

DE: 2490020 USU/Kanzia/Lat/660/Mana	rati ovington Plan	Trenco
	TallexingionPlan	818 Soundside Rd
Site Information:		Edenton, NC 27932
Project Customer: H and H Project Name:		
Lot/Block: 669	Subdivision: MANOR @ LEXIN	NGTON PLANTATION
Model:		
Address:		
City: CAMERON	State: NC	
General Truss Engineering Criteria & Des	ign Loads (Individual Truss De	esign
Drawings Show Special Loading Conditio	ns):	
Design Code: IRC2015/TPI2014	Design Program: MiT	ek 20/20 8.2
Wind Code: ASCE 7-10 Wind Speed: 130 mj	ph Design Method: MWI	FRS (Envelope)/C-C hybrid Wind ASCE 7-10
Roof Load: 40.0 psf	Floor Load: N/A psf	
Mean Roof Height (feet): 25	Exposure Category: C	2

No.	Seal#	Truss Name	Date
1	I43516040	A01	11/6/20
2	143516041	A02	11/6/20
3	143516042	A03	11/6/20
4	143516043	A04	11/6/20
5	143516044		11/6/20
<u>6</u>	143516045	B01	11/6/20
7		B02	11/6/20
8	143516047	C01	11/6/20
9	143516048	C02	11/6/20
10	143516049	H01	11/6/20
11	143516050	H02	11/6/20
12	143516051	H03	11/6/20
13	143516052	H04	11/6/20
14	143516053	J01	11/6/20
40	143516054	J02	11/6/20
10		JU3	11/6/20

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource-Sumter,SC.

Truss Design Engineer's Name: Sevier, Scott

My license renewal date for the state of North Carolina is December 31, 2020.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Sevier, Scott

November 6,2020



⊢						37-11-0 37-11-0						
OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.15	TC	0.08	Vert(LL)	0.00	22	n/r	120	MT20	244/190
CDL	10.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	0.00	22	n/r	120		
CLL	0.0 *	Rep Stress Incr	YES	WB	0.17	Horz(CT)	0.01	22	n/a	n/a		
CDL	10.0	Code IRC2015/TF	PI2014	Matri	x-S						Weight: 259 lb	FT = 20%

BOT CHORD

TOP CHORD 2x4 SP No.2 2x6 SP No.2 BOT CHORD 2x4 SP No.3 OTHERS

REACTIONS. All bearings 37-11-0.

Max Horz 2=-186(LC 13) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 34, 35, 37, 38, 39, 40, 41, 32, 31, 29, 28, 27, 26, 25, 22 except 42=-118(LC 12), 24=-115(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 33, 34, 35, 37, 38, 39, 40, 41, 42, 32, 31, 29, 28, 27, 26, 25, 24, 22

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 10-11=-97/253, 11-12=-115/303, 12-13=-115/303, 13-14=-97/253

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 34, 35, 37, 38, 39, 40, 41, 32, 31, 29, 28, 27, 26, 25, 22 except (jt=lb) 42=118, 24=115.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANS/TPH1</u> Quality Criteria, DSB-89 and BCSI Building Component
 Satisfies
 Ansi/TPH Qu

 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





L	9-9-0	18-11-8	28-2-0	37-11-0
I	9-9-0	9-2-8	9-2-8	9-9-0
Plate Offsets (X,Y)	[3:0-5-0,0-4-8], [5:0-5-0,0-4-8]			
LOADING (psf)	SPACING- 2-0	-0 CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.1	5 TC 0.42	Vert(LL) -0.17 10-12 >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.1	5 BC 0.96	Vert(CT) -0.42 10-12 >999 240	MT20HS 187/143
BCLL 0.0 *	Rep Stress Incr YE	S WB 0.54	Horz(CT) 0.16 6 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	1 Matrix-AS	Wind(LL) 0.18 12-15 >999 240	Weight: 209 lb FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

5-10, 3-10

Rigid ceiling directly applied

1 Row at midpt

LUMBER-

TOP CHORD2x6 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=184(LC 12) Max Uplift 2=-429(LC 12), 6=-429(LC 13) Max Grav 2=1556(LC 1), 6=1556(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3058/1294, 3-4=-2107/996, 4-5=-2107/996, 5-6=-3058/1294

BOT CHORD 2-12=-1044/2737, 10-12=-1046/2734, 8-10=-1047/2734, 6-8=-1046/2737

WEBS 4-10=-362/1011, 5-10=-1020/565, 5-8=0/372, 3-10=-1020/565, 3-12=0/372

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are MT20 plates unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=429, 6=429.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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ł		7-1-0 7-1-0	13-9-9 6-8-9			24-1-7 10-3-14				30-10-0 6-8-9	37-1 7-1	1-0 -0	
LOADING TCLL TCDL BCLL BCDL	(psf) 20.0 10.0 0.0 * 10.0	SPACING Plate Grip Lumber DO Rep Stress Code IRC	- 2-0-0 DOL 1.15 DL 1.15 s Incr YES 2015/TPI2014	CSI. TC BC WB Matrix-	0.47 0.68 0.61 -AS	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (-0.18 -0.50 15 0.10 0.19	(loc) 16 5-16 10 17	l/defl >999 >902 n/a >999	L/d 360 240 n/a 240	PLATES MT20 MT20HS Weight: 241 lb	GRIP 244/190 187/143 FT = 20%	
LUMBER	-					BRACING-							

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

- TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x6 SP No.2

 WEBS
 2x4 SP No.3 *Except*

 20-21: 2x4 SP No.2
- REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=186(LC 12) Max Uplift 2=-336(LC 12), 10=-336(LC 13) Max Grav 2=1669(LC 1), 10=1669(LC 1)
- FORCES.
 (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-3402/1114, 3-5=-2851/876, 5-6=-2838/1031, 6-7=-2838/1031, 7-9=-2851/876, 9-10=-3402/1114

 BOT CHORD
 2-19=-902/3071, 17-19=-902/3071, 16-17=-320/1951, 15-16=-320/1951, 14-15=-320/1951, 12-14=-907/3071, 10-12=-907/3071
- WEBS 6-21=-344/1113, 14-21=-370/1098, 7-14=-376/343, 9-14=-588/422, 17-20=-370/1098, 6-20=-344/1113, 5-17=-376/343, 3-17=-588/422

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) 200.0lb AC unit load placed on the bottom chord, 18-11-8 from left end, supported at two points, 5-0-0 apart.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 336 lb uplift at joint 2 and 336 lb uplift at joint 10.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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		7-1-0	13-9-9			24-1-7	24-1-7			30-10-0	37-11-0	
		7-1-0 '	6-8-9			10-3-14				6-8-9	7-1	-0 '
Plate Offs	ets (X,Y)	[2:0-0-12,Edge], [1	0:0-0-12,Edge]									
LOADING TCLL TCDL BCLL BCDL	(psf) 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip D Lumber DOI Rep Stress Code IRC20	2-5-8 IOL 1.15 L 1.15 Incr NO 015/TPI2014	CSI TC BC WB Mate	0.44 0.97 0.83 rix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.17 -0.50 0.13 0.19	(loc) 16 15-16 10 17	l/defl >999 >908 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 275 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHO BOT CHO WEBS	- DRD 2x6 SP DRD 2x6 SP 2x4 SP 20-21: :	No.2 No.2 No.3 *Except* 2x4 SP No.2				BRACING- TOP CHOR BOT CHOR	:D :D	Structu Rigid c	ıral wood eiling dire	sheathing c ectly appliec	directly applied or 3-2-9 d d or 6-6-3 oc bracing.	oc purlins.
REACTIC	DNS. (size Max H Max U Max G	e) 2=0-3-8, 10=0- orz 2=226(LC 12) plift 2=-428(LC 12) rav 2=2013(LC 1),	-3-8 , 10=-428(LC 13) 10=2013(LC 1)									
FORCES TOP CHC	. (lb) - Max. DRD 2-3=- 9-10=	Comp./Max. Ten 4235/1467, 3-5=-34 4235/1467	All forces 250 (lb) or 490/1152, 5-6=-3472/	less excep 1344, 6-7=	t when shown -3472/1344, 7	n. 7-9=-3490/1152,						
BOT CHC	0RD 2-19=	-1208/3858, 17-19	=-1208/3858, 16-17=	-446/2378	15-16=-446/2	2378,						
WEBS	6-21= 17-20	=-446/2378, 12-14 =-461/1356, 14-21=)=-491/1342, 6-20=	=-1211/3858, 10-12= -491/1342, 7-14=-455 -461/1356, 5-17=-455	5/423, 9-14 5/423, 3-17	5 =-813/539, 9- ′=-813/539, 3-	12=0/257, 19=0/257						
NOTES- 1) Unbala 2) Wind: <i>J</i> MWFR MWFR 3) 200.0lt 4) All plat 5) This tru	ASCE 7-10; V S (envelope) S for reaction AC unit load es are 2x4 M uss has been	loads have been o ult=130mph (3-sec gable end zone an s shown; Lumber E placed on the bott T20 unless otherwis designed for a 10.0	considered for this der ond gust) Vasd=103n d C-C Exterior(2) zon OOL=1.60 plate grip D om chord, 18-11-8 fro se indicated.	sign. hph; TCDL e; end vert OL=1.60 m left end, e load nond	=6.0psf; BCD ical left and rig supported at	L=6.0psf; h=25ft; C ght exposed;C-C fc two points, 5-0-0 a h any other live loa	Cat. II; E or mem opart. Ids.	Exp C; E bers an	inclosed; d forces	&	ALOP SES	AROLIN

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 428 lb uplift at joint 2 and 428 lb uplift at joint 10.

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l	<u>8-9-0</u> 8-9-0		<u>17-11-8</u> 9-2-8	9	7-2-0 -2-8		<u>35-11-0</u> 8-9-0	
Plate Offsets (X,Y)	[3:0-5-0,0-4-8], [5:0-5-0,0	-4-8]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 1.15 1.15 YES I2014	CSI. TC 0.34 BC 0.98 WB 0.51 Matrix-AS	DEFL. in ((Vert(LL) -0.17 10 Vert(CT) -0.42 10 Horz(CT) 0.14 Wind(LL) 0.16	loc) l/defl -12 >999 -12 >999 6 n/a -12 >999	L/d 360 240 n/a 240	PLATES MT20 MT20HS Weight: 199 lb	GRIP 244/190 187/143 FT = 20%
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x4 SF	2 No.2 2 No.2			BRACING- TOP CHORD St BOT CHORD Ri	ructural wood s gid ceiling dired	heathing directl	y applied.	

WEBS

1 Row at midpt

5-10, 3-10

WEBS 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=175(LC 12) Max Uplift 2=-408(LC 12), 6=-408(LC 13) Max Grav 2=1476(LC 1), 6=1476(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2924/1238, 3-4=-2007/940, 4-5=-2007/940, 5-6=-2924/1238

BOT CHORD 2-12=-1006/2621, 10-12=-1008/2618, 8-10=-1010/2618, 6-8=-1008/2621

WFBS 4-10=-304/914, 5-10=-977/541, 5-8=0/349, 3-10=-977/541, 3-12=0/349

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are MT20 plates unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=408, 6=408.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10 except (it=lb) 16=107, 17=103, 18=134, 14=106, 13=103, 12=128.

10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

TRENCIO AMITEK Affiliate 818 Soundside Road

Edenton, NC 27932

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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	H&H/Kenzie/Lot/669/ManoratLexingtonPlan	
						l43516048
2489030	C02	Common Girder	1	2		
				–	Job Reference (optional)	
Builders FirstSource, S	umter, SC - 29153,			8.240 s Ma	r 9 2020 MiTek Industries, Inc. Thu Nov 5 14:07:49 2020	Page 2

ID:2zFdWTuXpJBB?1X9xOWCpYycTNe-yHE2MtBM3ZYKN5YX_22bTpBK5zlK?Ak9sqwgdlyMCre

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 9-12=-20

Concentrated Loads (lb)

Vert: 6=-1536(B) 15=-1649(B) 16=-1649(B) 17=-1649(B) 18=-1988(B) 19=-1988(B) 20=-1536(B) 21=-1536(B)

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Plate Offsets (X,Y)-- [5:0-3-0,Edge]

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.09 BC 0.04 WB 0.05 Matrix-S	DEFL. ir Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) 0.00	n (loc) l/defl 9 n/r 9 n/r 10 n/a	L/d 120 120 n/a	PLATES MT20 Weight: 63 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	P No.2 P No.2 P No.3		BRACING- TOP CHORD BOT CHORD	Structural wood except end vert Rigid ceiling dir	sheathing dir icals. ectly applied o	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,

WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3

REACTIONS. All bearings 10-10-0. Max Horz 15=-174(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 15, 13, 12 except 14=-166(LC 12), 11=-165(LC 13) Max Grav All reactions 250 lb or less at joint(s) 15, 10, 13, 14, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 13, 12 except (jt=lb) 14=166, 11=165.

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 Satisfies
 Ansi/TPH Qu

 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

NOTES-1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=108, 2=131.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (jt=lb) 2=131.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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Edenton, NC 27932

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (jt=lb) 2=124.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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Edenton, NC 27932

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2, 7.

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Plate Offsets (X,Y)-	[4:Edge,0-1-14]			_
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d PLATES GRIP	
TCLL 20.0	Plate Grip DOL 1.15	TC 0.12	Vert(LL) -0.00 7 >999 360 MT20 244/190	
TCDL 10.0	Lumber DOL 1.15	BC 0.12	Vert(CT) -0.01 4-7 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 2 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MP	Wind(LL) 0.01 4-7 >999 240 Weight: 18 lb FT = 20%	
LUMBER-			BRACING-	

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=63(LC 11)

Max Uplift 2=-106(LC 8), 4=-54(LC 12) Max Grav 2=211(LC 1), 4=146(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

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NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=106.

Structural wood sheathing directly applied or 3-11-8 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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	ŀ			2-11- 2-11-	-8 -8					3-11-8 1-0-0	
Plate Offsets (X,Y)	[2:0-3-6,0-0-1]										
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0	.10	Vert(LL)	-0.00	9	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0	.09	Vert(CT)	-0.00	9	>999	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0	.06	Horz(CT)	0.00	2	n/a	n/a		
BCDL 10.0	Code IRC2015/TI	PI2014	Matrix-N	/IP	Wind(LL)	0.00	9	>999	240	Weight: 19 lb	FT = 20%
LUMBER-					BRACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x6 SP No.2 BOT CHORD WEBS 2x4 SP No.3

REACTIONS. (size) 4=0-1-8, 2=0-3-8 Max Horz 2=46(LC 11) Max Uplift 2=-77(LC 8)

Max Grav 4=282(LC 2), 2=247(LC 1)

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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.
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2-3=-258/16 TOP CHORD

WEBS 3-5=-321/12

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) gable end zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60 3) C-C wind load user defined.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 10) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S)

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf) Vert: 1-3=-60, 3-4=-80, 5-7=-20
 - Concentrated Loads (lb)
 - Vert: 3=-150
- 2) Dead + 0.75 Roof Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15

Continued on page 2

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Structural wood sheathing directly applied or 3-11-8 oc purlins,

except end verticals, and 2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Job	Truss	Truss Type	Qty	Ply	H&H/Kenzie/Lo	ot/669/ManoratLexingtor	nPlan	142516055
2489030	J03	Half Hip	7	1				143510055
Duildere FiretSource	Sumtor SC 20152			240 a Ma	Job Reference	(optional)	V E 14:07:E4 2020	Daga 2
Builders FirstSource, 3	buinter, SC - 29155,	ID:2zFd\	م /TuXpJBB?	1X9xOWC	pYycTNe-IE1xP	aFVu6AdUtRUnbdmAsi	JFc_YHgb7u?6eRlz	Page 2 yMCrZ
LOAD CASE(S) Uniform Loads (plf) Vert: 1-3=-50, 3 Concentrated Loads (lb) Vert: 4=-15 3=- 3) Dead + Uninhabitable A Uniform Loads (plf) Vert: 1-3=-20, 3 Concentrated Loads (lb) Vert: 3=-130 4) Dead + 0.6 C-C Wind (P Uniform Loads (plf) Vert: 1-2=80, 2- Horz: 1-2=-92, 2 Concentrated Loads (lb) Vert: 3=-130 5) Dead + 0.6 C-C Wind (P Uniform Loads (plf) Vert: 1-2=59, 2-	1-4=-99, 5-7=-20 130 130 14: Without Storage: Lumber 1-4=-40, 5-7=-40 Pos. Internal) Case 1: Lumber 13=66, 3-4=59, 5-7=-12 2-3=-78, 3-4=83, 4-5=53 Pos. Internal) Case 2: Lumber 13=66, 3-4=59, 5-7=-12	ID:2zFdV Increase=1.25, Plate Increase=1.25 Increase=1.60, Plate Increase=1.60 Increase=1.60, Plate Increase=1.60	VTuXpJBB?	1X9xOWC	pYycTNe-IE1xP	aFVu6AdUtRUnbdmAsı	JFc_YHgb7u?6eRlz	yMCrZ
Horz: 1-2=-71, 2 Concentrated Loads (Ib)	2-3=-78, 3-4=83, 4-5=-34							
Vert: 3=-130								
6) Dead + 0.6 C-C Wind (N Uniform Loads (plf) Vert: 1-2=1, 2-3 Horz: 1-2=-21, 2 Concentrated Loads (lb)	leg. Internal) Case 1: Lumber =-48, 3-4=-52, 5-7=-20 2-3=28, 3-4=-20, 4-5=-49	Increase=1.60, Plate Increase=1.60						
Vert: 3=-130	lea Internal) Case 2: Lumber	Increase-1.60 Plate Increase-1.60						
Uniform Loads (plf) Vert: 1-2=-41, 2 Horz: 1-2=21, 2 Concentrated Loads (lb)	-3=-48, 3-4=-52, 5-7=-20 -3=28, 3-4=-20, 4-5=38	IIICI6256=1.00, Flate IIICI6256=1.00						
Vert: 3=-130 8) Dead + 0.6 MWFRS Wir	nd (Pos. Internal) Left [,] Lumbe	r Increase=1.60. Plate Increase=1.60						
Uniform Loads (plf) Vert: 1-2=57, 2- Horz: 1-2=-69, 2	3=40, 3-4=28, 5-7=-12 2-3=-52, 3-4=52, 4-5=28	a increase - 1.00, 1 late increase - 1.00						
Vert: 3=-130								
9) Dead + 0.6 MWFRS Wir Uniform Loads (plf) Vert: 1-2=14, 2- Horz: 1-2=-26, 2	nd (Pos. Internal) Right: Lumb 3=22, 3-4=28, 5-7=-12 2-3=-34, 3-4=52, 4-5=-21	per Increase=1.60, Plate Increase=1.60						
Concentrated Loads (lb)								
10) Dead + 0.6 MWFRS W Uniform Loads (plf)	/ind (Neg. Internal) Left: Lumb 2-3=17 3-4=5 5-7=-20	per Increase=1.60, Plate Increase=1.60						
Horz: 1-2=-45, Concentrated Loads (Ik Vert: 3=-130	, 2-3=-37, 3-4=37, 4-5=13 b)							
11) Dead + 0.6 MWFRS W	/ind (Neg. Internal) Right: Lun	nber Increase=1.60, Plate Increase=1.60						
Uniform Loads (plf) Vert: 1-2=6, 2- Horz: 1-2=-26,	-3=-1, 3-4=5, 5-7=-20 , 2-3=-19, 3-4=37, 4-5=-36							
Concentrated Loads (It	o)							
12) Dead + 0.6 MWFRS W Uniform Loads (plf)	/ind (Pos. Internal) 1st Paralle	el: Lumber Increase=1.60, Plate Increase=1.6)					
Vert: 1-2=33, 2 Horz: 1-2=-45, Concentrated Loads (Ik	2-3=40, 3-4=6, 5-7=-12 , 2-3=-52, 3-4=30, 4-5=25 o)							
Vert: 3=-130 13) Dead + 0.6 MWFRS W	/ind (Pos. Internal) 2nd Paralle	el: Lumber Increase=1.60, Plate Increase=1.6	0					
Vert: 1-2=10, 2 Horz: 1-2=-22,	2-3=18, 3-4=6, 5-7=-12 , 2-3=-30, 3-4=30, 4-5=-18							
Concentrated Loads (It))							
14) Dead + 0.6 MWFRS W Uniform Loads (plf)	/ind (Pos. Internal) 3rd Paralle	el: Lumber Increase=1.60, Plate Increase=1.6	0					
vert: 1-2=33, 2 Horz: 1-2=-45, Concentrated Loads (It	2-3=40, 3-4=6, 5-7=-12 , 2-3=-52, 3-4=30, 4-5=25 5)							

Vert: 3=-130

Continued on page 3

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Job	Truss	Truss Type	Qty	Ply	H&H/Kenzie/Lot/669/ManoratLexingtonPlan	
2480020	102		7	1	143516055	
2469030	505		1	'	Job Reference (optional)	
Builders FirstSource, S	Sumter, SC - 29153,		{ [1] Yn IBB2	3.240 s Ma	r 9 2020 MiTek Industries, Inc. Thu Nov 5 14:07:54 2020 Page 3	
LOAD CASE(S) 15) Deed + 0.6 MWERS Wind (Res. Internel) 4th Decellel: Lumber Increase, 1.60, Plate Increase, 1.60						
Uniform Loads (plf)						
Vert: 1-2=10, 2-3=18, 3-4=6, 5-7=-12						
Horz: 1-2=-22, 2-3=-30, 3-4=30, 4-5=-18 Concentrated Loads (lb)						
Vert: 3=-130						
16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60						
Vert: 1-2=25, 2-3=17, 3-4=-17, 5-7=-20						
Horz: 1-2=-45, 2-3=-37, 3-4=15, 4-5=10						
Concentrated Loads (Ib) Vert: 3=-130						
17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60						
Uniform Loads (plf)	Uniform Loads (plf)					
Horz: 1-2=-22, 2-3=-15, 3-4=-17, 3-7=-20						
Concentrated Loads (lb)						
Vert: 3=-130 18) Dead: Lumber Increase=0.90. Plate Increase=0.90 Plt_metal=0.90						
Uniform Loads (plf)						
Vert: 1-3=-20, 3-4=-40, 5-7=-20						
Vert: 3=-130						
19) Dead + 0.75 Roof Live	(bal.) + 0.75 Attic Floor + 0.7	5(0.6 MWFRS Wind (Neg. Int) Left): Lumber In	crease=1.	.60, Plate	Increase=1.60	
Uniform Loads (plf) Vert: 1-217, 2-322, 3-465, 5-720						
Horz: 1-2=-33, 2-3=-28, 3-4=28, 4-5=9						
Concentrated Loads (lb)						
20) Dead + 0.75 Roof Live	(bal.) + 0.75 Attic Floor + 0.7	5(0.6 MWFRS Wind (Neg. Int) Right): Lumber	ncrease=	1.60, Plate	e Increase=1.60	
Vert: 1-2=-30, 2-3=-36, 3-4=-65, 5-7=-20 Horz: 1-2=-20, 2-3=-14, 3-4=28, 4-5=-27						
Concentrated Loads (II))					
Vert: 4=-15 3=-130 21) Dead + 0.75 Roof Live (bal) + 0.75 Attic Floor + 0.75(0.6 MWERS Wind (Neg. Int) 1st Parallel): Lumber Increase-1.60 Plate Increase-1.60						
Uniform Loads (plf)						
Vert: 1-2=-17, 2-3=-22, 3-4=-82, 5-7=-20						
Horz: 1-2=-33, 2-3=-28, 3-4=11, 4-5=8 Concentrated Loads (lb)						
Vert: 4=-15 3=-130						
22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60						
Vert: 1-2=-33, 2-3=-39, 3-4=-82, 5-7=-20						
Horz: 1-2=-17, 2-3=-11, 3-4=11, 4-5=-25						
Vert: 4=-15 3=	-130					
23) 1st Dead + Roof Live (unbalanced): Lumber Increas	e=1.15, Plate Increase=1.15				
Uniform Loads (plf) Vert: 1-3=-60	3-4=-80 5-7=-20					
Concentrated Loads (II)					
Vert: 3=-150	(unbalanced): Lumber Increa	so-1.15 Plata Increase-1.15				
Uniform Loads (plf)	(unbalanceu). Lumber increa	SE=1.13, Flate Increase=1.13				
Vert: 1-3=-20,	3-4=-80, 5-7=-20					
Concentrated Loads (II Vert: 3=-150))					
25) 3rd Dead + 0.75 Roof I	_ive (unbalanced) + 0.75 Attic	c Floor: Lumber Increase=1.15, Plate Increase=	1.15			
Uniform Loads (pit)						
Concentrated Loads (Ib)						
Vert: 4=-15 3=	-130	Electric umber Increases - 4.45, Distaile	4 4 5			
∠o) 4th Dead + 0.75 Roof I Uniform Loads (blf)	Live (unbalanced) + 0.75 Attic	: FIOUR: LUMDER INCREASE=1.15, Plate Increase=	1.15			
Vert: 1-3=-20,	3-4=-99, 5-7=-20					
Concentrated Loads (II) -130					
voit. 4 =-10 0=						

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

