

Trenco

818 Soundside Rd Edenton, NC 27932

Re: J0920-4494

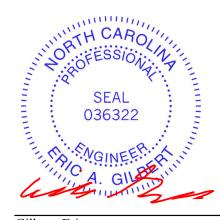
Precision/Lot 40 Summerlin/Harnett

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15101717 thru E15101742

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



November 16,2020

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

9-4-0

Scale: 1/8"=1

5-9-11

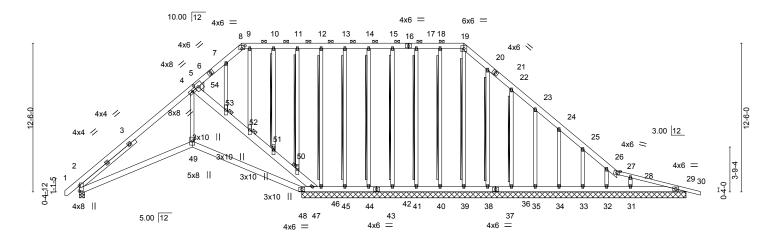


Plate Off	sets (X.Y)	[5:0-2-4,0-0-0], [8:0-3-0,0		<u>0</u> 15	-41	9-0-0		9-0-0 0-2-3	5-1-5
1 1010 011	0010 (71,1)	[0.0 2 1,0 0 0], [0.0 0 0,0	0 0], [0 1.0 2	10,0 0 12], [0 1.0 0 1,0 2	1				
LOADIN	G (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc) l	/defl L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC 0.30	Vert(LL)	-0.06 48-49 >	999 360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC 0.29	Vert(CT)	-0.14 2-49 >	999 240		
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.29	Horz(CT)	0.08 29	n/a n/a		
BCDL	10.0	Code IRC2015/Ti	PI2014	Matrix-S	Wind(LL)	0.05 2-49 >	999 240	Weight: 538	3 lb FT = 20%

27-8-8

LUMBER-TOP CHORD 2x6 SP No 1 *Except*

9-6-0

9-6-0

27-30: 2x4 SP No.1 **BOT CHORD** 2x6 SP No.1

WEBS 2x4 SP No.2 *Except* 4-47: 2x8 SP No.1

OTHERS 2x4 SP No.2 SLIDER

Left 2x4 SP No.2 - 6-2-10

BRACING-TOP CHORD

BOT CHORD

WEBS

JOINTS

Rigid ceiling directly applied or 6-0-0 oc bracing, Except:

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

10-0-0 oc bracing: 2-49,48-49,47-48. T-Brace:

2x4 SPF No.2 - 19-39, 18-40, 17-41, 15-42

, 14-44, 13-45, 12-46, 11-50, 10-51, 20-38, 22-36

45-1β-11 51-0-0

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails. 6in o.c. with 3in minimum end distance.

Brace must cover 90% of web length. 1 Brace at Jt(s): 50, 51, 52, 53

2-0-0 oc purlins (6-0-0 max.): 8-19.

REACTIONS. All bearings 32-3-8 except (jt=length) 2=0-5-8.

(lb) - Max Horz 2=-379(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 40, 41, 42, 44, 45, 38 except

2=-176(LC 12), 29=-138(LC 9), 47=-164(LC 13), 46=-706(LC 9), 36=-123(LC 13),

35=-111(LC 13), 34=-107(LC 13), 33=-143(LC 13), 31=-124(LC 9)

18-8-8

Max Grav All reactions 250 lb or less at joint(s) 39, 40, 41, 42, 44, 45, 38, 36,

35, 34, 33, 32 except 2=924(LC 1), 29=262(LC 24), 47=766(LC 3), 46=940(LC

22), 31=389(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown TOP CHORD

2-4=-1509/285, 4-5=-481/511, 5-7=-186/308, 7-8=-215/324, 8-9=-186/297,

9-10=-186/297, 10-11=-186/297, 11-12=-186/297, 12-13=-186/297, 13-14=-186/297

14-15=-186/297, 15-17=-186/297, 17-18=-186/297, 18-19=-187/297, 19-20=-209/333,

20-22=-183/253, 25-26=-258/239, 27-28=-251/210, 28-29=-292/238 2-49=-387/1293, 48-49=-376/1266, 47-48=-331/1136, 46-47=-233/319, 45-46=-233/319,

44-45=-233/319, 42-44=-233/319, 41-42=-233/319, 40-41=-233/319, 39-40=-233/319,

38-39=-233/319, 36-38=-233/319, 35-36=-233/319, 34-35=-233/319, 33-34=-233/319, 32-33=-233/319, 31-32=-233/319, 29-31=-233/319

4-49=-166/1138, 4-54=-1484/706, 53-54=-1267/466, 52-53=-1347/507, 51-52=-1395/487,

50-51=-1389/505, 47-50=-1401/522, 12-46=-367/158, 5-54=-393/428, 28-31=-267/168

WFBS

BOT CHORD

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Provide adequate drainage to prevent water ponding.



November 16,2020



Job	Truss	Truss Type	Qty	Ply	Precision/Lot 40 Summerlin/Harnett
J0920-4494	A01-GE	GABLE	1	1	E15101717
00020-4404	AUT-OL	OABLE		'	Job Reference (optional)

Comtech, Inc,

Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:51:59 2020 Page 2 ID:52SygMJAaHxrWTaExlrbuZyFiSD-Zm?DTaGOiCQ2bJwOT4nYB6DzXViMHlrD5FbwdAyldhE

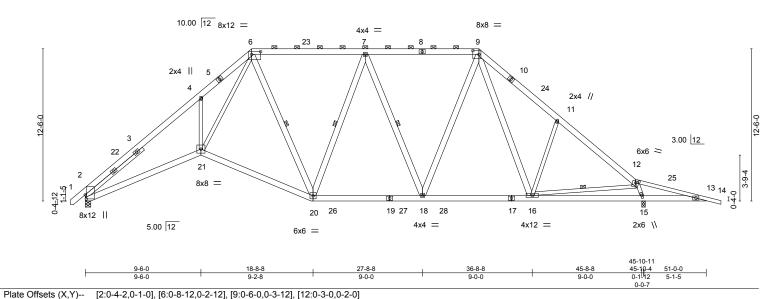
NOTES-

- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 40, 41, 42, 44, 45, 38 except (jt=lb) 2=176, 29=138, 47=164, 46=706, 36=123, 35=111, 34=107, 33=143, 31=124.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

Ply Truss Type Qty E15101718 J0920-4494 A02 5 PIGGYBACK BASE Job Reference (optional) Fayetteville, NC - 28314, 8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:01 2020 Page 1 Comtech, Inc. ID:52SygMJAaHxrWTaExIrbuZyFiSD-W87_uGlfEpgmqd4naVp0HXJAalJrl5JWYY40h3yldhC 1-2-8

Scale = 1:94.6

Precision/Lot 40 Summerlin/Harnett



DEFL.

(loc)

I/defl

L/d

PLATES

GRIP

244/190

FT = 20%

TCLL 20.0 Plate Grip DOL 1.15 TC 0.77 Vert(LL) -0.19 18-20 >999 360 MT20 **TCDL** вс 10.0 Lumber DOL 1.15 0.55 Vert(CT) -0.34 20-21 >999 240 **BCLL** 0.0 Rep Stress Incr YES WB 0.68 Horz(CT) 0.22 15 n/a n/a 240 Weight: 462 lb

BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Wind(LL) 0.09 21 >999 LUMBER-**BRACING-**

2x6 SP No.1 *Except* TOP CHORD TOP CHORD Structural wood sheathing directly applied or 2-8-3 oc purlins, except 12-14: 2x4 SP No.1 2-0-0 oc purlins (5-7-2 max.): 6-9. **BOT CHORD BOT CHORD** 2x6 SP No.1

Rigid ceiling directly applied or 6-0-0 oc bracing. **WEBS** 2x6 SP No.1 *Except* **WEBS** 1 Row at midpt 6-20, 7-20, 7-18, 12-16 4-21,6-21,11-16,12-16,12-15: 2x4 SP No.2

SLIDER Left 2x4 SP No.2 -x 6-2-10

Max Grav 2=1868(LC 1), 15=2359(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown 2-4=-3975/649, 4-6=-3719/921, 6-7=-1724/538, 7-9=-1774/525, 9-11=-2221/573, TOP CHORD

11-12=-2317/408, 12-13=-1129/1108

Max Uplift 2=-60(LC 12), 15=-107(LC 13)

(size) 2=0-5-8, 15=0-3-8 Max Horz 2=-297(LC 10)

SPACING-

BOT CHORD 2-21=-307/3220, 20-21=-115/1965, 18-20=-109/1841, 16-18=-21/1509, 15-16=-84/809,

2-0-0

13-15=-1048/1156

WEBS 4-21=-396/402, 6-21=-479/2565, 6-20=-304/140, 7-20=-419/210, 7-18=-438/232, 9-18=-116/785, 9-16=-137/529, 11-16=-498/319, 12-16=-890/1613, 12-15=-2377/805

LOADING (psf)

REACTIONS.

Job

Truss

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-1 to 3-3-12, Interior(1) 3-3-12 to 13-8-0, Exterior(2) 13-8-0 to 18-0-13, Interior(1) 18-0-13 to 32-4-0, Exterior(2) 32-4-0 to 36-8-12, Interior(1) 36-8-12 to 52-2-8 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 4x6 MT20 unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb)
- 15=107 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



November 16,2020

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty Ply Precision/Lot 40 Summerlin/Harnett E15101719 J0920-4494 A03 PIGGYBACK BASE Job Reference (optional) Fayetteville, NC - 28314, 8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:03 2020 Page 1 Comtech, Inc, ID:52SygMJAaHxrWTaExlrbuZyFiSD-SXFklyJvmRwU4wD9iwrUMyOcw60HD1Cp0sZ7mxyIdhA 13-8-0 23-0-0 32-4-0 38-9-2 45-2-5 51-0-0

9-4-0

23-0-0

9-4-0

Scale = 1:94.6

5-9-11

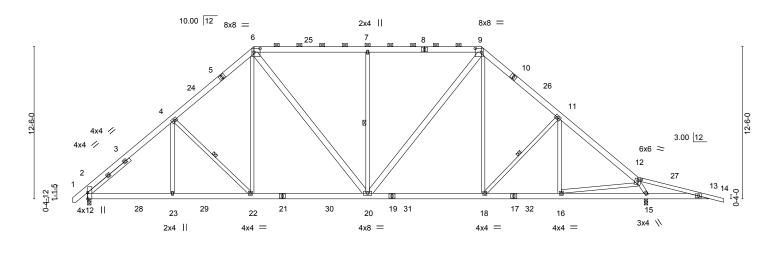


Plate Offsets (X,Y) [2:0-6-3,Edge], [6:0-6-0,0-3-12], [9:0-6-0,0-3-12], [12:0-3-0,0-2-0]										
LOADIN	G (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC 0.40	Vert(LL)	-0.14 20-22	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC 0.49	Vert(CT)	-0.23 20-22	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.53	Horz(CT)	0.07 15	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matrix-S	Wind(LL)	0.04 20	>999	240	Weight: 442 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

9-4-0

38-9-2

2-0-0 oc purlins (5-2-0 max.): 6-9.

6-0-0 oc bracing: 13-15.

1 Row at midpt

45-2-5

Structural wood sheathing directly applied or 4-4-8 oc purlins, except

4-22, 7-20, 11-18

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

0-9-11

5-0-0

LUMBER-

TOP CHORD 2x6 SP No.1 *Except*

12-14: 2x4 SP No.1 2x6 SP No.1

6-11-12

BOT CHORD WEBS 2x4 SP No.2 *Except*

6-20,9-20: 2x6 SP No.1

SLIDER Left 2x4 SP No.2 -x 4-5-13

REACTIONS. (size) 2=0-3-8, 15=0-3-8 Max Horz 2=-296(LC 10)

Max Uplift 2=-58(LC 12), 15=-107(LC 13) Max Grav 2=2148(LC 2), 15=2458(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-2817/509, 4-6=-2345/584, 6-7=-2056/584, 7-9=-2057/584, 9-11=-2294/543,

11-12=-2417/379, 12-13=-1007/1031

BOT CHORD 2-23=-209/2111, 22-23=-209/2111, 20-22=-56/1752, 18-20=-26/1670, 16-18=-131/1807, 15-16=0/724, 13-15=-965/1034

6-8-4

13-8-0

6-8-4

WEBS 4-23=0/345, 4-22=-499/240, 6-22=-72/734, 6-20=-176/605, 7-20=-628/301,

9-20=-191/694, 9-18=-40/670, 11-18=-441/184, 11-16=-204/262, 12-16=-694/1407,

12-15=-2565/807

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-1 to 3-3-12, Interior(1) 3-3-12 to 13-8-0, Exterior(2) 13-8-0 to 18-0-13, Interior(1) 18-0-13 to 32-4-0, Exterior(2) 32-4-0 to 36-8-12, Interior(1) 36-8-12 to 52-2-8 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 4x6 MT20 unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 15=107
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



November 16,2020



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTesk go connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/PTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information, evaluable from Trus Plate betting. 2570 Crisis Highways. fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TPH Qu Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



23-0-0

9-4-0

23-0-0

9-4-0

Fayetteville, NC - 28314, Comtech, Inc.

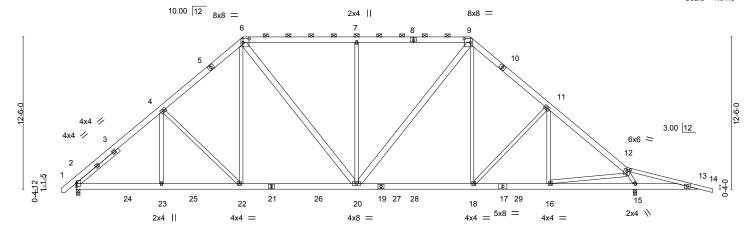
13-8-0

6-8-4

13-8-0

8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:06 2020 Page 1 ID:52SygMJAaHxrWTaExlrbuZyFiSD-s6wtxzMn3MJ3xOykN3PB_b09CJxXQR5FiqonMGyldh7 32-4-0 38-9-2 45-2-5 51-0-0 6-5-2 6-5-2 5-9-11

Scale = 1:94.6



		0 11 12	0 0 4	0 + 0	,	7 + 0	002	002 0011	0 0 0
Plate Offse	ets (X,Y)	[6:0-6-0,0-3-12], [9:0-6-0	,0-3-12], [12:0	-3-0,0-2-0]					
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	I/defl L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC 0.27	Vert(LL)	-0.26 18-20	>999 360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC 0.90	Vert(CT)	-0.45 18-20	>999 240		
BCLL	0.0 *	Rep Stress Incr	NO	WB 0.33	Horz(CT)	0.05 15	n/a n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matrix-S	Wind(LL)	0.15 18-20	>999 240	Weight: 883 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

9-4-0

LUMBER-

TOP CHORD 2x6 SP No 1 *Except*

12-14: 2x4 SP No.1

6-11-12

BOT CHORD 2x6 SP No.1 *Except* 17-19: 2x6 SP 2400F 2.0E

WEBS 2x4 SP No.2 *Except*

6-20,9-20: 2x6 SP No.1

SLIDER Left 2x4 SP No.2 -x 4-5-13

REACTIONS. (size) 2=0-3-8, 15=0-3-8

Max Horz 2=-296(LC 25)

Max Uplift 2=-132(LC 8), 15=-218(LC 9) Max Grav 2=2795(LC 2), 15=3433(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-3772/188, 4-6=-3265/258, 6-7=-3373/281, 7-9=-3374/281, 9-11=-3900/323, 11-12=-3721/199, 12-13=-557/949

2-23=-212/2701, 22-23=-212/2701, 20-22=-138/2449, 18-20=-40/2891, 16-18=-31/2821,

15-16=-53/1156, 13-15=-885/574 **WEBS**

4-23=0/386, 4-22=-491/217, 6-22=-137/741, 6-20=-289/1587, 7-20=-611/232, 9-20=-210/850, 9-18=-142/1445, 11-18=-379/319, 11-16=-596/200, 12-16=-313/1849,

12-15=-3582/313

NOTES-

BOT CHORD

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); cantilever right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Provide adequate drainage to prevent water ponding.
- 6) All plates are 4x6 MT20 unless otherwise indicated.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=132, 15=218.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDE MITER REFERENCE FACE MITERS AND INCLUDE MITER REFERENCE FACE MITERS AND INCLUDE MITERS REPORTED FOR THE MITERS AND INCLUDE MITERS AN fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP/1 Qu Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



46-0₇0

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

2-0-0 oc purlins (6-0-0 max.): 6-9.

6-0-0 oc bracing: 13-15.

5-0-0

November 16,2020



Job	Truss	Truss Type	Qty	Ply	Precision/Lot 40 Summerlin/Harnett
J0920-4494	A04	PIGGYBACK BASE	1		E15101720
J0920-4494	A04	PIGGIBACK BASE		2	Job Reference (optional)

Comtech, Inc,

Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:06 2020 Page 2 ID:52SygMJAaHxrWTaExlrbuZyFiSD-s6wtxzMn3MJ3xOykN3PB_b09CJxXQR5FiqonMGyldh7

NOTES-

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1622 lb down and 193 lb up at 27-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

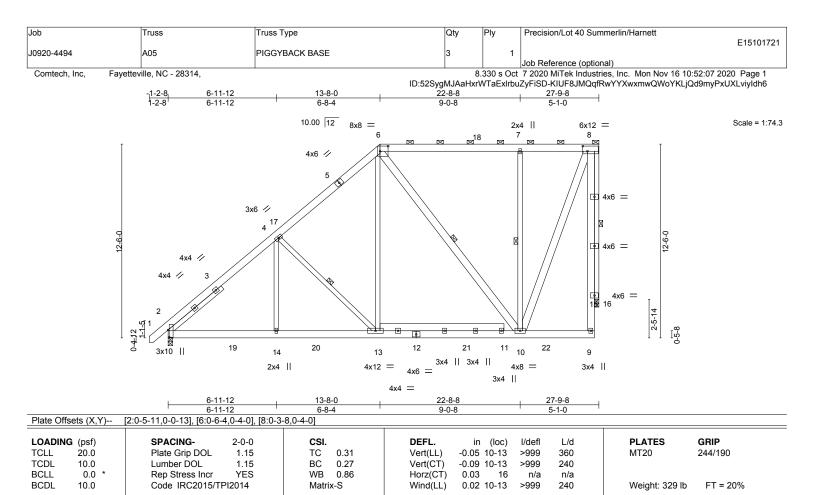
LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-60, 6-9=-60, 9-12=-60, 12-14=-60, 2-13=-20

Concentrated Loads (lb) Vert: 28=-1445(B)





BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

2x4 SP No.2 *Except* **WEBS** 8-9,6-10,8-10: 2x6 SP No.1

OTHERS 2x4 SP No.2

SLIDER Left 2x4 SP No.2 -x 4-5-13

REACTIONS. (size) 2=0-3-8, 16=0-3-8

Max Horz 2=400(LC 12) Max Uplift 2=-7(LC 12), 16=-143(LC 9)

Max Grav 2=1359(LC 19), 16=1275(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-4=-1606/165, 4-6=-1133/237, 6-7=-469/136, 7-8=-468/136 TOP CHORD

BOT CHORD 2-14=-466/1223, 13-14=-466/1223, 10-13=-275/800

WEBS 4-14=0/388, 4-13=-580/272, 6-13=-94/776, 6-10=-597/228, 7-10=-507/264,

8-10=-354/1194, 8-16=-1276/336

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-1 to 3-3-12, Interior(1) 3-3-12 to 13-8-0, Exterior(2) 13-8-0 to 19-10-11, Interior(1) 19-10-11 to 27-3-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Bearing at joint(s) 16 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb)
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

4-13, 6-10, 7-10, 8-16

except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-8.

Rigid ceiling directly applied or 10-0-0 oc bracing.

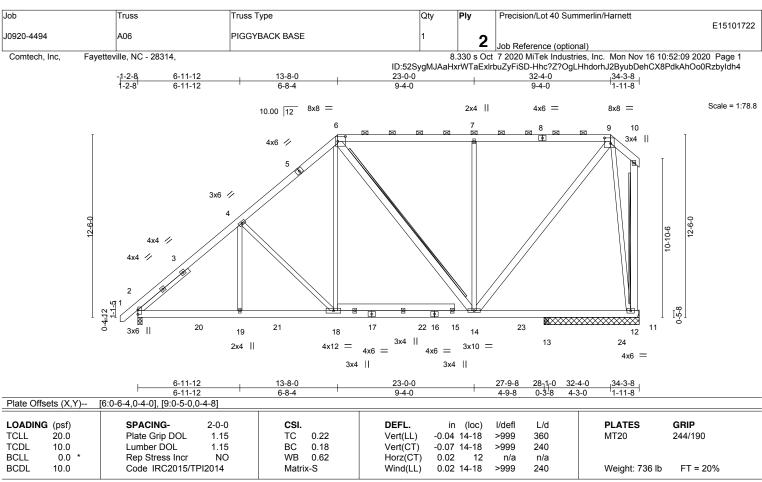
November 16,2020



Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/THI Quality Criteria, DSB-89 and BCSI Building Component Safety Information, evaluable from Tune Bloth perfitting 2670 (Crips Highways). fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TPH Qu Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LUMBER-

TOP CHORD 2x6 SP No 1 BOT CHORD 2x6 SP No.1

WFBS 2x4 SP No.2 *Except* 6-14,9-14,10-12: 2x6 SP No.1

SLIDER Left 2x4 SP No.2 -x 4-5-13 **BRACING-**TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-9. Rigid ceiling directly applied or 10-0-0 oc bracing.

T-Brace: 2x4 SPF No.2 - 10-12 2x6 SPF No.2 - 6-14

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 6-6-0 except (jt=length) 2=0-3-8, 13=0-3-8.

Max Horz 2=371(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 12 except 11=-607(LC 14),

13=-209(LC 5)

Max Grav All reactions 250 lb or less at joint(s) except 2=1563(LC 15), 12=1796(LC

2), 12=1277(LC 1), 13=2690(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1934/51, 4-6=-1479/140, 6-7=-891/139, 7-9=-892/139

BOT CHORD 2-19=-280/1450, 18-19=-280/1450, 14-18=-154/1048

WEBS 4-19=0/373, 4-18=-552/210, 6-18=-47/790, 6-14=-366/94, 7-14=-644/226,

9-14=-175/1181, 9-12=-1215/239

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to

ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12 except (jt=lb) 11=607, 13=209.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2356 lb down and 270 lb up at 27-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

COnthus drininga gerdry design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information, evaluable from Trus Blots petitive, 2870 Crist Michael ND 20081. fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP/1 Qu Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



November 16,2020



Ply Job Truss Truss Type Qty Precision/Lot 40 Summerlin/Harnett E15101722 J0920-4494 A06 PIGGYBACK BASE Job Reference (optional)

Comtech, Inc,

Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:09 2020 Page 2 ID:52SygMJAaHxrWTaExlrbuZyFiSD-Hhc?Z?OgLHhdorhJ2ByubDehCX8PdkAhOo0Rzbyldh4

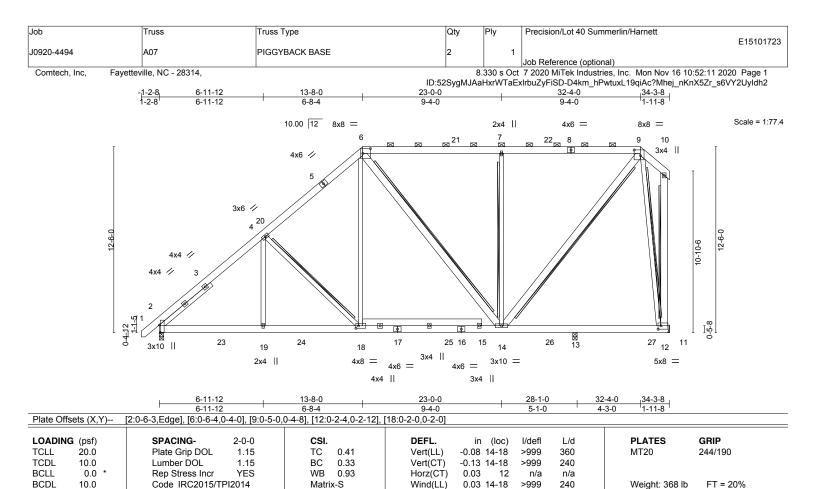
LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-60, 6-9=-60, 9-10=-60, 2-11=-20

Concentrated Loads (lb) Vert: 13=-2057(F)





LUMBER-

TOP CHORD 2x6 SP No 1 BOT CHORD 2x6 SP No.1

2x4 SP No.2 *Except* WFBS 6-14,9-14,10-12: 2x6 SP No.1

SLIDER Left 2x4 SP No.2 -x 4-5-13 **BRACING-**TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 5-7-13 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-9.

Rigid ceiling directly applied or 10-0-0 oc bracing. T-Brace:

2x4 SPF No.2 - 4-18, 7-14, 9-14, 10-12,

9-12 2x6 SPF No.2 - 6-14

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. (size) 2=0-3-8, 12=Mechanical, 13=0-3-8

Max Horz 2=371(LC 12)

Max Uplift 2=-41(LC 12), 12=-163(LC 9)

Max Grav 2=1561(LC 19), 12=1340(LC 2), 13=428(LC 18)

FORCES. - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1932/318, 4-6=-1477/394, 6-7=-888/346, 7-9=-888/347 **BOT CHORD** 2-19=-522/1448, 18-19=-522/1448, 14-18=-338/1046

WEBS 4-19=0/373, 4-18=-552/261, 6-18=-90/791, 6-14=-370/93, 7-14=-644/319,

9-14=-358/1172, 9-12=-1208/484

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-1 to 3-3-12, Interior(1) 3-3-12 to 13-8-0, Exterior(2) 13-8-0 to 19-10-11, Interior(1) 19-10-11 to 32-4-0, Exterior(2) 32-4-0 to 33-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb)
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



November 16,2020

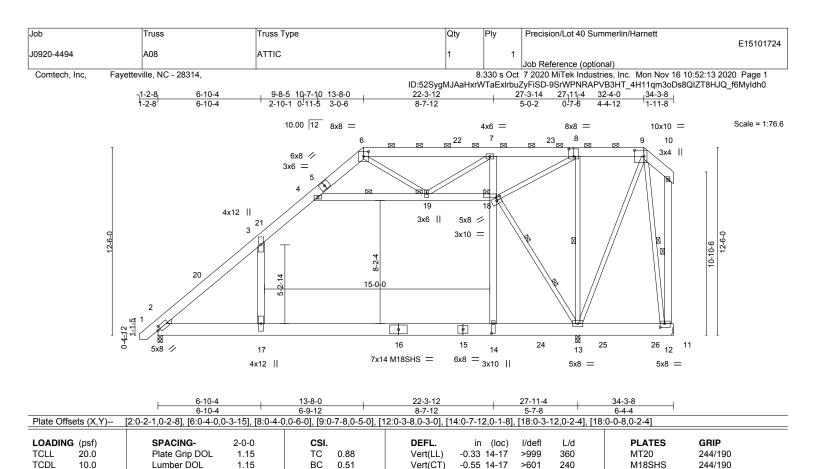


MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/THI Quality Criteria, DSB-89 and BCSI Building Component Safety Information, evaluable from Tune Bloth perfitting 2670 (Crips Highways). fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSVTP/1 Qu Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LUMBER-

BCLL

BCDL

TOP CHORD 2x8 SP No.1

0.0

10.0

BOT CHORD 2x10 SP 2400F 2 0F **WEBS** 2x4 SP No.2 *Except*

3-17,4-18,7-14,10-12: 2x6 SP No.1

WEDGE

Left: 2x4 SP No.3

BRACING-TOP CHORD

Horz(CT)

Wind(LL)

0.02

0.10 14-17

BOT CHORD WEBS

JOINTS

Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-9.

Weight: 440 lb

FT = 20%

Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt 4-19, 8-13, 10-12, 9-12

13-18 2 Rows at 1/3 pts

n/a

240

1 Brace at Jt(s): 18, 19

12

n/a

>999

REACTIONS. (size) 2=0-3-8, 13=0-3-8, 12=Mechanical

Rep Stress Incr

Code IRC2015/TPI2014

Max Horz 2=367(LC 12)

Max Uplift 13=-1(LC 8), 12=-80(LC 12)

Max Grav 2=2017(LC 20), 13=1769(LC 27), 12=851(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

2-3=-2559/0, 3-4=-1766/63, 4-6=-373/154, 6-7=-253/101, 7-8=-193/304, 8-9=-312/126 TOP CHORD

YES

WB

Matrix-S

0.95

BOT CHORD 2-17=-127/1763, 14-17=-127/1763, 13-14=-130/1733

WEBS $3-17=0/1039,\ 4-19=-1852/72,\ 18-19=-1899/374,\ 14-18=0/1861,\ 7-18=-700/316,$ 8-18=-703/364, 9-13=-110/564, 9-12=-570/176, 6-19=-32/307, 7-19=-101/321,

13-18=-2741/127

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-11 to 3-5-2, Interior(1) 3-5-2 to 13-8-0, Exterior(2) 13-8-0 to 19-10-11, Interior(1) 19-10-11 to 32-4-0, Exterior(2) 32-4-0 to 33-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Ceiling dead load (10.0 psf) on member(s). 3-4, 4-19, 18-19; Wall dead load (5.0psf) on member(s).3-17, 14-18
- 8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 14-17
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13, 12.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) Attic room checked for L/360 deflection.



November 16,2020

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDE MITER REFERENCE FACE MITERS AND INCLUDE MITER REFERENCE FACE MITERS AND INCLUDE MITERS REPORTED FOR THE MITERS AND INCLUDE MITERS AN fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP/1 Qu Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Vert(CT)

Horz(CT)

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

WFBS

JOINTS

-0.55 14-17

0.10 14-17

12

0.02

>602

>999

1 Row at midpt

n/a

240

n/a

240

(Switched from sheeted: Spacing > 2-8-0).

1 Brace at Jt(s): 6, 9, 18, 10, 19

2-0-0 oc purlins (6-0-0 max.), except end verticals

Rigid ceiling directly applied or 10-0-0 oc bracing.

10-12, 13-18

LUMBER-

TCDL

BCLL

BCDL

TOP CHORD 2x8 SP 2400F 2.0E BOT CHORD 2x10 SP 2400F 2 0F WFBS 2x4 SP No.2 *Except*

10.0

0.0

10.0

3-17,4-18,7-14,10-12: 2x6 SP No.1, 13-18: 2x4 SP No.1 WEDGE

Lumber DOL

Rep Stress Incr

Code IRC2015/TPI2014

Left: 2x4 SP No.3

REACTIONS.

(size) 2=0-3-8, 13=0-3-8, 12=Mechanical Max Horz 2=781(LC 12)

Max Uplift 13=-50(LC 8), 12=-170(LC 12)

Max Grav 2=4318(LC 20), 13=3629(LC 27), 12=1953(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 2-3=-5521/0, 3-4=-3799/134, 4-6=-809/320, 6-7=-556/200, 7-8=-430/890, 8-9=-719/273,

1.15

NO

BC

WB 0.94

Matrix-S

0.58

10-12=-257/114

BOT CHORD 2-17=-268/3802, 14-17=-268/3802, 13-14=-266/3788

WEBS 3-17=0/2279, 4-19=-3979/142, 18-19=-4480/844, 14-18=0/3910, 7-18=-1701/716,

8-18=-1787/781, 8-13=-450/534, 9-13=-243/1355, 9-12=-1322/382, 6-19=-42/655,

7-19=-299/1090, 13-18=-5929/242

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x8 - 2 rows staggered at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.

Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-11 to 3-5-2, Interior(1) 3-5-2 to 13-8-0, Exterior(2) 13-8-0 to 19-10-11, Interior(1) 19-10-11 to 32-4-0, Exterior(2) 32-4-0 to 33-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Provide adequate drainage to prevent water ponding.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Ceiling dead load (10.0 psf) on member(s). 3-4, 4-19, 18-19; Wall dead load (5.0psf) on member(s).3-17, 14-18
- 10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 14-17
- 11) Refer to girder(s) for truss to truss connections.

M WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTesk go comectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



M18SHS

Weight: 881 lb

244/190

FT = 20%

November 16,2020



Job	Truss	Truss Type	Qty	Ply	Precision/Lot 40 Summerlin/Harnett
J0920-4494	400	ATTIC	2	_	E15101725
J0920-4494	A09	ATTIC	3	2	Job Reference (optional)

Comtech, Inc,

Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:15 2020 Page 2 ID:52SygMJAaHxrWTaExlrbuZyFiSD-5rzHq2SRx7RnWm8TPR3IrUudtx4c1NiamkTmBFyIdh_

NOTES-

- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13 except (jt=lb) 12=170.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) Attic room checked for L/360 deflection.



	6-10-4	13-8-0	22-3-12	27-11-4	34-3-8
	6-10-4	6-9-12	8-7-12	5-7-8	6-4-4
Plate Offsets (X,Y)	[2:0-2-0,0-2-4], [5:0-4-0,Edge], [6:0	0-4-0,0-3-15], [8:0-4-0,0-6-	0], [9:0-4-0,0-3-15], [14:0-8-0,0-1	I-8], [17:0-6-4,0-1-8],	[18:0-4-8,0-2-12]

JOINTS

LOADING (psf)	SPACING- 5-1-8	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.72	Vert(LL) -0.39 14-17 >847 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.70	Vert(CT) -0.66 14-17 >502 240	M18SHS 244/190
BCLL 0.0 *	Rep Stress Incr NO	WB 0.74	Horz(CT) 0.02 12 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.12 14-17 >999 240	Weight: 863 lb FT = 20%

LUMBER-**BRACING-**TOP CHORD

TOP CHORD 2x8 SP 2400F 2.0E BOT CHORD 2x10 SP 2400F 2 0F

BOT CHORD WFBS 2x4 SP No.2 *Except* 3-17,4-18,7-14,10-12,19-20: 2x6 SP No.1, 13-18: 2x4 SP 2400F 2.0E **WEBS**

WEDGE

Left: 2x6 SP No.2

REACTIONS. (size) 2=0-3-8, 13=0-3-8, 12=Mechanical Max Horz 2=941(LC 12)

Max Grav 2=4886(LC 20), 13=6465(LC 2), 12=1446(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown TOP CHORD 2-3=-6241/0, 3-4=-4150/0, 4-6=-758/550, 6-7=-845/1106, 7-8=-1541/2847,

8-9=-346/256, 9-10=-297/432, 12-20=-619/24, 10-20=-287/330 2-17=-827/4206, 14-17=-827/4206, 13-14=-821/4188, 12-13=-330/316

BOT CHORD WEBS 3-17=0/2727, 4-21=-4771/101, 18-21=-6277/1709, 14-18=0/4707, 7-18=-2337/1174,

8-18=-2871/1323, 13-19=-854/754, 8-19=-390/990, 9-19=-542/644, 9-20=-465/181,

6-21=-746/1085, 7-21=-867/2181, 13-18=-7879/932, 19-20=-537/457

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x8 - 2 rows staggered at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.

Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-11 to 3-5-2, Interior(1) 3-5-2 to 13-8-0, Exterior(2) 13-8-0 to 19-10-11, Interior(1) 19-10-11 to 32-4-0, Exterior(2) 32-4-0 to 33-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Provide adequate drainage to prevent water ponding.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Ceiling dead load (10.0 psf) on member(s). 3-4, 4-21, 18-21, 19-20; Wall dead load (5.0psf) on member(s). 3-17, 14-18, 13-19
- 10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 14-17, 12-13
- 11) Refer to girder(s) for truss to truss connections

Continued AMNRAGE My design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTesk go comectors. This design is based only upon parameters shown, and is for an individual building ocomponent, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



2-0-0 oc purlins (6-0-0 max.), except end verticals

Rigid ceiling directly applied or 10-0-0 oc bracing.

Fasten (2X) T and I braces to narrow edge of web with 10d

(0.131"x3") nails, 6in o.c., with 3in minimum end distance.

2x6 SPF No.2 - 13-18

(Switched from sheeted: Spacing > 2-8-0).

Brace must cover 90% of web length.

1 Brace at Jt(s): 6, 9, 18, 19, 10, 20, 21

T-Brace:

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Job	Truss	Truss Type	Qty	Ply	Precision/Lot 40 Summerlin/Harnett
J0920-4494	A09-A	ATTIC	1		E15101726
00020-4404	A05-A	ATTIO		2	Job Reference (optional)

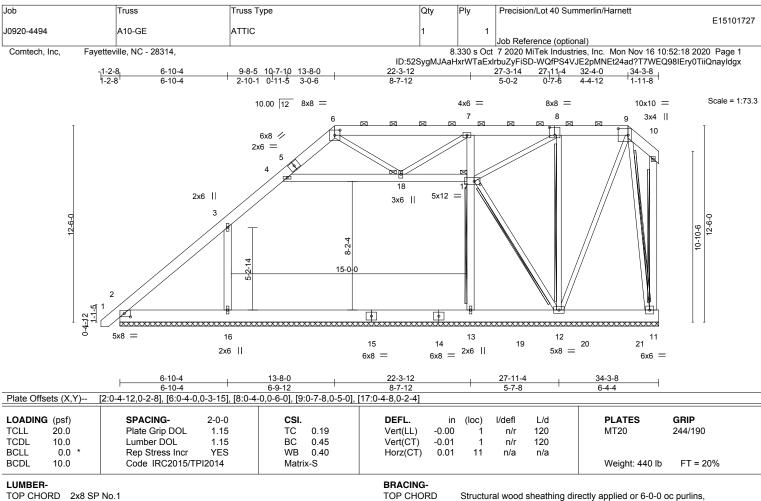
Comtech, Inc, Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:17 2020 Page 2 ID:52SygMJAaHxrWTaExlrbuZyFiSD-2D51EkUhTkhVl4IrWs6mwvzwQllDVKLtE2ytE7yldgy

NOTES-

- 13) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 14) Attic room checked for L/360 deflection.





2x8 SP No 1 BOT CHORD 2x10 SP No.1

WFBS 2x4 SP No.2 *Except*

3-16,4-17,7-13,10-11: 2x6 SP No.1

TOP CHORD

BOT CHORD

WEBS

T-Brace:

Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SPF No.2 - 13-17, 8-12, 10-11, 9-11,

12-17

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-9.

1 Brace at Jt(s): 17, 18

JOINTS

REACTIONS. All bearings 34-3-8

Max Horz 2=533(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 13, 11 except 16=-253(LC 12),

12=-227(LC 9)

Max Grav All reactions 250 lb or less at joint(s) except 2=607(LC 24), 16=1378(LC

20), 13=1295(LC 27), 12=698(LC 1), 11=410(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-697/25, 3-4=-750/140, 4-6=-514/172

BOT CHORD 2-16=-172/426, 13-16=-172/426, 12-13=-173/429

WEBS 3-16=-734/385, 17-18=-596/232, 13-17=-480/260, 7-17=-777/327, 8-17=-303/115,

7-18=-43/483, 12-17=-694/271

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Ceiling dead load (10.0 psf) on member(s). 3-4, 4-18, 17-18; Wall dead load (5.0psf) on member(s).3-16, 13-17
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13, 11 except (jt=lb) 16=253, 12=227.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 11) Attic room checked for L/360 deflection.



November 16,2020

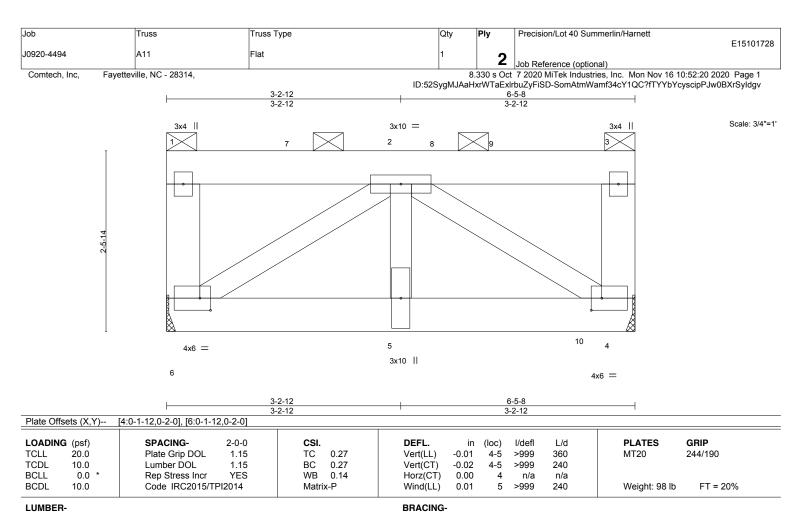
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTesk go comectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





TOP CHORD

BOT CHORD

2-0-0 oc purlins: 1-3. except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

2x4 SP No.2 *Except* WFBS 1-6,3-4: 2x6 SP No.1

REACTIONS. (size) 6=Mechanical, 4=Mechanical

Max Uplift 6=-175(LC 8), 4=-271(LC 8) Max Grav 6=1692(LC 2), 4=2438(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-6=-491/101

BOT CHORD 5-6=-227/1711 4-5=-227/1711 2-4=-2066/274, 2-6=-2066/274 WFBS

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

 3) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) and C-C Exterior(2) 0-2-12 to 4-7-9, Interior(1) 4-7-9 to 6-2-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=175, 4=271.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1225 lb down and 180 lb up at 1-9-12, and 1225 lb down and 180 lb up at 3-9-12 on top chord, and 1261 lb down and 157 lb up at 5-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15



November 16,2020

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Job	Truss	Truss Type	Qty	Ply	Precision/Lot 40 Summerlin/Harnett
10000 4404	Δ11	E			E15101728
J0920-4494	A11	Flat	1	2	Job Reference (optional)

Comtech, Inc,

Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:20 2020 Page 2 ID:52SygMJAaHxrWTaExlrbuZyFiSD-SomAtmWamf34cY1QC?fTYYbYcyscipPJw0BXrSyldgv

LOAD CASE(S) Standard Uniform Loads (plf)

Vert: 1-3=-60, 4-6=-20 Concentrated Loads (lb) Vert: 7=-1015 8=-1015 10=-1061



Job Truss Truss Type Qty Ply Precision/Lot 40 Summerlin/Harnett E15101729 GABLE J0920-4494 B1-GE Job Reference (optional) Fayetteville, NC - 28314, 8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:21 2020 Page 1 Comtech, Inc. ID:52SygMJAaHxrWTaExlrbuZyFiSD-w?KY46XCXzBwEhbdliAi5l8n4MGkRHkS9gw4Nvyldgu 9-6-0 9-6-0 19-0-0 9-6-0 Scale = 1:52.7 5x5 = 8 10.00 12 5 10 4x6 // 4x6 19 5x8 || 17 20 16 5.00 | 12 3x6 II 3x6 22 14 9-6-0 19-0-0 9-6-0 9-6-0

Plate Offsets (X,Y)--[3:0-2-0,0-2-0], [11:0-2-0,0-2-0] LOADING (psf) SPACING-2-0-0 CSI. DEFL. (loc) I/defl L/d **PLATES GRIP TCLL** 20.0 Plate Grip DOL 1.15 TC 0.04 Vert(LL) -0.00 12 n/r 120 MT20 244/190 **TCDL** -0.00 10.0 Lumber DOL 1.15 BC 0.02 Vert(CT) 13 n/r 120 **BCLL** 0.0 Rep Stress Incr YES WB 0.07 0.00 12 Horz(CT) n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Weight: 154 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No 1 2x4 SP No 2 OTHERS

SLIDER Left 2x4 SP No.2 -x 1-10-12, Right 2x4 SP No.2 -x 1-10-12

REACTIONS. All bearings 19-0-0.

(lb) - Max Horz 2=-264(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 18, 12, 19, 17 except 2=-213(LC 8), 20=-117(LC 12), 21=-117(LC

12), 22=-215(LC 12), 16=-120(LC 13), 15=-115(LC 13), 14=-188(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 12, 19, 20, 21, 22, 17, 16, 15, 14 except 2=300(LC 20),

18=405(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-340/258, 11-12=-259/177

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 12, 19, 17 except (jt=lb) 2=213, 20=117, 21=117, 22=215, 16=120, 15=115, 14=188.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 18, 19, 20, 21, 22, 17, 16, 15, 14.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

November 16,2020



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



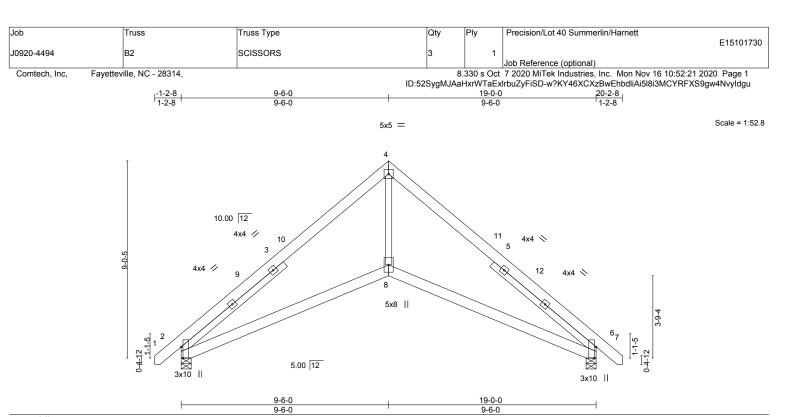


Plate Offs	sets (X,Y)	[2:0-6-3,Edge], [6:0-6-3,Edge]		
LOADING	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.36	Vert(LL) -0.06 6-8 >999 360 MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.29	Vert(CT) -0.13 6-8 >999 240
BCLL	0.0 *	Rep Stress Incr YES	WB 0.21	Horz(CT) 0.05 6 n/a n/a
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) -0.04 8 >999 240 Weight: 140 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No 1 BOT CHORD 2x6 SP No.1 2x4 SP No 2 WFBS

SLIDER Left 2x4 SP No.2 -x 6-2-10, Right 2x4 SP No.2 -x 6-2-10

REACTIONS. (size) 2=0-5-8, 6=0-5-8

Max Horz 2=-212(LC 10)

Max Uplift 2=-46(LC 12), 6=-46(LC 13) Max Grav 2=822(LC 1), 6=822(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1228/128, 4-6=-1228/123

BOT CHORD 2-8=0/957, 6-8=0/952

WEBS 4-8=0/912

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-1 to 3-3-12, Interior(1) 3-3-12 to 9-6-0, Exterior(2) 9-6-0 to 13-10-13, Interior(1) 13-10-13 to 20-1-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

November 16,2020



Design valid for use only with MiTek® connectors. This design is based only upon parameters and not is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information

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Job Truss Truss Type Qty Ply Precision/Lot 40 Summerlin/Harnett E15101731 ATTIC J0920-4494 C1-GE Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:22 2020 Page 1 ID:52SygMJAaHxrWTaExlrbuZyFiSD-OBuwlSYqHGKnsrApJQhxdzgpGmQdAcMcNKgdwLyldgt

18-3-4 7-11-4 8₁6-1 2-3-8 0-6-12 15-4-15 15-11₁12 11-11-8 23-11-0 3-5-7 3-5-7 0-6-12 2-3-8

Scale = 1:80.3

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 8-11-2 oc bracing.

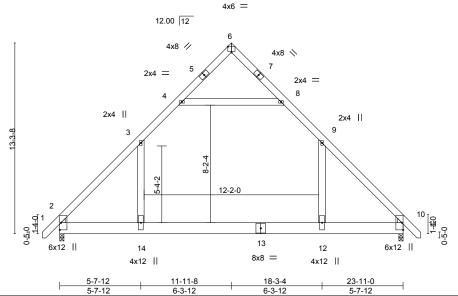


Plate Offsets (X,Y)-- [2:0-1-1,0-3-6], [2:0-0-8,0-0-8], [6:0-3-0,Edge], [10:0-0-8,0-0-8], [10:0-1-1,0-3-6]

LOADING	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.58	Vert(LL) -0.28 12-14 >999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.81	Vert(CT) -0.48 12-14 >593 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.63	Horz(CT) 0.01 10 n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.13 12-14 >999 240	Weight: 227 lb FT = 20%

BRACING-

TOP CHORD **BOT CHORD**

LUMBER-

TOP CHORD 2x6 SP No 1 *Except*

1-5.7-11: 2x6 SP 2400F 2.0E

BOT CHORD 2x10 SP No.1 **WEBS** 2x6 SP No.1

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 10=0-3-8

Max Horz 2=-391(LC 10)

Max Grav 2=1601(LC 20), 10=1601(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2166/4, 3-4=-1166/186, 8-9=-1165/186, 9-10=-2165/3

BOT CHORD 2-14=0/1268, 12-14=0/1268, 10-12=0/1268 **WEBS** 9-12=0/1051, 3-14=0/1051, 4-8=-1360/239

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Ceiling dead load (10.0 psf) on member(s). 3-4, 8-9, 4-8; Wall dead load (5.0psf) on member(s).9-12, 3-14
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
- 7) Attic room checked for L/360 deflection.



November 16,2020



Job Truss Truss Type Qty Ply Precision/Lot 40 Summerlin/Harnett E15101732 C2 ATTIC J0920-4494 3 Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:23 2020 Page 1 ID:52SygMJAaHxrWTaExIrbuZyFiSD-sNSIVnZS2aSeT?I?t7CAAADvBAINv3alc_PBSnyldgs

18-3-4 7-11-4 8-6-1 15-4-15 15-11-12 11-11-8 23-11-0 5-7-12 2-3-8 0-6-12 3-5-7 3-5-7 0-6-12 2-3-8

Scale = 1:80.3 4x6 = 12.00 12 5

Structural wood sheathing directly applied.

Rigid ceiling directly applied or 8-2-13 oc bracing.

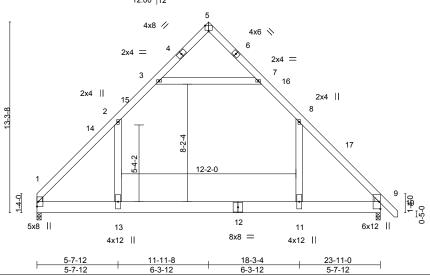


Plate Offsets (X,Y)- [1:0-1-1,0-3-6], [1:0-0-8,0-0-8], [5:0-3-0,Edge], [9:0-0-8,0-0-8], [9:0-1-1,0-3-6]

LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.96	Vert(LL) -0.30 11-13 >936 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.84	Vert(CT) -0.52 11-13 >544 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.63	Horz(CT) 0.01 9 n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.10 11-13 >999 240	Weight: 224 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No 1 *Except* 6-10: 2x6 SP 2400F 2.0E

BOT CHORD 2x10 SP No.1 **WEBS** 2x6 SP No.1

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 9=0-3-8

Max Horz 1=-309(LC 8)

Max Grav 1=1549(LC 21), 9=1608(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2094/0, 2-3=-1164/148, 7-8=-1152/141, 8-9=-2143/0

BOT CHORD 1-13=0/1237, 11-13=0/1237, 9-11=0/1237 **WEBS** 8-11=0/1061, 2-13=0/984, 3-7=-1374/163

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 11-11-8, Exterior(2) 11-11-8 to 16-4-5, Interior(1) 16-4-5 to 25-0-2 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Ceiling dead load (10.0 psf) on member(s). 2-3, 7-8, 3-7; Wall dead load (5.0psf) on member(s).8-11, 2-13
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13
- 7) Attic room checked for L/360 deflection.



November 16,2020



Job Truss Truss Type Qty Ply Precision/Lot 40 Summerlin/Harnett E15101733 СЗ ATTIC J0920-4494 6 Job Reference (optional)

Comtech, Inc.

Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:24 2020 Page 1 ID:52SygMJAaHxrWTaExlrbuZyFiSD-Ka0gi7Z4puaV59KBRqkPiOm3WZ56eWmvrd9k_Dyldgr

Structural wood sheathing directly applied.

Rigid ceiling directly applied or 7-6-8 oc bracing.

18-3-4 15-4-15 15-11₁12 11-11-8 23-11-0 2-3-8 0-6-12 3-5-7 3-5-7 0-6-12 2-3-8 4x6 =

Scale = 1:80.3

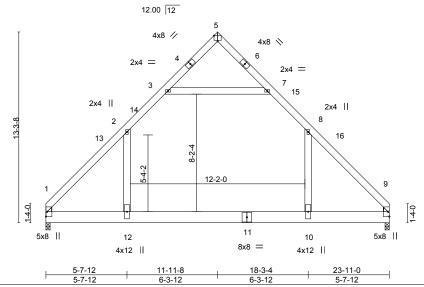


Plate Offsets (X,Y)-- [1:0-1-1,0-3-6], [1:0-0-8,0-0-8], [5:0-3-0,Edge], [9:0-0-8,0-0-8], [9:0-1-1,0-3-6]

LOADING	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl	L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.98	Vert(LL) -0.33 10-12 >869	360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.88	Vert(CT) -0.56 10-12 >502	240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.63	Horz(CT) 0.01 9 n/a	n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.11 10-12 >999	240	Weight: 220 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No 1 BOT CHORD 2x10 SP No.1 WFBS 2x6 SP No.1

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 9=0-3-8

Max Horz 1=302(LC 9)

Max Grav 1=1551(LC 21), 9=1551(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2096/0, 2-3=-1155/148, 7-8=-1154/148, 8-9=-2095/0

1-12=0/1226, 10-12=0/1226, 9-10=0/1226 BOT CHORD WEBS 8-10=0/994, 2-12=0/994, 3-7=-1375/166

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 11-11-8, Exterior(2) 11-11-8 to 16-4-5, Interior(1) 16-4-5 to 23-9-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Ceiling dead load (10.0 psf) on member(s). 2-3, 7-8, 3-7; Wall dead load (5.0psf) on member(s).8-10, 2-12
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 10-12
- 7) Attic room checked for L/360 deflection.





Job Truss Truss Type Qty Ρlγ Precision/Lot 40 Summerlin/Harnett E15101734 J0920-4494 C4 ATTIC GIRDER 2 Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:26 2020 Page 1 ID:52SygMJAaHxrWTaExlrbuZyFiSD-Hy8R7pbKLVqDKTUaYFmtoprU7Npu6UFClxer36yldgp

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

15-11-12 14-11-10 18-3-4 3-0-2 1-0-1 2-3-8 7-11-4 8-11-6 11-11-8 2-3-8 1-0-1 3-0-2 23-11-0

Scale = 1:80.3

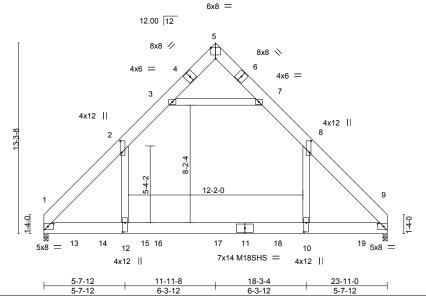


Plate Offsets (X,Y)-- [1:0-4-13,0-2-8], [2:0-9-12,0-1-4], [5:0-4-0,Edge], [8:0-9-12,0-1-4], [9:0-4-13,0-2-8], [10:0-7-12,0-2-0], [12:0-7-12,0-2-0]

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.66	Vert(LL) -0.28 10-12 >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.66	Vert(CT) -0.52 10-12 >544 240	M18SHS 244/190
BCLL 0.0 *	Rep Stress Incr NO	WB 0.38	Horz(CT) 0.02 9 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.17 10-12 >999 240	Weight: 546 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x10 SP 2400F 2.0E BOT CHORD 2x10 SP 2400F 2.0E

WFBS 2x6 SP No.1

REACTIONS. (size) 1=0-3-8, 9=0-3-8 Max Horz 1=-296(LC 24)

Max Uplift 1=-326(LC 9)

Max Grav 1=6904(LC 2), 9=5926(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-7702/124, 2-3=-3309/82, 3-5=-131/1512, 5-7=-33/1253, 7-8=-3568/181,

8-9=-7405/6

BOT CHORD 1-12=0/4375, 10-12=0/4418, 9-10=0/4381 **WEBS** 8-10=0/5178, 2-12=-234/5952, 3-7=-6311/204

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc.

Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-6-0 oc.

- Webs connected as follows: 2x6 2 rows staggered at 0-9-0 oc. 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Ceiling dead load (10.0 psf) on member(s). 2-3, 7-8, 3-7; Wall dead load (5.0psf) on member(s).8-10, 2-12
- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 10-12
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=326
- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1320 lb down and 183 lb up at 2-0-12, 1320 lb down and 183 lb up at 4-0-12, 747 lb down and 100 lb up at 6-0-12, 1678 lb down and 217 lb up at 7-11-0, 1678 lb down and 217 lb up at 12-1-0, and 1370 lb down at 16-3-0, and 1682 lb down and 213 lb up at 22-1-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 12) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

WARNING - Verify design parameters and HEAD NOTES ON THIS AND INCLUDED WITTER REFERENCE FACE MILE AND INCLUDED WITTER REFERENC fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TPH Qu Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



November 16,2020



Job	Truss	Truss Type	Qty	Ply	Precision/Lot 40 Summerlin/Harnett
J0920-4494	C4	ATTIC GIRDER	1		E15101734
00020-4404	04	ATTIO OINDER	'	2	Job Reference (optional)

Comtech, Inc,

Fayetteville, NC - 28314,

8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:26 2020 Page 2 ID:52SygMJAaHxrWTaExlrbuZyFiSD-Hy8R7pbKLVqDKTUaYFmtoprU7Npu6UFClxer36yldgp

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)
Vert: 1-2=-60, 2-3=-80, 3-5=-60, 5-7=-60, 7-8=-80, 8-9=-60, 1-12=-20, 10-12=-40, 9-10=-20, 3-7=-20

Drag: 8-10=-10, 2-12=-10

Concentrated Loads (lb)

Vert: 13=-1222(B) 14=-1222(B) 15=-607(B) 16=-1371(B) 17=-1371(B) 18=-945(B) 19=-1375(B)



Job Truss Truss Type Qty Ply Precision/Lot 40 Summerlin/Harnett E15101735 ROOF SPECIAL J0920-4494 M1 3 Job Reference (optional) Fayetteville, NC - 28314, 8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:26 2020 Page 1 Comtech, Inc. ID:52SygMJAaHxrWTaExlrbuZyFiSD-Hy8R7pbKLVqDKTUaYFmtopra8NwH6NkClxer36yldgp 5-9-11 5-9-11 Scale = 1:39.3 3x6 || 10.00 12 10 6x6 = 3.00 12 3 1-9-7 8 7 6 4x8 = 2x4 || 12_T7_Γ8

Plate Offsets (X,Y)	[2:0-2-12,0-0-7], [3:0-3-0,0-2-0]

LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.27	Vert(LL) -0.	04 8	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.25	Vert(CT) -0.	09 7-8	>999	240		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.79	Horz(CT) 0.	01 6	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.	09 7-8	>999	240	Weight: 86 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1 *Except* TOP CHORD 3-5: 2x6 SP No.1

BOT CHORD 2x6 SP No.1 **WEBS** 2x4 SP No.2 *Except* 4-7: 2x6 SP No.1

REACTIONS.

(size) 2=0-3-0, 6=0-3-8 Max Horz 2=225(LC 12)

Max Uplift 2=-182(LC 8), 6=-178(LC 9) Max Grav 2=578(LC 1), 6=481(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1260/945, 4-7=-259/190 TOP CHORD **BOT CHORD** 2-8=-1158/1186, 7-8=-1119/1170 WEBS 3-8=-353/280, 3-7=-1151/1093

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 12-4-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=182, 6=178.



Structural wood sheathing directly applied or 5-2-1 oc purlins,

Rigid ceiling directly applied or 7-1-3 oc bracing.

except end verticals.



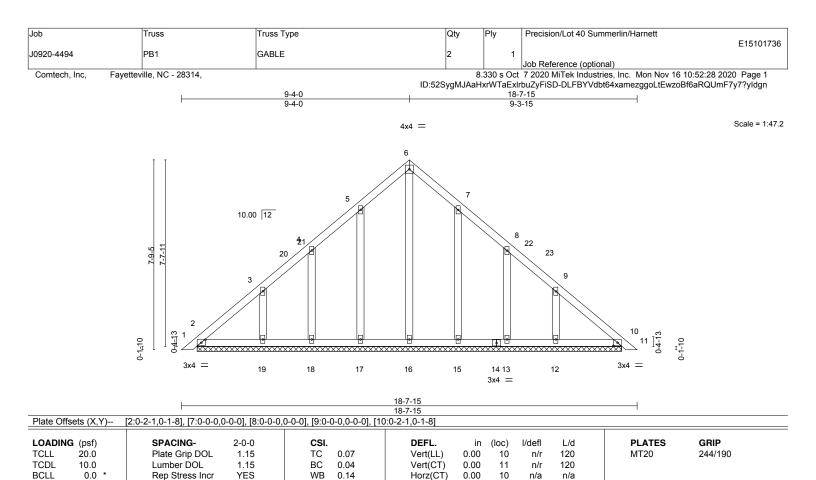
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BRACING-

TOP CHORD

BOT CHORD

n/a

LUMBER-

BCDL

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No 1

10.0

OTHERS 2x4 SP No.2

REACTIONS. All bearings 17-4-8. (lb) - Max Horz 2=-182(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 17, 18, 19, 15, 13, 12

Max Grav All reactions 250 lb or less at joint(s) 2, 16, 17, 18, 10, 19, 15, 13, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code IRC2015/TPI2014

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-2-12 to 4-7-9, Interior(1) 4-7-9 to 9-4-0, Exterior(2) 9-4-0 to 13-8-12, Interior(1) 13-8-12 to 18-5-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-S

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 17, 18, 19, 15, 13. 12.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Weight: 106 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

FT = 20%

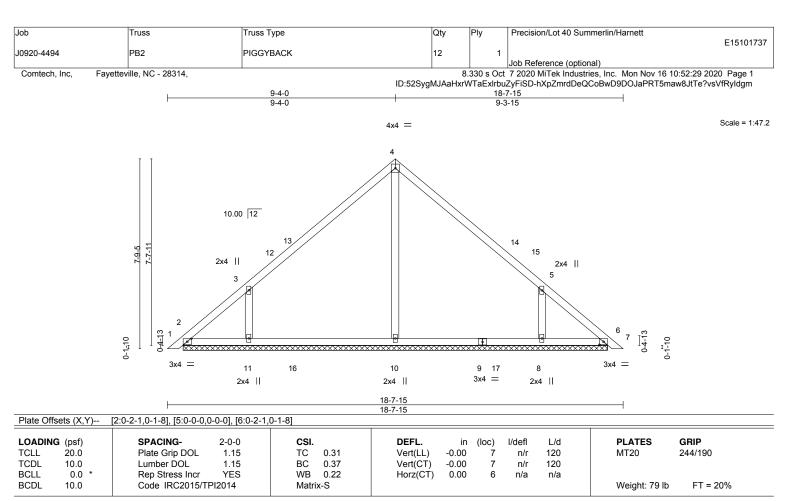
November 16,2020



Design valid for use only with MiTesk go connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/PTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information, evaluable from Trus Plate betting. 2570 Crisis Highways. fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TPH Qu Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No 1

2x4 SP No.2 OTHERS

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 17-4-8

(lb) - Max Horz 2=-182(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 11=-187(LC 12), 8=-187(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 6 except 10=624(LC 19), 11=558(LC 19), 8=557(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 3-11=-463/320, 5-8=-463/320

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-2-12 to 4-7-9, Interior(1) 4-7-9 to 9-4-0, Exterior(2) 9-4-0 to 13-8-12, Interior(1) 13-8-12 to 18-5-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 11=187, 8=187.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MID 20601



Job Truss Truss Type Qty Ply Precision/Lot 40 Summerlin/Harnett E15101738 PB3 J0920-4494 PIGGYBACK 3 Job Reference (optional) Fayetteville, NC - 28314, 8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:30 2020 Page 1 Comtech, Inc. ID:52SygMJAaHxrWTaExlrbuZyFiSD-9jNyzBerPkKfp4nLn5qpyf?GV_HL2J0nDZc3Ctyldgl 9-4-0 9-4-0 Scale = 1:47.0 4x4 = 10.00 12 3x4 || 5 2x4 ||

2x4 || Plate Offsets (X,Y)--[2:0-2-1,0-1-8]

SPACING-

Plate Grip DOL

Rep Stress Incr

Code IRC2015/TPI2014

Lumber DOL

0-1₁10

DEFL. in (loc) I/defl L/d Vert(LL) -0.00 n/r 120 -0.00 Vert(CT) 120 n/r 0.00 Horz(CT) n/a n/a

2x4 ||

PLATES GRIP MT20 244/190

Weight: 69 lb FT = 20%

LUMBER-

TCLL

TCDL

BCLL

BCDL

WFBS

LOADING (psf)

20.0

10.0

0.0

10.0

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x6 SP No.1 **OTHERS** 2x4 SP No.2 **BRACING-**

BOT CHORD

14-1-8 14-1-8

> TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

7 6

3x4 ||

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 13-5-12

(lb) - Max Horz 2=176(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 7, 2 except 9=-187(LC 12)

3x4

2-0-0

1.15

1.15

YES

Max Grav All reactions 250 lb or less at joint(s) 2 except 7=306(LC 20), 8=647(LC 19), 9=575(LC 19)

9

CSI.

TC

вс

WB

Matrix-S

0.31

0.31

0.26

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 4-8=-285/67, 3-9=-481/339

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ff; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-2-12 to 4-7-9, Interior(1) 4-7-9 to 9-4-0, Exterior(2) 9-4-0 to 13-9-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 2 except (jt=lb) 9 = 187.
- 6) Non Standard bearing condition. Review required.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building



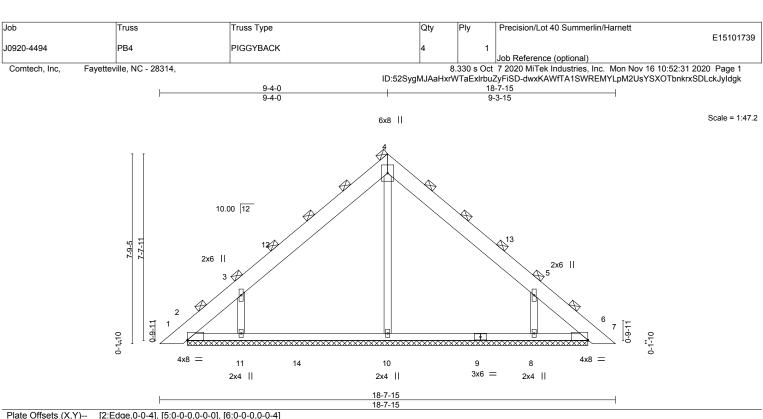
November 16,2020







Edenton, NC 27932



1 late of	10010 (71, 1)	[L.Lago, 0 0 1], [0.0 0 0,0 0 0], [0.0 0	0,0 0 1]	
LOADIN	IG (psf)	SPACING- 5-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.23	Vert(LL) -0.00 7 n/r 120 MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.95	Vert(CT) 0.00 6 n/r 120
BCLL	0.0 *	Rep Stress Incr NO	WB 0.42	Horz(CT) 0.01 6 n/a n/a
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Weight: 114 lb FT = 20%

LUMBER-TOP CHORD

2x8 SP No 1 2x4 SP No 1

BOT CHORD 2x4 SP No.2 OTHERS

BRACING-TOP CHORD

BOT CHORD

2-0-0 oc purlins (6-0-0 max.)

(Switched from sheeted: Spacing > 2-8-0).

Rigid ceiling directly applied or 7-2-13 oc bracing.

REACTIONS. All bearings 16-4-13.

(lb) - Max Horz 2=442(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 6 except 2=-207(LC 8), 11=-473(LC 12), 8=-463(LC 13) Max Grav All reactions 250 lb or less at joint(s) except 2=384(LC 20), 10=1466(LC 19), 11=1332(LC 19), 8=1332(LC 20), 6=311(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-548/363, 3-4=-627/458, 4-5=-588/466, 5-6=-415/185 TOP CHORD **BOT CHORD** 2-11=-95/311. 10-11=-94/290. 8-10=-94/290. 6-8=-79/300

WEBS 4-10=-505/6, 3-11=-1111/786, 5-8=-1113/785

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-11 to 4-10-7, Interior(1) 4-10-7 to 9-4-0, Exterior(2) 9-4-0 to 13-8-12, Interior(1) 13-8-12 to 18-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (jt=lb) 2=207, 11=473, 8=463.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





Job Truss Truss Type Qty Ply Precision/Lot 40 Summerlin/Harnett E15101740 J0920-4494 VA1 **GABLE** Job Reference (optional)

5-9-14 5-9-14

Fayetteville, NC - 28314, Comtech, Inc.

8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:31 2020 Page 1 ID:52SygMJAaHxrWTaExlrbuZyFiSD-dwxKAWfTA1SWREMYLpM2UsYVEOg?nqjxSDLckJyldgk 11-7-13 5-9-15

Scale = 1:35.3 3x4 =

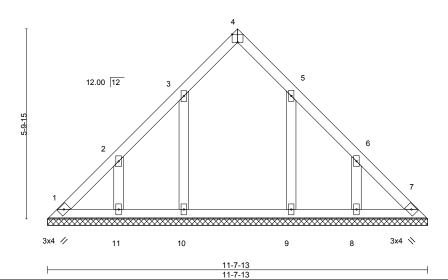


Plate Offsets (X,Y) [4:0-2-0,Edge]							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.06 BC 0.09 WB 0.04	DEFL. in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 7 n/a n/a	PLATES GRIP MT20 244/190			
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 56 lb FT = 20%			

LUMBER-

TOP CHORD 2x4 SP No 1 BOT CHORD 2x4 SP No.1

2x4 SP No.2 OTHERS

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 11-7-13

(lb) - Max Horz 1=-164(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7 except 10=-109(LC 12), 11=-160(LC 12), 9=-105(LC 13), 8=-161(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 11, 8 except 10=298(LC 19), 9=293(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb) 10=109, 11=160, 9=105, 8=161.



November 16,2020





Job Truss Truss Type Qty Ply Precision/Lot 40 Summerlin/Harnett E15101741 J0920-4494 VALLEY VA2 Job Reference (optional) Fayetteville, NC - 28314, 8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:32 2020 Page 1 Comtech, Inc. ID:52SygMJAaHxrWTaExlrbuZyFiSD-56ViOsg5xLaN2OxkvWtH145clo_YWHn4ht59GmyIdgj 4-7-7 4-7-7 Scale = 1:30.4 4x4 = 2 12.00 12 3x4 // 3x4 \ 2x4 || 9-2-14 9-2-14 LOADING (psf) SPACING-2-0-0 CSI. **DEFL** in (loc) I/defl L/d **PLATES** GRIP **TCLL** 20.0 Plate Grip DOL 1.15 TC 0.20 Vert(LL) n/a n/a 999 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 вс 0.14 Vert(CT) n/a n/a 999 **BCLL** WB 0.05 0.0 Rep Stress Incr YES Horz(CT) 0.00 3 n/a n/a Code IRC2015/TPI2014 FT = 20% **BCDL** 10.0 Matrix-S Weight: 38 lb

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 **OTHERS**

(size) 1=9-2-14, 3=9-2-14, 4=9-2-14

Max Horz 1=-102(LC 8)

Max Uplift 1=-25(LC 13), 3=-25(LC 13)

Max Grav 1=194(LC 1), 3=194(LC 1), 4=296(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



Job Truss Truss Type Qty Ply Precision/Lot 40 Summerlin/Harnett E15101742 J0920-4494 VA3 VALLEY Job Reference (optional) Fayetteville, NC - 28314, 8.330 s Oct 7 2020 MiTek Industries, Inc. Mon Nov 16 10:52:33 2020 Page 1 Comtech, Inc. ID:52SygMJAaHxrWTaExIrbuZyFiSD-al34bCgjifiDgXWwSDOWaHdptCM6FkeEvXqjpCyIdgi 5-10-14 2-11-7 Scale = 1:20.5 4x4 = 2 12.00 12 3x4 // 3x4 💉 2x4 || 5-10-14 5-10-14 LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) I/defl L/d **PLATES** GRIP **TCLL** 20.0 Plate Grip DOL 1.15 TC 0.11 Vert(LL) n/a n/a 999 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 вс 0.05 Vert(CT) n/a n/a 999 **BCLL** WB 0.01 0.0 Rep Stress Incr YES Horz(CT) 0.00 3 n/a n/a Code IRC2015/TPI2014 **BCDL** 10.0 Matrix-P Weight: 23 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 **OTHERS**

(size) 1=5-10-14, 3=5-10-14, 4=5-10-14

Max Horz 1=-62(LC 8)

Max Uplift 1=-23(LC 13), 3=-23(LC 13)

Max Grav 1=127(LC 1), 3=127(LC 1), 4=163(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 5-10-14 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

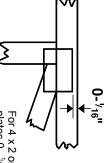


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- ¹/18" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

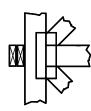
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



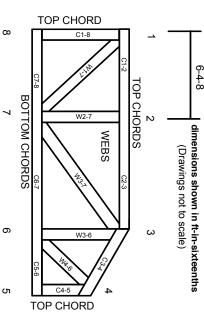
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.
Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

ANSI/TPI1: DSB-89:

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Iniury

- Damage or Personal Injury

 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.

4

- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.

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- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

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 Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.

9

- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.

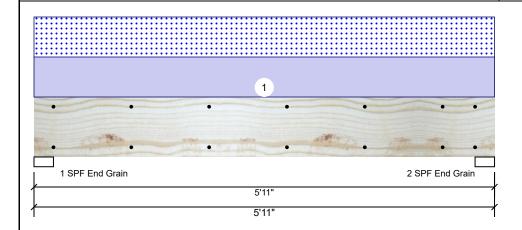


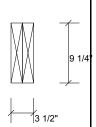
Client: Project: Address: Date: 11/16/2020 Designer:

Neal Baggett Job Name: Lot 40 Summerlin

Project #:

Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED Level: Level





Page 1 of 10

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal Temperature: Temp <= 100°F

Application: Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Wind Brg Live Dead Snow Const 0 1764 1742 0 0 1 2 0 1764 1742 0 0

Analysis Results

Comb. Analysis Actual Location Allowed Capacity Case 2'11 1/2" 14423 ft-lb Moment 4550 ft-lb 0.315 (32%) D+S L Unbraced 4550 ft-lb 2'11 1/2" 11027 ft-lb 0.413 (41%) D+S L 2370 lb Shear 11 1/2" 7943 lb 0.298 (30%) D+S L LL Defl inch 0.035 (L/1894) 2'11 1/2" 0.139 (L/480) 0.250 (25%) S L TL Defl inch 0.071 (L/941) 2'11 1/2" 0.185 (L/360) 0.380 (38%) D+S L

Bearings

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.000" 1764 / 1742 3506 L D+S End Grain 2 - SPF 3.000" 1764 / 1742 D+S 3506 L End Grain

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

Uniform

Load Type ID Trib Width Side Dead 0.9 Comments Location Live 1 Snow 1.15 Wind 1.6 Const. 1.25

589 PLF

0 PLF

589 PLF

Top

Self Weight 7 PLF

1

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 12/11/2021

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

0 PLF

0 PLF

A02





isDesign™

Client:

Project: Address:

11/16/2020 Designer:

Neal Baggett Job Name: Lot 40 Summerlin

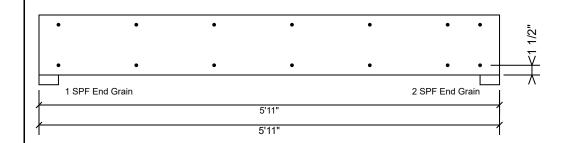
Project #:

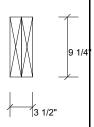
Kerto-S LVL

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 2 of 10

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

1 3		•	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	163.7 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 12/11/2021

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info







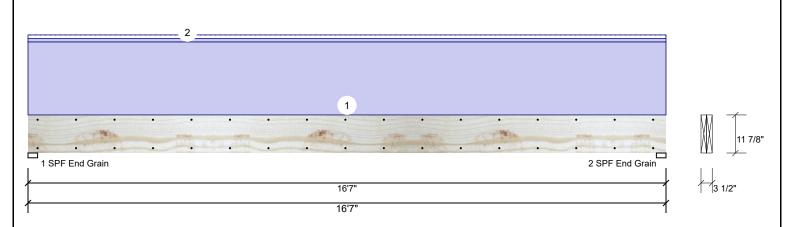
Client: Project: Address: Date: 11/16/2020

Designer: Neal Baggett Job Name: Lot 40 Summerlin

Project #:

1.750" X 11.875" 2-Ply - PASSED **Kerto-S LVL**

Level: Level



Member Inform	nation						Reaction	ns UNPAT	TERNE	D lb (Uplift))		
Type:	Girder		Applicat	ion: F	loor		Brg	Live	Dead	d Snow		Wind	Const
Plies:	2		Design I	Method: A	SD		1	0	190	1 83		0	0
Moisture Condition	: Dry		Building	Code: IE	BC/IRC 2015		2	0	190	1 83		0	0
Deflection LL:	480		Load Sh	aring: N	lo								
Deflection TL:	360		Deck:	N	lot Checked								
Importance:	Normal												
Temperature:	Temp <= 100)°F											
							Bearing	s					
							Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb
							1 - SPF	3.000"	22%	1901 / 83	1984	L	D+S
							End						
Analysis Result	:S						Grain						
Analysis Ac	tual	Location	Allowed	Capacity	Comb.	Case	2 - SPF	3.000"	22%	1901 / 83	1984	L	D+S
Moment 752	28 ft-lb	8'3 1/2"	17919 ft-lb	0.420 (42%) D	Uniform	End Grain						
Unbraced 785	6 ft-lb	8'3 1/2"	7862 ft-lb	0.999	D+S	L							

Uniform

1

L

TL Defl inch 0.402 (L/484) **Design Notes**

LL Defl inch

Shear

1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".

15'4 7/8" 7980 lb

(100%)

8'3 9/16" 0.405 (L/480) 0.040 (4%) S

8'3 9/16" 0.540 (L/360) 0.740 (74%) D+S

0.204 (20%) D

- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 12'4 1/2" o.c.
- 6 Bottom braced at bearings.

1631 lb

(L/11572)

0.017

7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	210 PLF	0 PLF	0 PLF	0 PLF	0 PLF	C1-GE
2	Tie-In	0-0-0 to 16-7-0	(Span)1-0-0	Тор	20 PSF	0 PSF	20 PSF	0 PSF	0 PSF	RAKE OH
Ì	Self Weight				9 PLF					

Notes

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 2 Damaged Beams must not be used

Handling & Installation

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 3 of 10

This design is valid until 12/11/2021

Manufacturer Info



isDesign™

Client: Project: Address:

11/16/2020 Designer: Neal Baggett

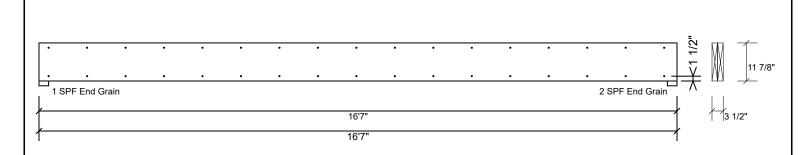
Job Name: Lot 40 Summerlin

Page 4 of 10

Project #:

1.750" X 11.875" 2-Ply - PASSED **Kerto-S LVL**

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

rasterrain pries asing E	TOWS OF TOU BOX Halls (CTEONS) at
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

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 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is valid until 12/11/2021

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633







Client: Project: Address:

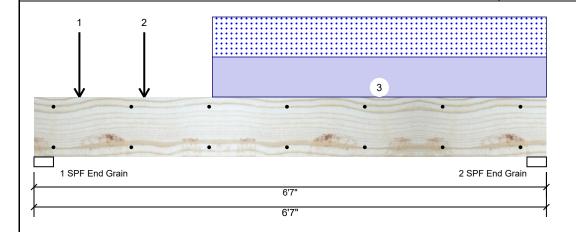
11/16/2020 Designer: Neal Baggett

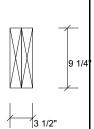
Job Name: Lot 40 Summerlin

Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL**

Level: Level





Page 5 of 10

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal Temperature: Temp <= 100°F

Application: Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Live Wind Brg Dead Snow Const 0 1806 1783 0 0 1 2 0 1366 1342 0 0

Analysis Results

Analysis Actual Location Allowed Capacity Comb. Case 3'1 3/4" 14423 ft-lb Moment 4163 ft-lb 0.289 (29%) D+S L Unbraced 4163 ft-lb 3'1 3/4" 10370 ft-lb 0.401 (40%) D+S L 2930 lb Shear 11 1/2" 7943 lb 0.369 (37%) D+S L LL Defl inch 0.039 (L/1911) 3'2 11/16" 0.155 (L/480) 0.250 (25%) S L TL Defl inch 0.079 (L/948) 3'2 11/16" 0.207 (L/360) 0.380 (38%) D+S L

Bearings

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.000" 1806 / 1783 3589 L D+S End Grain 2 - SPF 3.000" 1366 / 1342 2708 L D+S End Grain

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Point	0-7-0		Тор	670 lb	0 lb	670 lb	0 lb	0 lb	A05	
2	Point	1-5-0		Тор	781 lb	0 lb	781 lb	0 lb	0 lb	A06	
3	Part. Uniform	2-3-8 to 6-7-0		Тор	390 PLF	0 PLF	390 PLF	0 PLF	0 PLF	A07	
	Self Weight				7 PLF						

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 12/11/2021

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info





isDesign™

Client: Project: Address: 11/16/2020

Designer: Neal Baggett Job Name: Lot 40 Summerlin

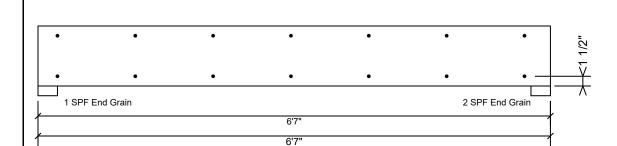
Level: Level

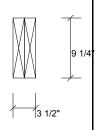
Project #:

Kerto-S LVL

1.750" X 9.250"

2-Ply - PASSED





Page 6 of 10

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

1 3		•	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	163.7 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

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This design is valid until 12/11/2021





Client: Project: Address:

11/16/2020 Designer: Neal Baggett

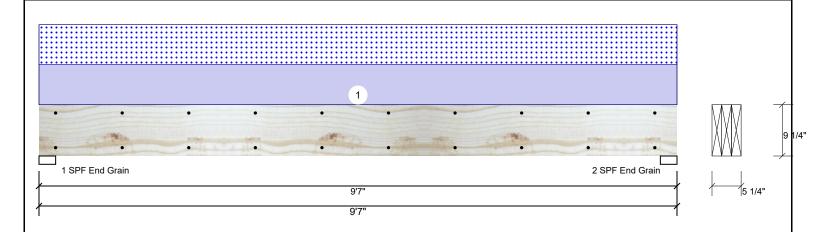
Job Name: Lot 40 Summerlin

Page 7 of 10

Project #:

1.750" X 9.250" 3-Ply - PASSED **Kerto-S LVL**

Level: Level



Member Information										
Type:	Girder									
Plies:	3									
Moisture Condition:	Dry									
Deflection LL:	480									
Deflection TL:	360									
Importance:	Normal									
Temperature:	Temp <= 100°F									

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: Yes Deck: Not Checked

Reaction	Reactions UNPATTERNED Ib (Uplift)											
Brg	Live	Dead	Snow	Wind	Const							
1	0	2289	2238	0	0							
2	0	2289	2238	0	0							

Analysis Results Capacity Analysis Actual Location Allowed Comb. Case Moment 10014 ft-lb 4'9 1/2" 22500 ft-lb 0.445 (45%) D+S L Unbraced 10014 ft-lb 4'9 1/2" 11921 ft-lb 0.840 (84%) D+S L Shear 3622 lb 8'7 1/2" 11914 lb 0.304 (30%) D+S L LL Defl inch 0.121 (L/914) 4'9 1/2" 0.230 (L/480) 0.520 (52%) S L TL Defl inch 0.244 (L/452) 4'9 1/2" 0.307 (L/360) 0.800 (80%) D+S L

	Bearing	5					
I	Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
	1 - SPF End Grain	3.000"	33%	2289 / 2238	4527	L	D+S
	2 - SPF End Grain	3.000"	33%	2289 / 2238	4527	L	D+S

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	467 PLF	0 PLF	467 PLF	0 PLF	0 PLF	
	Self Weight				11 PLF					

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 12/11/2021

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Manufacturer Info





isDesign™

Client: Project: Address:

11/16/2020 Designer: Neal Baggett

Job Name: Lot 40 Summerlin

Page 8 of 10

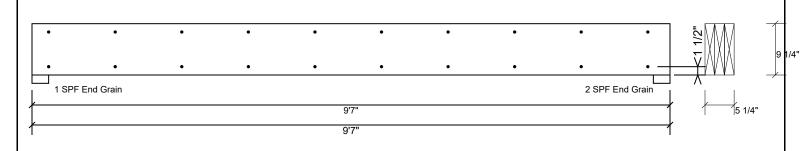
Project #:

Kerto-S LVL

1.750" X 9.250"

3-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Informing & Installation

 I. VIL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation

- - This design is valid until 12/11/2021

For flat roofs provide proper drainage to prevent ponding

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ICC-ES: ESR-3633

Manufacturer Info







Client: Project: Address:

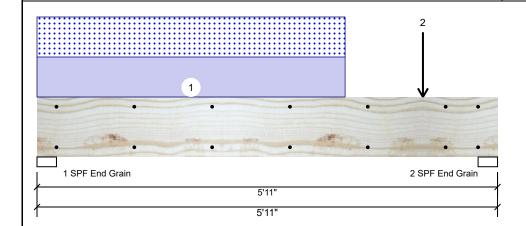
11/16/2020 Designer: Neal Baggett

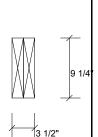
Job Name: Lot 40 Summerlin

Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL**

Level: Level





Page 9 of 10

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal Temperature: Temp <= 100°F

Application: Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	0	1653	1632	0	0
2	0	1920	1899	0	0

Bearings

End Grain

Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.000" 1653 / 1632 3286 L D+S End Grain 2 - SPF 3.000" 1920 / 1899 3818 L D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4395 ft-lb	3' 7/16"	14423 ft-lb	0.305 (30%)	D+S	L
Unbraced	4395 ft-lb	3' 7/16"	11027 ft-lb	0.399 (40%)	D+S	L
Shear	3811 lb	4'11 1/2"	7943 lb	0.480 (48%)	D+S	L
LL Defl inch	0.034 (L/1933)	3' 1/16"	0.139 (L/480)	0.250 (25%)	S	L
TL Defl inch	0.069 (L/961)	3' 1/16"	0.185 (L/360)	0.370 (37%)	D+S	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Part. Uniform	0-0-0 to 3-11-8		Тор	537 PLF	0 PLF	537 PLF	0 PLF	0 PLF	A03
2	Point	4-11-8		Тор	1405 lb	0 lb	1405 lb	0 lb	0 lb	A04
	Self Weight				7 PLF					

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This design is valid until 12/11/2021

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Manufacturer Info





isDesign™

Client: Project:

Designer: Address:

Project #:

Neal Baggett Job Name: Lot 40 Summerlin

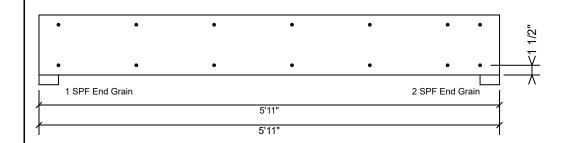
11/16/2020

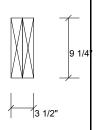
Kerto-S LVL

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 10 of 10

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	163.7 PLF	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

Notes

NOtes
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Handling & Installation

- Handling & Installation

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Manufacturer Info



