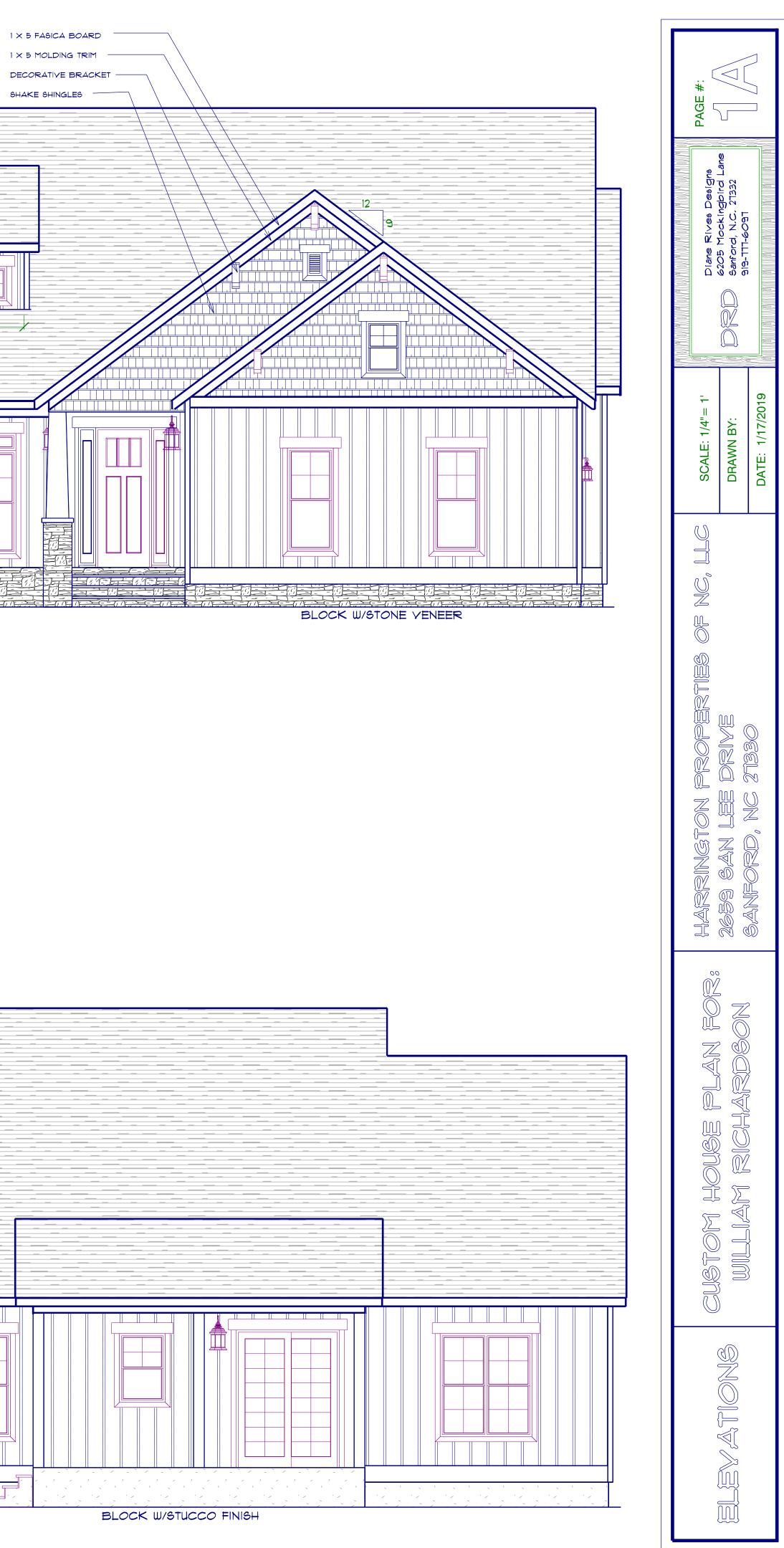
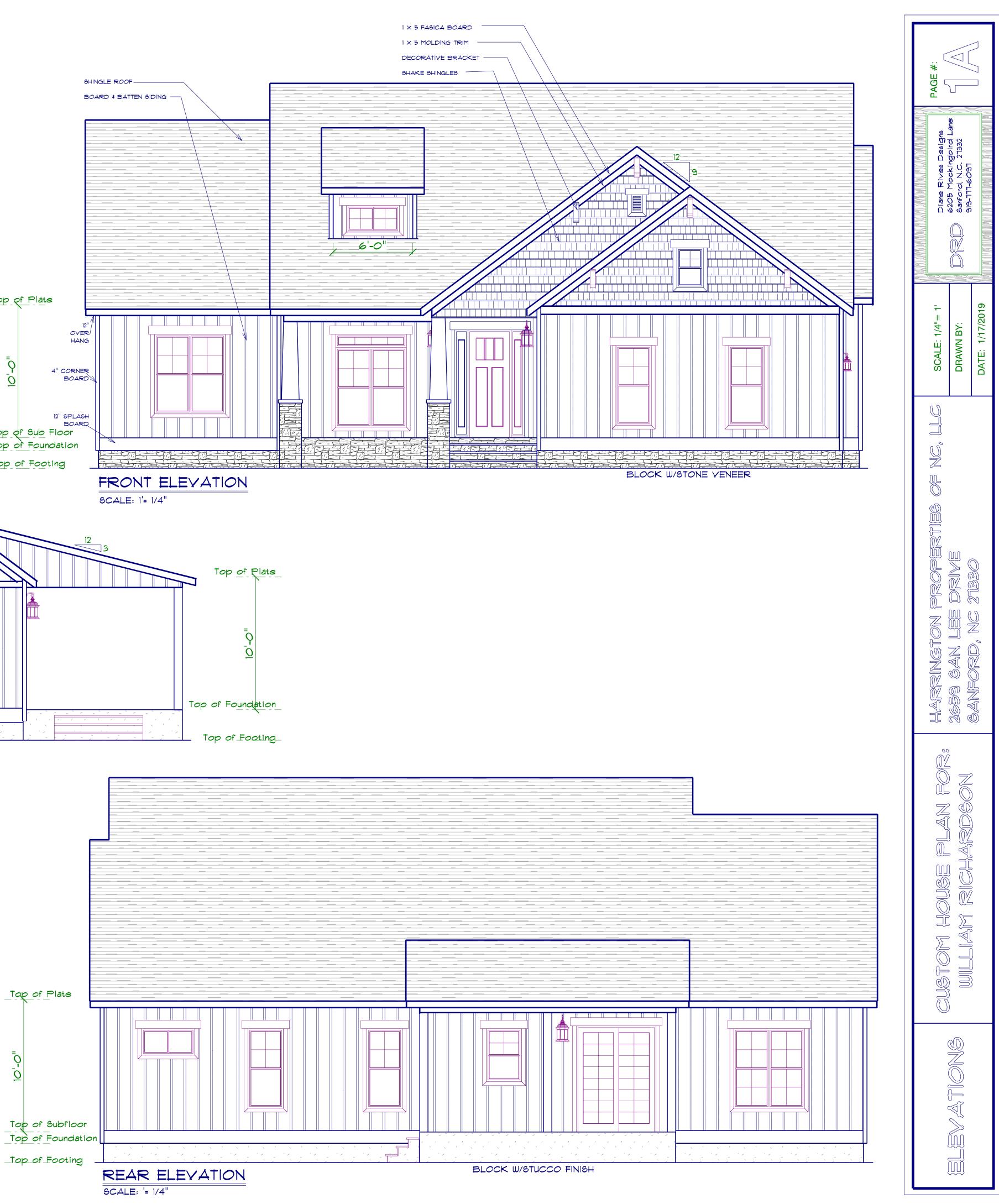




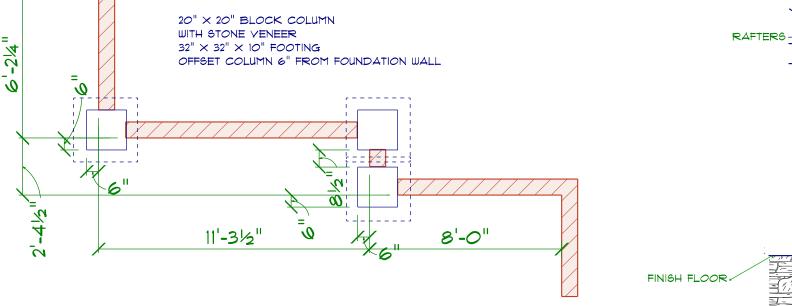
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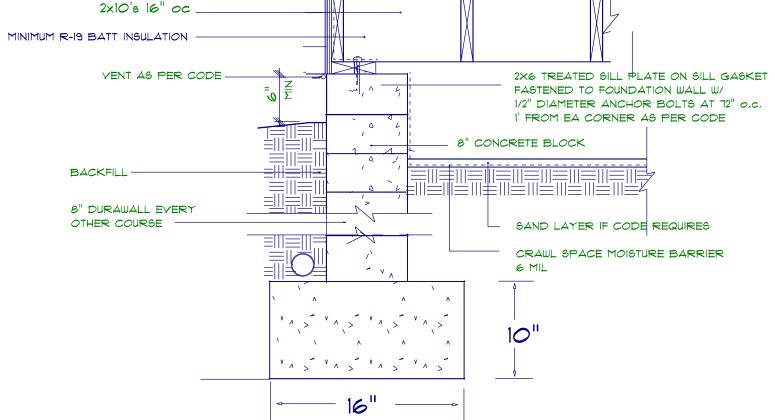


## PORCH COLUMN PLACEMENT

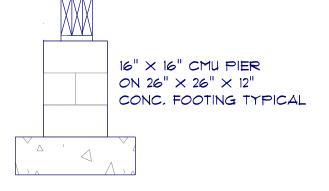








# CONCRETE BLOCK PIER DETAIL



Termite Soil Treatment: Treat entire slab area soil or crawl space surface before vapor barrier is installed and slab is poured with a state approved termiticide. Termiticide should be applied by a licensed and certified pest control professional by the state of North Carolina.

OF 6' OC AND NO MORE THAN I' FROM EA CORNER.

ALL ANCHOR BOLTS TO BE 12" LONG, 1/2" DIA. A36 UNO ANCHOR BOLTS SHALL BE SPACE AT A MAX

PROVIDE 6 MIL POLY VAPOR BARRIER TO COVER GROUND SURFACE IN CRAWL SPACE

FOOTING WIDTHS ARE BASED ON A LOAD-BEARING SOIL CAPACITY OF 2000 PSI.

FILL MIX TO HAVE 1/2" MAX AGGREGATE SIZE.

FOUNDATION CONCRETE MIX TO HAVE 1-1/2" MAX AGGREGATE SIZE, CONCRETE

PROVIDE WATER PROOFING AND PERIMETER DRAINS AS REQUIRED.

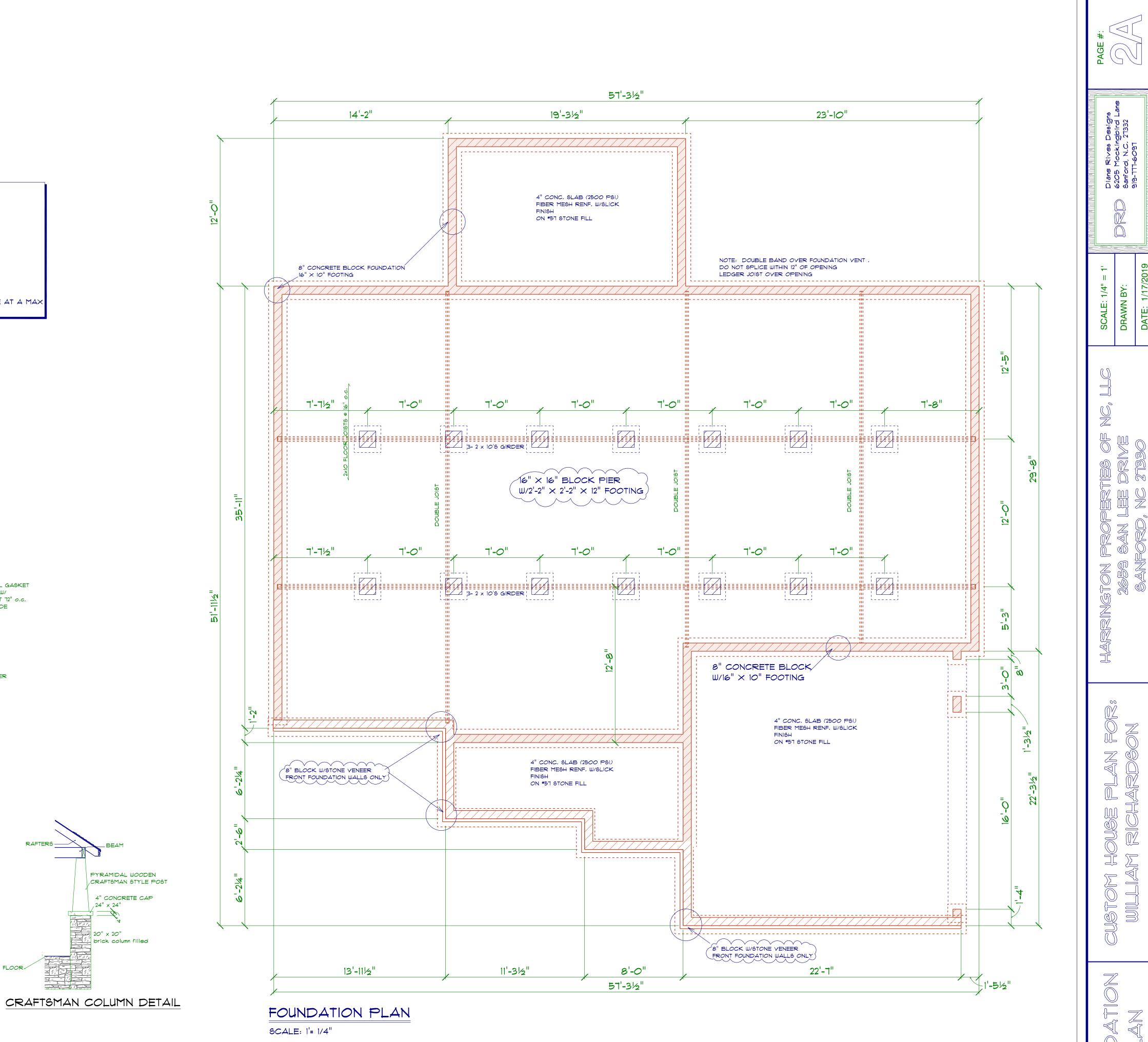
THE 28 DAY COMPRESSIVE STRENGTH OF ALL FOOTINGS IS 3000 PSI

FOUNDATION NOTES: ALL FOOTINGS SHALL BEAR ON ORIGINAL UNDISTURBED SOIL.

 $3-2 \times 10^{\circ}$  GIRDER 2 × 10'S 16" OC JOIST 2- 2 × 10'5 DBL JOIST

TYPICAL WALL: 8" BLOCK W/ 16" × 10" FOOTING





## GENERAL FRAMING NOTES:

ALL LUMBER IN CONTACT WITH CONCRETE OR MAGONRY SHALL BE PRESSURE TREATED

FRAMING LUMBER SHALL BE SYP \*2 GRADE AND/OR SPRUCE PINE FIR \*1 AND/OR \*2, KILN DRIED.

WHERE PRE-ENGINEERED JOISTS ARE USED, JOIST MANUFACTURER SHALL

PROVIDE SHOP DRAWINGS, WHICH BEAR SEAL OF A N.C. ENGINEER.

STUDS AND JOISTS SHALL NOT BE CUT TO INSTALL PLUMBING OR WIRING WITHOUT ADDING METAL OR WOOD SIDE PANELS TO STRENGTHEN THE MEMBER TO ITS ORIGINAL CAPACITY.

NAIL MULTIPLE MEMBERS WITH 2 ROWS OF 16d NAILS STAGGERED 32" OC AN USE

3-16d NAILS 2" IN AT EACH END. DOUBLE ALL STUDS UNDER ROOF POST DOWNS UNO. NAIL FLOOR JOISTS TO SILL PLATE WITH 8d TOE NAILS.

ALL EXPOSED FRAMING ON PORCHES AND DECKS SHALL BE PRESSURE TREATED.

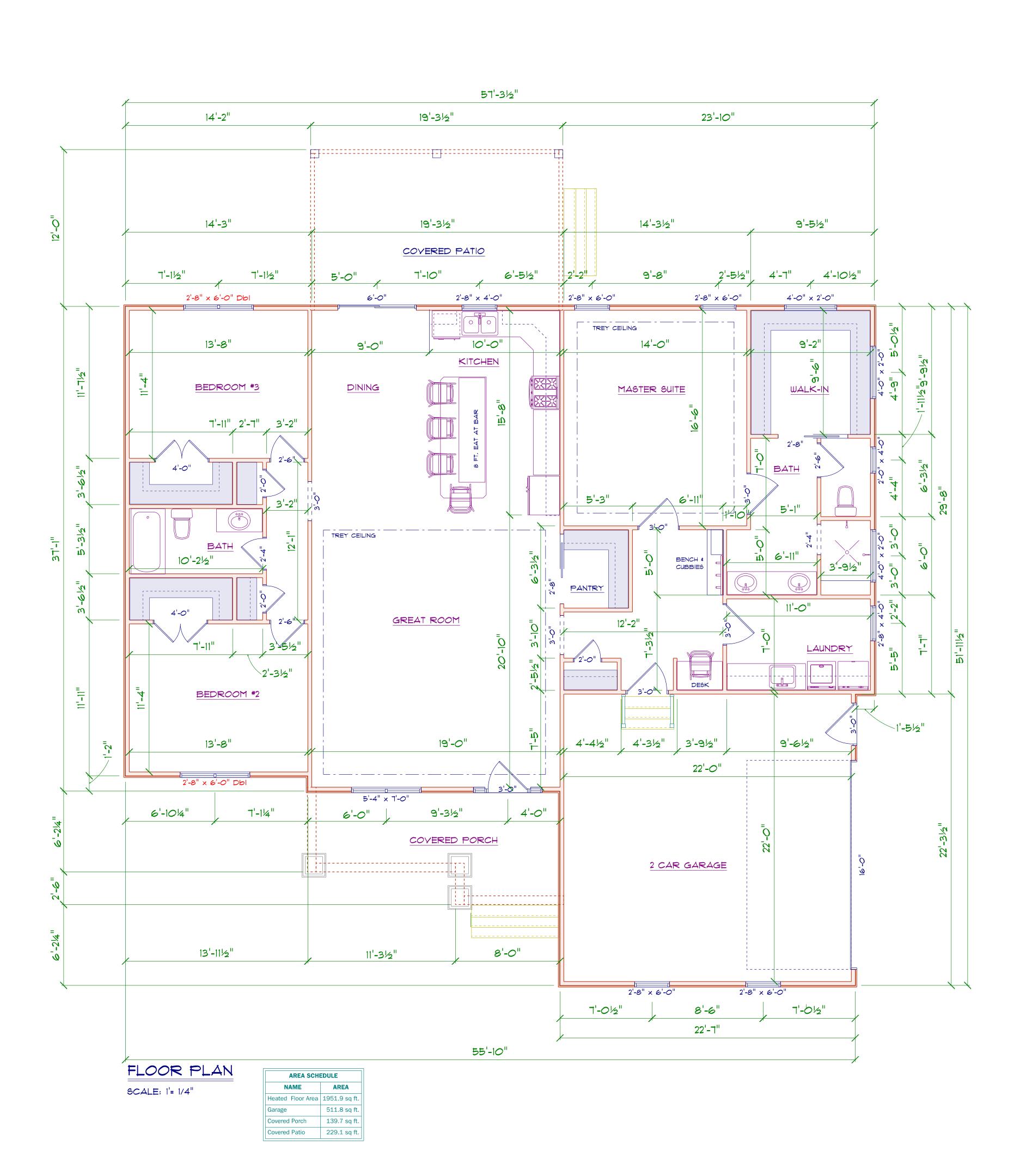
PROVIDE WATERPROOFING AND DRAINS AS REQUIRED.

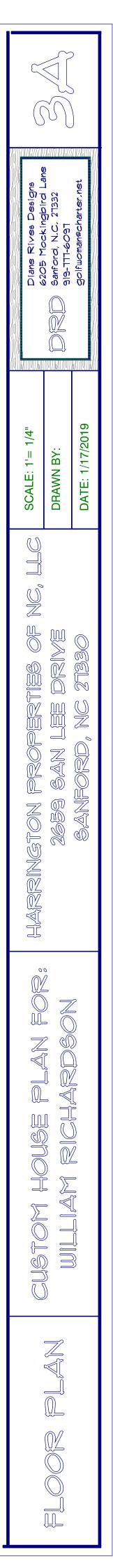
ALL FRAMING TO BE 16" OC UNO. WALL FRAMING DIMENSIONS ARE BASED ON 2 × 4 STUDS UNO. DOUBLE STUDS UNDER ALL HEADERS.

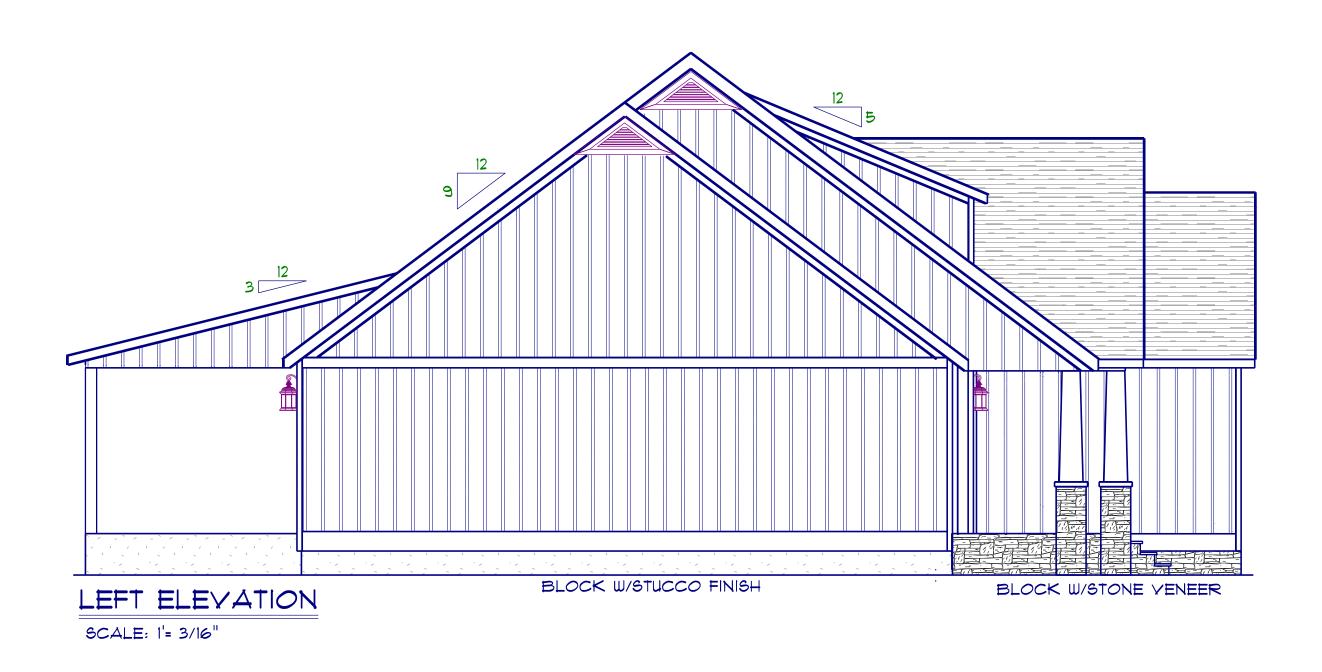
LVL'S AND TJI'S TO BE SIZED BY OTHERS

EXTERIOR WALLS IN LIVING AREAS ARE 2 × 4

			OPENING SCHEDULE	
SIZE	HINGE	COUNT	LIBRARY NAME	ТҮРЕ
3'-0"	L	1	Exterior Door\Colonial	DOOR
3'-0"	R	1	Exterior Door\Tall Exterior Door\Colonial	DOOR
6'-0"	RN	1	Exterior Door\Tall Exterior Door\Patio	SLIDING DOOF
16'-0"	U	1	Garage\Tall Garage	GARAGE
2'-0"	L	2	Interior Door\Colonial	DOOR
2'-0"	R	1	Interior Door\Colonial	DOOR
2'-4"	L	1	Interior Door\Colonial	DOOR
2'-6"	L	2	Interior Door\Colonial	DOOR
2'-6"	R	1	Interior Door\Colonial	DOOR
4'-0"	LR	2	Interior Door\Colonial	DOOR
2'-8"	N	2	Interior Door\Pocket	POCKET
3'-0"	L 2 Interior Door\Tall Interior Door\Colonial [		DOOR	
3'-0"	R	1	Interior Door\Tall Interior Door\Colonial	DOOR
1'-0" x 8'-0"	N	2	Manufacturer\Therma-Tru\American Style Collection	WINDOW
3'-0"	R	1	Manufacturer\Therma-Tru\American Style Collection	DOOR
2'-8" x 4'-0"	U	1	Window\Double Hung	WINDOW
2'-8" x 4'-0"	U	1	Window\Double Hung	WINDOW
2'-8" x 6'-0"	U	4	Window\Double Hung	WINDOW
2'-8" x 6'-0" Dbl	UU	1	Window\Double Hung	WINDOW
2'-8" x 6'-0" Dbl	UU	1	Window\Double Hung	WINDOW
2'-0" x 4'-0"	U	1	Window\Double Hung	WINDOW
2'-0" x 3'-0"	U	1	Window\Double Hung	WINDOW
5'-4" x 7'-0"	NA	1	Window\Double Hung	COMBINED UN
4'-0" x 2'-0"	NL	3	Window\Slider	WINDOW
4'-0" x 2'-0"	NL	1	Window\Slider	WINDOW





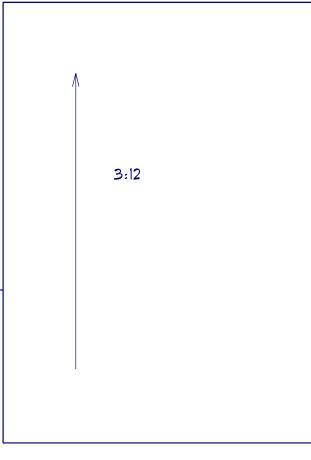


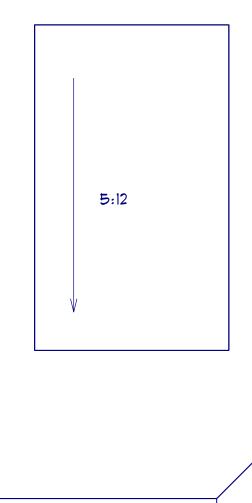
## ROOF NOTES:

TRUSSES, BRACINGS, BRIDGING AND CONNECTORS ARE TO BE DESIGNED BY THE TRUSS MANUFACTURER. IDENTIFY LUMBER BY OFFICIAL GRADE MARKINGS.

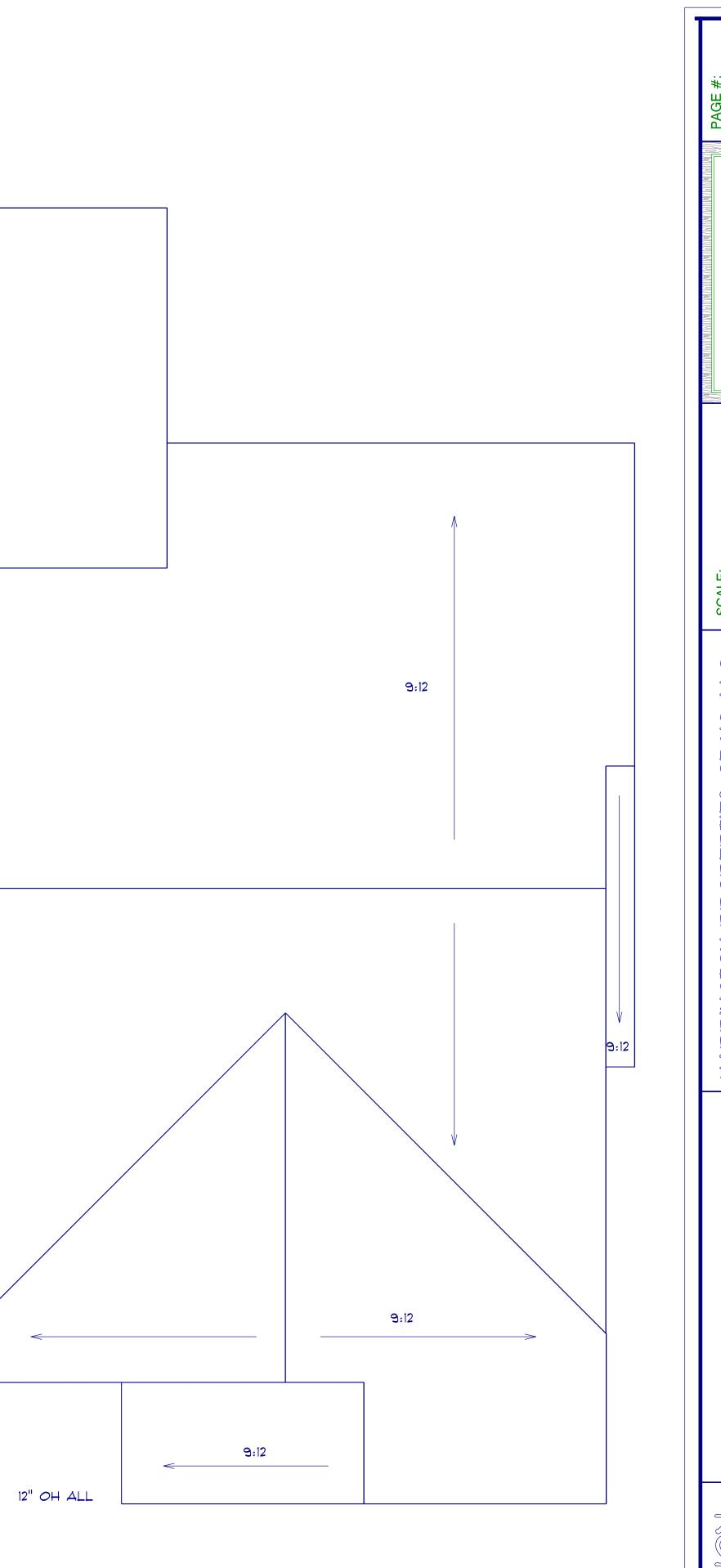
DO NOT CUT OR REMOVE CHORDS OR OTHER TRUSS MEMBERS. DO NOT NOTCH OR DRILL TRUSS MEMBERS.

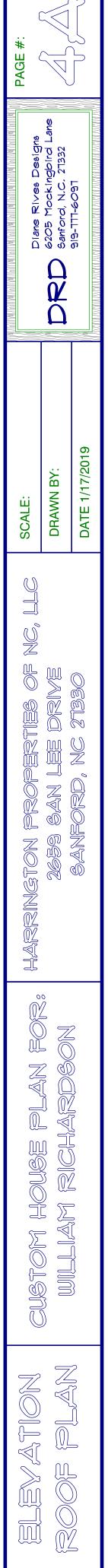
WHERE PRE-ENGINEERED ROOF TRUSSES ARE USED, TRUSS MANUFACTURER SHALL PROVIDE SHOP DRAWINGS, WHICH BEAR SEAL OF A N. C. REGISTERED ENGINEER.

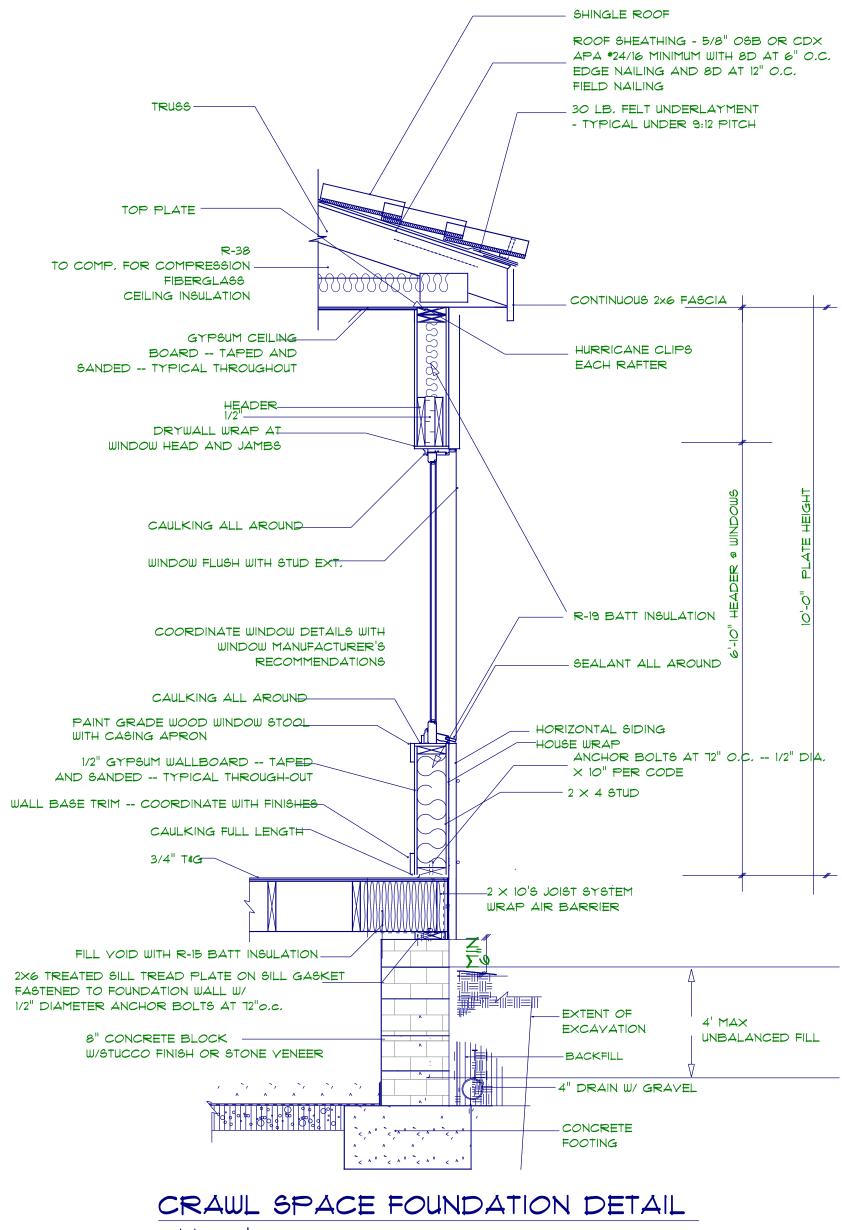




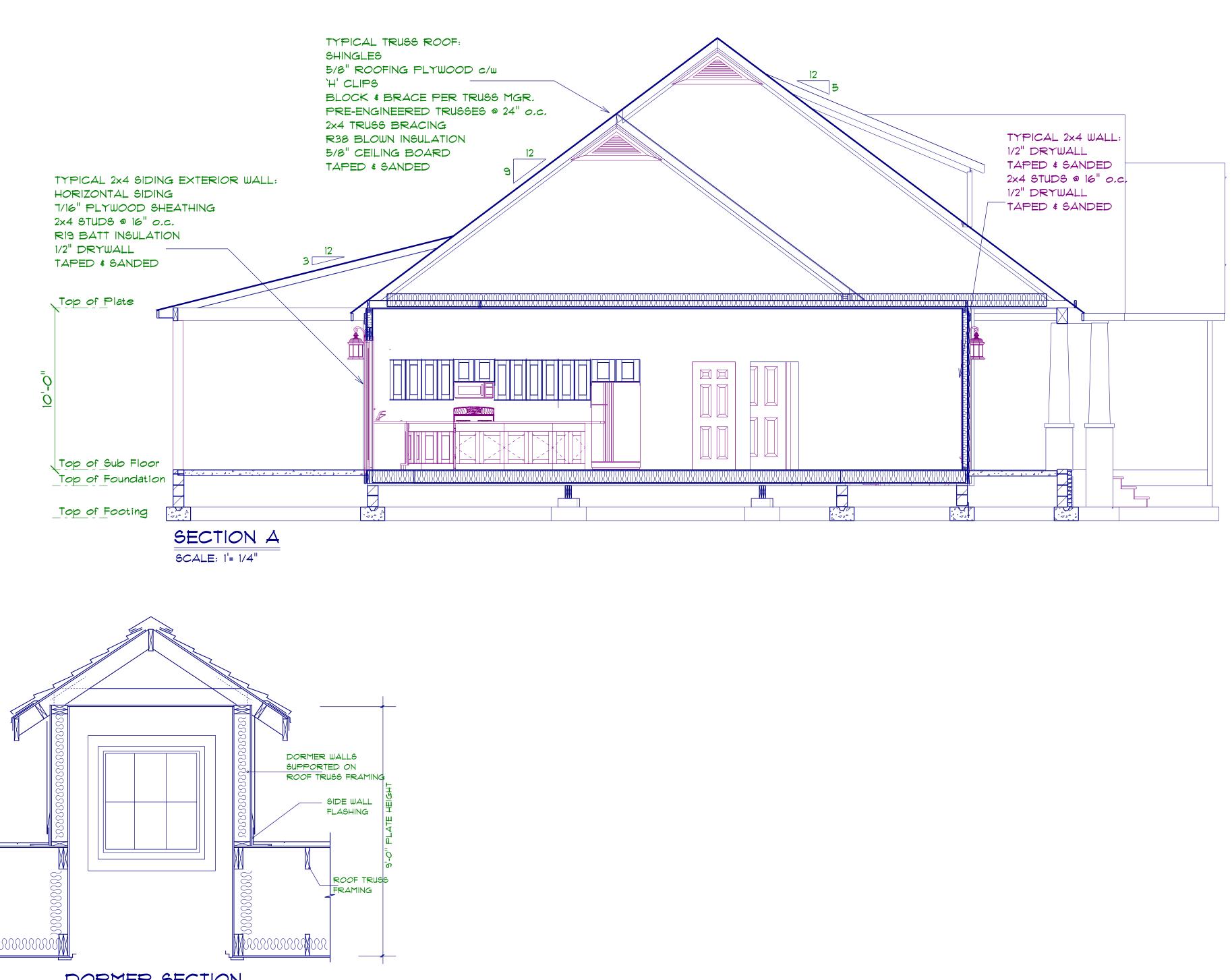
**ROOF PLAN** SCALE: 1'= 1/4"





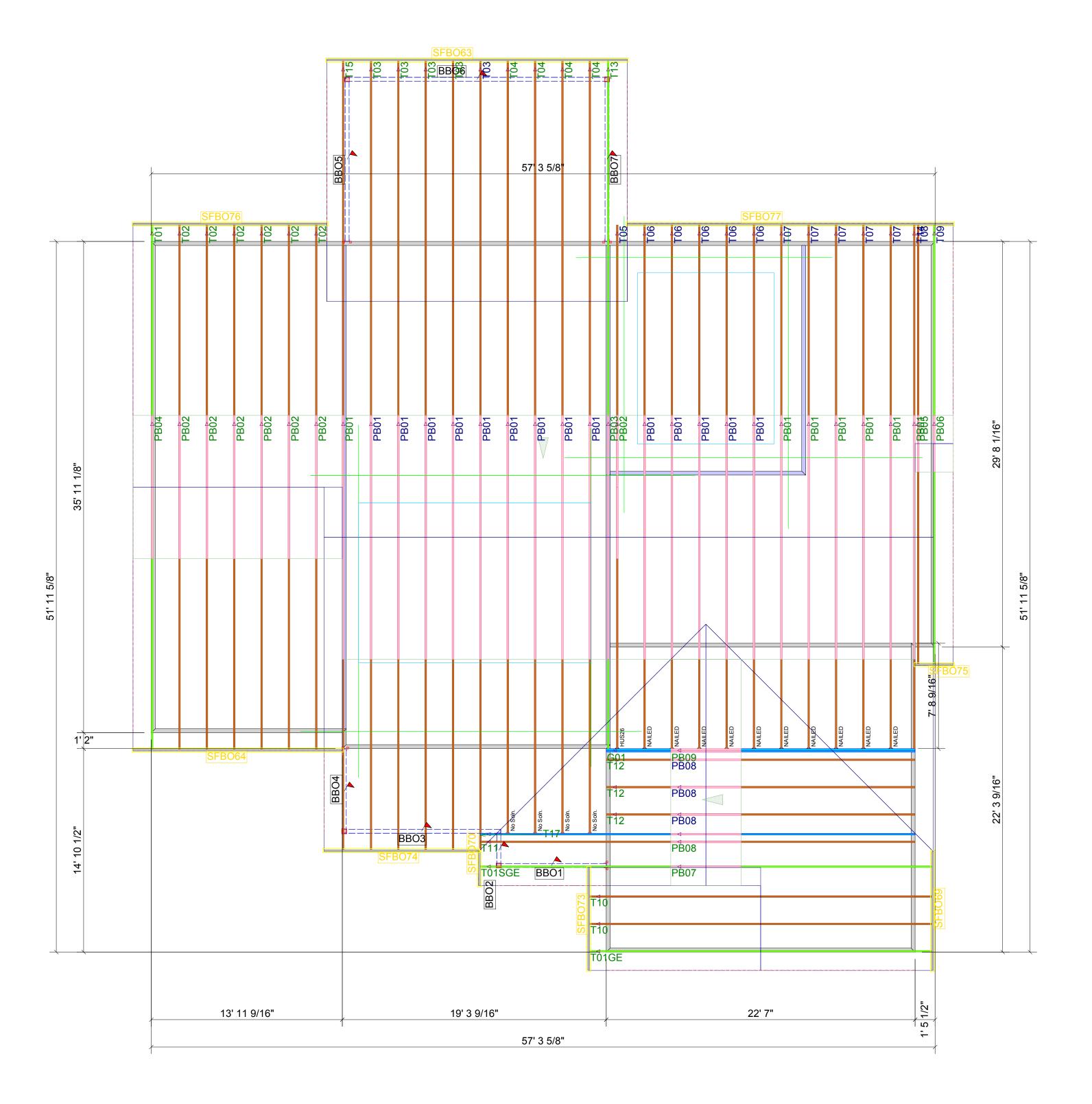


not to scale



DORMER SECTION

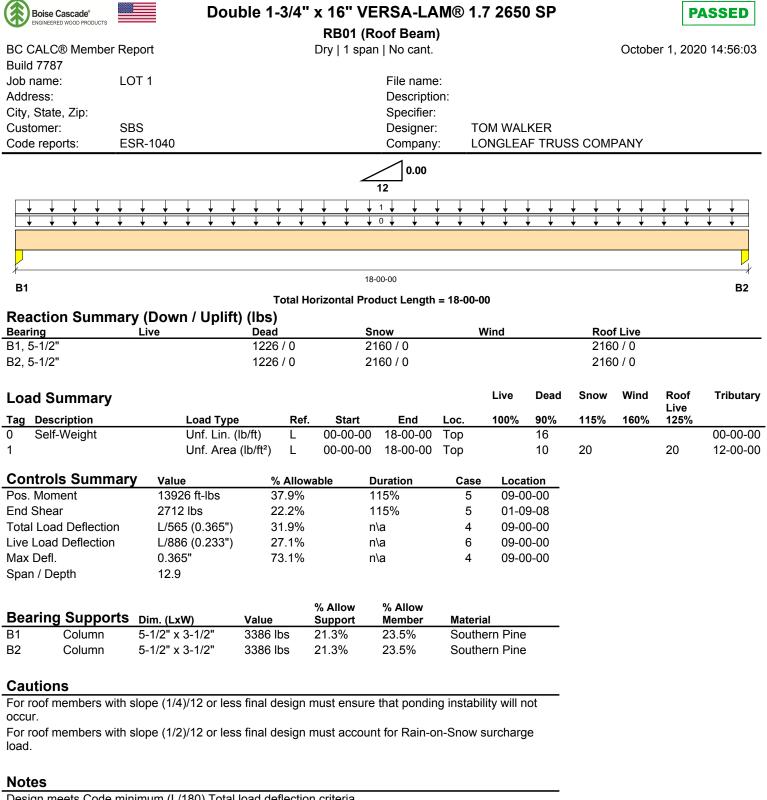




Floor Area: 0 SF Floor Plywood: 0

Roof Area: 3914.57 SF Roof Plywood: 90 sheets Roof Shingles: 49 Squares ROOF TRUSS LAYOUT

NOTE	IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR TRUSSES TO SUPPORTING STRUCTURE PERREACTIONS SHOWN ON TRUSS ENGINEERING.	SPECIAL CONSIDERATIONS FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO FABRICATION.	THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRFIBED IN WTCA1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY	FOR THE CONSTRUCTION, DESIGN, DRAWINGS, DOCUMENTS INCLUDING THE INSTALLATION AND BRACING OF TRUSSES MANUFACTURED BY THIS COMPANY. SEE http://support.sbcindustry.com/pubs/TTBDResp-D	
SANF	ONGLEAF RUSS C		Date: 4476 Hwy. 21 W West End. NC 27376	/ / (910) 673-4711	
SERVICE BUILDING SUPPLY	Project: <b>RICHARDSON</b>	Induer.	#: Designer:	P20-04027	



Design meets Code minimum (L/180) Total load deflection criteria.

Design meets Code minimum (L/240) Live load deflection criteria.

Design meets arbitrary (0.5") Maximum Total load deflection criteria.

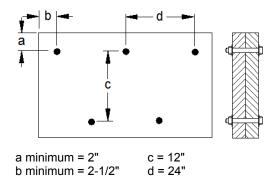
BC CALC® analysis is based on IBC 2009.

Design based on Dry Service Condition.

Calculations assume member is fully braced.

Boise Cascade*		Double 1-3/4" x 16" VERSA-LAM® 1.7	2650 SP	PASSED
	0013	RB01 (Roof Beam)	E	
BC CALC® Mem	ber Report	Dry   1 span   No cant.	October 1, 2	020 14:56:03
Build 7787				
Job name:	LOT 1	File name:		
Address:		Description:		
City, State, Zip:		Specifier:		
Customer:	SBS	Designer: TO	M WALKER	
Code reports:	ESR-1040	Company: LO	NGLEAF TRUSS COMPANY	

## **Connection Diagram: Full Length of Member**



Bolts are assumed to be Grade A307 or Grade 2 or higher. Connectors are: 1/2 in. Staggered Through Bolt

### Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER® , AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM<sup>™</sup>, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



## RE: P20-04027 - RICHARDSON

Site Information:

Project Customer: Project Name: Lot/Block: Model: Address: City:

Subdivision:

Design Program: MiTek 20/20 8.3

Design Method: User defined

Floor Load: N/A psf

Exposure Category: B

Trenco 818 Soundside Rd Edenton, NC 27932

State:

## General Truss Engineering Criteria & Design Loads (Individual Truss Design **Drawings Show Special Loading Conditions):**

Design Code: IRC2018/TPI2014 Wind Code: N/A Wind Speed: 130 mph Roof Load: 40.0 psf

Mean Roof Height (feet): 12

No.	Seal#	Truss Name	Date
1	E14419867 E14419868		5/19/20 5/19/20
2345678910	E14419869 E14419870	PB02 PB03	5/19/20 5/19/20
5	E14419871	PB04	5/19/20
6 7	E14419872 E14419873	PB06	5/19/20 5/19/20
8 9	E14419874 E14419875	PB07 PB08	5/19/20 5/19/20
10 11	E14419876 E14419877	PB09 T01	5/19/20 5/19/20
12	E14419878	T01GE	5/19/20
13 14	E14419879 E14419880	T02	5/19/20 5/19/20
16	E14419881 E14419882	T03 T04	5/19/20 5/19/20
17 18	E14419883 E14419884		5/19/20 5/19/20
19	E14419885 E14419886	T07	5/19/20
20 21	E14419887		5/19/20 5/19/20
21 22 23	E14419888 E14419889	T10 T11	5/19/20 5/19/20
25	E14419890 E14419891	T12 T13	5/19/20 5/19/20
26 27	E14419892 E14419893	T14	5/19/20
28	E14419893 E14419894	T17	5/19/20 5/19/20

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters

My license renewal date for the state of North Carolina is December 31, 2020 **IMPORTANT NOTE:** The seal on these truss component designs is a certificate that the engineer named is licensed in the jurisdiction (a) by the true that the engineer named is licensed in the jurisdiction (a) by the true that the engineer named is licensed in the jurisdiction (a) by the true that the engineer named is licensed in the jurisdiction (a) by the true that the engineer named is licensed in the jurisdiction (a) by the true that the engineer named is licensed in the jurisdiction (a) by the true the true of shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

May 19,2020

Job	Truss	Truss Type	Qty	Ply	RICHARDSON	
P20-04027	G01	Piggyback Base Girder	1	2		E14419867
Longleaf Truss Company	, West End, NC - 27376,			.330 s Ma		Tue May 19 07:20:33 2020 Page 1
	L	4-8-10 9-10-5	14-0-1	18-1-12	22-7-0	SEBSfHf8wkR0cpwmHmaF4HzF?pi
	, I	4-8-10 5-1-11	4-1-12	4-1-12	4-5-4	
	0.00	4x8 =	4x4 =			Scale = 1:67.7
	9.00 12 I	$2_{15} \boxtimes 16$	3 #1			T
			3x4 🕅			
	3x4 1/		4 3x4	*		
	I M			17		
		.    \\		$\backslash$		0
	11-0-0				3x4 1 6 x 18	11-0-0
	7-5-8					
	P				3x4 🕅 7	
				0.0	-5- -5- -8-	
	⊠ 19 14		11 <sup>23</sup> <sup>24</sup> 10 <sup>25</sup>	26	9 27 28 8	
	2x4    HUS2		8 = NAILED 3x4 = NAILED NAILE		Ax4 = NAILED 2x4    NAILED	
		4-8-10 9-10-5		18-1-12	1 22-7-0	
Plate Offsets (X,Y) [	1:0-1-12.0-1-8]. [2:0-6-0.0-2-	4-8-10 5-1-11 D], [5:0-1-12,0-1-8], [6:0-1-12,0-1-8]	4-1-12	4-1-12	4-5-4	
LOADING (psf)	SPACING-	2-0-0 <b>CSI</b> .	DEFL.	in	l (loc) l/defl L/d	PLATES GRIP
TCLL (roof) 20. Snow (Pf/Pg) 16.5/15.0	Plate Grin DC		35 Vert(LL)	-0.03	10-11 >999 240 10-11 >999 180	MT20 244/190
TCDL 10. BCLL 0.	n * Rep Stress In	cr NO WB 0.3				Woight: $415 \text{ lb}$ ET = $20\%$
BCDL 10.	Code IRC20	8/TPI2014 Matrix-S				Weight: 415 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP			BRACING- TOP CHORD		ed or 6-0-0 oc purlins, except	end verticals, and 2-0-0 oc
BOT CHORD 2x6 SP WEBS 2x4 SP			BOT CHORD		(6-0-0 max.): 2-3. eiling directly applied or 6-0-0	oc bracing.
REACTIONS. (size	) 14=0-3-8, 8=0-3-8					
	orz 14=-282(LC 10) lift 14=-212(LC 12), 8=-190(	LC 12)				
Max Gr	av 14=2757(LC 34), 8=1794	(LC 34)				
( )		250 (lb) or less except when shown 1320/256, 5-6=-1799/261, 6-7=-198				
1-14=	-1748/212, 7-8=-1631/186 =-57/722, 10-11=-59/1363, 9					
WEBS 2-13=	-348/129, 2-11=-117/641, 3-	1=-180/510, 5-11=-809/163, 5-10=-	-151/620,			
	-270/88, 1-13=-147/1239, 7-9	=-113/1484				
NOTES- 1) 2-ply truss to be conr	nected together with 10d (0.1	31"x3") nails as follows:				
	d as follows: 2x4 - 1 row at 0 octed as follows: 2x6 - 2 rows					
	ollows: 2x4 - 1 row at 0-9-0 or red equally applied to all plie	c. s, except if noted as front (F) or bac	k (B) face in the LOAD (	CASE(S)	section. Ply to	
ply connections have		only loads noted as (F) or (B), unless				"TH CARO
4) Wind: ASCE 7-16; Vi	ult=130mph (3-second gust)	/asd=103mph; TCDL=6.0psf; BCDI rer left and right exposed ; end verti				R. EESSIG: 24
plate grip DOL=1.60		=1.15 Plate DOL=1.15); Pg=15.0 p			CH.	et Turc
DOL=1.15); Is=1.0; F	ough Cat B; Partially Exp.; C	e=1.0; Cs=1.00; Ct=1.10, Lu=50-0-				SEAL
6) Unbalanced snow loa	less than 0.500/12 in accord ds have been considered fo	this design.			vposed v 2-0-0 wide	036322
8) This truss has been of		n chord live load nonconcurrent with				
		0.0psf on the bottom chord in all are embers, with BCDL = 10.0psf.	as where a rectangle 3-	6-0 tall by	/ 2-0-0 wide	WGINEER ON ST
10) Provide mechanical 14=212, 8=190.	connection (by others) of tru	ss to bearing plate capable of withs	tanding 100 lb uplift at jo	oint(s) exc	cept (jt=lb)	CA GILBE
11) This truss is design referenced standard		18 International Residential Code se	ections R502.11.1 and F	802.10.2	and	May 10 2020
		ne size or the orientation of the purli	in along the top and/or b	ottom cho	ord.	May 19,2020
🛕 WARNING - Verify de		ON THIS AND INCLUDED MITEK REFERENCE				ENGINEERING BY
a truss system. Before us	e, the building designer must verify	is based only upon parameters shown, and i he applicability of design parameters and pro lividual truss web and/or chord members only	perly incorporate this design ir	to the overa	11	<b>TRENCO</b>
is always required for sta fabrication, storage, deliv	bility and to prevent collapse with po ery, erection and bracing of trusses	ssible personal injury and property damage. I and truss systems, see ANSI/TPI1 0	For general guidance regardin Quality Criteria, DSB-89 and I	g the		A MiTek Affiliate 818 Soundside Road
Salety mormation ava	nadio nom muss riale institute, 218	N. Lee Street, Suite 312, Alexandria, VA 223	17.			Edenton, NC 27932

[	Job	Truss	Truss Type	Qty	Ply	RICHARDSON
						E14419867
	P20-04027	G01	Piggyback Base Girder	1	2	
					<b>_</b>	Job Reference (optional)
	Longleaf Truss Company,	West End, NC - 27376,		8.3	330 s Mar	23 2020 MiTek Industries, Inc. Tue May 19 07:20:33 2020 Page 2
			ID:gkUVx	24QO5glB	KvEjRcTh	WzFI?4-tOcX8xu2wazwHzOB6EBSfHf8wkR0cpwmHmaF4HzF?pi

#### NOTES-

13) Use Simpson Strong-Tie HUS26 (14-10d Girder, 4-10d Truss) or equivalent at 0-9-10 from the left end to connect truss(es) to back face of bottom chord.

14) Fill all nail holes where hanger is in contact with lumber.

15) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

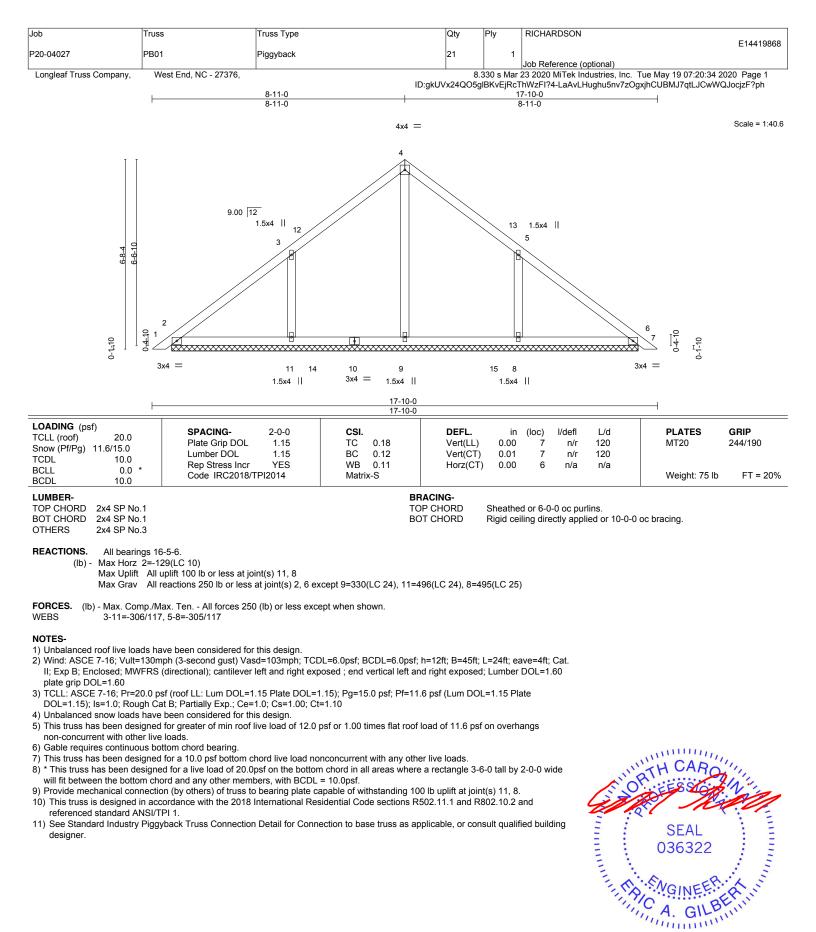
Uniform Loads (plf) Vert: 1-2=-43, 2-3=-53, 3-7=-43, 8-14=-20

Concentrated Loads (lb)

Vert: 12=-152(B) 13=-152(B) 19=-1106(B) 20=-152(B) 22=-152(B) 23=-152(B) 24=-164(B) 25=-164(B) 25=-164(B) 27=-164(B) 28=-164(B) 28=-

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses safe truss systems, see **ANSVTPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



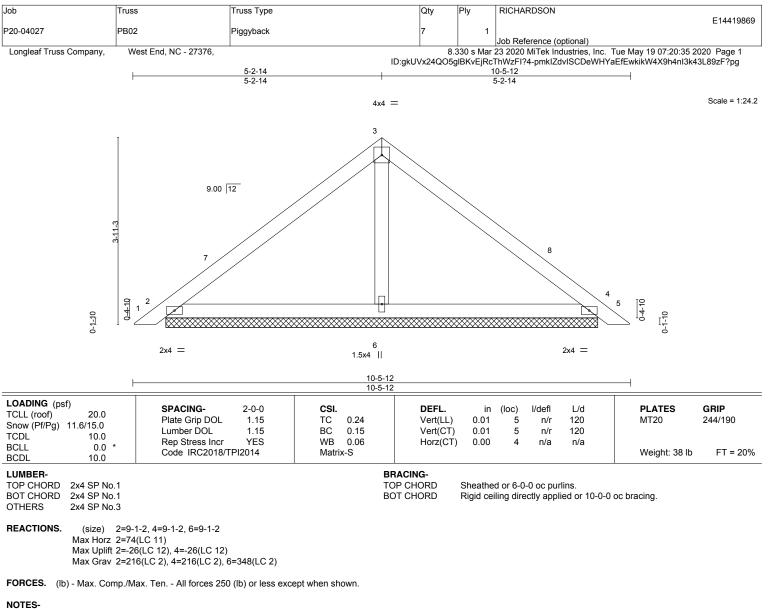


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANST/PTI Quality criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. RENCO

May 19,2020



Edenton, NC 27932

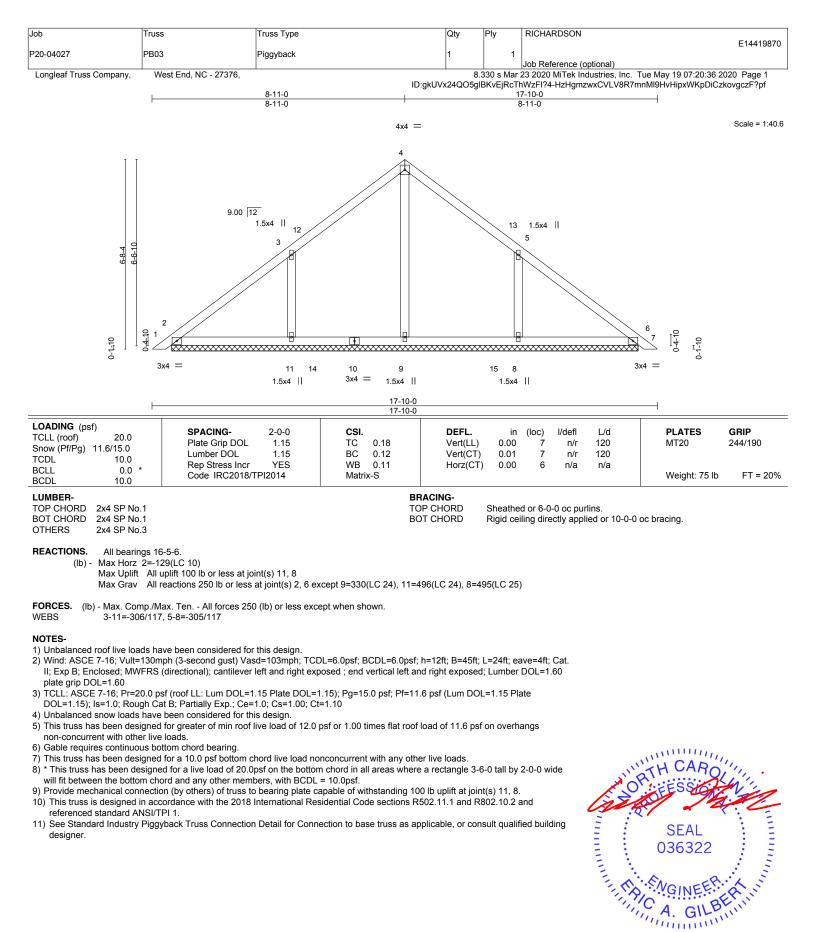


- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



818 Soundside Road Edenton, NC 27932

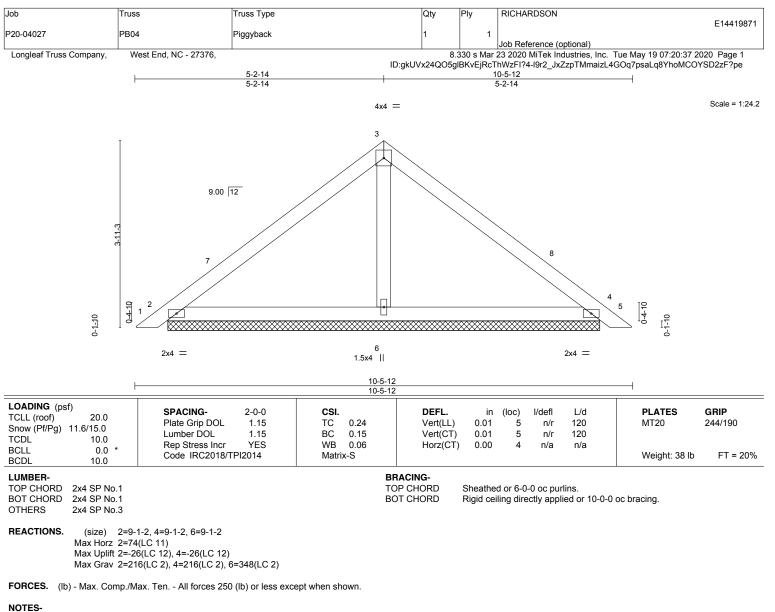
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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A MiTek At

May 19,2020

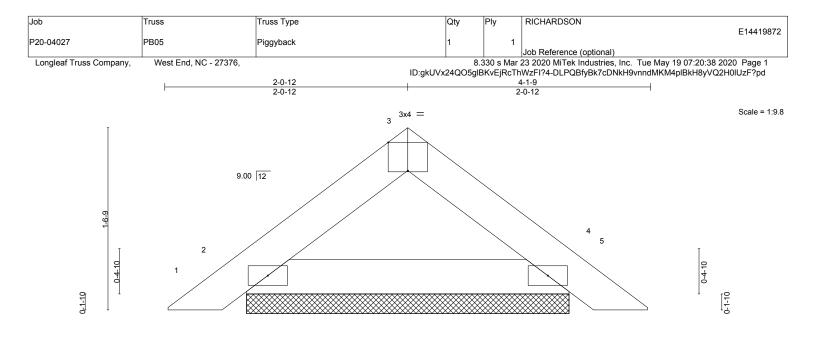


- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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2x4 =

2x4 =

Sheathed or 4-1-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.02 BC 0.06 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 4 4 4	l/defl n/r n/r n/a	L/d 120 120 n/a	<b>PLATES</b> MT20	<b>GRIP</b> 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2018/TPI2014	Matrix-P						Weight: 12 lb	FT = 20%

TOP CHORD

BOT CHORD

4-1-9 4-1-9

LUMBER-

TOP CHORD 2x4 SP No 1 BOT CHORD 2x4 SP No.1

REACTIONS. (size) 2=2-8-14, 4=2-8-14 Max Horz 2=-27(LC 10) Max Uplift 2=-11(LC 12), 4=-11(LC 12) Max Grav 2=136(LC 2), 4=136(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

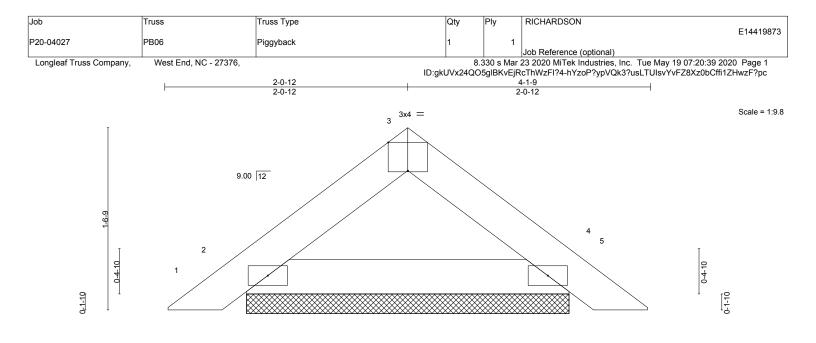
#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4. 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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2x4 =

2x4 =

Sheathed or 4-1-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD

BOT CHORD

LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.02 BC 0.06 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 4 4 4	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2018/TPI2014	Matrix-P						Weight: 12 lb	FT = 20%

4-1-9 4-1-9

LUMBER-

TOP CHORD 2x4 SP No 1 BOT CHORD 2x4 SP No.1

REACTIONS. (size) 2=2-8-14, 4=2-8-14 Max Horz 2=-27(LC 10) Max Uplift 2=-11(LC 12), 4=-11(LC 12) Max Grav 2=136(LC 2), 4=136(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

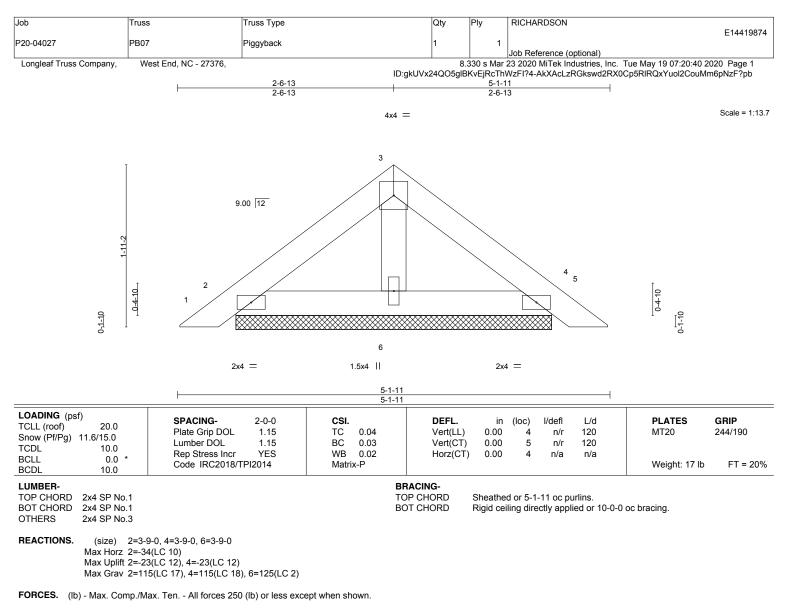
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4. 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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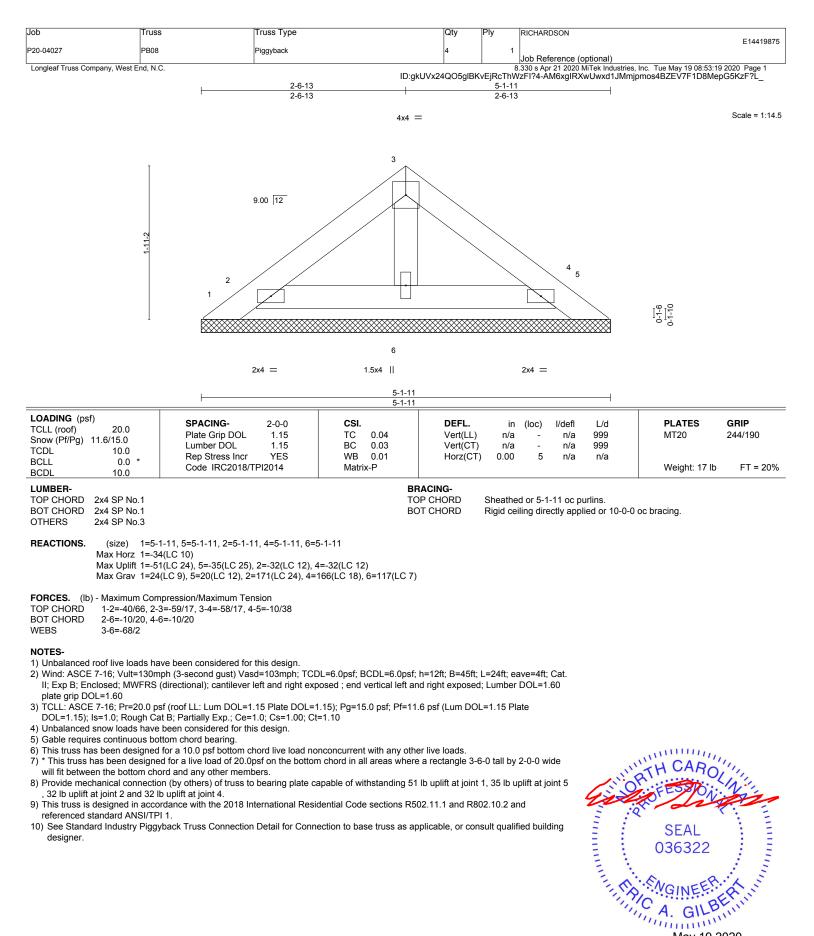
#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



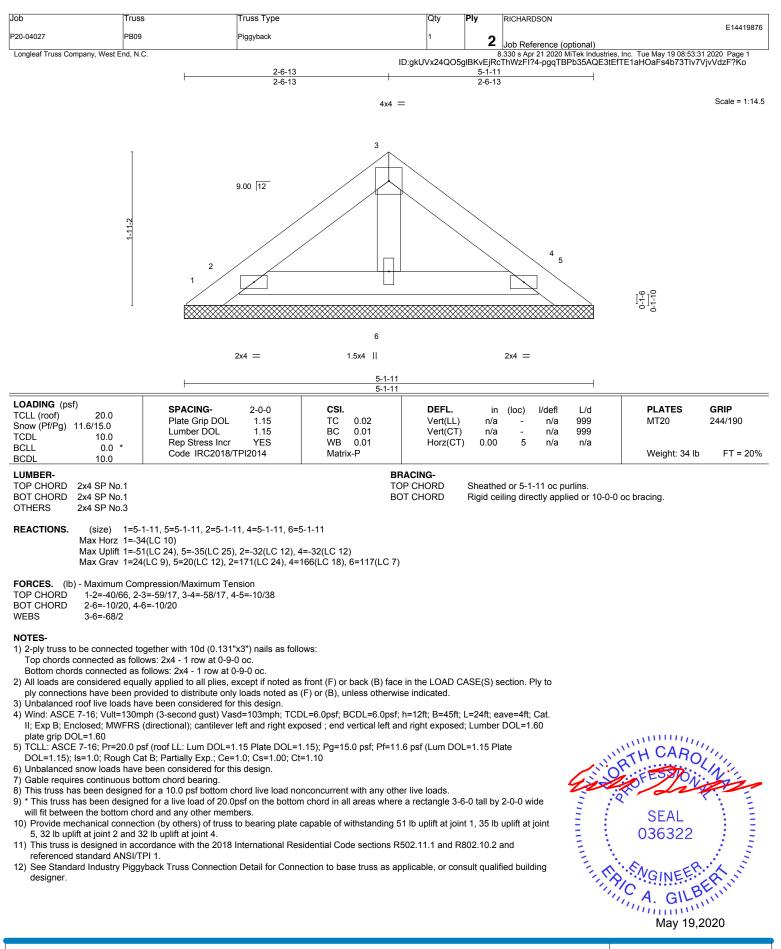
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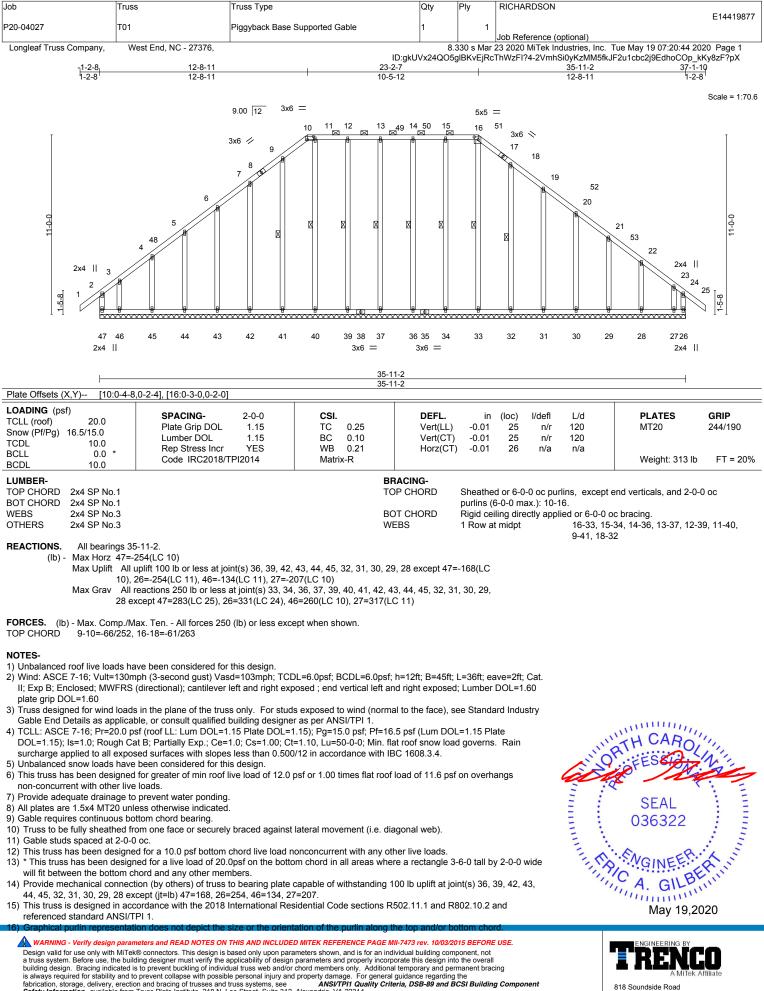


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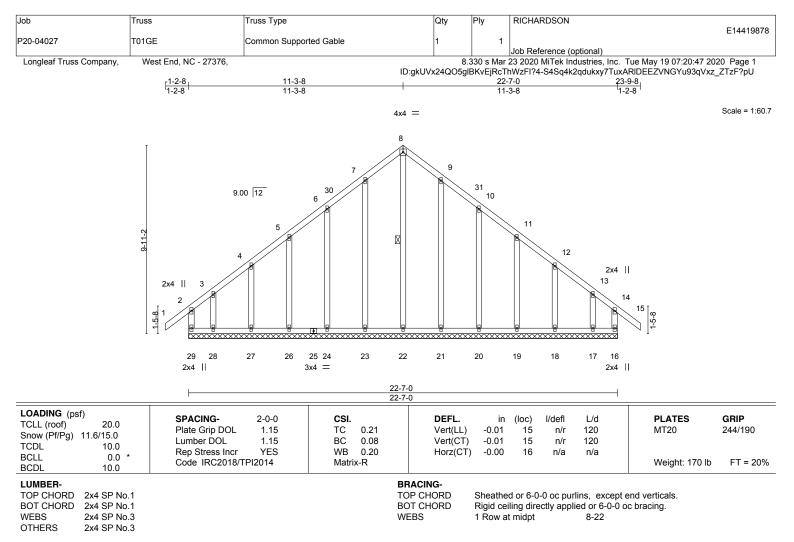




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fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Que** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



#### REACTIONS. All bearings 22-7-0.

(lb) - Max Horz 29=220(LC 11)

 Max Uplift
 All uplift 100 lb or less at joint(s) 16, 23, 24, 26, 27, 21, 20, 19, 18, 17 except 29=-125(LC 10), 28=-101(LC 11)

 Max Grav
 All reactions 250 lb or less at joint(s) 29, 16, 23, 24, 26, 27, 28, 21, 20, 19, 18, 17 except

22=265(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

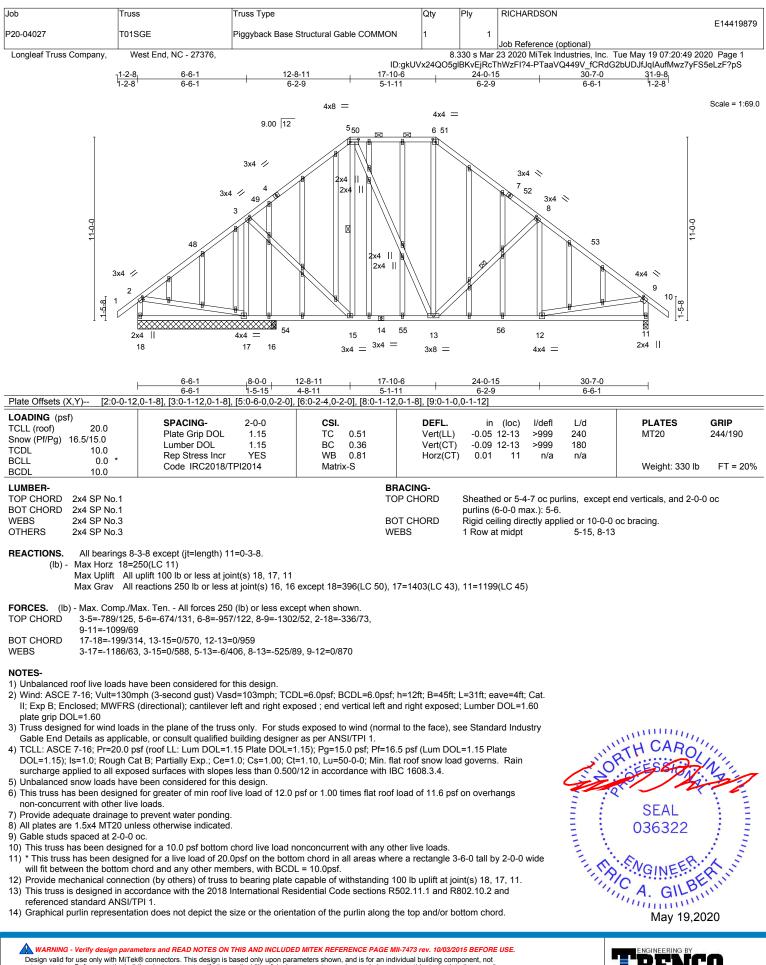
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- All plates are 1.5x4 MT20 unless otherwise indicated.
- B) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 23, 24, 26, 27, 21, 20, 19, 18, 17 except (jt=lb) 29=125, 28=101.
  14) This is designed in accordance with the 2019 laterational Besidential Cade sections D502 11 (1 and D802 10 and 10 and
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



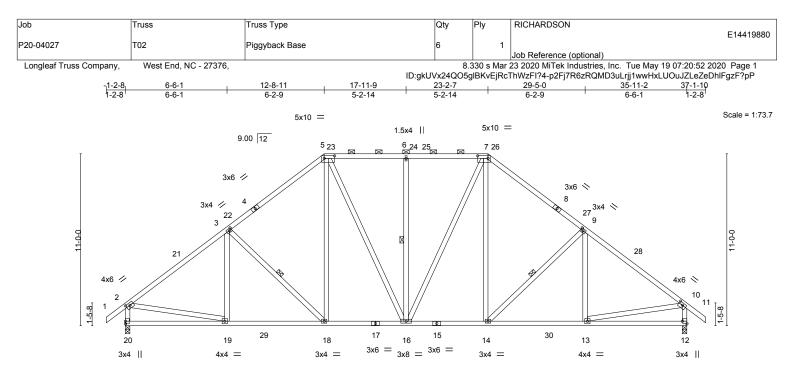
TRENCO A MiTek Atfiliate 818 Soundside Road

Edenton, NC 27932

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	6-6-1	12-8-11	17-11-9	23-2-7	29-5-0	35-11-2	
	6-6-1	6-2-9	5-2-14	5-2-14	6-2-9	6-6-1	
Plate Offsets (X,Y)							

LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.52 BC 0.43 WB 0.56 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.08 13-14 >999 -0.15 13-14 >999 0.05 12 n/a	L/d 240 180 n/a	<b>PLATES</b> MT20 Weight: 265 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WEBS 2x4 SP No.3	-3-8, 12=0-3-8	то	T CHORD	Sheathed or 4-3-13 oc pu purlins (5-2-5 max.): 5-7. Rigid ceiling directly appli 1 Row at midpt	<i>,</i> ,	oc bracing.	-0 oc

Max Horz 20=254(LC 11) Max Uplift 20=33(LC 12), 12=-33(LC 12) Max Grav 20=1734(LC 24), 12=1734(LC 25)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1962/49, 3-5=-1702/118, 5-6=-1387/128, 6-7=-1387/128, 7-9=-1702/118, 9-10=-1962/49, 2-20=-1632/66, 10-12=-1632/66

BOT CHORD 19-20=-193/342, 18-19=0/1649, 16-18=0/1378, 14-16=0/1317, 13-14=0/1483 WEBS 3-18=-442/90, 5-18=0/545, 5-16=-30/396, 6-16=-491/59, 7-16=-30/396, 7-14=0/546, 9-14=-442/90, 2-19=0/1387, 10-13=0/1390

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=36ft; eave=5ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

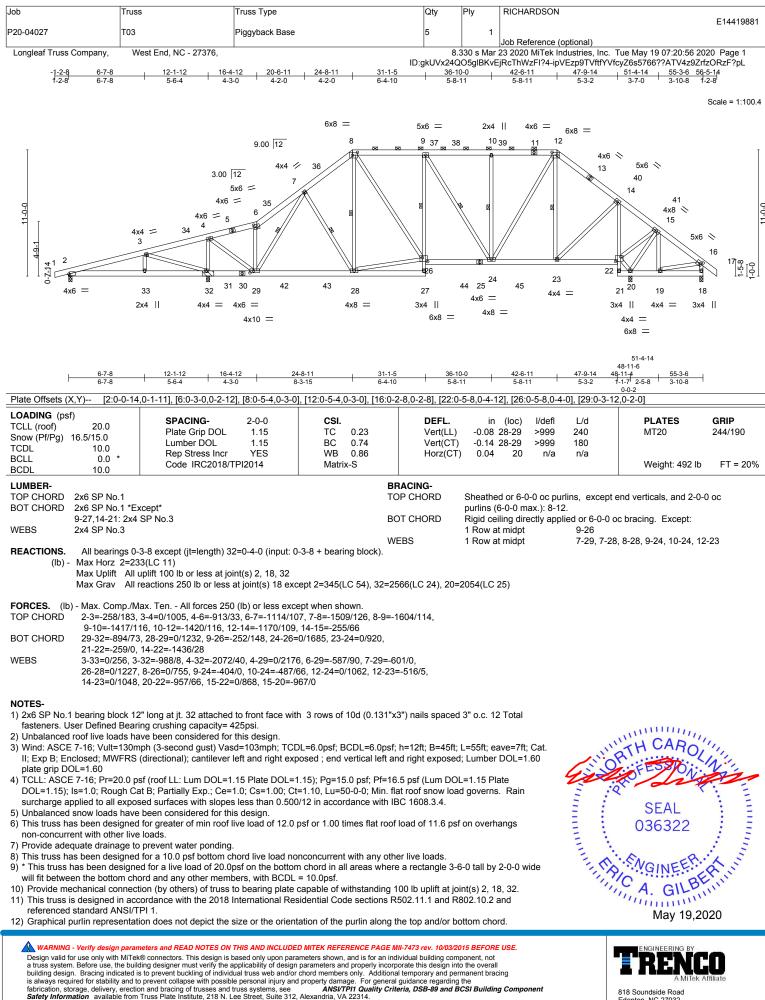
6) Provide adequate drainage to prevent water ponding.

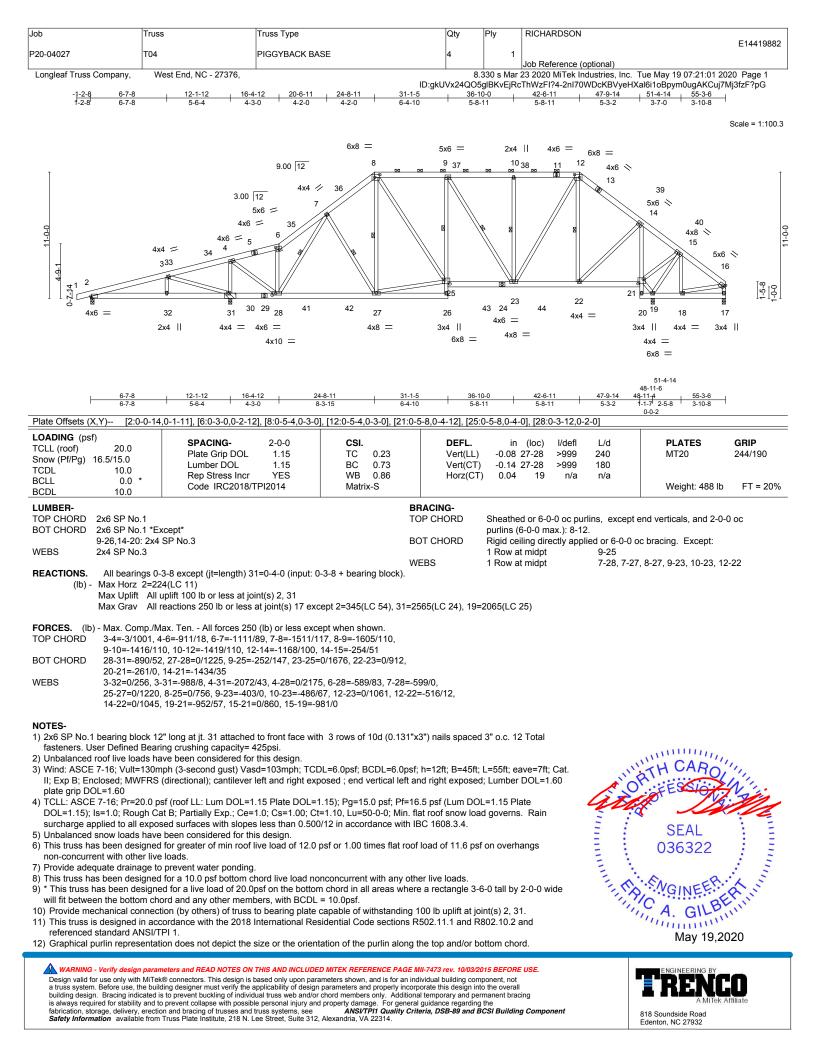
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20, 12.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

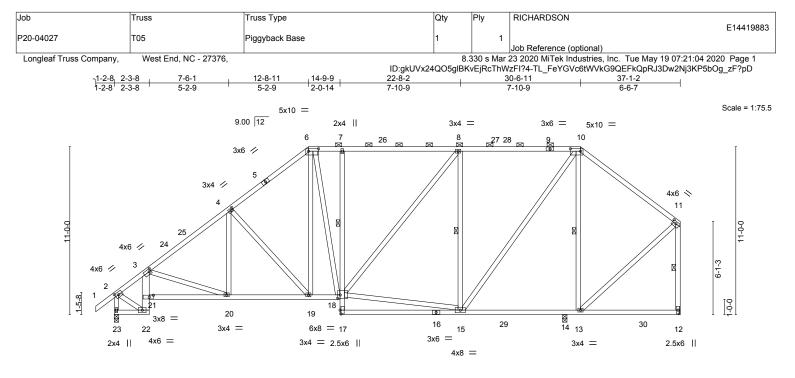


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	2-3-8	7-6-1	12-8-11	14-9-9		2-8-2	1		-8-1	30 <sub>0</sub> -6-		37-1-2		
	2-3-8	5-2-9	5-2-9	2-0-14		-10-9			1-15	0-10-	-	6-6-7		
Plate Offsets (2	X,Y) [2:0-2-1/	4,0-2-0], [3:0-1-8,0-2	-0], [4:0-1-12,0-1-8]	, [6:0-8-0,0-	2-0], [10:0	<u>-8-0,0-2-0], [1</u>	1:0-3-0	),0-1-12],	[17:0-3	-0,0-0-12	], [18:0-2-0	0,0-2-12	], [21:0-3-4,0-1-8]	
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL	20.0	SPACING- Plate Grip DO Lumber DOL Rep Stress Ir	1.15	CSI. TC BC WB	0.86 0.69 0.73	Vei Vei	FL. rt(LL) rt(CT) rz(CT)	-0.13 1 -0.27 1		l/defl >999 >999 n/a	L/d 240 180 n/a		PLATES MT20	<b>GRIP</b> 244/190
BCLL BCDL	0.0 * 10.0	Code IRC20		Matr		10	12(01)	0.11	12	n/a	11/a		Weight: 301 lb	FT = 20%
LUMBER-						BRACING-								
TOP CHORD 2x4 SP No.1 TOP CHORD Sheathed or 4-3-9 oc purlins, except end verticals, and 2-0-0 oc									) oc					
BOT CHORD	CHORD 2x4 SP No.1 *Except* purlins (3-11-2 max.): 6-10.													
WEBS	3-22: 2x6 SP No.1, 7-17: 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0 VEBS 2x4 SP No.3 6-0-0 oc bracing: 21-22.						-0 oc bra	acing, Except:						
						WEBS		1 Row at			7-18 8-15 10	7-13 11.	-12	
REACTIONS.														
FORCES (Ib	) - Max Comp /	Max. Ten All forces	250 (lb) or less exc	ent when st	าดพท									
TOP CHORD	2-3=-1384/36 8-10=-1261/1	, 3-4=-2086/58, 4-6= 34, 10-11=-859/119,	-1694/117, 6-7=-14 2-23=-1653/48, 11-	15/126, 7-8 12=-1108/5	=-1416/12 <sup>:</sup> 8	,								
BOT CHORD		32, 21-22=-539/0, 3- 77, 7-18=-484/63, 14			19-20=-47	/1723,								
WEBS	WEBS 4-20=0/288, 4-19=-620/81, 6-19=-5/620, 6-18=-32/549, 15-18=-19/1119, 8-18=-22/390, 8-15=-850/90, 10-15=0/1136, 10-13=-770/39, 2-22=0/1161, 11-13=0/822													
NOTES-	NOTES-													
	1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=37ft; eave=5ft; Cat.													

- II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 23, 12.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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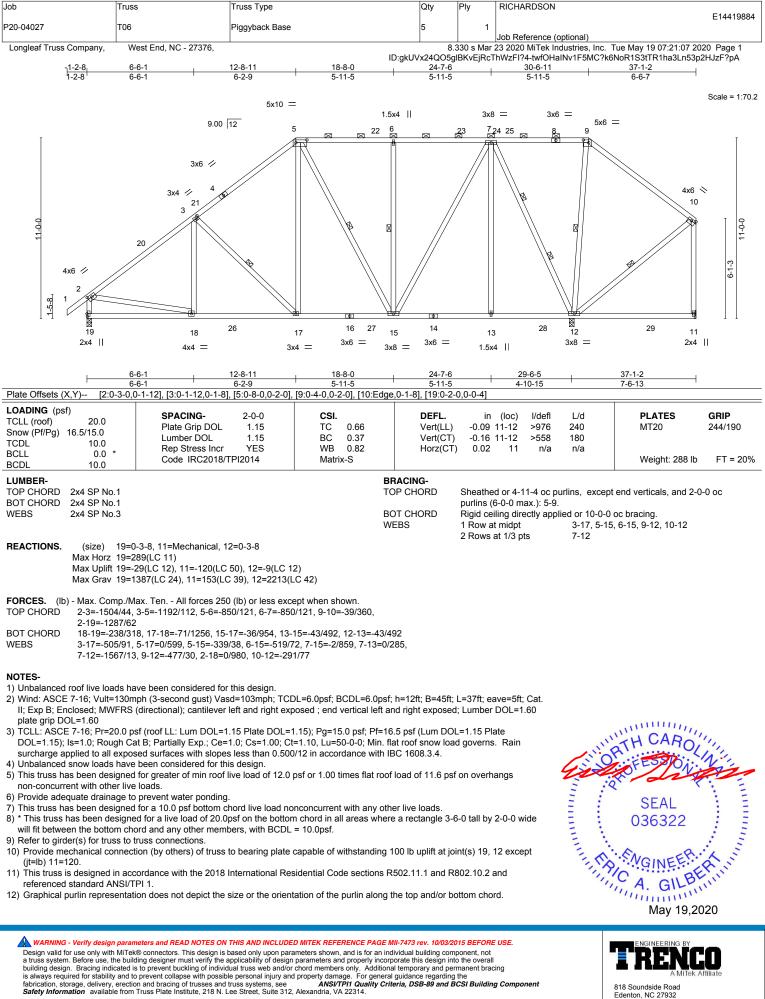
Contraction of the

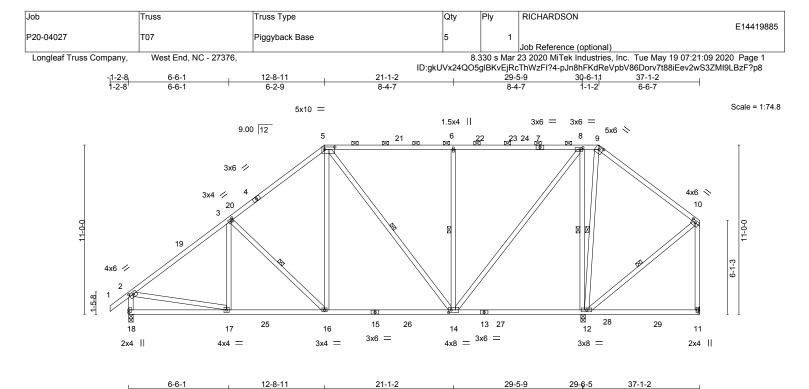
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	6-6-1	6-2-9	1	8-4-7		8-4-7	7	0-0-	11	7-6-13	1	
Plate Offsets (X,Y) [2:0-3-0,	,0-1-12], [3:0-1-12,0-1-8	, [5:0-8-0,0-2-0],	[8:0-2-4,0-1	-8], [9:0-3-0	,0-2-2], [10:0-3-0,0	)-1-12], [14	4:0-2-0	),0-2-0]				
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/	2-0-0 1.15 1.15 YES TPI2014	<b>CSI.</b> TC BC WB Matrix	1.00 0.58 0.97 <-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.14 1: -0.23 14 0.02		l/defl >999 >999 n/a	L/d 240 180 n/a		PLATES MT20 Weight: 275 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-				в	RACING-							
TOP CHORD 2x4 SP No.1				Т					ourlins, e	xcept end	verticals, and 2-	0-0 oc
BOT CHORD 2x4 SP No.1				_		purlins (2-		,				
WEBS 2x4 SP No.3				В		Rigid ceilii 6-0-0 oc b	0	2 11	ed or 10-	-0-0 oc bra	acing, Except:	
				W	/EBS	1 Row at r	midpt		3-16, 5	5-14, 6-14	, 8-12, 9-12, 10- <sup>-</sup>	12

REACTIONS. (size) 18=0-3-8, 11=Mechanical, 12=0-3-8 Max Horz 18=289(LC 11) Max Uplift 18=-29(LC 12), 11=-86(LC 50), 12=-6(LC 12)

Max Grav 18=1406(LC 24), 11=184(LC 39), 12=2154(LC 42)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-1523/45, 3-5=-1229/111, 5-6=-798/122, 6-8=-798/122, 9-10=-70/304, 2-18=-1301/64 BOT CHORD 17-18=-237/325, 16-17=-70/1271, 14-16=-31/987
- WEBS 3-16=-485/93, 5-16=0/652, 5-14=-440/17, 6-14=-776/108, 8-14=-5/1479, 8-12=-1669/56, 9-12=-425/23, 2-17=0/989

#### NOTES-

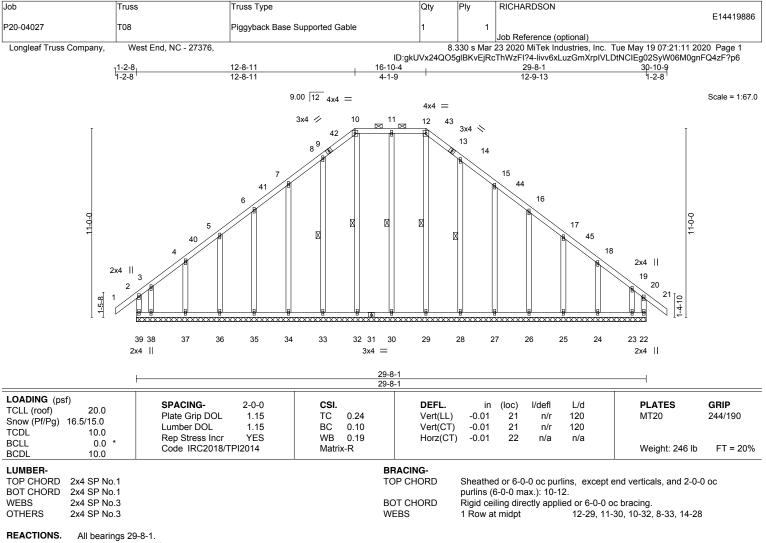
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=37ft; eave=5ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 11, 12. 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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(lb) - Max Horz 39=-249(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 33, 34, 35, 36, 37, 28, 27, 26, 25, 24 except 39=-237(LC 10), 22=-203(LC 11), 38=-193(LC 11), 23=-164(LC 10)

Max Grav All reactions 250 lb or less at joint(s) 29, 30, 32, 33, 34, 35, 36, 37, 28, 27, 26, 25, 24 except 39=324(LC 25), 22=296(LC 24), 38=302(LC 10), 23=273(LC 11)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 8-10=-67/268, 12-14=-58/269

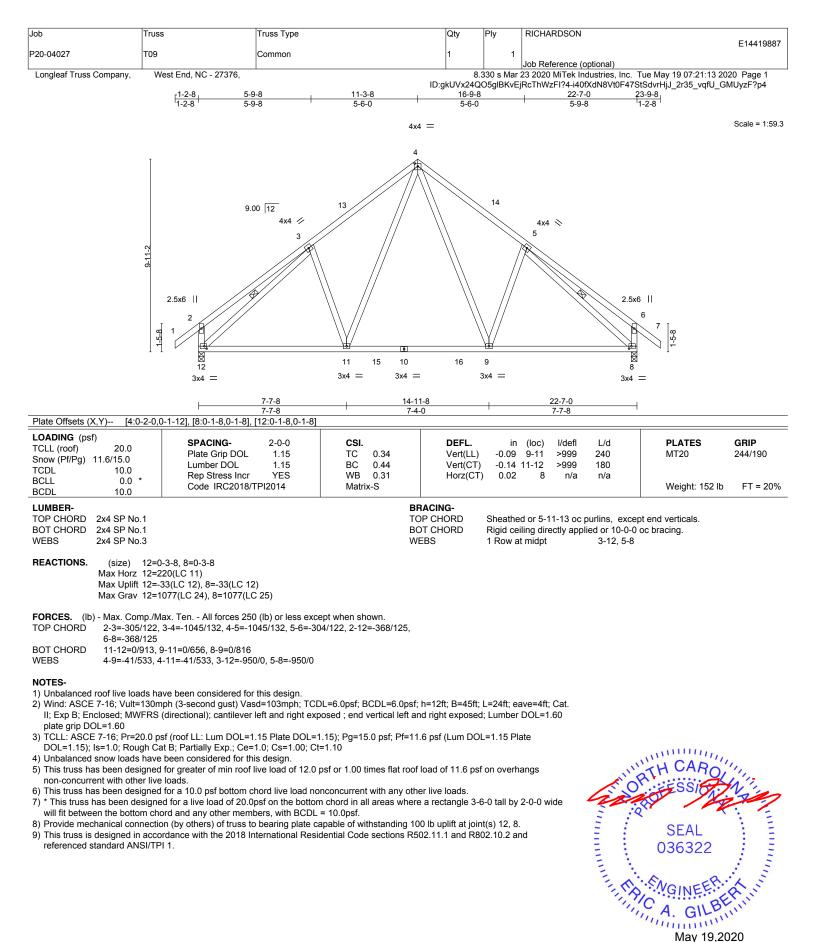
#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=30ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 1.5x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 11) Gable studs spaced at 2-0-0 oc.
- 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 13) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 33, 34, 35, 36, 37, 28, 27, 26, 25, 24 except (jt=lb) 39=237, 22=203, 38=193, 23=164.
- 15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

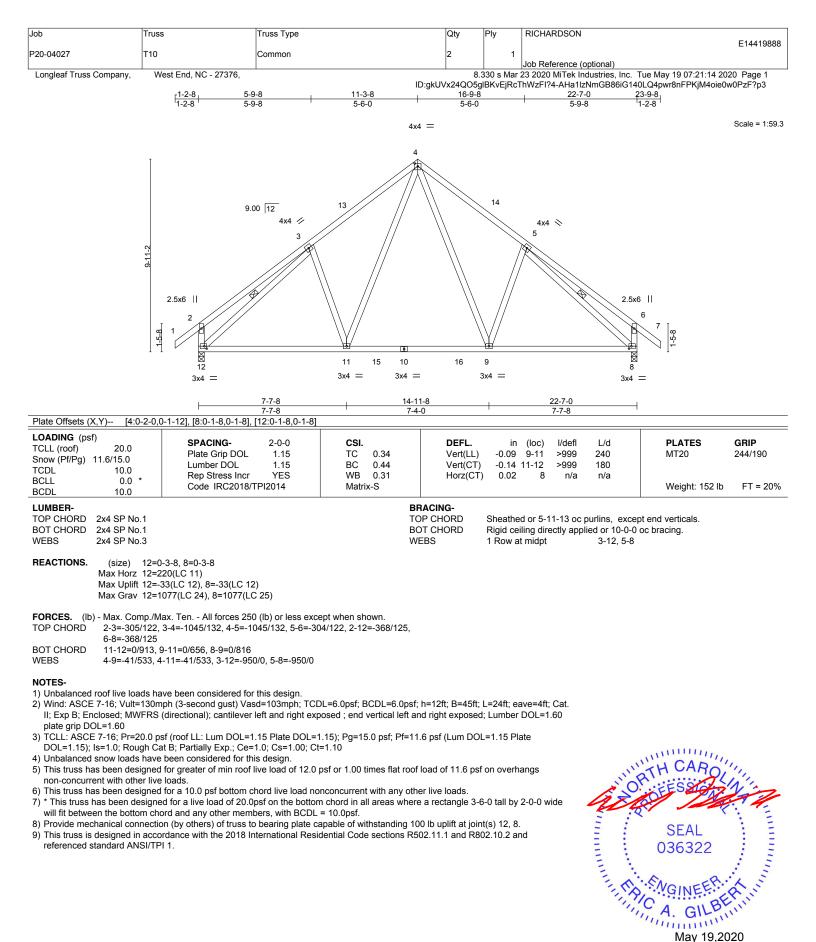
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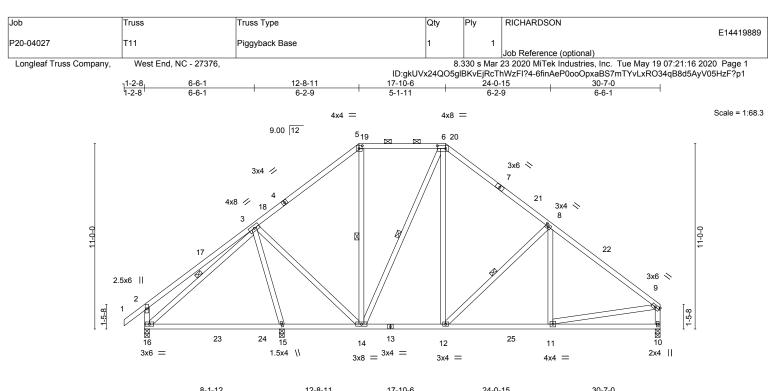


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Show (Pt/Pg)         16.5/15.0         Lumber DOL         1.15         BC         0.44         Vert(CT)         -0.23         15-16         >426         180           TCDL         10.0         Rep Stress Incr         YES         WB         0.82         Horz(CT)         0.02         10         n/a         n/a		8-1-12		10-6	24-0-15	30-7-0			
LOADING (psf) TCLL (roof)         SPACING- 20.0 Snow (Pf/Pg)         SPACING- 16.5/15.0 TCDL         2-0-0 Plate Grip DOL         CSI.         DEFL.         in         (loc)         I/defl         L/d         PLATES         GRIP           Snow (Pf/Pg)         16.5/15.0 TCDL         Plate Grip DOL         1.15         TC         0.52         Vert(LL)         -0.14         15-16         >710         240         MT20         244/15           BCLL         0.0 * BCDL         0.0 * Code IRC2018/TPI2014         WB         0.82         WB         0.02         10         n/a         n/a         Weight: 218 lb         FT =           LUMBER- TOP CHORD         2x4 SP No.1         Sheathed or 5-4-7 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.         Sheathed or 5-4-7 oc purlins, except end verticals, and 2-0-0 oc		8-1-12	4-6-14 5-1	I-11	6-2-9	6-6-1	I		
TCLL (roof)       20.0 Snow (Pf/Pg)       SPACING- 20.0 16.5/15.0 TCDL       2-0-0 Plate Grip DOL       CSI. 1.15 TC       DEFL.       in (loc)       I/deft       PLATES       GHIP         Now (Pf/Pg)       16.5/15.0 TCDL       Plate Grip DOL       1.15 Lumber DOL       TC       0.52       Vert(LL)       -0.14       15-16       >710       240       MT20       244/15         BCLL       0.0       Rep Stress Incr       YES Code IRC2018/TPI2014       WB       0.82       Horz(CT)       0.02       10       n/a       n/a         LUMBER- TOP CHORD       2x4 SP No.1       Sheathed or 5-4-7 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.       Sheathed or 5-4-7 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.	Plate Offsets (X,Y) [5:0-2-4,0	)-2-0], [6:0-6-0,0-2-0], [8:0-1-12,0-1-8],	[9:Edge,0-1-8]						
TOP CHORD     2x4 SP No.1     TOP CHORD     Sheathed or 5-4-7 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.	TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 *	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	TC 0.52 BC 0.44 WB 0.82	Vert(LL) Vert(CT)	-0.14 15-16 >710 -0.23 15-16 >426	240 180	MT20	<b>GRIP</b> 244/190 FT = 20%	
	TOP CHORD 2x4 SP No.1			OP CHORD		· · ·	nd verticals, and 2-0-	) oc	
6-0-0 oc bracing: 14-15.			BC	OT CHORD	CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:				
WEBS         1 Row at midpt         5-14, 6-14, 8-12, 3-16           REACTIONS.         (size)         15=0-3-8, 16=0-3-8, 10=0-3-8	( ,		W	EBS	1 Row at midpt	5-14, 6-14, 8	8-12, 3-16		
Max Horz 16=242(LC 11) Max Uplift 16=-48(LC 12), 10=-5(LC 12)		( - )							

Max Grav 15=1364(LC 43), 16=539(LC 24), 10=1100(LC 45)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-345/137, 3-5=-695/132, 5-6=-436/138, 6-8=-903/127, 8-9=-1250/54, 2-16=-403/136, 9-10=-1000/38

 BOT CHORD
 15-16=-66/345, 12-14=0/632, 11-12=0/918

 WEBS
 3-15=-1102/72, 3-14=-0/639, 6-14=-482/0, 6-12=0/619, 8-12=-537/91, 9-11=0/835

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=31ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

b) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) Unbalanced snow loads have been considered for this design.

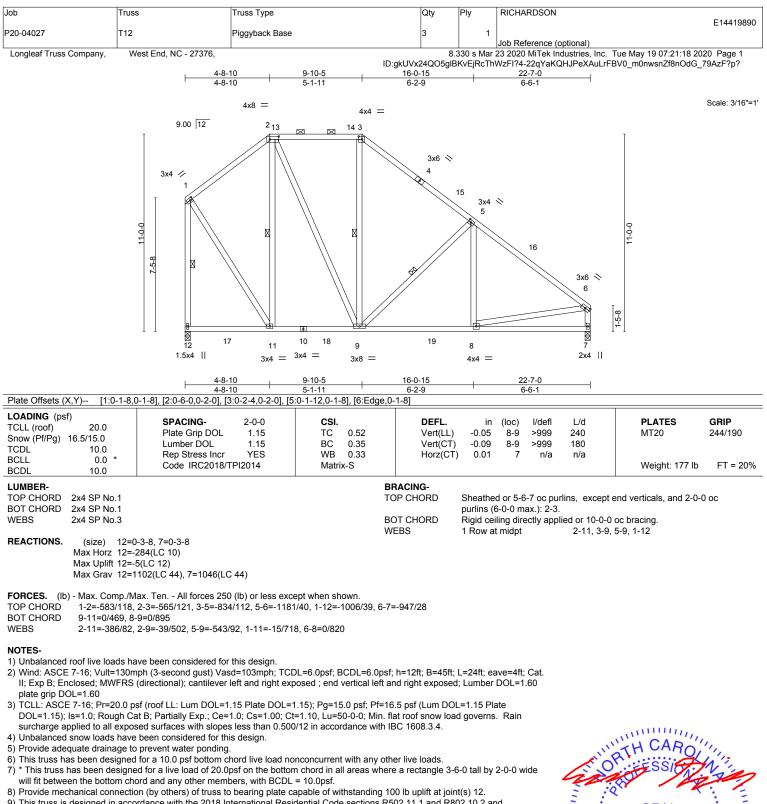
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
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Edenton, NC 27932

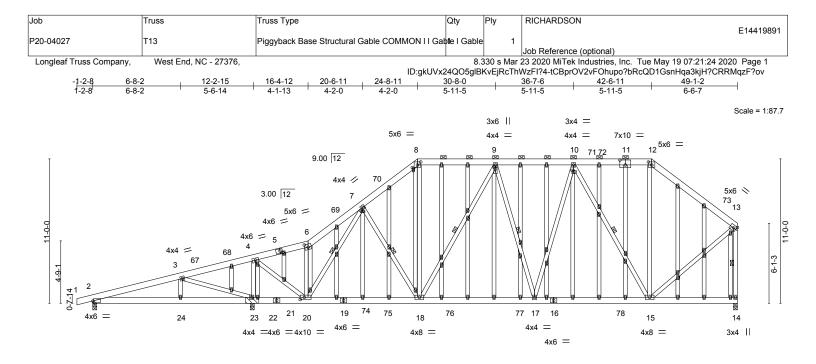


 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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F	6-8-2 6-8-2	12-2-15	12-3-8 16-4-12	24-8-11 8-3-15	33-7-1		42-6-1 8-11-0		<u>49-1-2</u> 6-6-7	
Plate Offsets (X	(,Y) [2:0-0-14	,0-1-11], [5:0-2-3,0-		], [6:0-2-0,0-0-14], [7:0						
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 1 TCDL BCLL	20.0 16.5/15.0 10.0 0.0 *	SPACING- Plate Grip Do Lumber DOL Rep Stress II Code IRC20	. 1.15 ncr YES	CSI. TC 0.37 BC 0.38 WB 0.87 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.08 15-17 -0.14 15-17 0.03 14	>999 24 >999 14	./d 40 80 )/a	PLATES MT20 Weight: 621 lb	<b>GRIP</b> 244/190 FT = 20%
BCDL	10.0				BRACING-					
TOP CHORD					TOP CHORD			, except end	verticals, and 2-0-0	D OC
BOT CHORD WEBS	2x6 SP No.1 2x4 SP No.3				BOT CHORD	purlins (6-0-0 m Rigid ceiling dir	,	or 6 0 0 oc br	acing	
	2x4 SP No.3				WEBS	1 Row at midpt			18, 9-17, 10-15, 13	-14
REACTIONS.       (size)       2=0-3-8, 14=0-3-8, 23=(0-3-8 + bearing block) (req. 0-4-2)         Max Horz       2=264(LC 11)         Max Uplift       2=-25(LC 12), 23=-8(LC 12)         Max Grav       2=354(LC 54), 14=1616(LC 25), 23=2614(LC 24)										
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       3-4=-25/972, 4-6=-943/8, 6-7=-1157/76, 7-8=-1589/109, 8-9=-1275/110, 9-10=-1486/108, 10-12=-954/116, 12-13=-1198/96, 13-14=-1538/32										
BOT CHORD       20-23892/46, 18-2019/1270, 17-18=0/1532, 15-17=0/1396         WEBS       3-24=0/256, 3-23994/10, 4-232111/46, 4-20=0/2200, 6-20=-604/77, 7-20=-652/0, 8-18=-3/684, 9-18=-504/26, 10-17=0/460, 10-15=-983/15, 12-15=0/406, 13-15=0/1193										
fasteners. Us 2) Unbalanced 3) Wind: ASCE II; Exp B; End plate grip DC 4) Truss design Gable End D 5) TCLL: ASCE DOL=1.15); 1 surcharge ap 6) Unbalanced 7) This truss ha non-concurre 8) Provide adec 9) All plates are 10) Gable studs 11) This truss h 12) * This truss will fit betwee	ser Defined Beari roof live loads ha 7-16; Vult=130m closed; MWFRS JL=1.60 ed for wind loads etails as applicat 7-16; Pr=20.0 pg s=1.0; Rough Ca oplied to all expos snow loads have s been designed ent with other live uate drainage to 2x4 MT20 unless s spaced at 2-0-0 has been design een the bottom cl	ng crushing capaci ve been considered ph (3-second gust) (directional); cantile in the plane of the ple, or consult quality of (roof LL: Lum DC t B; Partially Exp.; ded surfaces with sl been considered for for greater of min r loads. prevent water pome s otherwise indicate oc. d for a 10.0 psf bott ed for a live load of bord and any other	ty= 425psi. d for this design. Vasd=103mph; TCl ever left and right exit truss only. For stud fied building designe 0L=1.15 Plate DOL= Ce=1.0; Cs=1.00; C opes less than 0.500 or this design. oof live load of 12.0 ding. ed. tom chord live load if f20.0psf on the bott members, with BCD	h 3 rows of 10d (0.131 DL=6.0psf; BCDL=6.0p bosed ; end vertical left is exposed to wind (noi rr as per ANSI/TPI 1. 1.15); Pg=15.0 psf; Pf= t=1.10, Lu=50-0-0; Min 0/12 in accordance with psf or 1.00 times flat ro nonconcurrent with any om chord in all areas w L = 10.0psf. capable of withstanding	sf; h=12ft; B=45ft; and right exposed rmal to the face), so 16.5 psf (Lum DOI flat roof snow load BC 1608.3.4. bof load of 11.6 psf other live loads. here a rectangle 3-	L=49ft; eave=6ft ; Lumber DOL=1 ee Standard Indu L=1.15 Plate d governs. Rain on overhangs -6-0 tall by 2-0-0	.60 ustry	AND	SEAL 036322	
Continued on pa	ge 2		io boag plato		5					
Design valid fo a truss system	r use only with MiTek . Before use, the build	® connectors. This designing designer must verify	gn is based only upon par the applicability of design	D MITEK REFERENCE PAG ameters shown, and is for an a parameters and properly inc chord members only. Additi	individual building comporate this design into	oonent, not the overall			RENC	0

a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



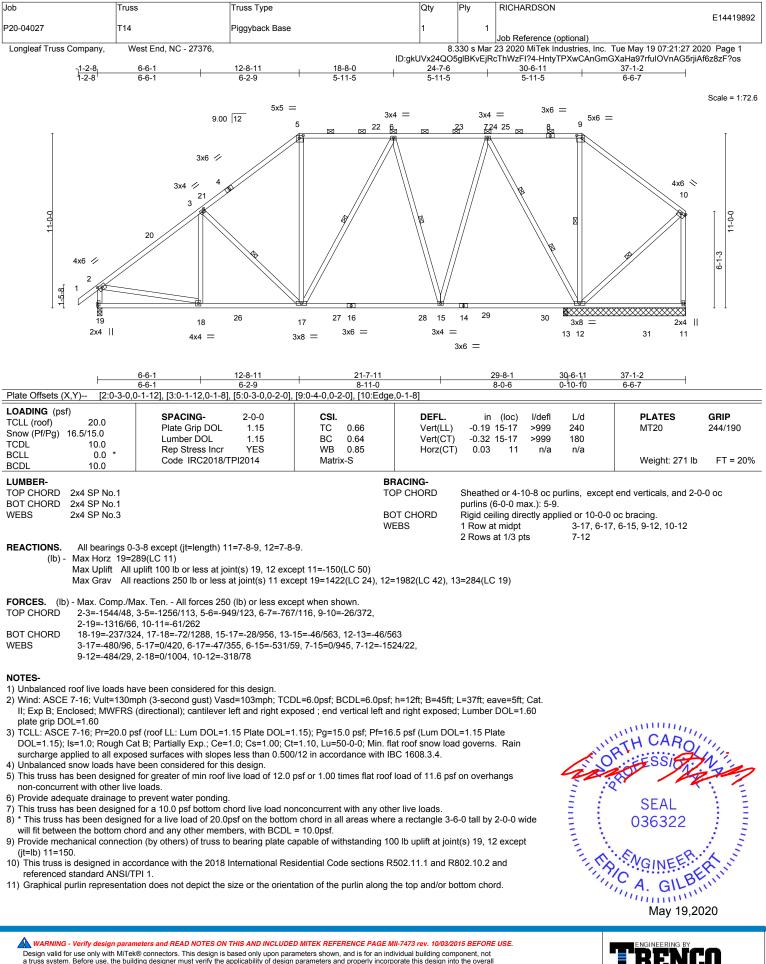
Job	Truss	Truss Type	Qty	Ply	RICHARDSON			
P20-04027	T13	Piggyback Base Structural Gable COMMON I I Gal		1	E14419891			
F 20-04027					Job Reference (optional)			
Longleaf Truss Company	y, West End, NC - 27376,	8.330 s Mar 23 2020 MiTek Industries, Inc. Tue May 19 07:21:25 2020 Page 2						
		ID:gkUVx24QO5glBKvEjRcThWzFl?4-LOIB2jWggZXYWzNB997fmEp1XhApoByQEsA?uGzF?ou						

#### NOTES-

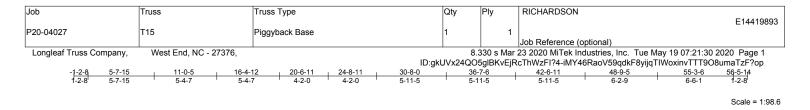
14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

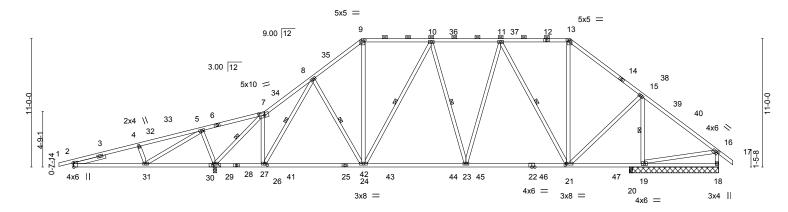
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSVTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





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ł	6-2-10	12-0-0 12-1 <sub>1</sub> 12 16-4-12	24-8-11	33-7-11	42-6-11	47-11-2	48-9-5 55-3-6		
Plate Offsets ()	6-2-10 (.Y) [2:0-4-4.Ed	<u>5-9-6 0-1<sup>1</sup>12 4-3-0 </u> ge], [7:0-5-12,0-2-8], [9:0-3-0,0-	<u>8-3-15</u> 2-0]. [13:0-3-0.0-2-0]. [16:0-3	<u>8-11-0</u> 3-0.0-1-12], [21:0-1	<u>8-11-0</u> 1-8.0-1-8], [26:0-2-12	<u>5-4-7</u> 2.0-1-8], [29:0-2-8.0	<u> </u>		
LOADING (pst TCLL (roof) Snow (Pf/Pg) TCDL	f) 20.0 16.5/15.0 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.64 BC 0.70 WB 0.79	DEFL. Vert(LL) Vert(CT) Horz(CT	in (loc) l/ -0.20 21-23 > -0.32 21-23 >	defl L/d 999 240 999 180 n/a n/a	PLATES MT20	<b>GRIP</b> 244/190	
BCLL BCDL	0.0 * 10.0	Code IRC2018/TPI2014	Matrix-S				Weight: 368 lb	FT = 20%	
LUMBER- TOP CHORD       BRACING- TOP CHORD         BOT CHORD       2x4 SP No.1         BOT CHORD       2x4 SP No.1         WEBS       2x4 SP No.3         SLIDER       Left 2x4 SP No.3 - x 2-10-5         BOT CHORD.       All bearings 0-3-8 except (jt=length) 29=0-4-10 (input: 0-3-8 + bearing block), 19=7-7-12, 18=7-7-12.         (ib) -       Max Uplift         All uplift       All uplift         Max Uplift       All uplift         100 or less at joint(s)       18, 20 except 29=-286(LC 12)         Max Grav       All reactions 250 lb or less at joint(s)									
FORCES. (Ib TOP CHORD BOT CHORD WEBS	9-10=-1143/24, 10-11=-1329/60, 11-13=-788/105, 13-15=-1004/90, 15-16=-65/283 BOT CHORD 2-31=-820/241, 29-31=-1835/470, 26-29=-310/732, 24-26=0/1112, 23-24=0/1417, 21-23=0/1274								
<ul> <li>fasteners. U</li> <li>2) Unbalanced</li> <li>3) Wind: ASCE</li> <li>II; Exp B; Enplate grip DC</li> <li>4) TCLL: ASCE</li> <li>DOL=1.15); surcharge al</li> <li>5) Unbalanced</li> <li>6) This truss hanon-concurr</li> <li>7) Provide ade</li> <li>8) All plates are</li> <li>9) This truss</li> <li>will fit betw</li> <li>11) Provide medite</li> <li>(jt=lb) 29=2</li> <li>12) This truss is</li> </ul>	ser Defined Bearing roof live loads have 7-16; Vult=130mpf iclosed; MWFRS (d) DL=1.60 57-16; Pr=20.0 psf Is=1.0; Rough Cat I pplied to all expose snow loads have b as been designed for ent with other live Ic quate drainage to p a 3x6 MT20 unless as been designed for a has been designed for a has been designed to has been designed can be bottom cho cchanical connection 286.	ong at jt. 29 attached to front fa g crushing capacity= 425psi. been considered for this desig (3-second gust) Vasd=103mpl irectional); cantilever left and rig (roof LL: Lum DOL=1.15 Plate I B; Partially Exp.; Ce=1.0; Cs=1. d surfaces with slopes less than een considered for this design. or greater of min roof live load of bads. revent water ponding. otherwise indicated. or a 10.0 psf bottom chord live lo d for a live load of 20.0psf on the rd and any other members, with n (by others) of truss to bearing dance with the 2018 Internation 1.	n. ; TCDL=6.0psf; BCDL=6.0p ht exposed ; end vertical left IOL=1.15); Pg=15.0 psf; Pf= 0; Ct=1.10, Lu=50-0-0; Min 0.500/12 in accordance with 12.0 psf or 1.00 times flat ro ad nonconcurrent with any of bottom chord in all areas w BCDL = 10.0psf. plate capable of withstandin	esf; h=12ft; B=45ft; t and right exposed 16.5 psf (Lum DO) flat roof snow loa b IBC 1608.3.4. Dof load of 11.6 psf other live loads. there a rectangle 3 g 100 lb uplift at jo	L=55ft; eave=7ft; Ca t; Lumber DOL=1.60 L=1.15 Plate d governs. Rain f on overhangs -6-0 tall by 2-0-0 wid int(s) 18, 20 except		SEAL 036322	A THE THE REAL PROPERTY OF THE	
Design valid for a truss system building desig is always requ fabrication, sto	or use only with MiTek® 1. Before use, the building 1. Bracing indicated is to 1. B	ers and READ NOTES ON THIS AND IN connectors. This design is based only up g designer must verify the applicability or prevent buckling of individual truss wet revent collapse with possible personal in and bracing of trusses and truss systems uss Plate Institute, 218 N. Lee Street, Su	on parameters shown, and is for an design parameters and properly inc and/or chord members only. Additi ury and property damage. For gene , see <b>ANS//TPI1 Quality</b> C	individual building com corporate this design int ional temporary and per eral guidance regarding	ponent, not o the overall manent bracing	t	ENGINEERING BY AMITEK A 818 Soundside Road Edenton, NC 27932	<b>D</b> ffiliate	

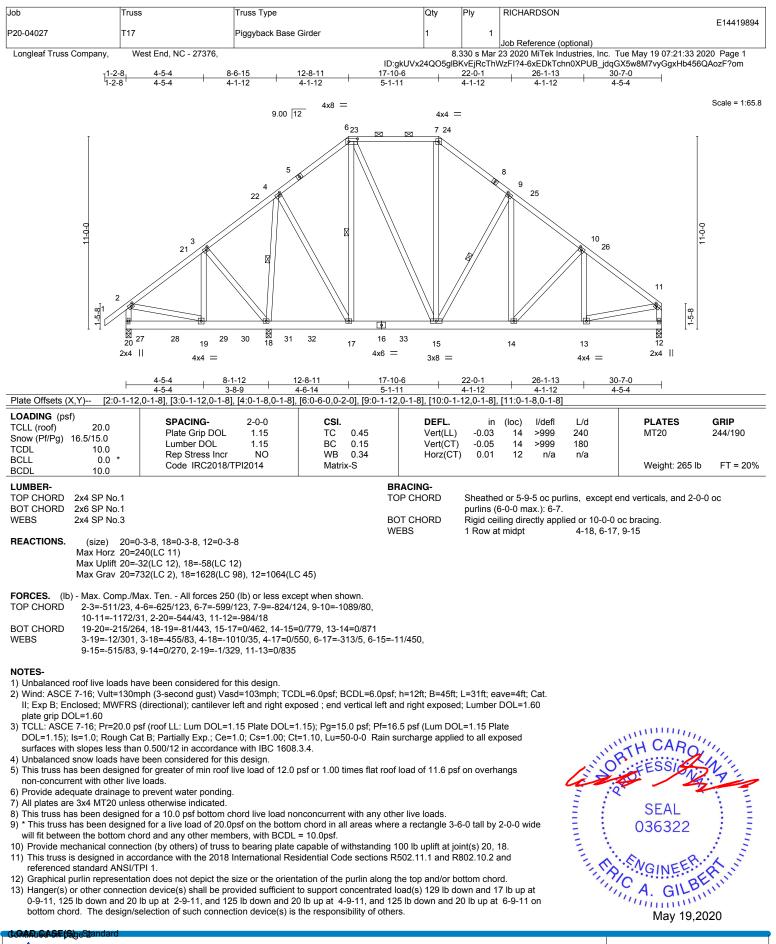
Job	Truss	Truss Type	Qty	Ply	RICHARDSON			
					E14419893			
P20-04027	T15	Piggyback Base	1	1				
					Job Reference (optional)			
Longleaf Truss Company,	West End, NC - 27376,	8.330 s Mar 23 2020 MiTek Industries, Inc. Tue May 19 07:21:30 2020 Page 2						
		ID:gkUVx24QO5glBKvEjRcThWzFI?4-iMY46RaoV59qdkF8yijqTIŴoxinvTTT9O8umaTzF?op						

#### NOTES-

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Job	Truss	Truss Type	Qty	Ply	RICHARDSON			
P20-04027	T17	Piggyback Base Girder	1	1	E14419894			
					Job Reference (optional)			
Longleaf Truss Company,	West End, NC - 27376,	8.330 s Mar 23 2020 MiTek Industries, Inc. Tue May 19 07:21:33 2020 Page 2						
		ID:gkUVx24QO5glBKvEjRcThWzFI?4-6xEDkTchn0XPUB_jdqGX5w8M7vyGgxHb456QAozF?om						

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-2=-43, 2-6=-43, 6-7=-53, 7-11=-43, 12-20=-20

Concentrated Loads (lb)

Vert: 27=-107 28=-104 29=-104 30=-104

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