST APPLIANCE.

a. THE PASSAGEWAY AND LEVEL SERVICE SPACE ARE NOT REQUIRED WHERE THE APPLIANCE CAN BE SERVICED AND REMOVED THROUGH THE REQUIRED OPENING.

b. WHERE THE PASSAGEWAY IS UNOBSTRUCTED AND NOT LESS THAN 6 FEET HIGH AND 22 INCHES WIDE FOR ITS ENTIRE LENGTH, THE PASSAGEWAY SHALL BE NOT MORE THAN 50 FEET LONG.

9. APPLIANCE ACCESS FOR INSPECTION SERVICE, REPAIR AND REPLACEMENT. APPLIANCES SHALL BE ACCESSIBLE FOR INSPECTION, SERVICE, REPAIR AND REPLACEMENT WITHOUT REMOVING PERMANENT CONSTRUCTION, OTHER APPLIANCES, OR ANY OTHER PIPING OR DUCTS NOT

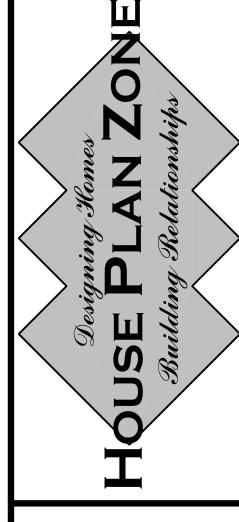
CONNECTED TO THE APPLIANCE BEING INSPECTED, SERVICED, REPAIRED OR REPLACED. A LEYEL WORKING SPACE AT LEAST 30 INCHES DEEP AND 30 INCHES WIDE SHALL BE PROVIDED IN FRONT OF THE CONTROL SIDE TO SERVICE AN APPLIANCE. M1305.1.1

10. EMERGENCY ESCAPE AND RESCUE OPENINGS SHALL BE OPERATIONAL FROM THE INSIDE OF THE ROOM WITHOUT THE USE OF KEYS, TOOLS OR SPECIAL KNOWLEDGE. WINDOW OPENING CONTROL DEVICES COMPLYING WITH ASTM F 2090 SHALL BE PERMITTED FOR USE ON WINDOWS SERVING AS A REQUIRED EMERGENCY ESCAPE AND RESCUE OPENING. ALL SLEEPING ROOMS TO HAVE AN EXTERIOR ACCESS THROUGH A DOOR OR WINDOW WITH A MINIMUM OF 5.7 SQUARE FEET NET CLEAR OPENING AS PER IRC 2018 R310.2.1. EXCEPTION: GRADE FLOOR OR BELOW GRADE OPENINGS SHALL HAVE A MINIMUM NET CLEAR OPENING OF 5 SQUARE FEET. MAXIMUM SILL HEIGHT TO BE 44 INCHES. MINIMUM NET CLEAR OPENING HEIGHT TO BE 24 INCHES. MINIMUM NET CLEAR OPENING WIDTH TO BE 20 INCHES.

11. ALL RETURN AIR GRILLS ARE TO BE LOCATED TO COMPLY WITH SECTION M1602 OF THE IRC 2018.

12. ALL SQUARE FOOTAGE MEASUREMENTS ARE APPROXIMATE AND MAY DIFFER FROM ACTUAL CONSTRUCTED RESIDENCE OR BUILDING.
13. FIRE SPRINKLER SYSTEM TO BE DESIGNED AND INSTALLED (IF REQUIRED BY LOCAL CODES) AS PER THE IRC 2018 AND BY A LICENSED PROFESSIONAL IN THE AREA OF CONSTRUCTION.

14. ALL BATHROOM EXHAUST VENTS SHALL BE VENTED DIRECTLY TO THE EXTERIOR OF THE HOME AND NOT INTO THE ATTIC. IRC 2018, M1505.2



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ort in the development of these plans and the completion of these con a codes and site specific conditions, House Plan Zone, LLC, assumed as resulting from errors, omissions or deficiencies in the design.

<u>D</u>

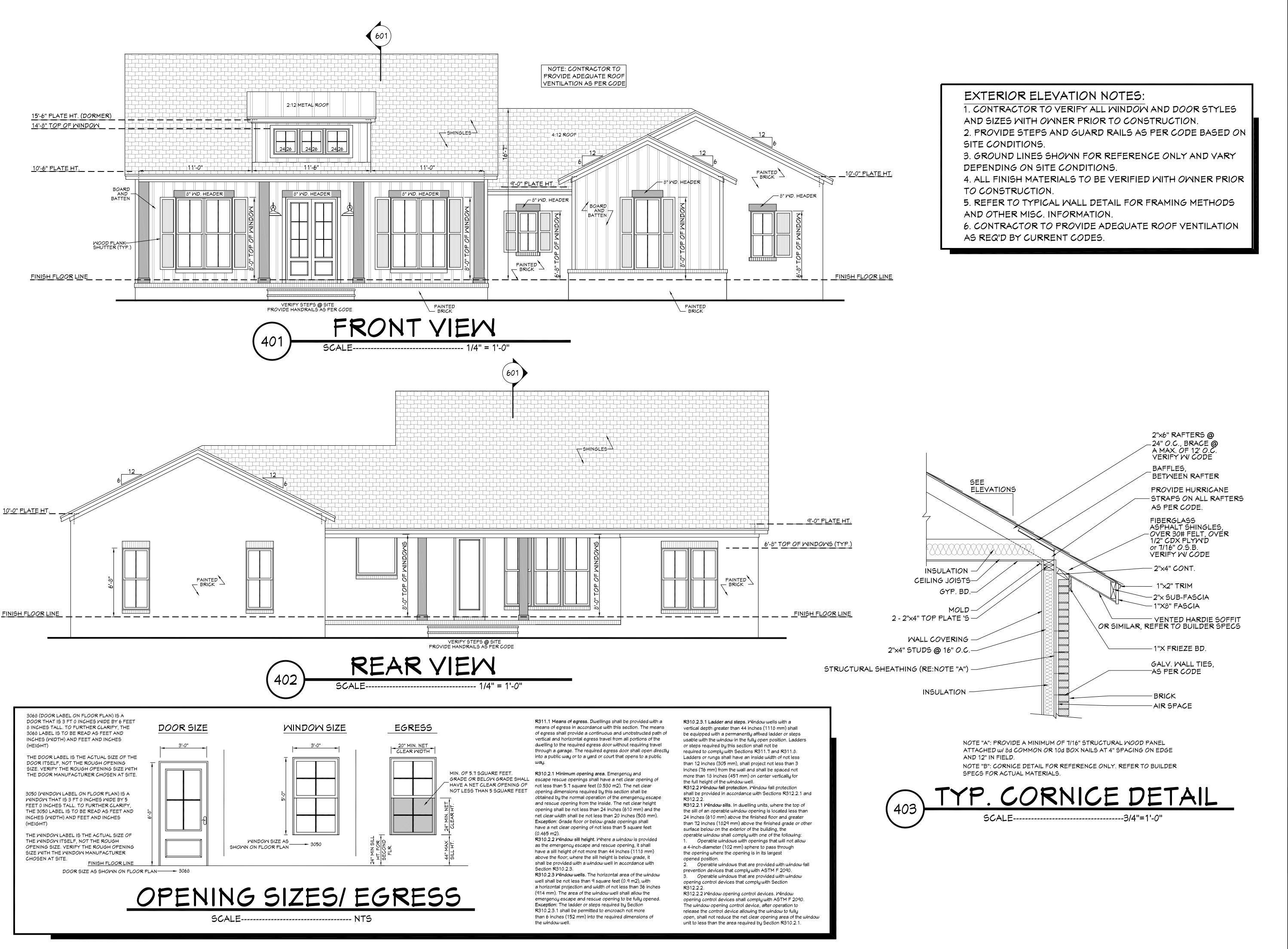
louse Plan Zone, LLC. has exercised great care and effort in ocuments. However, due to the great variance in building cocaponsibility for any damages, including structural failures restouse Plan Zone, LLC. highly recommends that these plans be addition to your local building officials prior to construction. After special conditions required building codes. All differences and second supplied building codes.

te: 08.29.19

Drawn By: B.L.L.

Project Name:

SHEET NUMBER



HOUSE PLAN ZONE

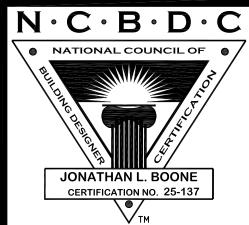
Building Relationships

Mebsite: www.HPZplans.com

Email: sales@hpzplans.com

Phone: 601.336.32**5**4

Fax: 1.800.**57**4.138**7**



The completion of these construction is Plan Zone, LLC. assumes no ilencies in the design.

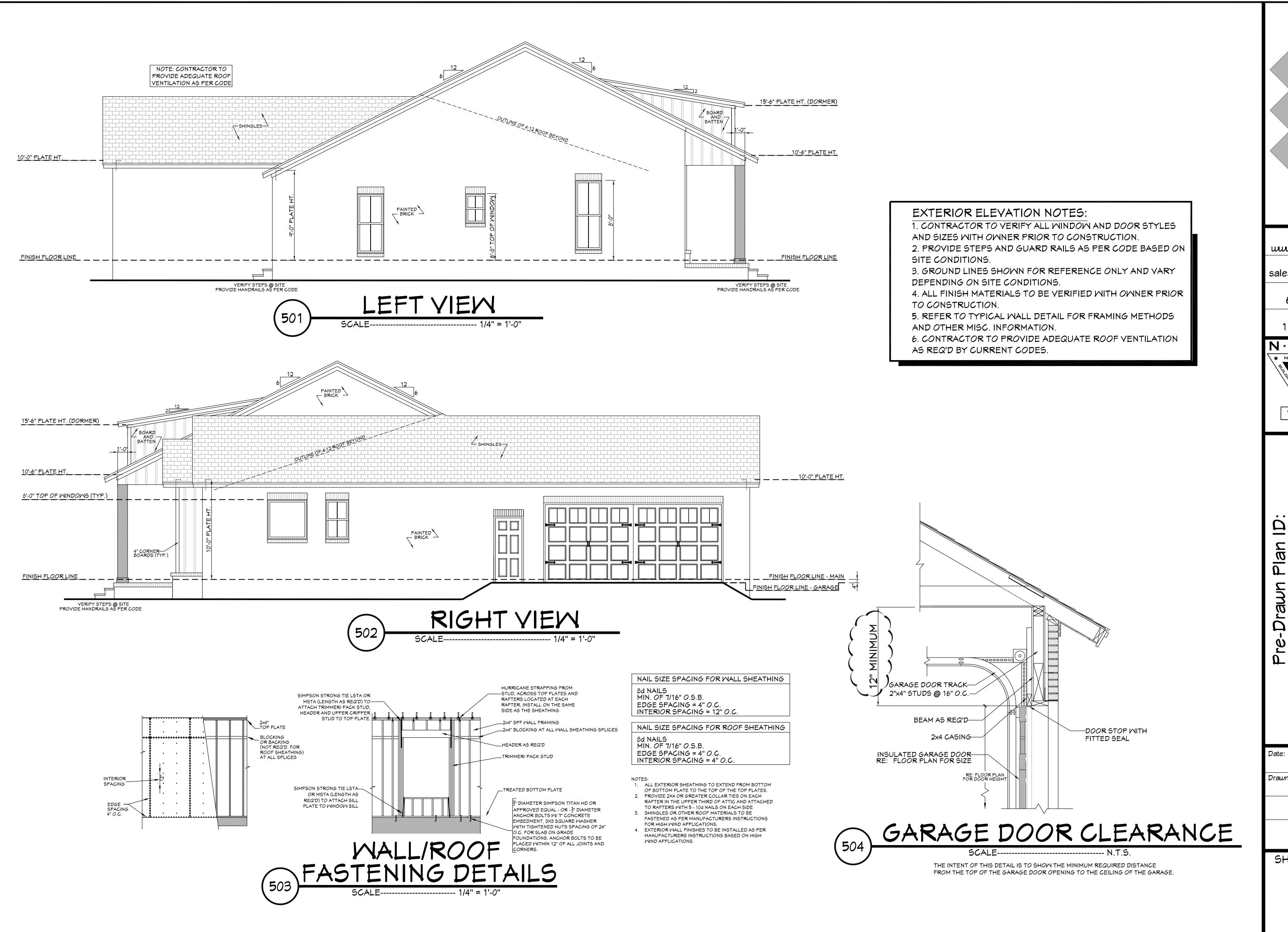
te:

08.29.19

Drawn By: B.L.L.

SHEET NUMBER

4



Designing Homes

HOUSE PLAN ZONE

Building Relationships

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Fax: 1.800.**57**4.1387



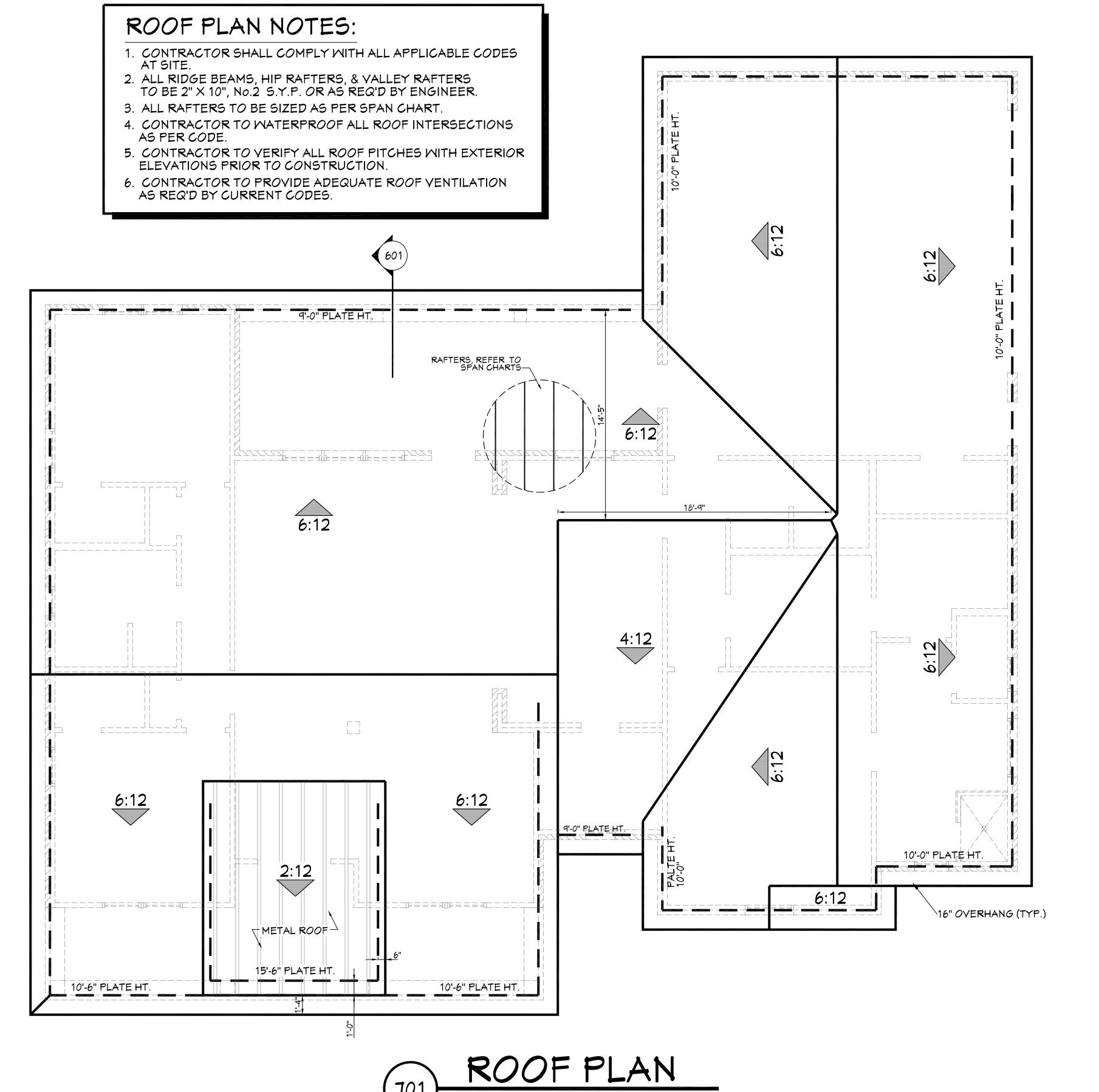
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B-2044R

e: 08.**2**9.19

Drawn By: B.L.L.

SHEET NUMBER

5



HIP/ VALLEY CONVERSION IF COMMON RAFTER ROOF THEN HIP/ VALLEY RAFTER ROOF PITCH BECOMES .. RISE/ RUN SLOPI 1/17 2/12 2/17 3/12 3/17 4/12 4/17 5/12 5/17 6/12 6/17 7/12 7/17 8/12 8/17 9/12 10/12 10/17 11/12 42° 11/17

CONVERSION CHART FOR SIMPLE ROOFS ONLY.

CHART DOES NOT APPLY FOR DUAL PITCH ROOFS.

12/17

12/12

ROOF PITCH	FACTOR
3/12	1.05
4/12	1.07
5/12	1.10
6/12	1.14
7/12	1.17
8/12	1.20
9/12	1.25
10/12	1.30
11/12	1.35
12/12	1.40
14/12	1.54
16/12	1.70

G JOIST SPANS FOR SOUTHERN PINE

CEILING JOIST SPANS FOR SOUTHERN PINE SPECIES (UNINHABITABLE ATTICS WITH LIMITED STORAGE, LIVE LOAD = 20psf, L/\(\triangle = 240\) DEAD LOAD = 10psf)

IF HABITABLE ATTIC SPACE IS DESIRED, REFER TO THE INTERNATIONAL RESIDENTIAL CODE, SPAN TABLES.

SIZE	SPACING (INCHES)	VISUALLY GRADED #2 SOUTHERN PINE (MAXIMUM CEILING JOIST SPANS) (FT IN.)
	12.0	9-3
2×4	16.0	8-0
	19.2	7-4
	24.0	6-7
	12.0	13-11
2×6	16.0	12-0
2.0	19.2	11-0
	24.0	9-10
	12.0	17-7
2×8	16.0	15-3
2 x 0	19.2	13-11
	24.0	12-6
	12.0	20-11
2 4 10	16.0	18-1
2×10	19.2	16-6
	24.0	14-9

The above tables are based on the IRC 2018 TABLE R802.5.1(2)

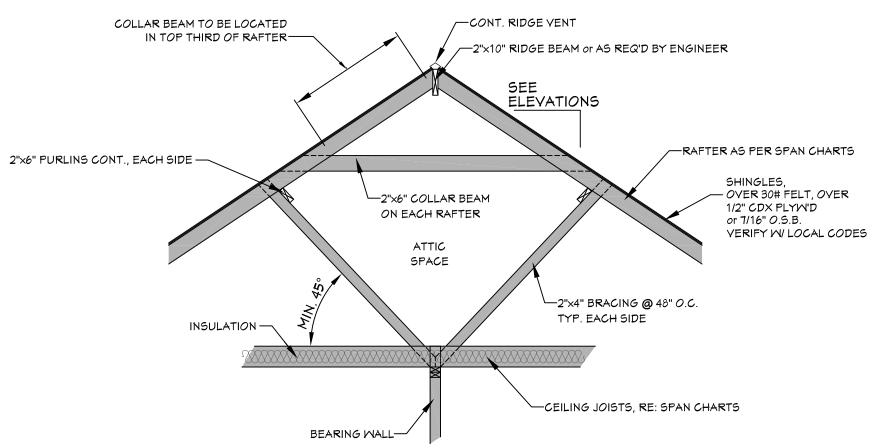
RAFTER SPANS

RAFTER SPANS FOR SOUTHERN PINE SPECIES LIVE LOAD=30psf, L/\(\triangle = 180\) DEAD LOAD = 10psf

SIZE SPACING (INCHES) SPANS (MAXIMUM RAFTER SPAN BETWEEN BRACING) (FT IN.) 12.0 12-11 16.0 11-2 19.2 10-2 24.0 9-2 12.0 16-4
16.0 11-2 19.2 10-2 24.0 9-2 12.0 16-4
X 19.2 10-2 24.0 9-2 12.0 16-4
24.0 9-2 12.0 16-4
12.0 4-2
O 16.0 14-2
X 19.2 12-11
24.0
12.0 19-5
12.0 19-5 16.0 16-10 X 19.2 15-4
CV 24.0 13-9
12.0 22-10
12.0 22-10 16.0 19-10
★ 19.2 18-1
(16-2)

NOTE

The above tables are based on the IRC 2018 TABLE R802.4.1(3)





HOUSE PLAN ZONE

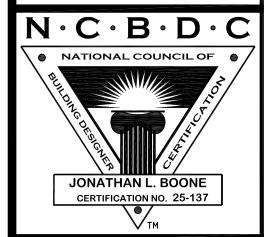
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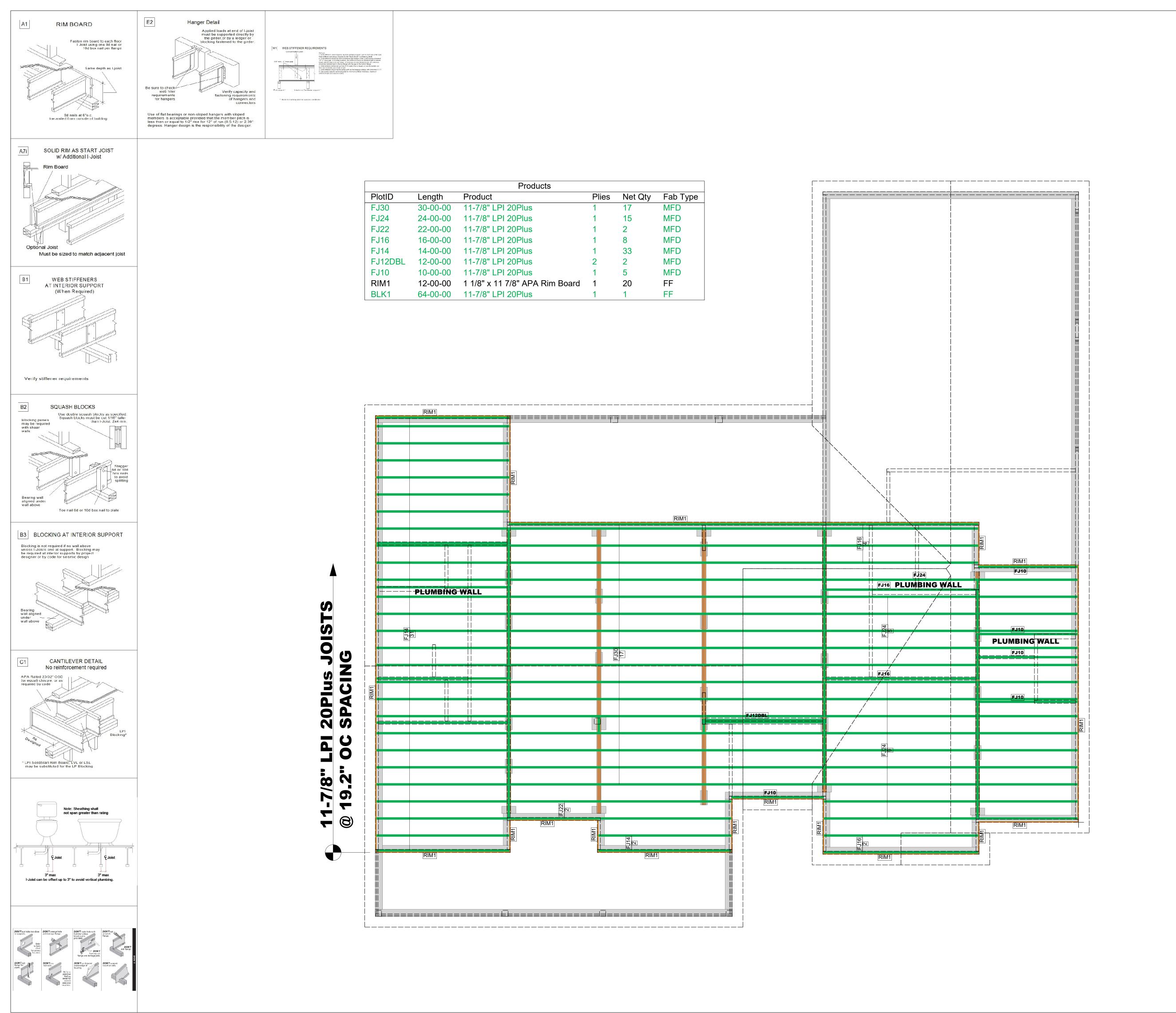
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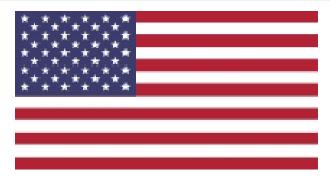
Date: 08.29.19
Drawn By:

B.L.L.

SHEET NUMBER







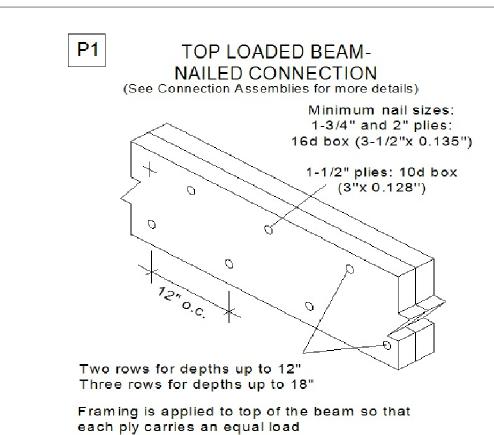
U.S. LUMBER

Important Notes WARNING: Failure to follow proper procedures for handling, storage and installation could result in unsatisfactory performance, unsafe

structures and possible collapse.

These instructions are offered as a guide to good practice in the handling, storage and installation of LP® SolidStart® I-Joists, LP SolidStart LVL & LP SolidStart LSL beams. They are, however, solely general recommendations and, in some instances, other or additional precautions may be desirable. In all cases, the procedures used should be as specified by the architect/engineer responsible for the entire building.

- This is not intended as a manual for selecting products and assumes that components and details have been specified correctly.
- Consult the LP SolidStart I-Joist, LP SolidStart LVL & LP SolidStart LSL brochures or contact your LP SolidStart products distributor for assistance.
- All rim joists, blocking, connections and temporary bracing must be installed before
- erectors are allowed on the structure.No loads other than the weight of the erectors are to be imposed on the structure
- before it is permanently sheathed.
- After sheathing, do not overload joists with construction materials exceeding design loads.
 LP SolidStart I-Joists, LP SolidStart LVL & LP SolidStart LSL beams must be used under dry, covered and well ventilated interior conditions in which the equivalent moisture content in lumber will not exceed 16%.



Customer Name:

BRAD CUMMINGS

Job Name: BURGESS/SPRING HILL CHURCH RD.

Designer:
Tony Huneycutt

Salesman:

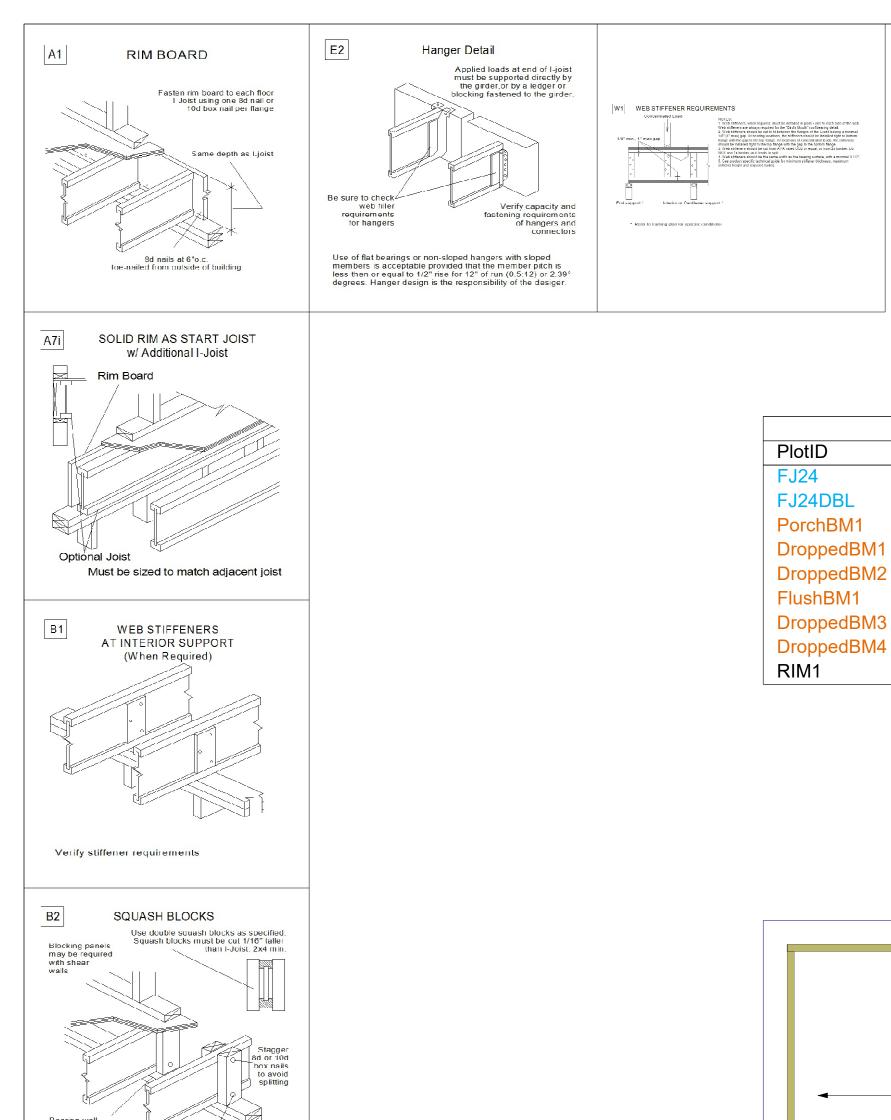
EDDIE BAUER

Scale:1/4" = 1'

Date:10/13/20

1ST FLOOR





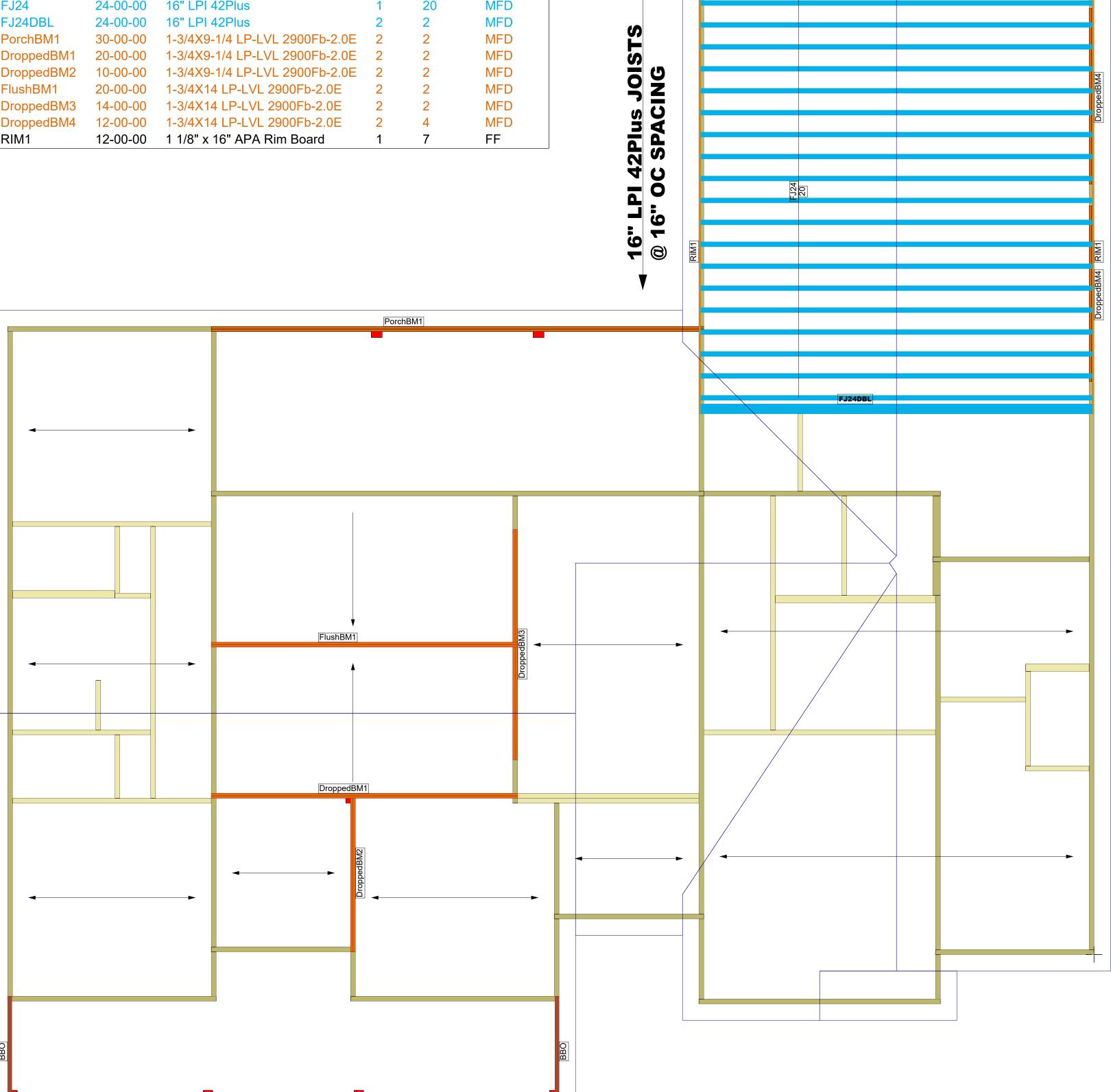
Toe nail 8d or 10d box nail to plate

B3 BLOCKING AT INTERIOR SUPPORT

CANTILEVER DETAIL
No reinforcement required

Note: Sheathing shall not span greater than rating

Blocking is not required if no wall above unless I-Joists end at support. Blocking may be required at interior supports by project designer or by code for seismic design



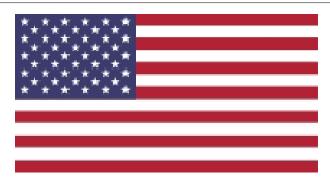
Fab Type

Products

Product

Length





U.S. LUMBER

Important Notes WARNING: Failure to follow proper procedures for handling, storage and installation could result in unsatisfactory performance, unsafe

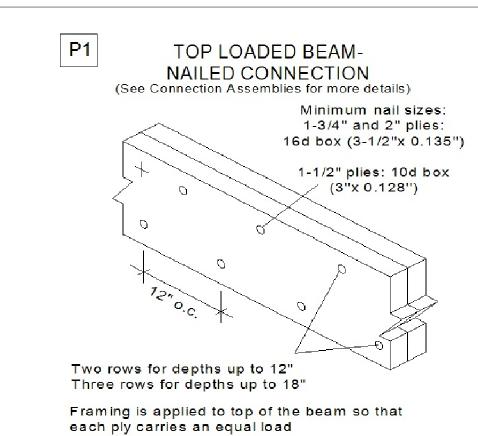
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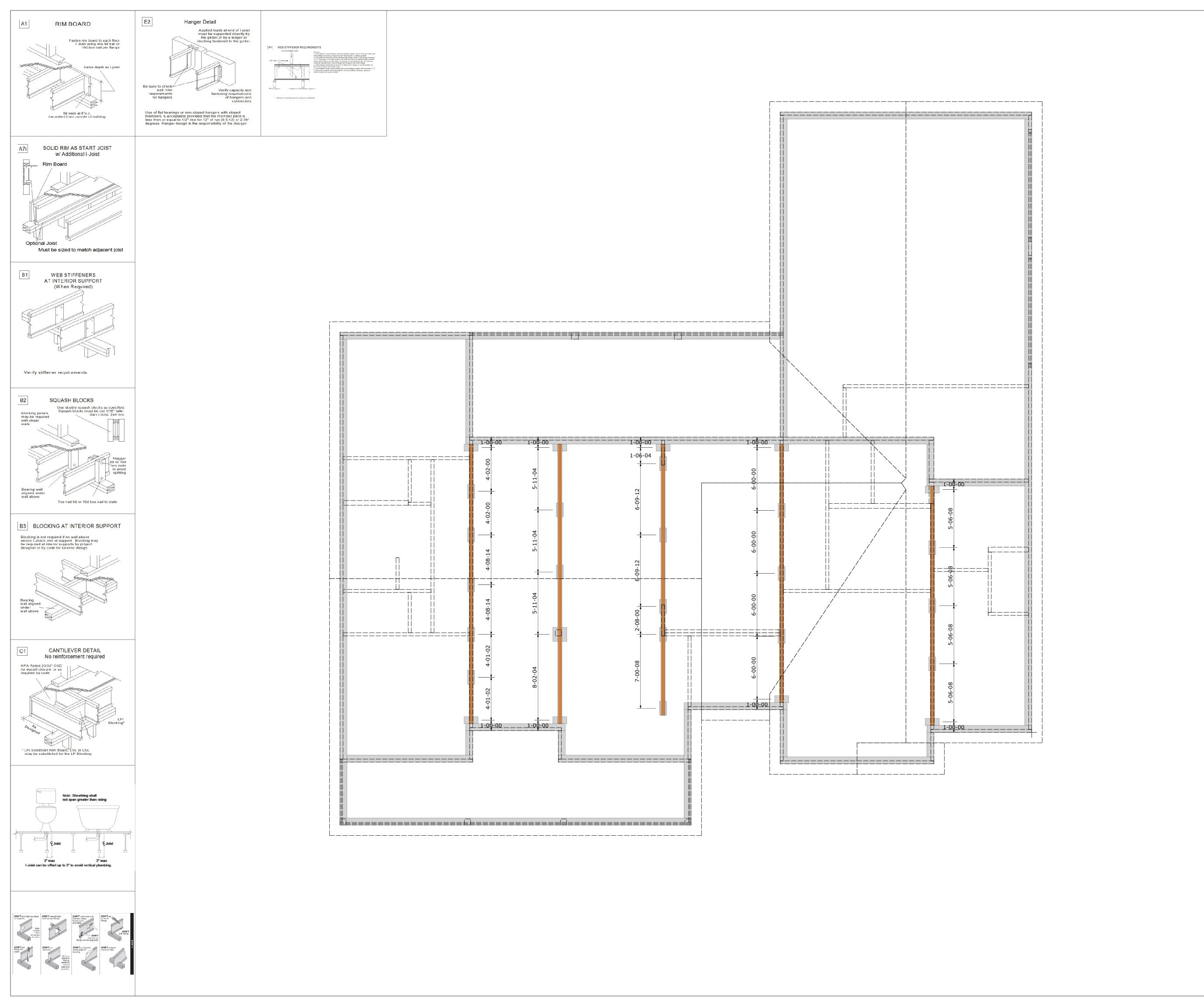
EDDIE BAUER

Scale:1/4" = 1'

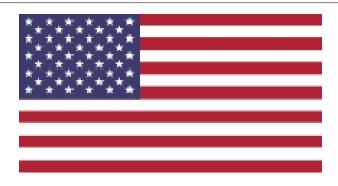
Date:10/13/20

2ND FLOOR









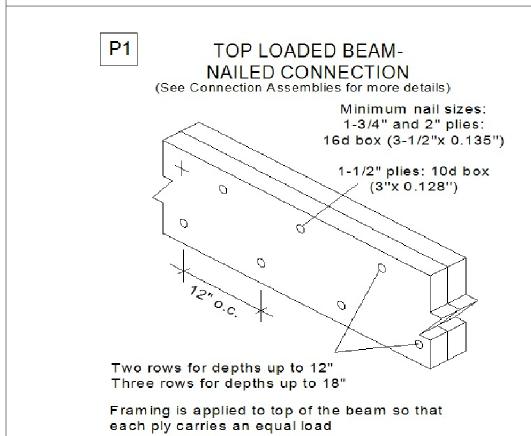
U.S. LUMBER

Important Notes | WARNING: Failure to follow proper procedures for handling, storage and installation could result in unsatisfactory performance, unsafe

These instructions are offered as a guide to good practice in the handling, storage and installation of LP® SolidStart® I-Joists, LP SolidStart LVL & LP SolidStart LSL beams. They are, however, solely general recommendations and, in some instances, other or additional precautions may be desirable. In all cases, the procedures used should be as specified by the architect/engineer responsible for the entire building.

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Customer Name:

BRAD CUMMINGS

Job Name: BURGESS/SPRING HILL CHURCH RD.

> Designer: **Tony Huneycutt**

> > Salesman:

EDDIE BAUER

Scale:1/4" = 1'

Date:10/13/20

1ST FLOOR



FIRST FLOOR FRAMING

1642	LF	12IJ	11-7/8" LF	PI 20Plus JO	DISTS	17/30'	15/24	' 2/22	8/16'	3	4926
						33/14'	2/12'	5/10'	+ 64' BLKG		
20	PCS	12RIM12	1-1/8" x	-1/8" x 11-7/8" x 12' RIM BOARD						43.3	866

5792

BONUS ROOM EWP

528	LF	16IJHD	16" LPI 42Plus JOISTS	22/24'		4.5	2376
7	PCS	16RIM12	1-1/8" x 16" x 12' RIM BO	DARD		58.8	411.6
120	LF	9LVL	1-3/4" x 9-1/4" LVL	2/30' 2/20' 2	2/10'	4.2	504
116	LF	14LVL	1-3/4" x 14" LVL	2/20' 2/14' 4	/12'	6.4	742.4

4034

Client: **Brad Cummings**

Project: Address:

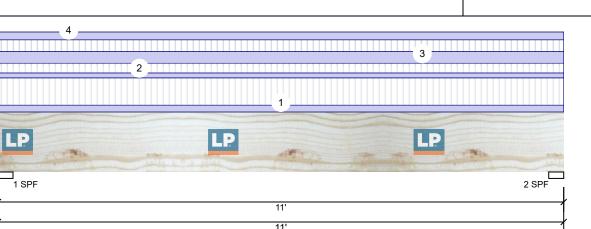
Date: 10/13/2020

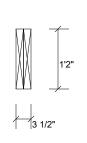
Input by:

Job Name: Spring Hill Church Rd

Project #:

LP-LVL 2900Fb-2.0E 1.750" X 14.000" 2-Ply - PASSED Level: Level 10' Garage Door Header





Page 1 of 1

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 600 Importance: Normal

Application: Design Method:

Building Code:

Floor ASD **IBC/IRC 2015**

Load Sharing: No Deck:

Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	2750	1837	0	0	0
2	2750	1837	0	0	0

Bearings

Bearing Lengt	th Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF 3.500	" 88%	1837 / 2750	4587	L	D+L	
2 - SPF 3.500	" 88%	1837 / 2750	4587	L	D+L	

Analysis Results

Temperature:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	11585 ft-lb	5'6"	27029 ft-lb	0.429 (43%)	D+L	L
Shear	3423 lb	1'4 3/4"	9310 lb	0.368 (37%)	D+L	L
LL Defl inch	0.103 (L/1227)	5'6"	0.264 (L/480)	0.390 (39%)	L	L
TL Defl inch	0.172 (L/735)	5'6"	0.211 (L/600)	0.820 (82%)	D+L	L

Design Notes

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.069", Long Term = 0.103"

Temp <= 100°F

- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be continuously braced.
- 7 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	70 PLF	280 PLF	0 PLF	0 PLF	0 PLF	Floor Load
2	Uniform			Тор	50 PLF	100 PLF	0 PLF	0 PLF	0 PLF	Attic Load
3	Uniform			Тор	120 PLF	120 PLF	0 PLF	0 PLF	0 PLF	Roof Load
4	Uniform			Тор	80 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Brick Load
	Self Weight				14 PLF					

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Manufacturer Info

Louisiana-Pacific Corp 414 Union Street, Suite 2000 Nashville, TN 37219 (888) 820-0325 www.lpcorp.com APA: PR-L280, ICC-ES: ESR-2403, LADBS: RR-25783, Florida: FL15228 **BMC/Locust Lumber Company** 312 E. Main Street, North Carolina 704-888-4411



Client: **Brad Cummings**

Project: Address:

Date: 10/13/2020

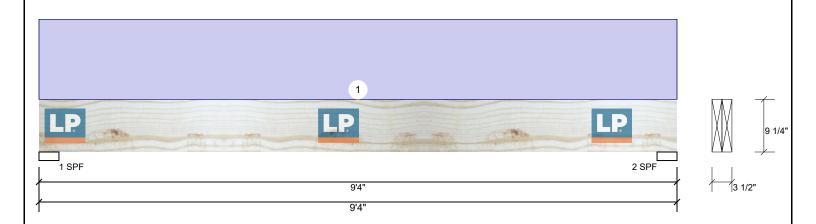
Input by:

Job Name: Spring Hill Church Rd

Project #:

LP-LVL 2900Fb-2.0E 1.750" X 9.250" Dining Room Header

2-Ply - PASSED Level: Level



Member Inforn	nation		Reaction	Reactions UNPATTERNED lb (Uplift)						
Туре:	Girder	Application:	Floor	Brg	Live	Dead	Snow	V	Vind	Const
Plies:	2	Design Method:	ASD	1	0	627	0		0	0
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015	2	0	627	0		0	0
Deflection LL:	480	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal									
Temperature:	Temp <= 100°F									
				Bearin	gs					
				Bearin	g Length	Cap. Re	eact D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPI	3.500"	12%	627 / 0	627	Uniform	D
				2 - SPI	3 500"	12%	627 / 0	627	Uniform	D

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1322 ft-lb	4'8"	11174 ft-lb	0.118 (12%)	D	Uniform
Shear	492 lb	1'	5536 lb	0.089 (9%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.045 (L/2351)	4'8"	0.444 (L/240)	0.100 (10%)	D	Uniform

Design Notes

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.045", Long Term = 0.068"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Ceiling Joists	
	Self Weight				9 PLF						

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Manufacturer Info

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Page 1 of 1





Client: **Brad Cummings**

Project: Address:

Date: 10/13/2020

Input by:

Job Name: Spring Hill Church Rd

Page 1 of 1

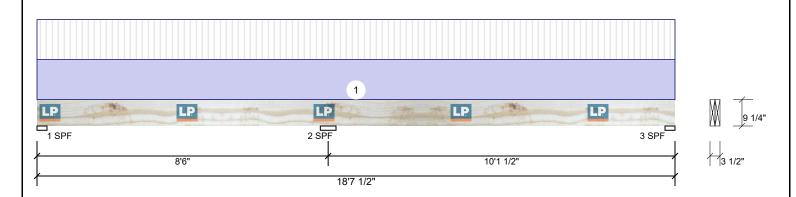
Project #:

Header @ Foyer LP-LVL 2900Fb-2.0E

1.750" X 9.250"

2-Ply - PASSED

Level: Level



Member Inform	nation			Reaction	s UNPAT	TERNE	D lb (Uplift)	ı	
Type:	Girder	Application:	Floor	Brg	Live	Dead	I Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	313	342	2 0	0	0
Moisture Condition	: Dry	Building Code:	IBC/IRC 2015	2	1135	1240	0	0	0
Deflection LL:	480	Load Sharing:	No	3	415	453	3 0	0	0
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal								
Temperature:	Temp <= 100°F								
				Bearings	5				
				Bearing	Length	Cap.	React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	3.500"	14%	337 / 389	727 L_	D+L
						29%	1249 / 1143	2391 LL	D+L
Analysis Result	alysis Results					17%	449 / 450	899 _L	D+L

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-2210 ft-lb	8'6"	12416 ft-lb	0.178 (18%)	D+L	LL
Pos Moment	1732 ft-lb	14'3 15/16"	12416 ft-lb	0.140 (14%)	D+L	_L
Shear	1097 lb	9'3 1/4"	6151 lb	0.178 (18%)	D+L	LL
LL Defl inch	0.036 (L/3257)	13'8 9/16"	0.247 (L/480)	0.150 (15%)	L	_L
TL Defl inch	0.067 (L/1769)	13'9 15/16"	0.495 (L/240)	0.140 (14%)	D+L	_L

Design Notes

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.031", Long Term = 0.046"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	100 PLF	100 PLF	0 PLF	0 PLF	0 PLF	Roof/Ceiling load from Great Room
	Self Weight				9 PLF					

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Manufacturer Info

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Client: Brad Cummings

Project: Address:

ent. Brad Cumming

Date: 10/13/2020

Input by:

Job Name: Spring Hill Church Rd

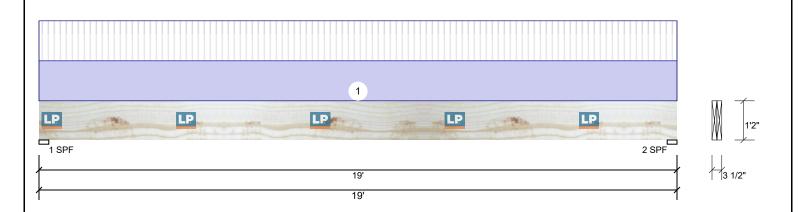
Page 1 of 1

Project #:

Ridge Beam in Great Room LP-LVL 2900Fb-2.0E

LVL 2900Fb-2.0E 1.750" X 14.000"

2-Ply - PASSED Level: Level



Member Info	rmation	Reactio	Reactions UNPATTERNED lb (Uplift)							
Type:	Girder	Application:	Roof	Brg	Live	Dead	Snow	W	/ind	Const
Plies:	2	Slope:	0/12	1	1805	1938	0		0	0
Moisture Condition	on: Dry	Design Method:	ASD	2	1805	1938	0		0	0
Deflection LL:	480	Building Code:	IBC/IRC 2015							
Deflection TL:	240	Load Sharing:	No							
Importance:	Normal	Deck:	Not Checked							
Temperature:	Temp <= 100°F									
				Bearing	S					
				Bearing	Length	Cap. Read	t D/L lb	Total I	Ld. Case	Ld. Comb.
				1 - SPF	3.500"	72% 193	8 / 1805	3743 l	L	D+L
				2 - SPF	3.500"	72% 193	8 / 1805	3743 I	L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	16933 ft-lb	9'6"	27029 ft-lb	0.626 (63%)	D+L	L
Shear	3193 lb	1'4 3/4"	9310 lb	0.343 (34%)	D+L	L
LL Defl inch	0.335 (L/664)	9'6 1/16"	0.464 (L/480)	0.720 (72%)	L	L
TL Defl inch	0.694 (L/320)	9'6 1/16"	0.927 (L/240)	0.750 (75%)	D+L	L

Design Notes

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.360", Long Term = 0.539"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 7'2 5/8" o.c.
- 7 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	190 PLF	190 PLF	0 PLF	0 PLF	0 PLF	Roof Load
	Self Weight				14 PLF					

Notes

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Client: **Brad Cummings**

Project: Address:

Date: 10/13/2020

Input by:

Job Name: Spring Hill Church Rd

Level: Level

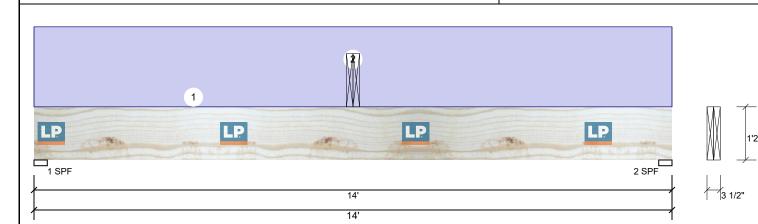
Project #:

Hdr. @ kit/Great Room

LP-LVL 2900Fb-2.0E

1.750" X 14.000"

2-Ply - PASSED



Member Information Reactions UNPATTERNED Ib (Uplift) Application: Brg Snow Wind Const Type: Floor Live Dead Plies: 2 Design Method: ASD 903 1767 0 0 0 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 903 1767 0 0 0 Deflection LL: 480 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Normal Temp <= 100°F Temperature: **Bearings** Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.500" 1767 / 902 2670 L D+L 2 - SPF 3.500" 51% 1767 / 903 2670 L D+L

Analysis Results

I	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	15286 ft-lb	7'	27029 ft-lb	0.566 (57%)	D+L	L
l	Shear	2511 lb	12'7 1/4"	9310 lb	0.270 (27%)	D+L	L
l	LL Defl inch	0.115 (L/1411)	7' 1/16"	0.339 (L/480)	0.340 (34%)	L	L
l	TL Defl inch	0.299 (L/544)	7' 1/16"	0.677 (L/240)	0.440 (44%)	D+L	L

Design Notes

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.184", Long Term = 0.276"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be continuously braced.

/ Bottom brace	ed at bearings.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	100 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Ceiling joists over Kitchen
2	Point	7-0-0		Тор	1938 lb	1805 lb	0 lb	0 lb	0 lb	Ridge Beam in Great Room Brg 2
	Bearing Length	0-3-8								
	Self Weight				14 PLF					

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Client: Project: Address:

Brad Cummings

Date:

10/13/2020

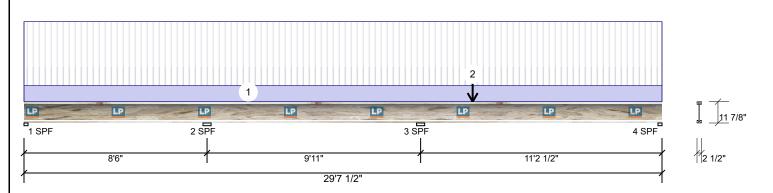
Input by:

Job Name: Spring Hill Church Rd

Project #:

Joists under Pantry-DR wall LPI 20Plus 11.875" - PASSED

Level: Level



Member Info	rmation			Reaction	ns UNPAT	TERNED	lb (Uplift)		
Type:	Joist	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const
Spacing:	19.2" o.c.	Design Method:	ASD	1	220	57	0	0	0
Moisture Condition	on: Dry	Building Code:	IBC/IRC 2015	2	614	142	0	0	0
Deflection LL:	480	Load Sharing:	No	3	765	287	0	0	0
Deflection TL:	240	Deck:	Not Checked	4	296	88	0	0	0
Importance:	Normal			'			_	-	-
Temperature:	Temp <= 100°F								
				Bearing	S				
				Bearing	Length	Cap. Re	eact D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	2.250"	31%	58 / 261	319 L_L	D+L
				2 - SPF	4.500"	34%	137 / 700	837 LL	D+L

Analysis F	Results
------------	---------

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-1133 ft-lb	18'5"	3755 ft-lb	0.302 (30%)	D+L	_LL
Pos Moment	974 ft-lb	24'6 3/4"	3755 ft-lb	0.260 (26%)	D+L	L_L
Shear	624 lb	18'5"	1485 lb	0.420 (42%)	D+L	_LL
LL Defl inch	0.068 (L/1967)	24'2 1/16"	0.277 (L/480)	0.240 (24%)	L	L_L
TL Defl inch	0.090 (L/1485)	24'1 1/16"	0.554 (L/240)	0.160 (16%)	D+L	L_L

Design Notes

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.022", Long Term = 0.033"
- 3 Top flange must be laterally braced at a maximum of 9'3" o.c.
- 4 Bottom flange must be laterally braced at a maximum of 8'7" o.c

4 Bottom hange must be laterally braced at a maximum of 67 6.c.									
ID	Load Type Lo	cation	Trib Width	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform		1-7-3	10 PSF	40 PSF	0 PSF	0 PSF	0 PSF	Floor Load
2	Point 2	20-10-0		100 lb	0 lb	0 lb	0 lb	0 lb	Bearing Wall Above
	Bearing Length	0-3-0							

3 - SPF 4.500"

4 - SPF 2.250"

45%

39%

292 / 813

86 / 319

1105 _LL

405 L_L

D+L

D+L

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Page 1 of 1





Client: **Brad Cummings**

Project: Address:

Date: 10/13/2020

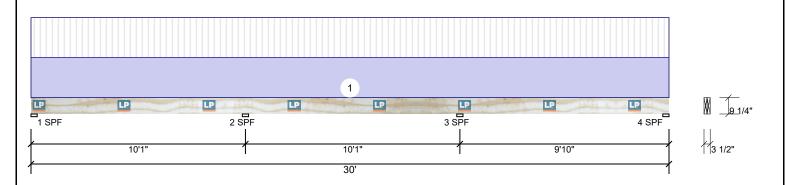
Input by:

Job Name: Spring Hill Church Rd

Project #:

LP-LVL 2900Fb-2.0E 1.750" X 9.250" 2-Ply - PASSED Rear Porch Header

Level: Level



Member Information					Reactions UNPATTERNED Ib (Uplift)						
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const		
Plies:	2	Design Method:	ASD	1	498	537	0	0	0		
Moisture Condition	: Dry	Building Code:	IBC/IRC 2015	2	1322	1424	0	0	0		
Deflection LL:	480	Load Sharing:	No	3	1295	1394	0	0	0		
Deflection TL: 240 Deck:	Deck:	Not Checked	4	485	523	0	0	0			
Importance:	Normal			'			•	-	-		
Temperature:	Temp <= 100°F										
				Bearing	S						
				Bearing	Length	Cap. Rea	ct D/L lb	Total Ld. Case	Ld. Comb.		
				1 - SPF	3.500"	21% 5	535 / 559	1094 L_L	D+L		
				2 - SPF	3.500"	55% 142	25 / 1431	2857 LL_	D+L		
Analysis Results				3 - SPF	3.500"	54% 139	96 / 1412	2808 _LL	D+L		
Analysis Actual Location Allowed Capacity Comb. Case					3.500"	21% 5	522 / 549	1071 L L	D+L		

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-2677 ft-lb	10'1"	12416 ft-lb	0.216 (22%)	D+L	LL_
Pos Moment	2158 ft-lb	4'4 11/16"	12416 ft-lb	0.174 (17%)	D+L	L_L
Shear	1308 lb	9'3 3/4"	6151 lb	0.213 (21%)	D+L	LL_
LL Defl inch	0.047 (L/2503)	4'11 11/16"	0.246 (L/480)	0.190 (19%)	L	L_L
TL Defl inch	0.084 (L/1415)	4'10"	0.493 (L/240)	0.170 (17%)	D+L	L_L

Design Notes

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.036", Long Term = 0.055"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.

	3										
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	120 PLF	120 PLF	0 PLF	0 PLF	0 PLF	Roof/Ceiling Load	
	Self Weight				9 PLF						

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