

Project Customer

Roof Load(psf):

SOUTH SCAN

20-10-0-10

Re: 201329RT1 Site Information

DrJ Engineering, LLC (FKA: DrJ Consulting, LLC) 6300 Enterprise Lane Madison, Wisconsin 53719

Paragon ID: 29955

			-			
	Lot/Block: Address:	48 44 WHITE HERON CT	S M	ubdivision: odel:	CAROLINA 2L-2873	LAKES
	City:	SANFORD	S	tate:	North Caro	lina
Nam	e Address and Licens	e # of the Building Designer,	if there is one, for th	e Building:		
	Name: -		License: -		State:	North Carolina
	Address: -		City: -			
Ger	eral Truss Engineerin	g Criteria & Design Loads (R	Refer to Individual Tru	ıss Design [rawing(s) ("٦	[DD[s]") for Special Loading Conditions):
	Design Code:	IRC 2015	Design Program:	20/20		
	Wind Code:	-	Exposure Catego	ory: -		
	Wind Speed(mph):	-	Floor Load:	-		
	Mean Roof Height(f	t): -				

The TDD(s) (also at times referred to as "Delegated Engineering Documents") referenced have been prepared based on the Construction Documents, specifications and written requirements prepared by the Building Designer indicating the nature and character of the Building, its applied load, its load paths and the structural resistance design requirements. The design criteria from the Construction Documents, specifications and written requirements have been transferred to DrJ Engineering, LLC (DrJ) byCarolina Structural Systems

Project Name: 201329RT1

The TDDs herein are specialty structural component designs and may be part of the Building's deferred or phased submittal documents. As a Truss Design Engineer (also referred to in some Jurisdictions as a "Delegated or Specialty Engineer"), the seal on this Cover Sheet and on any TDD represents an acceptance of professional engineering responsibility for the design of the single Truss only as depicted on the TDD pursuant to ANSI/TPI 1, the National Design Standard for Metal Plate Connected Wood Truss Construction ("TPI 1"). The Building Designer is responsible for and shall coordinate and review each of the TDDs and all notes for compatibility with the Construction Documents, specifications, written requirements, design loads and load path. Capitalized terms are as defined in TPI 1.

My license renewal date for North Carolina	is 12/31/2021
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COL# P-1038	

DrJ Consulting, PLLC

P-1038

Important Notice: Each TDD uses Metal Connector Plate (MCP) design values published by MCP manufacturers and lumber industry published design values (mechanically or visually graded as indicated) and their associated Specific Gravity (SG) values. These are incorporated into lumber design provisions and equations created by the American Wood Council (AWC) per the National Design Specification® (NDS®) for Wood Construction and input into modeling and analysis software that uses TPI 1 provisions. The lumber design values (and SG values as needed for connections) correspond to the lumber size and grade as defined on the TDD incorporating the design values (and SG values. The published lumber design values and associated SG values (a) are calculated and administered by the lumber rules writing agencies using a property range or bending correlation and are not tension proof tested, (b) are approved by the American Lumber Standards Committee (ALSC) as published design data that are representative of the strength and stiffness of specific grades and species/species groups of lumber, and (c) are further known by ALSC and the lumber rules writing agencies which do not have precise design values.

Neither the Truss Manufacturer nor the Truss Design Engineer can therefore verify or warrant that published lumber design values will exist within the lumber utilized in the Truss when manufactured and delivered.

Ryan Dexter

4/14/2021

The design assumptions, loading conditions, suitability and use of this set of TDDs for any Building shall be verified by both the Contractor and Building Designer. The approval of the Truss Submittal Package, including the TDDs and Truss Placement Diagrams ("TPDs") is the responsibility of the Building Designer and Contractor. Upon transmittal of the Truss Submittal Package and upon delivery of the Trusses to the Contractor, the Contractor shall read all notes and instructions in the TDDs and TPDs and review the practices and guidelines of Building Component Safety Information ("BCSI") and/or its summary sheets as published by the Truss Plate Institute and the Structural Building Component Association. The Truss Design Engineer is NOT the Building Designer, Truss System Engineer, or Structural Engineer of Record for any Building. Any field of use of the Truss, including applied loads, load paths, structural resistance requirements, handling, storage, installation and bracing, is the responsibility of the Building Designer and Contractor.

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1 of 1

	A03	PIGGYBACK BASE			5 1	lob Rot	oranca (antianal)	Paragon ID: 29955
					8.	330 s Dec 5 2019 MIT	ek Industries, Inc. Wed Apr 14 16; 32-0-0	31:42 2021 Page 1 P2996488
	-0-10-8 0-10-8	6-5-1 6-5-1	12-6-9 6-1-7	17-6-0 19-5 4-11-7 1-11	-7 24-0-12 -7 4-7-4	<u>28-8-0</u> 4-7-4	29-8-0 32-10-8 1-0-0 0-10-8 2-4-0	
REPAIR: 1) BRI	ЕАК ІН ВОТТОМ СН	IORD BETWEE	N JOINTS 15 8	k 16				
NO I DEQUAC	E - THIS REPAIR IS	VALID FOR T	HE DESIGN CO S MUST BE VE	RIFIED BY O	ROVIDED	IN THIS I	RUSS REPAIR I	DRAWING. IT'S
REF	ER TO ORIGINAL TI RUSS IS IN PLACE	RUSS DESIGN	DRAWING FC	R ADDITION	AL NOTES	BE SUPP	ORTING BEFOR	RE BEGINNING REPAIR.
	ESS OTHERWISE S	SPECIFIED, RE	MOVE ALL EL	ECTRICAL, M		AL, PLUM	BING, ETC. RUN	IS INTERFERING WITH
	AIR MATERIALS AND	JRE-ROUTE.	DO NOT COT,	DRILL, NOT	UR IVIC			5.
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		<u>¥ </u>	9 18 1	²⁹ ¹⁶	30 5v9 4	31 6x1	14 ¹⁴ ¹⁴	
	3x4	1 // 2x4	$3x4 \equiv 4x5$	 	12.00 12 6x8 12	5×	3 $3x6 =$	
A) AP	PLY 7/16" 24/16 SPA		B GUSSETS TO) FACH SIDE	OF TRUS	S AS SHO	WN ATTACH F	ACH GUSSET WITH (2)
ROWS OF	F 10d (3" X 0.131") N	IAILS: SPACEE	@ 4" OC INT	O ALL MEMB	ERS. DRI	/E NAILS	FHROUGH BOT	H GUSSETS AND
	STACCED SDACING			CK SIDE EOI	7 A NET 2			
LINCH.	STAGGER SPACIN		12-6-9	CK SIDE FO	R A NET 2'	OC SPAC	31-11-8 29-8-0, 32-0-0	USS MEMBER.
Offsets (X,Y)-	STAGGER SPACING	G FROM FROM 6-5-1 6-4-9	12-6-9 6-1-7	17-6-0 18-6-0 4-11-7 140-0	R A NET 2' + 24-0-12 5-6-12	28-8-0 4-7-4	31-11-8 29-8-0 1-0-0 2-3-8 2-3-8	USS MEMBER.
Offsets (X,Y)-	STAGGER SPACING 0-0-8 0-0-0-8 0-0-0-8 0-0-00000000	G FROM FROM	12-6-9 6-1-7 CSI. TC 0.87 BC 0.86	CK SIDE FO	24-0-12 5-6-12 In (loc) -0.47 14-15 -0.86 14-15	VC SPAC 28-8-0 4-7-4 Videfl L/d >822 240 >445 180	31-11-8 29-8-0 10-0 2-3-8 2-3-8 PLATES MT20	USS MEMBER.
Offsets (X,Y)-	STAGGER SPACING 0-0-6 0-0-8 0-0-9 0-0-0-0-0 0-0-8 0-0-	G FROM FROM 6-5-1 6-4-9 2-0-0 1.06 YES PI2014	12-6-9 6-1-7 CSI. TC 0.87 BC 0.86 WB 0.64 Matrix-MS	CK SIDE FO	RANET 2' 24-0-12 5-6-12 in (loc) -0.47 14-15 -0.86 14-15 0.18 11	Vdefl L/d >822 240 >425 180 n/a n/a	CING IN THE TR 31-11-8 29-8-0 10-0 	
Confisets (X,Y)- Confisets (X,Y)- ING (psf) 20.0 10.0 0.0 * 10.0	STAGGER SPACING 0-0-6 0-0-8 0-0-8 Plate Grip DOL Lumber DOL Lumber DOL SPACING- Plate Grip DOL Lumber DOL Stress Incr Code IRC2018/TF	C FROM FROM 6-5-1 6-4-9 2-0-0 1.05 YES PIZ014	12-6-9 6-1-7 CSI. TC 0.87 TC 0.86 WB 0.64 WB 0.64 Matrix-MS	DEFL. Vert(L) Vert(L) Vert(L) Vert(CT) Horz(CT) BRACING TOP CHC BOT CHC	R A NET 2' 24-0-12 5-6-12 in (loc) -0.47 14-15 -0.86 14-15 -0.86 14-15 11 -0.78 11 -0.78 11 -0.79 C put -0.79 C	28-8-0 4-7.4 4-7.4 Videfi Ld >822 240 >445 180 n/a n/a od sheathing directly a in(a) inequarks of the or 10-4 in(a)	PLATES 29:6:0 31-11-8 29:6:0 32-0:0 10:0 0-08 2-3-8 PLATES wrading vrading pplled or 3-0-12 oc purlims, except vrading	USS MEMBER.
Contraction Contra	STAGGER SPACING 0-0-6 0-0-5 0-0-5 0-0-5 0-0-5 0-0-5 Plate Grip DOL Lumber DOL DOL DOL DOL DOL DOL DOL DOL DOL DOL	G FROM FROM 6-5-1 6-4-9 2-0-0 1.15 YES PIZ014 333/0-3-8 (mln, 0-1-12)	12-6-9 6-1-7 CSI. TC 0.67 TC 0.66 WB 0.64 WB 0.64 Matrix-MS	CK SIDE FOI	In (loc) -0.47 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.7 14-15 -0.86 14-15 -0.7 14-15 -0.86 11 3-0 Structural wc 2-0-0 oc puri 1 NRD Rigid ceiling 1 Row at mic 1	28-8-0 4.7-4 4.7-4 Videfl Vide	Single in the transmission 31-11-8 29-8-0 32-000 110-0 0-048 2-3-8 Welcher welcher Welcher upplied or 3-0-12 oc purime, except 0-00 scharting, 3-17, 5-16, 6-16	USS MEMBER.
Content of the second s	STAGGER SPACING 0-0-6 0-0-5 0-0-5 Plate Grip DOL Lumber DOL Lumber DOL Rep Stress Incr Code IRC2018/TF pt* B4: 2x4 SP DSS e) 2=1333/0-3-8 (min. 0-1-12), 11=13 lorz 2=231(LC 11) plft 2=-612(LC 12), 11=67(LC 12) rav 2=1491(LC 17), 11=1508(LC 18)	G FROM FROM 6-5-1 6-4-9 2-0-0 1.00 1.15 YES PIZ014 333/0-3-8 (mln, 0-1-12)	12-6-9	DEFL. Vert(L1) Vert(L1) Vert(L1) Vert(CT) HorzCH BRACING TOP CH4 BOT CH4 WEBS	RANET 2' 24-0-12 5-6-12 0.17 14-15 -0.86 14-15 0.18 11 9RD Structural wc 2-0-0 oc puri 9RD Structural wc 1 Row at mic	OC SPAC 28-8-0 4.7-4 4.7-4 Videfl Vid	Single in the transmission 31-11-8 29-8-0 32-000 110-0 0-0-8 2-3-8 Welche welche 0-0-12 oc puritive, except -00 oc bracing, 3-17, 5-16, 6-16	USS MEMBER.
Contraction (X,Y)	STAGGER SPACING 0-0-6 0-0-6 0-0-6 0-0-6 Plate Grip DOL Lumber DOL Lumber DOL Rep Stress Incr Code IRC2018/TF pt* B4: 2x4 SP DSS e) 2=1333/0-3-8 (min. 0-1-12), 11=13 lorz 2=23(LC 12), 11=57(LC 12) rav 2=1491(LC 17), 11=1508(LC 18) CompJMax, Ten All forces 250 (lb) or less s =203107 10553 12, 14=0/1553 12, 14=0/1563 12, 24-5 =203107	G FROM FROM	12-6-9 6-1-7 CSL TC 0.87 BC 0.66 WB 0.64 WB 0.64 Matrix-MS	CK SIDE FO	RANET 2' 24-0-12 5-8-12 In (loc) -0.47 14-15 -0.86 14-15 -0.86 14-15 11 SPD Structural wc 2-0-0 cc puri NRD Structural wc 2-0-0 cc puri NRD Rigld celling 1 Row at mic	28-8-0 4.7-4 4.7-4 282 240 >445 180 n/a n/a n/a n/a 4.7-4	Single in the transmission 31-11-8 29-8-0 32-090 10-0 0-08 2-3-8 PLATES MT20 Values Poor bracing. 3-17, 5-16, 6-16 Values Values	USS MEMBER.
COTSets (X,Y) OTSets (X,Y) ING (psf) 20.0 10.0 0.0 10.0 10.0 ER- 44 SP No.2 X4 SP No.2 X4 SP No.2 X4 SP No.2 CTIONS. (lb/slz Max H Max G SES. (lb)-Max CHORD 2-269- S- CTIONS 2-3-19= S- CTIONS 2-50	STAGGER SPACING 0-0-8 0-0-1-12), 11=13 0-7 0-0-12, 11=13 0-7 0-0-12, 11=13 0-7 0-0-12, 11=13 0-7 0-0-12, 11=13 0-7 0-0-12, 11=13 0-7 0-0-12, 11=13 0-7 0-0-12, 11=15 0-7 0-0-12, 11=15 0-7 0-0-12, 11=15 0-7 0-0-12, 11=15 0-7 0-0-12, 11=15 0-7 0-0-12, 11=15 0-7 0-0-12, 11=15 0-12,	G FROM FROM 6-5-1 6-4-9 2-0-0 1.00 1.15 YES PIZ014 33300-3-8 (mln. 0-1-12) except when shown. =-1375/165, 5-271084/167, 22 =0/1158, 15-16=0 =0/1158, 15-16=0 =0/1158, 15-16=0 =0/1158, 15-16=0 =0/1158, 15-16=0 =0/1158, 15-29=0/1158, 15-16=0	12-6-9 6-1-7 CSI. TC 0.87 BC 0.66 WB 0.64 WB 0.64 Matrix-MS	CK SIDE FO	In (loc) -0.47 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 0.18 11 SPD Structural wc 2-00 oc purt NRD 10 RID 11 S- 12 14.15 -0.86 14-15 10 SPD Structural wc 2-00 oc purt 1 Row at mic 12, 8-9=-3210/142, 9-2109, 11-13=0/1500	28-8-0 4-7-4 4-7-4	CING IN THE TR	USS MEMBER.
CLINCH. Offsets (X,Y)- Offsets (X,Y)- ING (psf) 200 10.0 10.0 10.0 10.0 10.0 10.0 10.0	STAGGER SPACING 0-0-8 0-0-120, 11=-3 0-0-8 0-0-8 0-0-8 0-0-8 0-0-120, 11=-3 0-0-9 0-0-8 0-0-9 0-0-8 0-0-9 0-0-8 0-0-9 0-0-8 0-0-9 0-0-9 0-0-9 0-0-120, 11=-3 0-0-0-120, 17-29 0-0-0-120, 17-29 0-0-0-120, 17-29 0-0-0-120, 17-29 0-0-0-120, 17-29 0-0-0-120, 17-29 0-0-0-0-120, 17-29 0-0-0-0-0-120, 17-29 0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-	G FROM FROM 	12-6-9 6-1-7 CSI. TC 0.87 BC 0.86 WB 0.64 WB 0.64 Matrix-MS	CK SIDE FOI	R A NET 2' 24-0-12 5-8-12 In (loc) -0.47 14-15 -0.86 14-15 0.18 11 	28-8-0 4-7-4 4-7-4 4-7-4 282 240 >445 180 n/a n/a n/a 445 180 n/a	CING IN THE TR	USS MEMBER.
CUINCH. Offsets (X,Y)- Offsets (X,Y)- UNG (psf) 200 10.0 10.0 10.0 10.0 URR 4 SP No.2 4 SP No.2 4 SP No.2 4 SP No.2 5 CTIONS. (Ib/slzz 4 SP No.2 5 CTIONS. (Ib/slzz 5	STAGGER SPACING 0-0-8 0-0-9 0-0-8 0-0-9 0-0-8 0-0-9 0-0-0-0-0 0-0-0-0 0-0-0-0 0-0-0-0 0-0-0-0 0-0 0-0-0 0-0-0 0-0	G FROM FROM - 6-5-1 - 6-4-9 - 2-0-0 1.00 1.15 YES PIZ014 	12-6-9 6-1-7 CSI. TC 0.87 BC 0.86 WB 0.64 WB 0.64 Matrix-MS	CK SIDE FO	R A NET 2' 24-0-12 1 24-0-12 0.16 0.17 11 3- RD Structural we 2-0-0 cc puri 12, 8-9=-3210/142, 9- 2109, 11-13=0/1500 XS (directional) and C-C 2-10-8 zone; DOL=1.60 chord and any other	28-8-0 4-7-4 4-7-4 4-7-4 282 240 >445 180 n/a n/a n/a 445 180 n/a n/a n/a n/a 180/29, 0=-3136/29, Exter/or(2E)	CING IN THE TR	USS MEMBER.
CLINCH. Offsets (X,Y)- Offsets (X,Y)- ING (0,sf) 200 10.0 10.0 10.0 10.0 10.0 10.0 10.0	STAGGER SPACING 0-0-8 0-0-9 0-0-8 0-0-9 0-0-0-0-0-0-0-0 0-0-0-0-0-0 0-0-0-0-	G FROM FROM	12-6-9 6-1-7 CSI. TC 0.87 BC 0.86 WB 0.64 WB 0.64 Matrix-MS -28=-1084/167, 6-28=-1084/167 /1989, 15-30=0/1501, 30-31=0/1 70, 6-18=-1286/0, 8-15=-5776/13 25ft: B=45ft: L=32ft: eave=4ft; Cr to 19-5-7, Exterfor(2R) 19-5-7 to 3 4 MWFRS for reactions shown: Ilve loads. rectangle 3-6-0 tall by 2-0-0 wido upflr tal joint 2 and 5-0 tall by 2-0-0 wido upflr tal joint 2 and zeferer op and/or bottom chord.	CK SIDE FOI 17-6-0 18-6-0 4-11-7 10-0 DEFL Vert(L1) Vert(CT) Horz(CT) Horz(CT) BRACING TOP CHC BOT CHL WEBS 30-7-1535/149, 7-81541/ 501, 14-31=0/1501, 13-14=0, 3, 8-14=-16/1524 31-16/1524 at. II: Exp B: Enclosed: MWFF 24-28, 0 i. Lumber DOL=1.60 plate grip te will fit between the bottom of tool tool tool tool tool tool tool t	R A NET 2' 24-0-12 1. (loc) -0.47 14-15 -0.86 14-15 -0.86 14-15 0.18 11 SPD Structural wc 2-0-0 cp put NRD Rigld celling 1 Row at mic 12, 8-9=-3210/142, 9- 2109, 11-13=0/1500 KS (directional) and C-C (2-10-8 zone; DOL=1.60 shord and any other	28-8-0 4-7-4 4-7-4 4-7-4 282 240 >445 180 n/a n/a n/a 4.7-4	CING IN THE TR	USS MEMBER.
CLINCH. Offsets (X,Y)- UNG (psf) 200 000 000 000 000 000 000 000 000 00	STAGGER SPACING 0-0-8 0-05 0-0-8 0-05 0-0-8 0-05 0-0-8 0-05 0-0-8 0-05 0-0-8 0-05 SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TF pt* B4: 2x4 SP DSS e) 2=1333/0-3-8 (min. 0-1-12), 11=13 0/72 2=231(C.11) pt/ B4: 2x4 SP DSS comp.Max. Ten All forces 250 (b) or less 1- 1927/63, 3-26=-1842/94, 3-4=-1481/122, 4-5 -203167 vil=125mpl, 3-26=-1842/94, 3-4=-1481/122, 4-5 -203167 vil=125mpl, 3-26=-01/563, 17-26=0/1563, 3-27e=-10/571, 6-15=0/1 te loads have been considered for this design vil=10, 20, 54 vil=125mpl, 13-63, 17-18=0/1563, 17-28=0/273, 3-17=6=47/124, 5-17=-9/571, 6-15=0/1 te loads have been considered for this design vil=10, 20, 54 vil=125mpl, 13-63, 17-18=0/1563, 17-28=0/273, 3-17=647/124, 5-17=-9/571, 6-15=0/1 te loads have been considered for this design vil=10, 20, 55 Landome end will the 2014 02005 of the solor on designed for a low pload of 20, 0ps on the top 1= 10, 0ps f. Less on the load of 20, 0ps on the top 1= 10, 0ps f. Less ont depict the size or the or ndard	G FROM FROM - 6-5-1 - 6-4-9 - 2-0-0 1.00 1.15 YES PI2014 	12-6-9 6-1-7 TC 0.87 BC 0.86 WB 0.64 WB 0.64 Matrix-MS 0.86 V28=1084/167, 6-28=1084/167 0.93 V1989, 15-30=0/1501, 30-31=0/1 7/0, 6-18=-1288/0, 8-15=576/13 25ft: B=45ft: L=52ft: eave=4ft: Ci 19-5-7, 5tacfor(2F) 19-5-7 to V1998, 15-30=0.151, 10, 30-31=0/1 19-5-7, 5tacfor(2F) 19-5-7, to 110 MSR Stor reactions shown; 19 Well Rodts. 111 earl R052, 10.2, and reference op and/or bottom chord. 111 and R052, 10.2, and reference op and/or bottom chord.	CK SIDE FO	RANET 2' 24-0-12 12 5-8-12 0.16 0.17 14-15 0.18 11 SP SRD Structural we 2-00 oc put 12, 8-9=-3210/142, 9- 2109, 11-13=0/1500 XS (directional) and C-C 2-10-8 zone; DOL=1.60 chord and any other	28-8-0 4-7-4 4-7-4 28-8-0 4-7-4 28-8-2 240 ×422 240 ×42 180 r/a 100 r/a 101 r/a 102 steathing directly a 0d sheathing directly a 0frectly applied or 10-0 pt 0=-3136/29, Exterior(2E)	CING IN THE TR	USS MEMBER.
CLINCH. Offsets (X,Y)- DING (psf) 20.0 10.0 20.0 20.0 20.0 20.0 20.0 20.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code Plate Grip DOL Lumber DOL Rep Stress Incr Code Image: Space Stress Incr CompJMax, Ten All forces 250 (ID) or less Incr 2023167 Image: Space Stress Incr CompJMax, Ten All Stress 250 (ID) or less Incr 2023163, 3:26=-1842194, 3:4=-1481/122, 4:5=-203167 Image: Space Stress Incr 2023163 Image: Space Stress Incr 2023167 Image: Space Incr 2023167 <t< td=""><td>G FROM FROM</td><td>CSI. TC 0.87 TC 0.87 TC 0.87 TC 0.86 WB 0.64 WB 0.64 Matrix-MS Matrix-MS 7-28=1084/167, 6-28=1084/167 15.05, 30.31=0/1 TC 0.97 7/1589, 15-30=0/1501, 30-31=0/1 70, 6-18=1268/0, 8-15=576/13 TC 10.95-7, 10.30-31=0/1 2561; B=45ft; L=52ft; auve=4ft; CL 10.19-5-7, Extendor(2R) 19-5-7 to 8. MWFRS for reactions shown; Ilve loads. 11/1 and R802,10.2 and referer op and/or bottom chord. 11.1 and R802,10.2 and referer op and/or bottom chord. 11.1 and R802,10.2 and referer op and/or bottom chord.</td><td>CK SIDE FO</td><td>R A NET 2' 24-0-12 5-6-12 0.10 -0.47 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.70 or puti PRD RIgid celling 1 Row at mic 12, 8-9=-3210/142, 9- 2109, 11-13=0/1500 VS (directional) and C-C 210-8 zone; DOL=1.60 chord and any other</td><td>28-8-0 4-7-4 4-7-4 1/defl 2822 240 >445 180 r/a r/a</td><td>CING IN THE TR</td><td>USS MEMBER.</td></t<>	G FROM FROM	CSI. TC 0.87 TC 0.87 TC 0.87 TC 0.86 WB 0.64 WB 0.64 Matrix-MS Matrix-MS 7-28=1084/167, 6-28=1084/167 15.05, 30.31=0/1 TC 0.97 7/1589, 15-30=0/1501, 30-31=0/1 70, 6-18=1268/0, 8-15=576/13 TC 10.95-7, 10.30-31=0/1 2561; B=45ft; L=52ft; auve=4ft; CL 10.19-5-7, Extendor(2R) 19-5-7 to 8. MWFRS for reactions shown; Ilve loads. 11/1 and R802,10.2 and referer op and/or bottom chord. 11.1 and R802,10.2 and referer op and/or bottom chord. 11.1 and R802,10.2 and referer op and/or bottom chord.	CK SIDE FO	R A NET 2' 24-0-12 5-6-12 0.10 -0.47 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.70 or puti PRD RIgid celling 1 Row at mic 12, 8-9=-3210/142, 9- 2109, 11-13=0/1500 VS (directional) and C-C 210-8 zone; DOL=1.60 chord and any other	28-8-0 4-7-4 4-7-4 1/defl 2822 240 >445 180 r/a	CING IN THE TR	USS MEMBER.
CLINCH. Offsets (X,Y)- UNG (psf) 20.0 10.0 20.0 20.0 20.0 20.0 20.0 20.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code Plate Grip DOL Lumber DOL Rep Stress Incr Code Image: Space Stress Incr Code Plate Grip DOL Lumber DOL Rep Stress Incr Code pt 1433/0-3-8 (min. 0-1-12), 11=13 (0-12) pt 144: 2x4 SP DSS e) 2=1333/0-3-8 (min. 0-1-12), 11=13 (0-12) pt 154: 2x4 SP DSS e) 2=133/(0-11) pt 164: 2x4 SP DSS e) 2=1491(LC 17), 11=1508(LC 18) CompJMax. Ten All forces 250 (ID) or less 1- 1927/03, 3-276=-1842/94, 3-4=-1481/122, 4-5 -203167 pt 1553, 18-19=0/1563, 17-18=0/1563, 17-28= 0/273, 3-17=-547/124, 5-17=9/571, 6-15=0/1 releads have been considered for this design VUIE=1250m(L) 6-36=cond yeally Vasc=99mpt; netrodry 12-3-14 to 126-9. Extendor(2R) 12-6- gint exposed : end vertical eff and right expo frainage to prevent water ponding. I designed for a 1 ve load of 20.0pd line to a designed for a 1 ve load of 20.0pd line to 2) = 10.0pd. Loomection (by others) of truss to bearing pl ed in accordance with the 2018 International resentation does not depict the size or the or ndard	G FROM FROM 6-5-1 2-0-0 1.00 1.15 YES P2014 333/0-3-8 (mln, 0-1-12) except when shown. =-1375/165, 5-27=-1084/167, 21 =0/1158, 16-29=0/1158, 15-16=0 1948, 10-14=0/1284, 10-13=-156 TCDL=6.0psf; BCDL=6.0psf; h== 9 to 17-0-13, Interlor(1) 17-0-13 sed;C-C for members and forces ad nonconcurrent with any other action chord in al areas where a lat ecapable of withstanding of 7. If Residential Code sections R602 related on the purin along the 1	12-6-9 6-1-7 12-6-9 6-1-7 TC 0.47 CSI. 700 WB 0.64 WB 0.64 WB 0.64 Matrix-MS 9 7-28=-1084/167, 6-28=-1084/167 /1989, 15-30=0/1501, 30-31=0/1 7/0, 6-16=-1288/0, 8-15=-576/13 25ft: B=45ft: L=32ft: eave=4ft; Ci 19-5-7, Exterfor(2R) 19-5-7 to 26 MWFRS for reactions shown: I/vectangl: 3-60 tail by 2-0-0 wide uplift at joint 2 and 67 lb uplift a 11.1 and R802.10.2 and referer op and/or bottom chord.	CK SIDE FO	R A NET 2' 24-0-12 5-6-12 0.10 -0.47 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 -0.86 14-15 0.18 11 3- RD Structural wc 2-0-0 cp put RD RIG deeling 1 Row at mic 12, 8-9=-3210/142, 9- 2109, 11-13=0/1500 XS (directional) and C-C 210-8 zone; DOL=1.60 chord and any other	OC SPAC 28-8-0 4.7-4 4.7-4 Videfl Ld >822 240 >445 180 n/a n/a od sheathling directly applied or 10-q pt 0=-3136/29, Exterior(2E)	CING IN THE TR	USS MEMBER.

and lumber industry Engineer, or igner, whereas, DrJ Reference ENGINEERING LLC

Engineer, the seal on this TDD represents an acceptance of professional engineering responsibility for the design of the single Truss only as depicted pursuant to ANSI/TP1 1, the National Design Standard for Metal Plate Connected Wood Truss Construction ("TP1 1"). The resistance to load, by this Truss, is predicated on accurate Metal Connector Plate (MCP) design values published by MCP manufacturers and lumber industry published design values (mechanically or visually graded as indicated) and their associated Specific Gravity (SG) values. The Truss Design Engineer is NOT the Building Designer, Truss System Engineer, or Structural Engineer of Record for any Building. Any field use of the Truss, including applied design loads, load paths, and structural resistance requirements, is the responsibility of the Building Designer, Truss System Engineer, or Structural Engineer of Record for any Building. Any field use of the Truss, including applied design loads, load paths, and structural resistance requirements, is the responsibility of the Building Designer, there are shore this design's coversheet and DrJ Reference Sheet (rev. 06-17) are fully incorporated herein by reference and can be found at: pdf.paragontruss.com/jobFile/2996489 Copyright © 2020. Reproduction of this document, in any form, is prohibited without written permission from DrJ Engineering, LLC (DrJ) - Carolina Structural Systems



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General Safety Notes Failure to follow could cause property damage or personal injury.

1...The Truss Manufacturer (TM) isCarolina Structural Systems 2...Capitalized terms are as defined in ANSI/TPI 1, the National Design Standard for Metal Plate Connected Wood Truss Construction ("TPI 1").

3.□TPI 1 defines the responsibilities and duties of the Truss Designer (also known as Truss Design Engineer) and TM, unless otherwise defined by a Contract agreed upon in writing by the parties involved.

A.□Please thoroughly review the Truss Submittal Package ("TSP") and any related documents provided by the TM (e.g., Truss Design Drawings ("TDD"(s), Truss Placement Diagrams ("TPD"(s), cover sheets, details, detail notes, etc.). The approval of the contents of the TSP is the responsibility of the Building Designer "BD") and Contractor ("Gc"). The TSP shall be supplied to the GC/Truss Installer. The GC, after reviewing and/or approving the TSP, shall forward it to the BD for review. 5.□The GC shall review the entire TSP before unloading, storing, handling, installing, or bracing the Trusses. A review of the graphics alone is not sufficient. 6.□The TDD(s) (also at times referred to as "Delegated Engineering Documents")

6. DThe Tobses. A review of use graphics after is not sunicent. 6. DThe Tobses. A review of use graphics after is not sunicent. referenced have been prepared based on the Construction Documents, specifications and written requirements prepared by the BD indicating the nature and character of the Building, its applied load, its load paths and the structural resistance design requirements. The TDDs herein are specially structural component designs and may be part of the Building"s deferred or phased submittal documents. The design criteria from the Construction Documents, specifications and written requirements have been transferred to DrJ Engineering, LLC ("DrJ") by the TM.

7. □The seal of the Truss Design Engineer (also referred in some Jurisdictions as a "Delegated or Specialty Engineer") on any TDD represents an acceptance of professional engineering responsibility for the design of the single Truss only as depicted on the TDD pursuant to TPI 1.

B.⊡The TM and DrJ are NOT the BD, Truss System Engineer (as the term is defined in some Jurisdictions), or Structural Engineer of Record for any Building. 9....The BD is responsible for and shall coordinate and review each of the TDDs and all notes for compatibility with the Construction Documents, specifications, written requirements, design loads and load path. Any field of use of the Truss, including applied loads, load paths, structural resistance requirements, handling, storage, installation and bracing, shall be verified by and is the responsibility of the BD and GC. 10...Upon transmittal of the TSP and upon delivery of the Trusses to the GC, the GC shall read all notes and instructions in the TDDs and TPDs and review the practices and guidelines of Building Component Safety Information ("BCSI") and/or its summary sheets as published by the Truss Plate Institute (TPI) and the Structural Building Component Association (SBCA).

II.□The TSP which shall include the TDDs, TPDs, the appropriate BCSI summary sheets to facilitate proper Truss Temporary Lateral Restraint (LR) and Diagonal Bracing (DB), any Truss installation information and any related Truss details shall be reviewed by the BD for compatibility with the design of the Building, including submittal documents prepared by others, deferred and phased submittal documents. This review shall include a notation indicating that the reviewed documents have been found to be in general conformance with the design of the Building (or to make specific corrections noted and to return for review). In the absence of this notation, the TM will provide its Customer with the design assumptions used, according to the TMâ⊡s interpretation o the Construction Documents, specifications and written requirements, to design the individual structural building components (i.e., Trusses) per TPI 1.

.⊡The TM and DrJ shall be permitted to rely on the accuracy and completeness of the ilding Contract (if provided), the Construction Documents, specifications and itten requirements that have been furnished to the TM.

As set forth in TPI 1, the BD shall provide information that is sufficiently accurate reliable to be able to design the Trusses in the context of the following serviceability les including, but not limited to: (a) allowable vertical, horizontal or other required flection criteria: (b) any dead load, live load and in-service creep deflection criteria for ors or flat roofs subject to ponding loads; (c) any floor or roof camber requirements; any differential deflection criteria from Truss-to-Truss or Truss-to-adjacent structural mber; (e) any deflection and vibration criteria for floor Trusses including any ongback bridging requirements, and any dead load, live load, and in-service creep lection criteria for floor Trusses supporting stone or ceramic tile finishes; (f) moisture, perature, corrosive chemicals and gases expected to result in a wood moisture tent exceeding 19%, sustained temperatures exceeding 150A°F, and/or corrosion tential from wood preservatives or other sources that may be detrimental to Trusses. Due to the lateral thrust developed by scissors-type Trusses, if scissors-type usses are part of this Building design, consideration should be given to bearing wall nditions. Bearing walls supporting scissors-type Trusses should be designed in such anner that the walls will safely withstand the lateral forces of the Trusses. nsideration of effects on the design of the bearing and the associated wall or am/header assembly is not a part of this set of TDDs and is neither the ponsibility of the TM nor DrJ. Advice from the BD or any Registered Design fessional $(\hat{a} \square RDP\hat{a} \square)$ should be secured relative to these items if they are not provide he Construction Documents, specifications and written requirements.

15.□Unless specifically noted in writing otherwise, neither the TM nor DrJ have performed any of the following engineering services as it relates to the Building and are to be designed by others: (a) Masonry loading conditions relating to the Trusses, which require special engineering; (b) Areas of the Construction Documents that do not use Truss framing; and (c) All beam, header and related structural element designs. If any of the previously listed services are required by the Owner, the Owner's authorized agent, or the BD, please call 608-628-1453 for assistance. 16.□Where required by the Contract between the TM and GC or Owner or the

16. Where required by the Contract between the TM and GC or Owner or the Construction Documents, specifications and written requirements, a TPD will be provided that identifies the location of each Truss, as assumed by the TM based on their review of the Building Contract (if provided) and the Construction Documents, specifications and written requirements.

17. □When the TPD is provided, it serves only as a guide for Truss installation, it does not require the seal of any RDP.

18.□Truss manufacturing quality control shall be in accordance with TPI 1, Chapter 3, and monitored by a third party inspection agency.

19. □Unless specified by the BD in writing and noted on the TDD, these TDDs are not applicable for use with fire-retardant, preservative-treated, or green lumber. 20. □Plate type, size, orientation, and location dimensions indicated on a TDD are minimum plating requirements. □

21...DLmber used in the manufacturing of the Truss shall be of the species, size and grade, and in all respects, equal to or better than that specified on the TDD.

2.□Temporary LR and DB is required to be installed during construction for the purposes of holding Trusses in their proper location, plumb and in plane, until permanent individual Truss member LR, DB and Permanent Building Stability Bracing (PBSB) are completely installed (see BCSI-B1, BCSI-B2, BCSI-B3, BCSI-B7, and BCSI-B10 as applicable). 23.□Top chords must be sheathed or continuous LR members (i.e., purlins) shall be provided at the spacing indicated on TDD (e.g., 24 in. o.c. maximum). 24.□If no ceiling is installed or bottom chord LR is specified in writing by the BD, bottom chords require continuous LR at 10 ft. o.c. spacing, or less if specified on the TDD, along with DB as specified in BCSI-B1, BCSI-B3, BCSI-B7 or BCSI-B10 as applicable. 25.□Graphical representation of LR members (i.e., purlins), if shown on the TDD, do not depict the size or orientation of the restraint along the top and/or bottom chord and/or web members.

26 □ The size, connections and anchorage of the permanent continuous Truss chord and web member LR and DB must be designed by others in such a way as to support the imposed load along the clear span of the LR and DB, or as specified in BCSI-B1, BCSI-B3, BCSI-B7 or BCSI-B10 as applicable. □

27. □Additional PBSB for the Truss system (e.g., diagonal, X-bracing, etc.) may be required and is to be specified by the BD. DB in accordance with BCSI-B3 may be sufficient. □ 28. □The Trusses shall be examined upon delivery to the jobsite and also after they are erected and installed for: (a) dislodged or missing connectors, (b) cracked, dislodged or broken members, or (c) any other damage that may impair the structural integrity of the Trusses. Any unreported damage to any Truss during any part of the handling and installation process shall void the TMâ⊡s product warranty.

installation process shall void the TMâ⊡s product warranty. 29.⊡During Truss installation, never exceed the design loading shown on the TDD and never stack materials on Trusses with inadequate LR and DB (see BCSI-B4). Never overload any structural elements with stacks of building materials to a level greater than defined in BCSI-B4.

30. □Connections not shown on the TDDs are the responsibility of others. 31. □Do not cut or alter a Truss member, a Metal Connector Plate ("MCP") or any

31. □Do not cut or alter a Truss member, a Metal Connector Plate ("MCP") or an related structural element member without prior approval of an RDP or TD.

32.□Install and load Trusses vertically unless otherwise indicated in writing by the BD or as specifically defined on the TDD or TPD.

33. □Sheathing applied in the plane of the Truss is not considered in the design of the individual Truss (e.g., a gable-end Truss has no composite stiffness analysis performed) unless specifically noted. □

34.⊡Attachment of the purlin gable (i.e., hip frames or lay-in gables) to the supporting hip Trusses satisfies the LR and DB requirements for the top chord of the hip Trusses. Refer to the TDD for the individual Trusses braced in this manner. □

35. These Trusses are designed using the standard engineering analysis methods and associated software in accordance with TPI 1 and related proprietary information from the Design Program Manufacturer.

Each TDD uses MCP design values published by MCP manufacturers and lumber industry published design values (mechanically or visually graded as indicated). These are incorporated into lumber design provisions and equations created by the American Wood Council (AWC) per the National Design Specification® (NDS®) for Wood Construction and input into modeling/analysis software that uses TPI 1 provisions. The lumber design values correspond to the lumber size and grade as defined on the TDD incorporating the design values from the grade stamp identified by the TM on the lumber prior to cross cutting for manufacturing purposes. The published lumber design values (a) are calculated and administered by the lumber rules writing agencies using a property range or bending correlation, and are not tension proof tested, (b) are approved by the American Lumber Standards Committee (ALSC) as published design data that are representative of the strength and stiffness of specific grades and species/species groups of lumber, and (c) are further known by ALSC and the lumber rules writing agencies as individual visually or mechanically graded lumber pieces which do not have precise design values. Neither the TM nor the Truss Design Engineer can therefore verify or warrant that published lumber design values will exist within the lumber utilized in the Truss when manufactured and delivered.

The "WARNING" note found on the bottom of each TDD references this document by calling it the "DrJ Reference Sheet (rev. 01-16)"



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Numbering System

